10 seway Bay

Annex E Sewerage Impact Assessment



Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Sewerage Impact Assessment for Proposed Redevelopment at Caroline Hill Road, Causeway Bay

Report Ref

05 | 16 January 2024

This report takes into account the particular instructions and requirements of our client.

It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

Job number 285077

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Preliminary Design of Sewerage System from District Court Complex at Caroline Hill Road, ArchSD

1 Introduction

Sewerage Impact Assessment ("SIA") had been submitted in fulfilment of the Special Conditions (61) (a) of the Conditions of Sale of the Lot requiring for submission of a SIA and was approved by EPD and DSD on 5 January 2022 and 3 May 2022 respectively.

This revised SIA had been submitted to support the Fresh S16 Planning Application with the revised layout plan submission. The recommendation established in the previously approved SIA remains unchanged.

1.1 Reference Materials

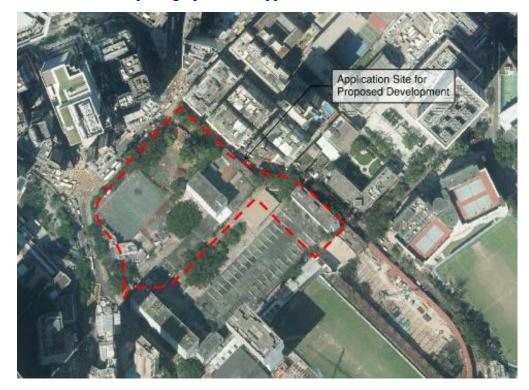
In evaluating the sewerage impact arising from the proposed development, the following sources of information have been specifically referred to:

- Environmental Protection Department (EPD) Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning No. : EPD/TP 1/05;
- DSD Sewerage Manual Key Planning Issues and Gravity Collection System; and
- Drainage Record Plans obtained from DSD.

2 The Development

For easy reference, a comparison table showing the difference between the Approved Scheme and the Proposed Scheme is shown in table below:

| Description | Approved Scheme | Proposed Scheme | Difference | | | | | |
|------------------------------------------------------|------------------------------------------------|-----------------------------------------------------------------------------------------------------------------------------------------------|-----------------------|--|--|--|--|--|
| Project Title | Proposed Redevelop Causeway Bay | Proposed Redevelopment at Caroline Hill Road, Causeway Bay | | | | | | |
| Description | • | Two 24-storeys office towers (Tower 1 and Tower 2) and one 18-storeys office tower (Tower 3) for office, retail and GIC facilities use. | | | | | | |
| Location | The site is located at (see Figure 1). | The site is located at Caroline Hill Road, Causeway Bay (see Figure 1). | | | | | | |
| Land Use Zoning | Commercial | Commercial | | | | | | |
| Site Area | 14,802 m ² | 14,802 m ² | Nil | | | | | |
| Office GFA | 85,000 m ² | 85,000 m ² | Nil | | | | | |
| Retail GFA | 10,000 m ² | 10,000 m ² | Nil | | | | | |
| GIC GFA | 5,000 m ² | 5,000 m ² | Nil | | | | | |
| GIC GFA (Performing Art & Cultural Facilities) | 2,000 m ² | - | -2,000 m ² | | | | | |



Below is an aerial photograph of the Application Site.

3 Description of Existing Environment and Baseline Conditions

3.1 Site Location and Topography

The Application site is located at Causeway Bay at the junction of Caroline Hill Road and Leighton Road. The Application Site covers approximately 14802m² of area. It was occupied by the ex-Electrical and Mechanical Services Department (EMSD) Headquarters, the ex-Civil Aid Service Headquarters, the ex-Post Office Recreation Club and the PCCW Recreation Club.

3.2 Approach and Methodology

The sewage generated from the proposed development will be collected and conveyed to the existing sewerage system via the existing manhole FMH7058242 and FMH7058644. In order to assess the associated sewerage impact, the capacities of the existing public sewers along Caroline Hill Road were checked.

The adequacy of sewerage capacity along Caroline Hill Road was determined by using the estimate of future sewage generation from the proposed development.

