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Causeway Bay

Annex E

Sewerage Impact Assessment



Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Sewerage Impact Assessment for Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Report Ref

05 | 16 January 2024

This report takes into account the particular instructions and requirements of our client.

It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

Job number 285077

Ove Arup & Partners Hong Kong Ltd
Level 5 Festival Walk
80 Tat Chee Avenue
Kowloon Tong
Kowloon
Hong Kong
www.arup.com

ARUP

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1 Introduction

Sewerage Impact Assessment (“SIA”) had been submitted in fulfilment of the Special Conditions (61) (a) of the Conditions of Sale of the Lot requiring for submission of a SIA and was approved by EPD and DSD on 5 January 2022 and 3 May 2022 respectively.

This revised SIA had been submitted to support the Fresh S16 Planning Application with the revised layout plan submission. The recommendation established in the previously approved SIA remains unchanged.

1.1 Reference Materials

In evaluating the sewerage impact arising from the proposed development, the following sources of information have been specifically referred to:

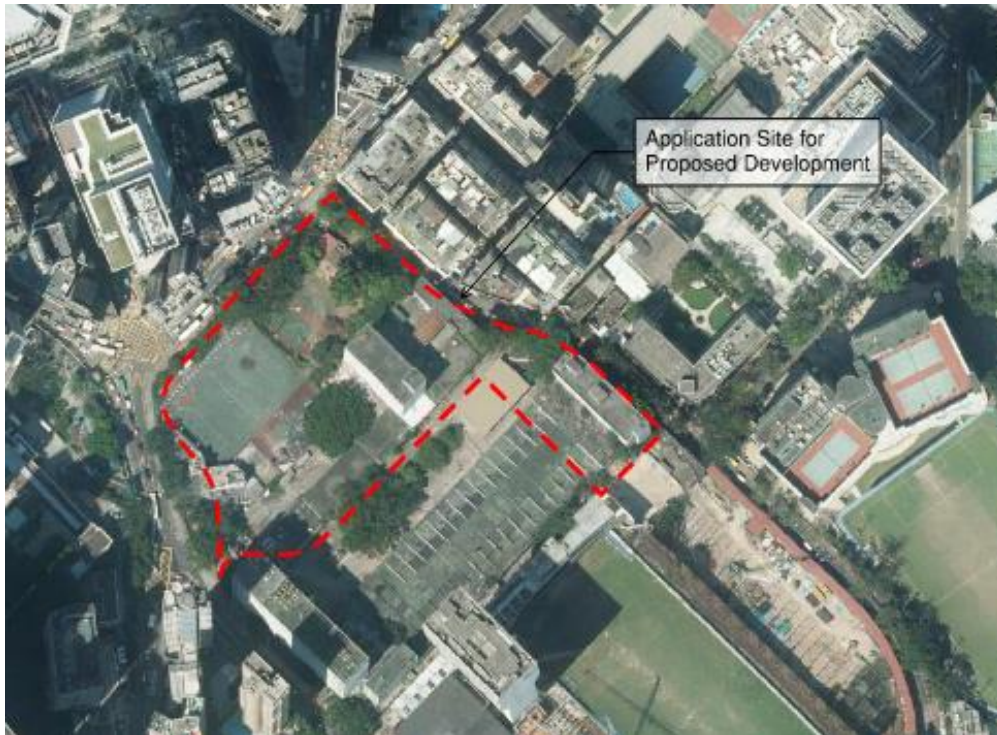
- Environmental Protection Department (EPD) Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning No. : EPD/TP 1/05;
- DSD Sewerage Manual – Key Planning Issues and Gravity Collection System; and
- Drainage Record Plans obtained from DSD.

2 The Development

For easy reference, a comparison table showing the difference between the Approved Scheme and the Proposed Scheme is shown in table below:

Description	Approved Scheme	Proposed Scheme	Difference
Project Title	Proposed Redevelopment at Caroline Hill Road, Causeway Bay		
Description	Two 24-storeys office towers (Tower 1 and Tower 2) and one 18-storeys office tower (Tower 3) for office, retail and GIC facilities use.		
Location	The site is located at Caroline Hill Road, Causeway Bay (see Figure 1).		
Land Use Zoning	Commercial		
Site Area	14,802 m ²	14,802 m ²	Nil
Office GFA	85,000 m ²	85,000 m ²	Nil
Retail GFA	10,000 m ²	10,000 m ²	Nil
GIC GFA	5,000 m ²	5,000 m ²	Nil
GIC GFA (Performing Art & Cultural Facilities)	2,000 m ²	0 m ²	-2,000 m ²

Below is an aerial photograph of the Application Site.



3 Description of Existing Environment and Baseline Conditions

3.1 Site Location and Topography

The Application site is located at Causeway Bay at the junction of Caroline Hill Road and Leighton Road. The Application Site covers approximately 14802m² of area. It was occupied by the ex-Electrical and Mechanical Services Department (EMSD) Headquarters, the ex-Civil Aid Service Headquarters, the ex-Post Office Recreation Club and the PCCW Recreation Club.

3.2 Approach and Methodology

The sewage generated from the proposed development will be collected and conveyed to the existing sewerage system via the existing manhole FMH7058242 and FMH7058644. In order to assess the associated sewerage impact, the capacities of the existing public sewers along Caroline Hill Road were checked.

The adequacy of sewerage capacity along Caroline Hill Road was determined by using the estimate of future sewage generation from the proposed development.

The global unit flow factors recommended in the EPD Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning No.: EPD/TP 1/05 have been adopted in the estimation.

3.3 Evaluation of Sewerage Impact

The sewage flow estimation from the proposed development, the unit flow factors as stated in EPD/TP 1/05 have been adopted.

The capacities of sewers have been calculated based on Colebrook-White equation, assuming full bore flow as below:

- (a) Pipe capacity is calculated based on the continuity equation

$$Q = AV$$

Where Q = pipe full flow capacity in m³/s

A = pipe cross-sectional area in m²

V = velocity at full bore flow in m/s

- (b) Velocity at full bore flow is based on the Colebrook-White equation

$$V = -\frac{1.49}{R^{0.375}} \left\{ \frac{ks}{14800R} + \left(\frac{1.255v}{R} \left(\frac{ks}{14800R} \right)^{0.5} \right)^{0.375} \right\}^{-0.25}$$

Where g = acceleration due to gravity in m/s²

R = hydraulic radius in m

S = pipe gradient

ks = pipe roughness in mm

v = kinematic viscosity of water in m²/s

- (c) The sewerage impact due to the proposed development on the two existing public sewers have been evaluated by calculating the estimated peak flow against the capacity of the existing public sewer. One is the existing public sewer with downstream sewer of 600Ø run along the South of Caroline Hill Road. Another one is the existing public sewer with downstream sewer of 500Ø run along the East of Caroline Hill Road.
- (d) The detailed calculations are provided in **Appendix B** for reference.