The global unit flow factors recommended in the EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning No.: EPD/TP 1/05 have been adopted in the estimation.

3.3 Evaluation of Sewerage Impact

The sewage flow estimation from the proposed development, the unit flow factors as stated in EPD/TP 1/05 have been adopted.

The capacities of sewers have been calculated based on Colebrook-White equation, assuming full bore flow as below:

(a) Pipe capacity is calculated based on the continuity equation

Q = AV

Where Q = pipe full flow capacity in m³/s

A = pipe cross-sectional area in m²

- V = velocity at full bore flow in m/s
- (b) Velocity at full bore flow is based on the Colebrook-White equation

 $V = -(32gRS)^{0.5} \log \{(ks/14800R) + (1.255v/R (32gRS)^{0.5})\}$

Where g = acceleration due to gravity in m/s^2

R = hydraulic radius in m

S = pipe gradient

ks = pipe roughness in mm

v = kinematic viscosity of water in m²/s

- (c) The sewerage impact due to the proposed development on the two existing public sewers have been evaluated by calculating the estimated peak flow against the capacity of the existing public sewer. One is the existing public sewer with downstream sewer of 600Ø run along the South of Caroline Hill Road. Another one is the existing public sewer with downstream sewer of 500Ø run along the East of Caroline Hill Road.
- (d) The detailed calculations are provided in **Appendix B** for reference.

3.4 Sewage Generation from the Proposed Development

The prediction for the proposed development sewage generation has been based on the information extracted from the development schedule (refer to Section 2). The quantity of sewage generated by the proposed is estimated from expected total population.

The table showing the sewage generation of the proposed development is calculated based on the guideline set in EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning and is shown on **Table B1** in **Appendix B** with the comparison between the Approved Scheme and the Proposed Scheme shown in the table below.

| Description | Approved Scheme | Proposed Scheme | Difference |
|----------------------------|--------------------|--------------------|--------------------|
| ADWF (m ³ /day) | <mark>991.7</mark> | <mark>973.2</mark> | <mark>-18.5</mark> |
| Catchment Inflow Factor | <mark>1.0</mark> | <mark>1.0</mark> | Nil |
| Contributing Population | <mark>3673</mark> | <mark>3604</mark> | <mark>-69</mark> |
| Global Peaking Factor | <mark>6</mark> | <mark>6</mark> | Nil |
| Peak Discharge, L/s | <mark>68.87</mark> | <mark>67.58</mark> | <mark>-1.29</mark> |

3.5 Impact of the Proposed Development

In order to assess the impact on the existing public sewer associated with the proposed development, the capacities of the existing public sewers have been checked and shown on **Tables B2** in **Appendix B**.

The estimation of sewage generation in the vicinity of the Application Site is based on the assumptions as below:

- 1) Existing public sewer information based on DSD drainage record plans and shown in **Figure 2-5**;
- 2) Existing development parameters in the vicinity of the proposed development are obtained from public domain and sewerage catchment plan shown in **Figure 6**;
- 3) Flow factors as per EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning; and
- 4) Global peaking factor with stormwater allowance is adopted as per EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.
- 5) The sewage flow and tentative sewerage system from Proposed District Court Complex is based on the design reference to the approved technical feasibility statement from ArchSD and shown in **Figure 7**.
- 6) As per ArchSD's current design, the sewer of FC tower is recommended to be connected to the existing sewerage manhole FSH7003584 and that of DC tower is recommended to be connected to the existing sewerage manhole FMH7009989.

The peak sewage flow from the proposed development is slightly reduced from 68.87 L/s to 67.58 L/s.

On the South side of Caroline Hill Road, it has proven that an existing public sewerage serving the Application Site comprising an existing 300Ø public gravity sewer running along the south of Caroline Hill Road and the downstream existing public sewer of 600Ø running along the Leighton Road has sufficient capacity to carry the estimated sewage from the Application Site.

On the East side of Caroline Hill Road, it has proven that an existing 400Ø public gravity sewer running along the east of Caroline Hill Road and the downstream

existing public sewer of 500Ø running along Leighton Road has sufficient capacity to carry the estimated sewage from the Application Site.

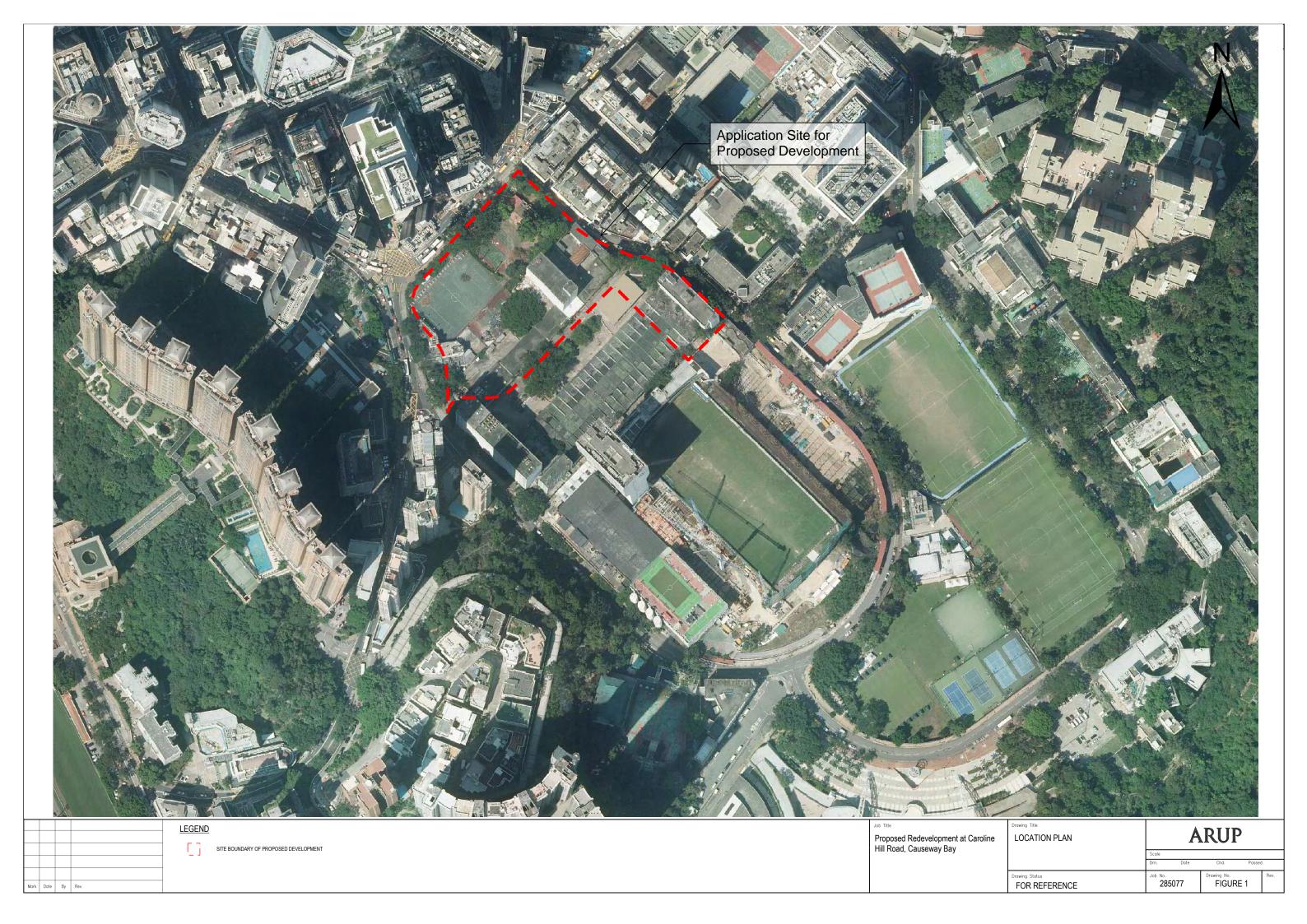
It is concluded that the proposed development would not result in any adverse sewerage impact to the existing public sewerage system. The capacities checking of the existing public sewers is shown in **Table B2**.