3.4 Sewage Generation from the Proposed Development

The prediction for the proposed development sewage generation has been based on the information extracted from the development schedule (refer to Section 2). The quantity of sewage generated by the proposed is estimated from expected total population.

The table showing the sewage generation of the proposed development is calculated based on the guideline set in EPD Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning and is shown on **Table B1 in Appendix B** with the comparison between the Approved Scheme and the Proposed Scheme shown in the table below.

Description	Approved Scheme	Proposed Scheme	Difference
ADWF (m ³ /day)	991.7	973.2	-18.5
Catchment Inflow Factor	1.0	1.0	Nil
Contributing Population	3673	3604	-69
Global Peaking Factor	6	6	Nil
Peak Discharge, L/s	68.87	67.58	-1.29

3.5 Impact of the Proposed Development

In order to assess the impact on the existing public sewer associated with the proposed development, the capacities of the existing public sewers have been checked and shown on **Tables B2** in **Appendix B**.

The estimation of sewage generation in the vicinity of the Application Site is based on the assumptions as below:

- 1) Existing public sewer information based on DSD drainage record plans and shown in **Figure 2-5**;
- 2) Existing development parameters in the vicinity of the proposed development are obtained from public domain and sewerage catchment plan shown in **Figure 6**;
- 3) Flow factors as per EPD Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning; and
- 4) Global peaking factor with stormwater allowance is adopted as per EPD Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning.
- 5) The sewage flow and tentative sewerage system from Proposed District Court Complex is based on the design reference to the approved technical feasibility statement from ArchSD and shown in **Figure 7**.
- 6) As per ArchSD's current design, the sewer of FC tower is recommended to be connected to the existing sewerage manhole FSH7003584 and that of DC tower is recommended to be connected to the existing sewerage manhole FMH7009989.

The peak sewage flow from the proposed development is slightly reduced from 68.87 L/s to 67.58 L/s.

On the South side of Caroline Hill Road, it has proven that an existing public sewerage serving the Application Site comprising an existing 300Ø public gravity sewer running along the south of Caroline Hill Road and the downstream existing public sewer of 600Ø running along the Leighton Road has sufficient capacity to carry the estimated sewage from the Application Site.

On the East side of Caroline Hill Road, it has proven that an existing 400Ø public gravity sewer running along the east of Caroline Hill Road and the downstream

existing public sewer of 500Ø running along Leighton Road has sufficient capacity to carry the estimated sewage from the Application Site.

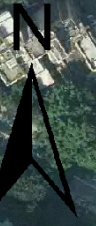
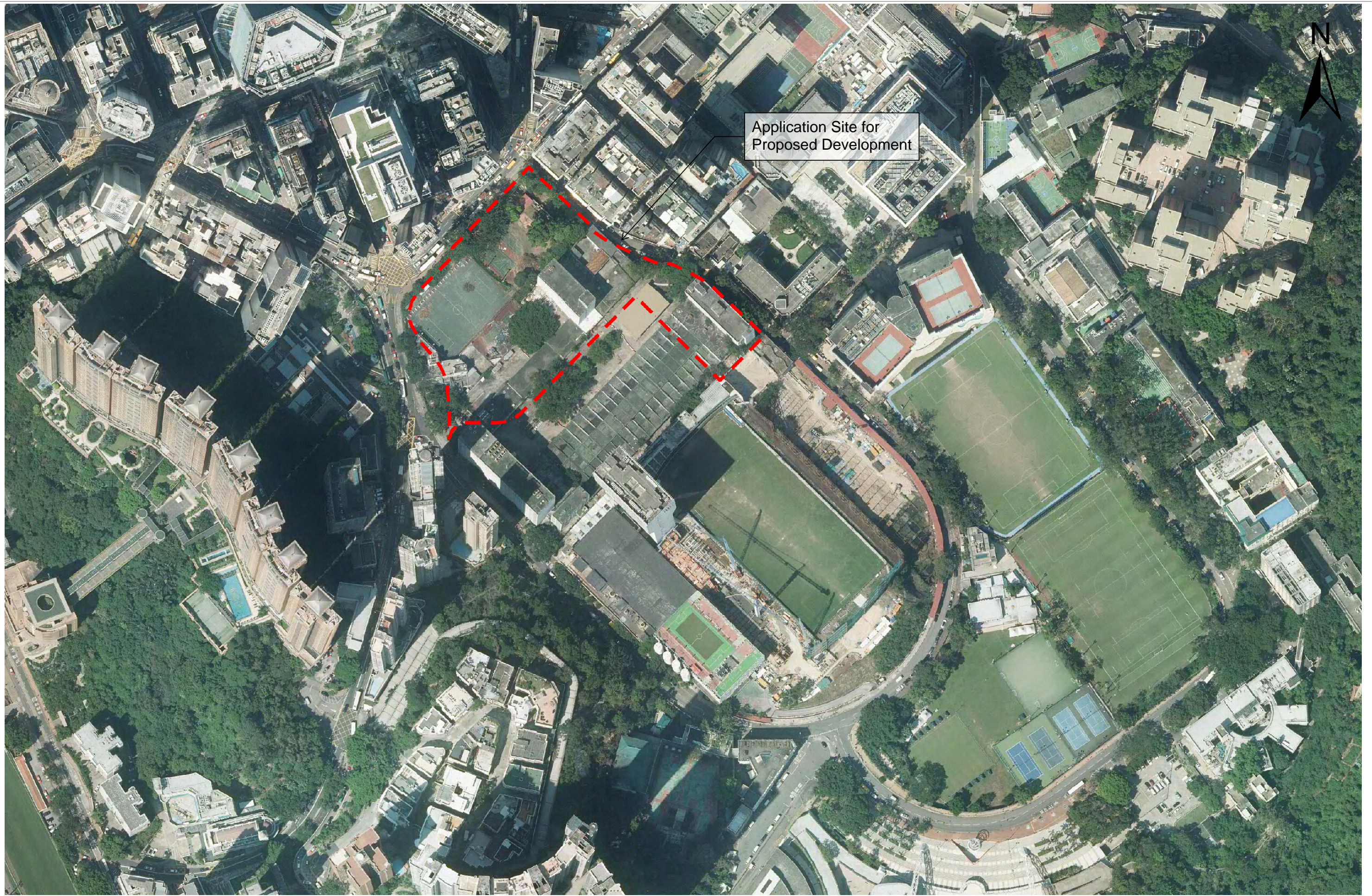
It is concluded that the proposed development would not result in any adverse sewerage impact to the existing public sewerage system. The capacities checking of the existing public sewers is shown in **Table B2**.