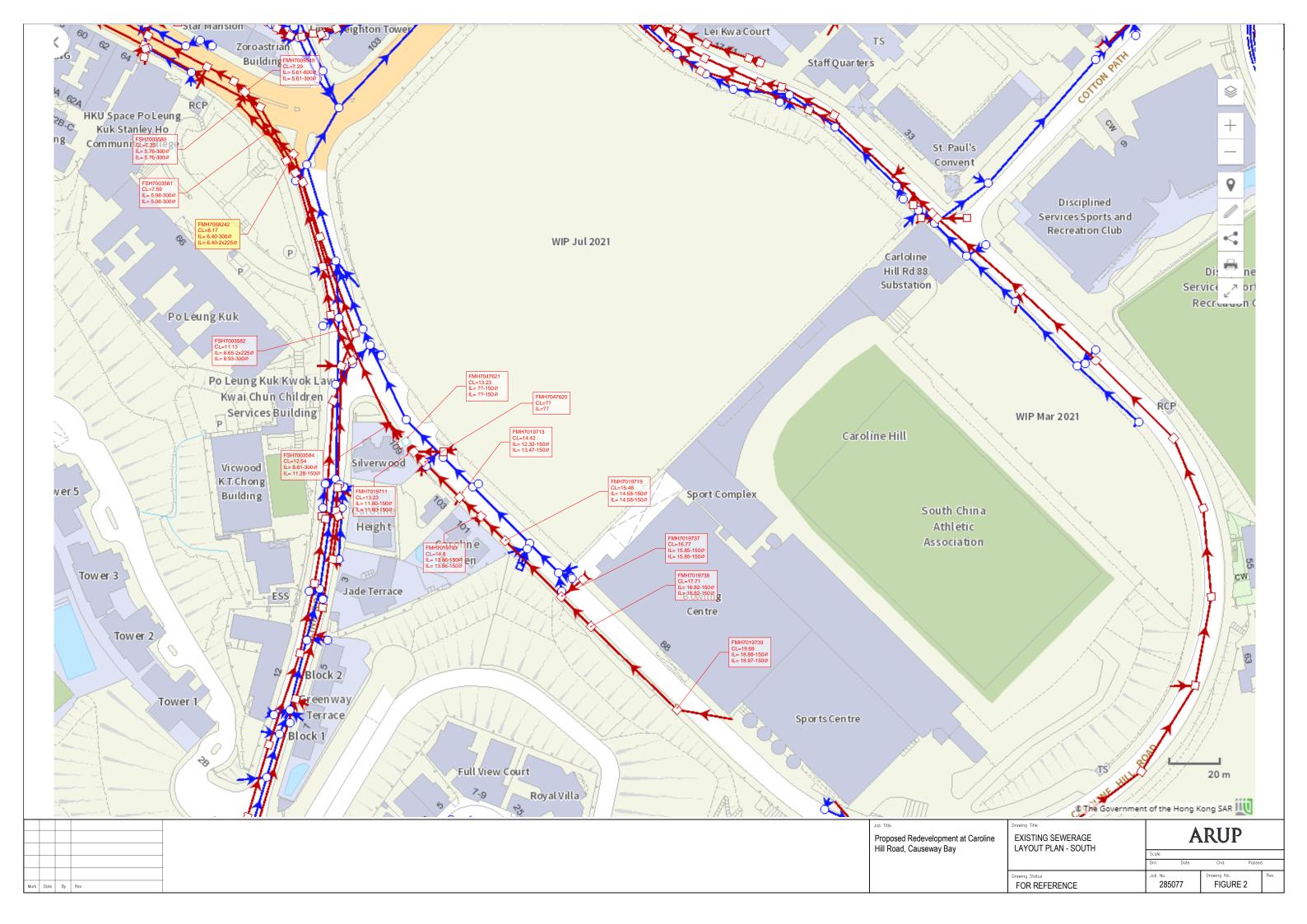
4 Conclusion

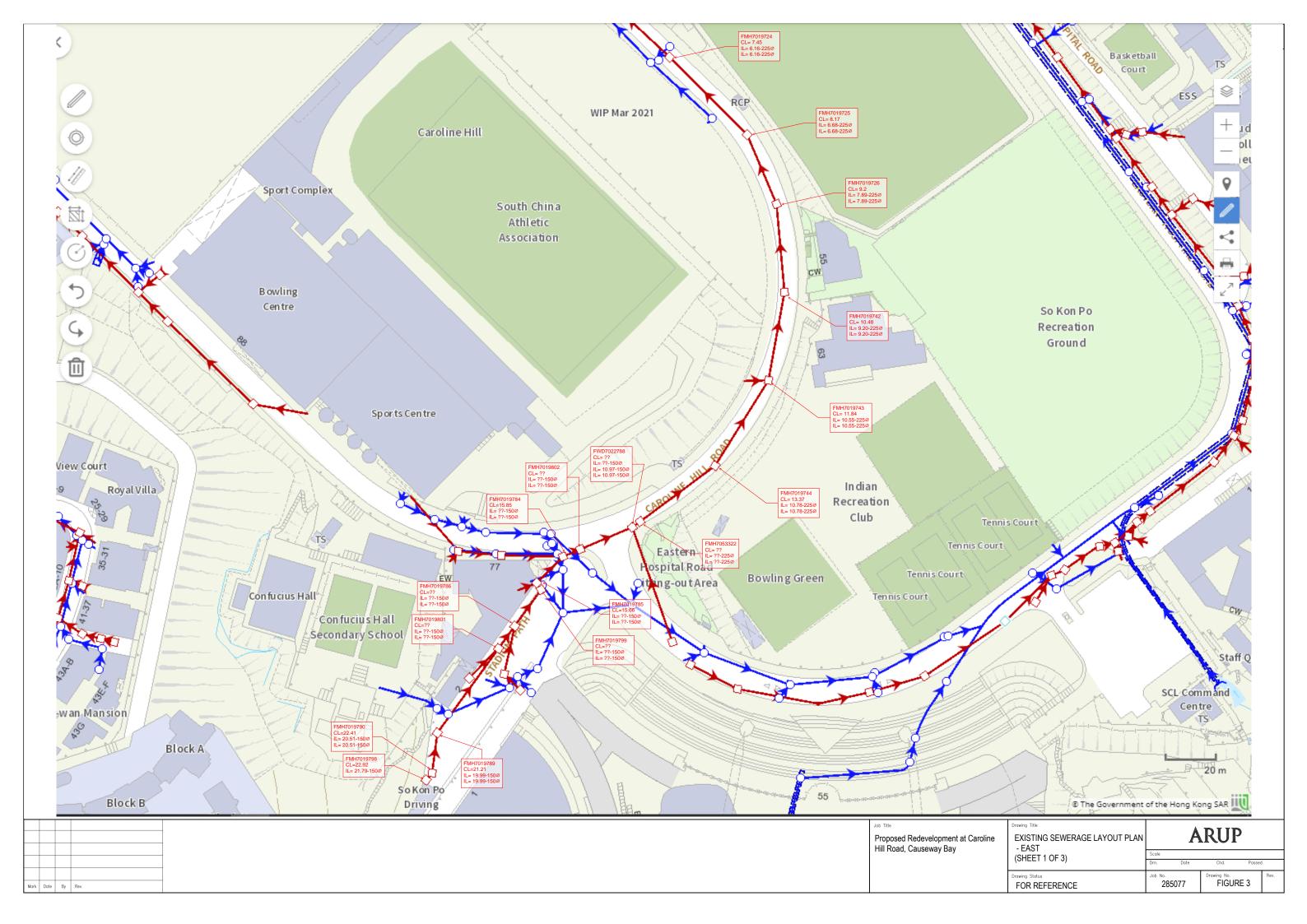
The peak sewage flow from the proposed development is slightly reduced from 68.87 L/s to 67.58 L/s. It is observed that the two existing public sewerage serving the Application Site through existing FMH7058644 and FMH7058242 running along the Leighton Road, have sufficient capacity to carry the estimated sewage from the Application Site. It is concluded that the proposed development would not result in any adverse sewerage impact to the existing public sewerage system.