4 Conclusion

The peak sewage flow from the proposed development is slightly reduced from 68.87 L/s to 67.58 L/s. It is observed that the two existing public sewerage serving the Application Site through existing FMH7058644 and FMH7058242 running along the Leighton Road, have sufficient capacity to carry the estimated sewage from the Application Site. It is concluded that the proposed development would not result in any adverse sewerage impact to the existing public sewerage system.

Appendix A

Figures



Application Site for
Proposed Development

LEGEND

 SITE BOUNDARY OF PROPOSED DEVELOPMENT

Mark	Date	By	Rev.

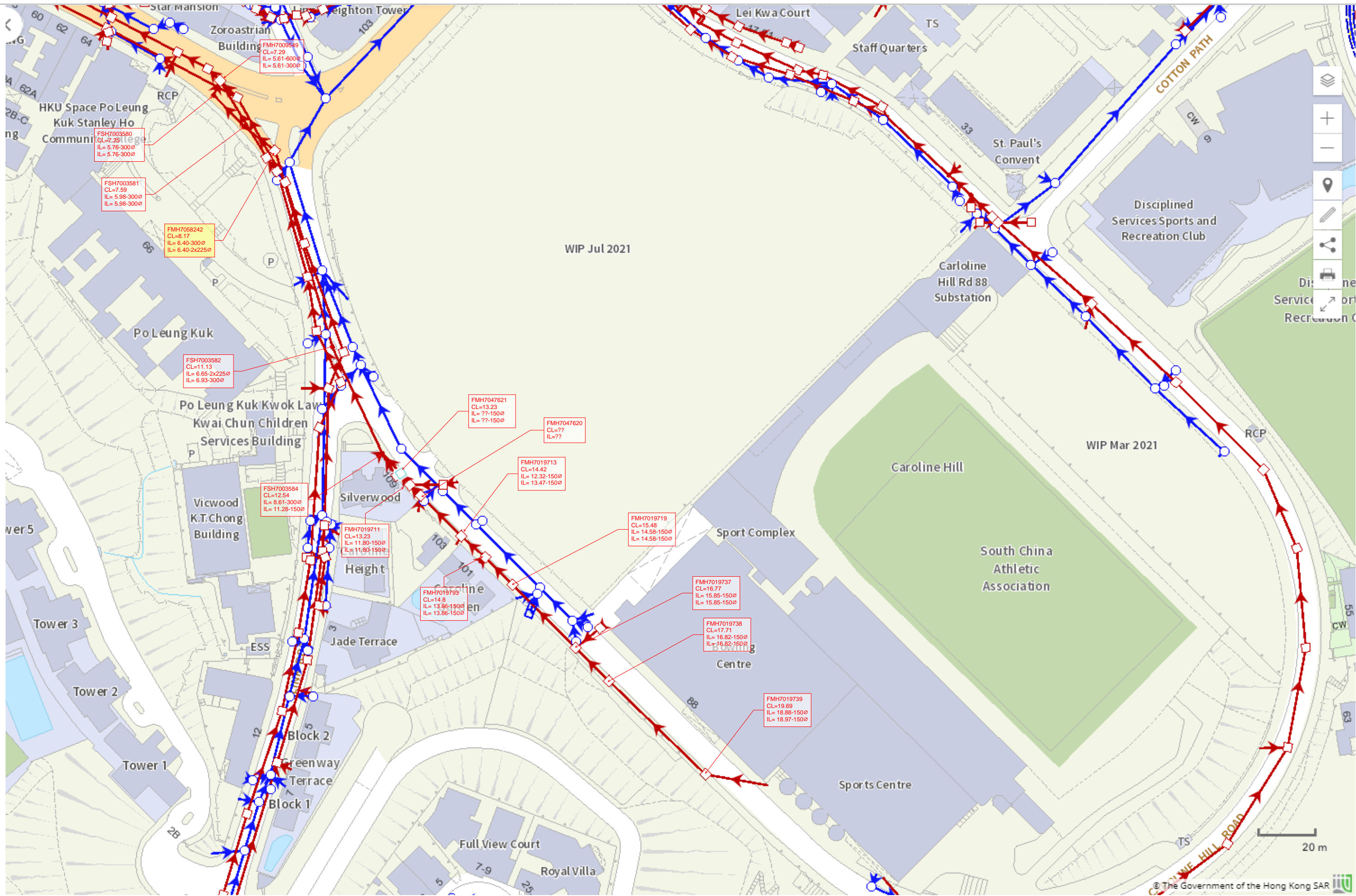
Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
LOCATION PLAN

ARUP

Drawing Status
FOR REFERENCE

Scale	
Dwn.	Date
Chd.	Passed
Job No. 285077	Drawing No. FIGURE 1
	Rev.



WIP Jul 2021

WIP Mar 2021

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Mark	Date	By	Rev.