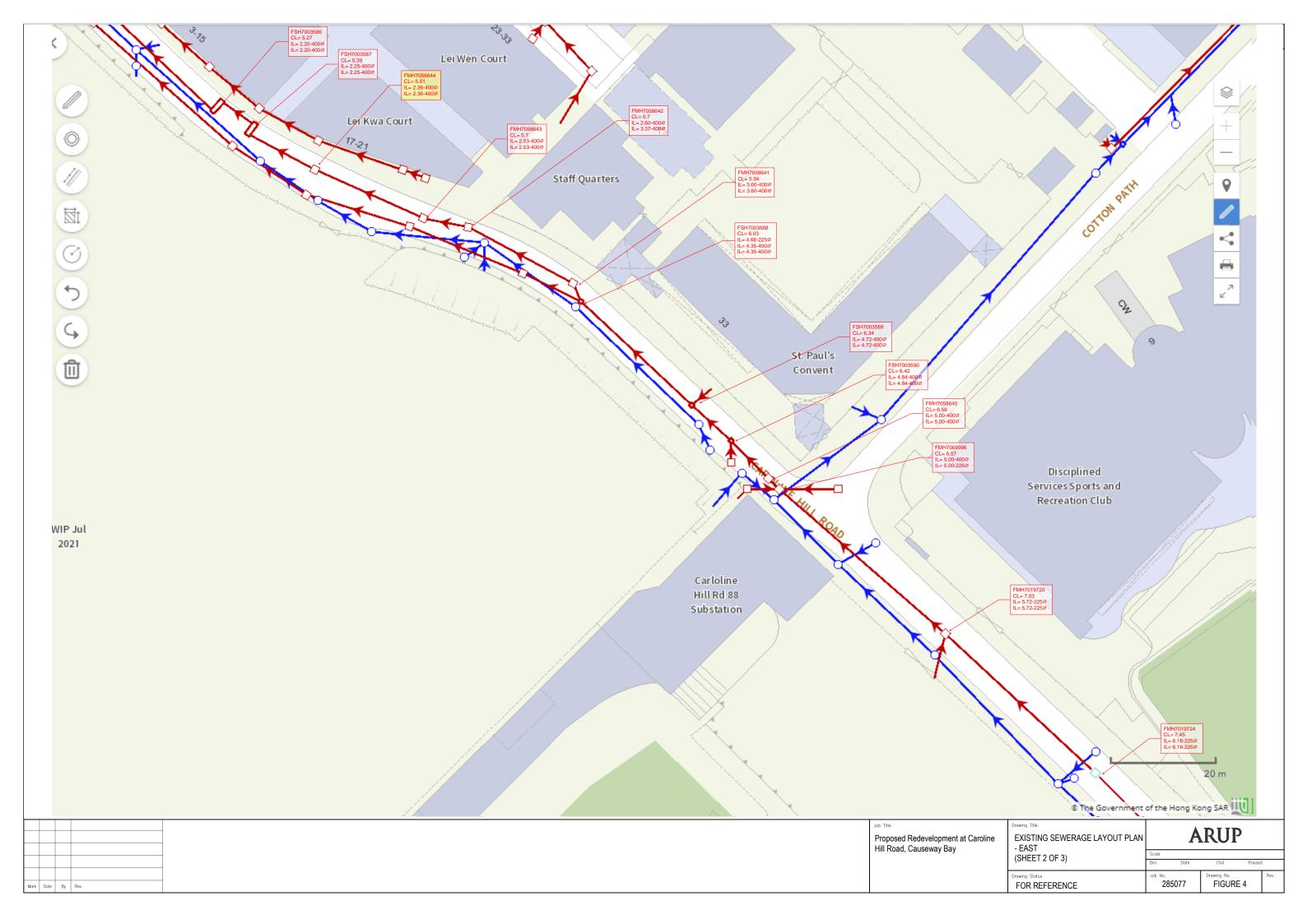
Appendix A