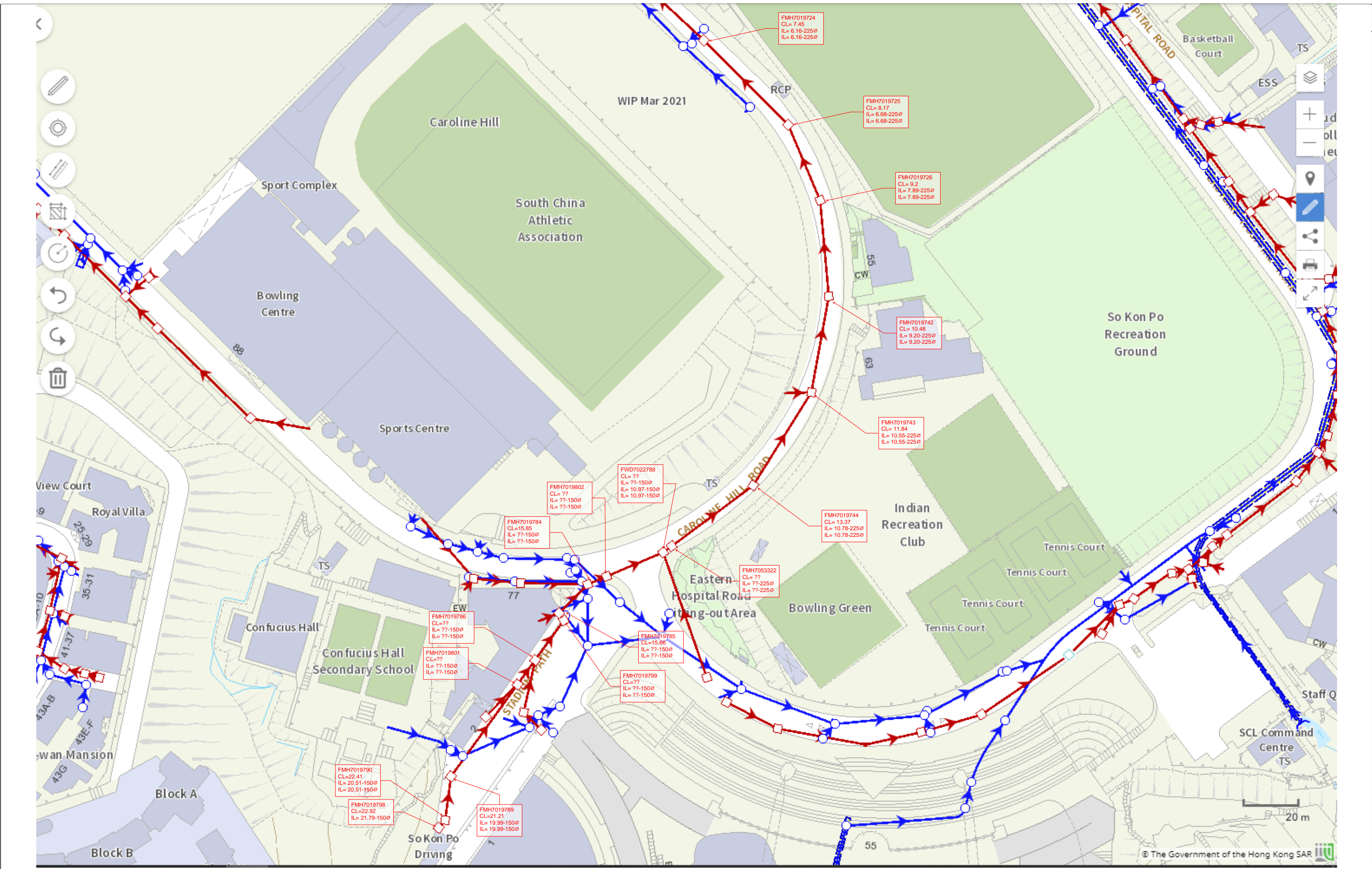
Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
EXISTING SEWERAGE LAYOUT PLAN - SOUTH

ARUP

Drawing Status
FOR REFERENCE

Scale	Job No.	Drawing No.	Rev.
	285077	FIGURE 2	

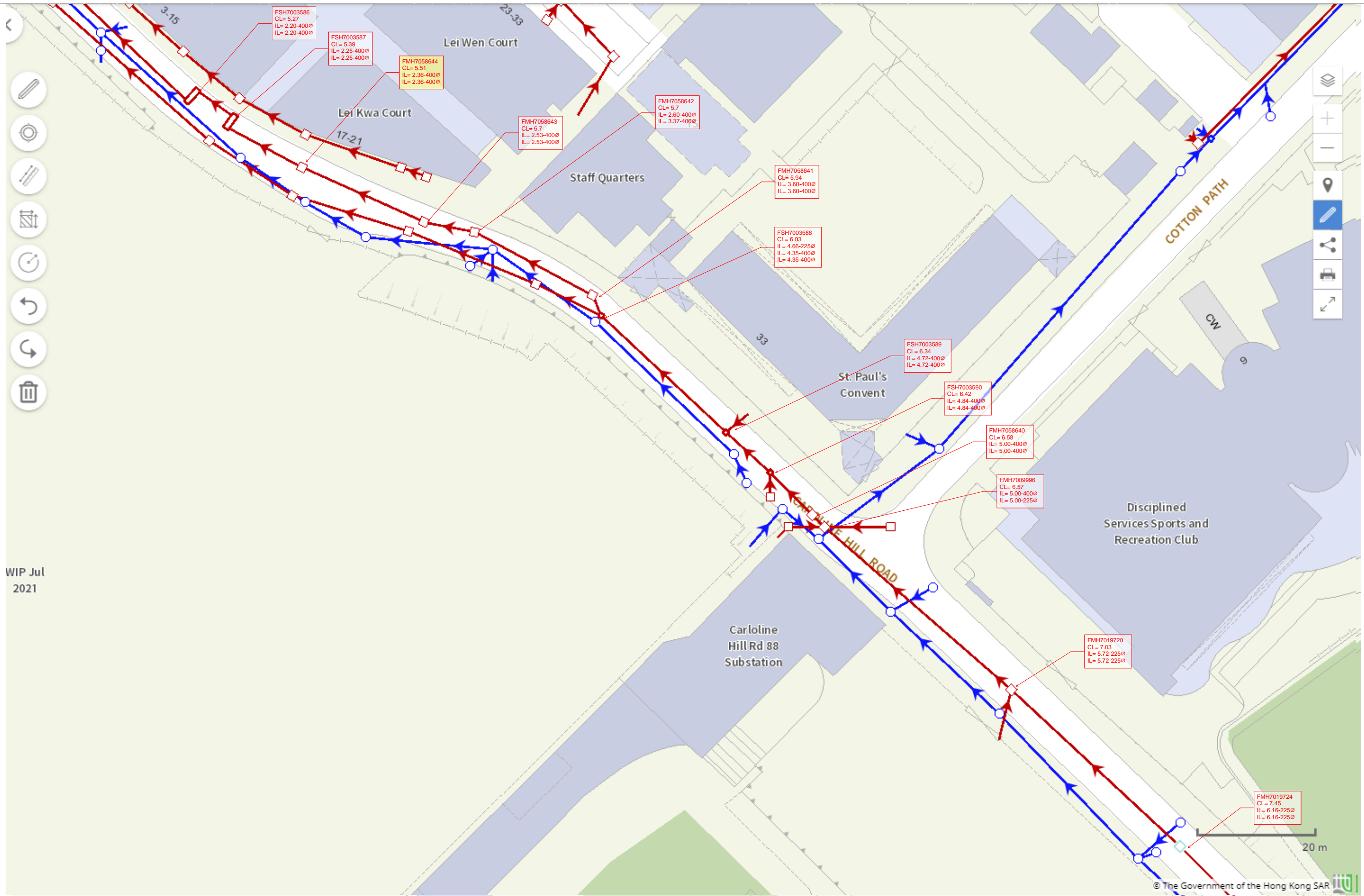


Mark	Date	By	Rev.

Job Title	Proposed Redevelopment at Caroline Hill Road, Causeway Bay
Drawing Title	EXISTING SEWERAGE LAYOUT PLAN - EAST (SHEET 1 OF 3)
Drawing Status	FOR REFERENCE

Scale	
Drn.	Date
Chd.	Passed
Job No.	285077
Drawing No.	FIGURE 3
Rev.	

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2021

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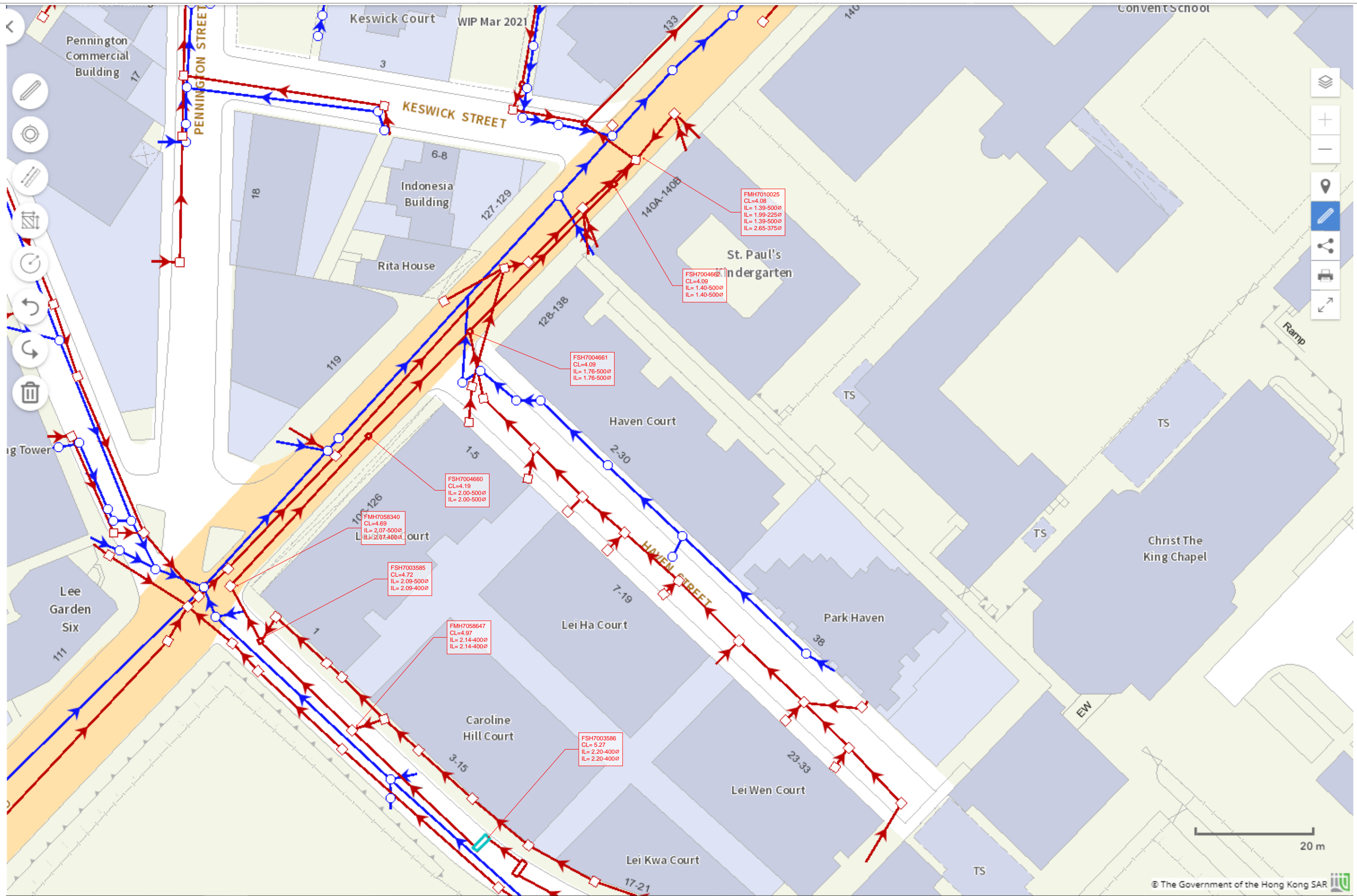
Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
EXISTING SEWERAGE LAYOUT PLAN - EAST (SHEET 2 OF 3)

Drawing Status
FOR REFERENCE

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Scale			
Dwn.	Date	Chd.	Passed
Job No.	Drawing No.		Rev.
285077	FIGURE 4		

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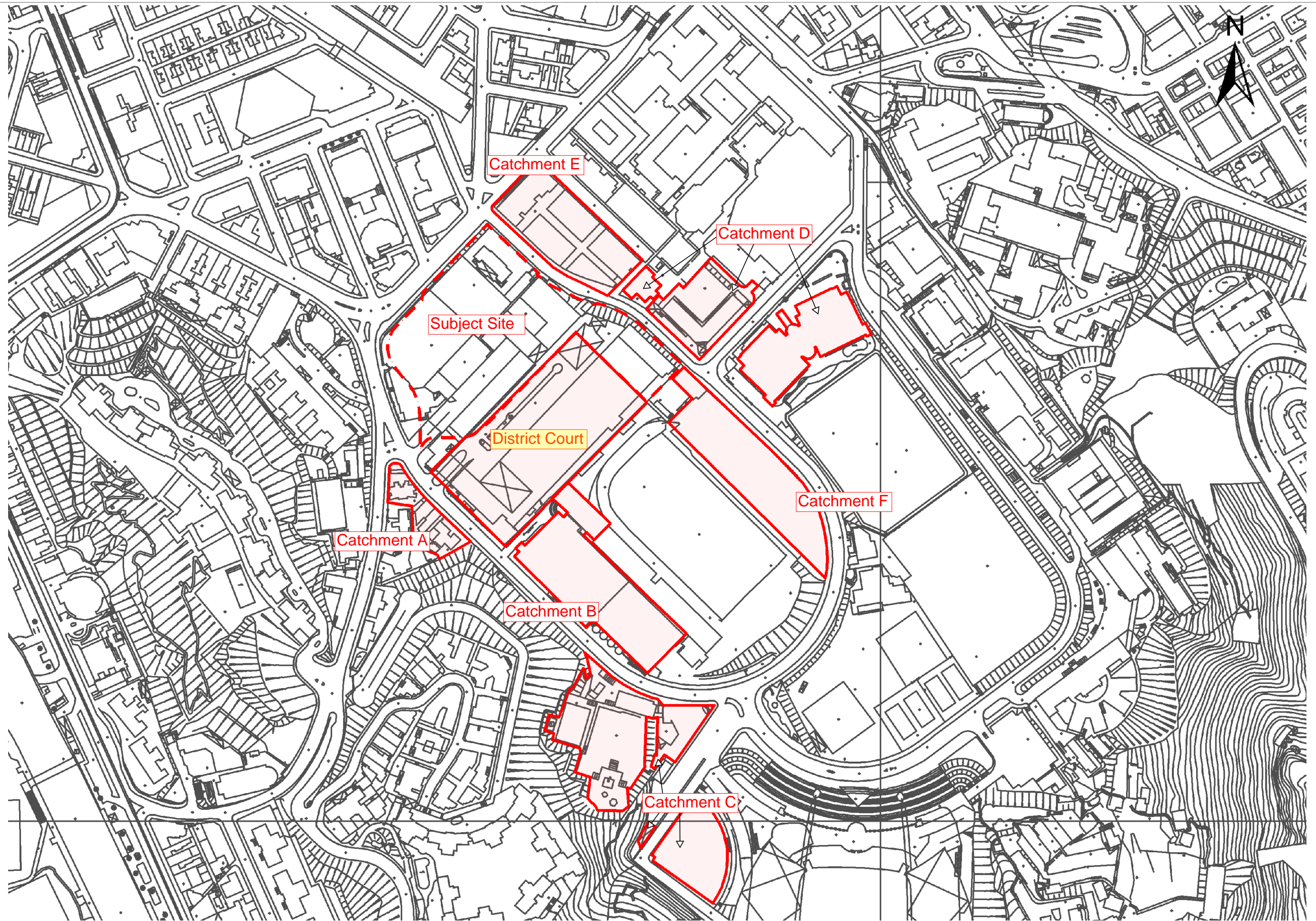
Mark	Date	By	Rev.

Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
EXISTING SEWERAGE LAYOUT PLAN - EAST (SHEET 3 OF 3)

Drawing Status
FOR REFERENCE

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Scale	Date	Chd.	Passed
Job No.	Drawing No.	Rev.	
285077	FIGURE 5		



LEGEND

SEWAGE CATCHMENT

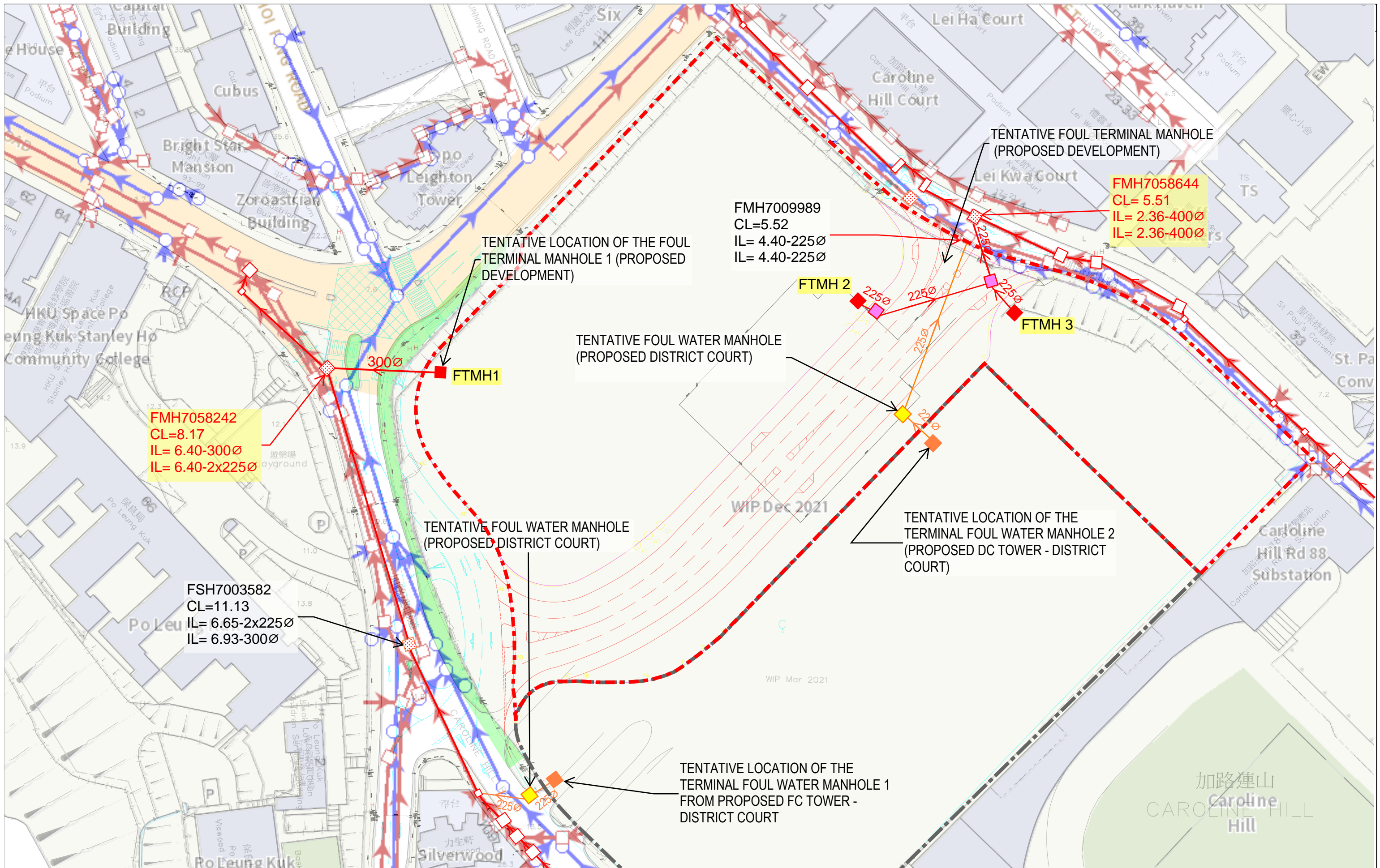
Mark	Date	By	Rev.

Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
SEWAGE CATCHMENT PLAN

ARUP

Scale		Chd.		Passed	
Dwn.	Date				
Drawing Status		Job No.	Drawing No.	Rev.	
FOR REFERENCE		285077	FIGURE 6		



Mark	Date	By	Rev.

LEGEND			
■	PROPOSED TERMINAL FOUL WATER MANHOLE (PROPOSED DEVELOPMENT)	←	PROPOSED SEWER (PROPOSED DEVELOPMENT)
■	PROPOSED TERMINAL FOUL WATER MANHOLE (PROPOSED DISTRICT COURT)	←	PROPOSED SEWER (PROPOSED DISTRICT COURT)
	SITE BOUNDARY OF PROPOSED DISTRICT COURT		SITE BOUNDARY OF PROPOSED DEVELOPMENT
	PROPOSED CONNECTION POINT (PROPOSED DEVELOPMENT)		PROPOSED CONNECTION POINT (PROPOSED DISTRICT COURT)
	PROPOSED FOUL WATER MANHOLE (PROPOSED DISTRICT COURT)		PROPOSED FOUL WATER MANHOLE OF PROPOSED DEVELOPMENT

Job Title
Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Drawing Title
Proposed Connection at Caroline Hill Road, Causeway Bay

Drawing Status
FOR REFERENCE

ARUP			
Scale	Dn. Date Chd. Passed		
Job No. 285077	Drawing No. FIGURE 7	Rev.	

Appendix B

Calculation

TABLE B1
Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption:

Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5
 Global Unit Flow Factors as per Tables T-2 and T-3
 Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

Development Schedule		
Sewage Flow Estimation for Caroline Hill Road - South	Estimation	Remark
Proposed Development		
(S) Subject Site		
GFA (m ²) for Office use	85000	
Worker Density (No. of Worker per 100m ²)	5.5	
No. of Employee	4675	
Unit flow factor (m ³ /person/day) - J6 Financial, Insurance, Real Estate & Business Services	0.08	
GFA (m ²) for F&B	10000	
Worker Density (No. of Worker per 100m ²)	3.5	
No. of Employee	350	
Unit flow factor (m ³ /person/day) - J10 Restaurant & Hotels	1.58	
GFA (m ²) for GIC	5000	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	165	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
Total ADWF, (m ³ /day)	973.2	
70% of Total ADWF, (m ³ /day)	681.2	
Total ADWF, (L/s)	11.26	
70% of Total ADWF, (L/s)	7.88	New Development
Catchment A		
A1 Silverwood		
Number of flats	81	
Population	219	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	59.05	
ADWF, (L/s)	0.68	P_{CF} = 1 included
A2 103 Caroline Hill Road (CHR)		
Number of flats	8	
Population	22	
Unit flow factor (m ³ /person/day) - Residential R2	0.270	
ADWF, (m ³ /day)	5.83	
ADWF, (L/s)	0.07	P_{CF} = 1 included
A3 Caroline Garden		
Number of flats	48	
Population	130	
Unit flow factor (m ³ /person/day) - Residential R2	0.270	
ADWF, (m ³ /day)	34.99	
ADWF, (L/s)	0.41	P_{CF} = 1 included
Catchment B		
B1 Bowling centre		
GFA (m ²)	5704	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	188	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	52.71	
ADWF, (L/s)	0.61	P_{CF} = 1 included
B2 Sport Complex		
GFA (m ²)	8352	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	276	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	77.17	
ADWF, (L/s)	0.89	P_{CF} = 1 included
B3 Sports Centre (50%)		
GFA (m ²)	6351	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	210	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	29.34	
ADWF, (L/s)	0.34	P_{CF} = 1 included
District Court		
District court - DC tower (connected to FSH7003584)		
ADWF, (m ³ /day)	119.46	
ADWF, (L/s)	3.32	According to approved technical feasibility statement from ArchSD
District court - FC tower (connected to FMH7009989)		
ADWF, (m ³ /day)	38.39	
ADWF, (L/s)	1.07	According to approved technical feasibility statement from ArchSD
Proposed Scenario Caroline Hill Road - South		
Total ADWF (m ³ /day)	1060	
Total ADWF (L/s)	12.27	
Contributing Population	3925	
Global Peaking Factor	6.00	
Total Peak Flow (L/s)	85.22	

Notes:
 Employment density shall refer to Commercial and Industrial Floor Space Utilization Survey published by PlanD.
 Office = 5.5 employee per 100m² of GFA
 Retail = 3.5 employee per 100m² of GFA
 Community, Social & Personal Services = 3.3 employee per 100m² of GFA

TABLE B1
Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption:

Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5
 Global Unit Flow Factors as per Tables T-2 and T-3
 Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

Development Schedule		
Sewage Flow Estimation for Caroline Hill Road - East	Estimation	Remark
Proposed Development		
(S) Subject Site		
GFA (m ²) for Office use	85000	
Worker Density (No. of Worker per 100m ²)	5.5	
No. of Employee	4675	
Unit flow factor (m ³ /person/day) - J6 Financial, Insurance, Real Estate & Business Services	0.08	
GFA (m ²) for Retail use	10000	
Worker Density (No. of Worker per 100m ²)	3.5	
No. of Employee	350	
Unit flow factor (m ³ /person/day) - J10 Restaurant & Hotels	1.58	
GFA (m ²) for GIC	5000	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	165	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
Total ADWF, (m³/day)	973.20	
30% of Total ADWF, (m³/day)	291.96	
Total ADWF, (L/s)	11.26	
30% of Total ADWF, (L/s)	3.38	New Development
Catchment B		
B3 Sports Centre (50%)		
GFA (m ²)	6351	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	210	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	29.34	
ADWF, (L/s)	0.34	P_{CIF} = 1 included
Catchment C		
C1 Confucius Hall Secondary School		
Number of students	360	
Number of staffs	29	
Unit flow factor (m ³ /person/day) - students	0.04	
Unit flow factor (m ³ /person/day) - staffs	0.28	
ADWF, (m ³ /day)	22.52	
ADWF, (L/s)	0.26	P_{CIF} = 1 included
C2 So Kon Po Driving Test Centre		
GFA (m ²)	357	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	12	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	3.30	
ADWF, (L/s)	0.04	P_{CIF} = 1 included
C3 Olypmic House		
GFA (m ²)	4343	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	143	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	40.13	
ADWF, (L/s)	0.46	P_{CIF} = 1 included

TABLE B1
Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption:

Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5

Global Unit Flow Factors as per Tables T-2 and T-3

Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

Development Schedule		
Sewage Flow Estimation for Caroline Hill Road - East	Estimation	Remark
Catchment D		
D1 Disciplined Services Sports and Recreation Club		
GFA (m ²)	10440	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	345	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	96.47	
ADWF, (L/s)	1.12	P_{CI}F = 1 included
D2 St. Paul Convent		
GFA (m ²)	1528	
Worker Density (No. of Worker per 100m ²)	3.3	
No. of Employee	50	
Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services	0.28	
ADWF, (m ³ /day)	14.12	
ADWF, (L/s)	0.16	P_{CI}F = 1 included
Staff Quarters (D)		
D3 Number of units	25	
Population	68	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	18.23	
ADWF, (L/s)	0.21	P_{CI}F = 1 included
Catchment E		
E1 Leishun Court		
Number of flats	120	
Population	324	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	87.48	
ADWF, (L/s)	1.01	P_{CI}F = 1 included
E2 Caroline Hill Court		
Number of flats	1146	
Population	3094	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	835.43	
ADWF, (L/s)	9.67	P_{CI}F = 1 included
E3 Lei Kwa Court		
Number of flats	56	
Population	151	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	40.82	
ADWF, (L/s)	0.47	P_{CI}F = 1 included
E4 Lei Ha Court		
Number of flats	120	
Population	324	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	87.48	
ADWF, (L/s)	1.01	P_{CI}F = 1 included
E5 Lei Wen Court		
Number of flats	146	
Population	394	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	106.43	
ADWF, (L/s)	1.23	P_{CI}F = 1 included
Catchment F		
F1 Staff Quarters		
Number of units	35	
Population	95	
Unit flow factor (m ³ /person/day) - Residential R2	0.27	
ADWF, (m ³ /day)	25.52	
ADWF, (L/s)	0.30	P_{CI}F = 1 included
Proposed Scenario Caroline Hill Road - East		
Total ADWF (m ³ /day)	1655	
Total ADWF (L/s)	19.16	
Contributing Population	6131	
Global Peaking Factor	5.00	
Total Peak Flow (L/s)	95.80	

Notes:

Employment density shall refer to Commercial and Industrial Floor Space Utilization Survey published by PlanD.

Office = 5.5 employee per 100m² of GFA

Retails = 3.5 employee per 100m² of GFA

Community, Social & Personal Services = 3.3 employee per 100m² of GFA

Appendix C

Preliminary Design of sewerage
system from District Court
Complex at Caroline Hill Road,
ArchSD

Hydraulic Calculation of Foul Water Connection Pipe (for information only)

	Population density (approx.)	Daily Sewage discharged capacity/ head (liter)	Flow Period in a day (hour)	DWF (m3/day)	DWF (lit/sec)	Peak Factor	Peak Discharge Flow Rate (lit/sec)
Tower DC	2172	55	10	119.46	3.32	6	19.91
Tower FC	698	55	10	38.39	1.07	6	6.40
Total							26.31

By Manning Formula

$$V = \left(\frac{HMD^{2/3} \times S^{1/2}}{n} \right)$$

Where:

- V = mean velocity (m/s)
- s = slope of the total energy line
- n = Manning's roughness coefficient
(For Vitrified Clay pipe: n = 0.012 value)

HMD = Hydraulic Mean Depth (HMD = 0.25 x d @ 1/2 bore capacity)

A = Sectional Area (0.393 x d² @ 1/2 bore capacity)

For Ductile Iron Pipe in Tower DC : Internal Diameter = 225mm

HMD	0.05625
Pipe size	225 m
Slope of the total energy line	0.01
Manning's roughness coefficient, n	0.012 (for Ductile Iron pipe)
V=	1.223 m/s

Q= V x A
 where
 Q = discharge flow rate capacity (l/s)
 A = area of pipe work (m2)
 24.31 l/s

According to the calculation (1/2 bore capacity)
 24.31 l/s > 19.91 l/s

Thus, a 225mm Ductile Iron pipe with gradient 1:100 shall cater the site discharge 1/2 bore capacity at 1.223m/s velocity.

For Ductile Iron Pipe in Tower FC : Internal Diameter = 225mm

HMD	0.05625
Pipe size	225 m
Slope of the total energy line	0.01
Manning's roughness coefficient, n	0.012 (for Ductile Iron pipe)
V=	1.223 m/s

Q= V x A
 where
 Q = discharge flow rate capacity (l/s)
 A = area of pipe work (m2)
 24.31 l/s

According to the calculation (1/2 bore capacity)
 24.31 l/s > 6.40 l/s

200mm Ductile Iron pipe with gradient 1:100 shall cater the site discharge 1/2 bore capacity at 1.131m/s velocity.

The connection to DSD manhole shall be 225mm at least. Thus, 225mm discharge pipe shall be connected to DSD government manhole.

For Ductile Iron Pipe in Tower FC : Internal Diameter = 200mm

HMD	0.05
Pipe size	200 m
Slope of the total energy line	0.01
Manning's roughness coefficient, n	0.012 (for Ductile Iron pipe)
V=	1.131 m/s

Q= V x A
 where
 Q = discharge flow rate capacity (l/s)
 A = area of pipe work (m2)
 17.76 l/s

According to the calculation (1/2 bore capacity)
 17.76 l/s > 6.40 l/s

100mm	1:40
150mm	1:70
200mm	1:70
225mm	1:100
250mm	1:120
300mm	1:150

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