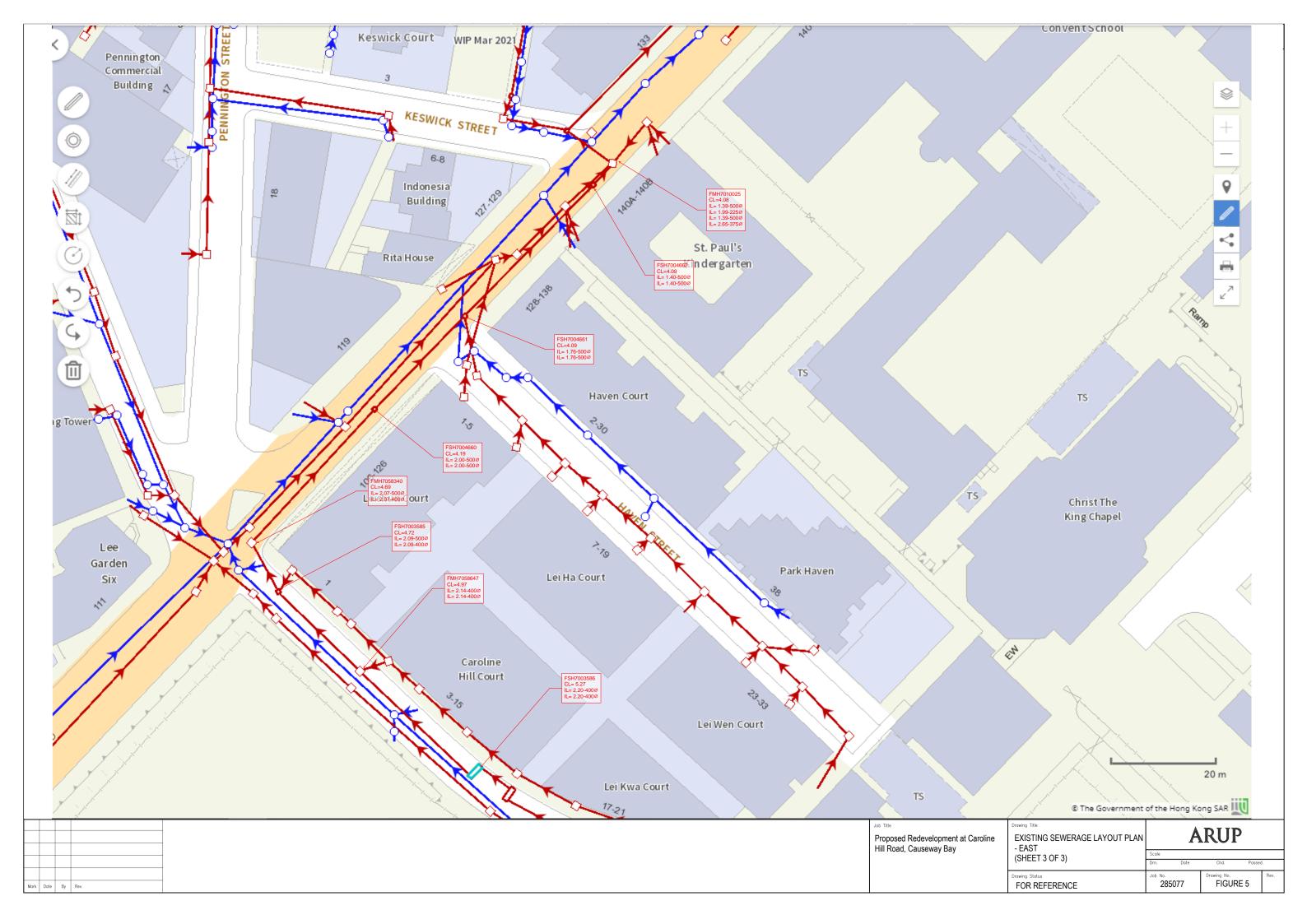
Figures

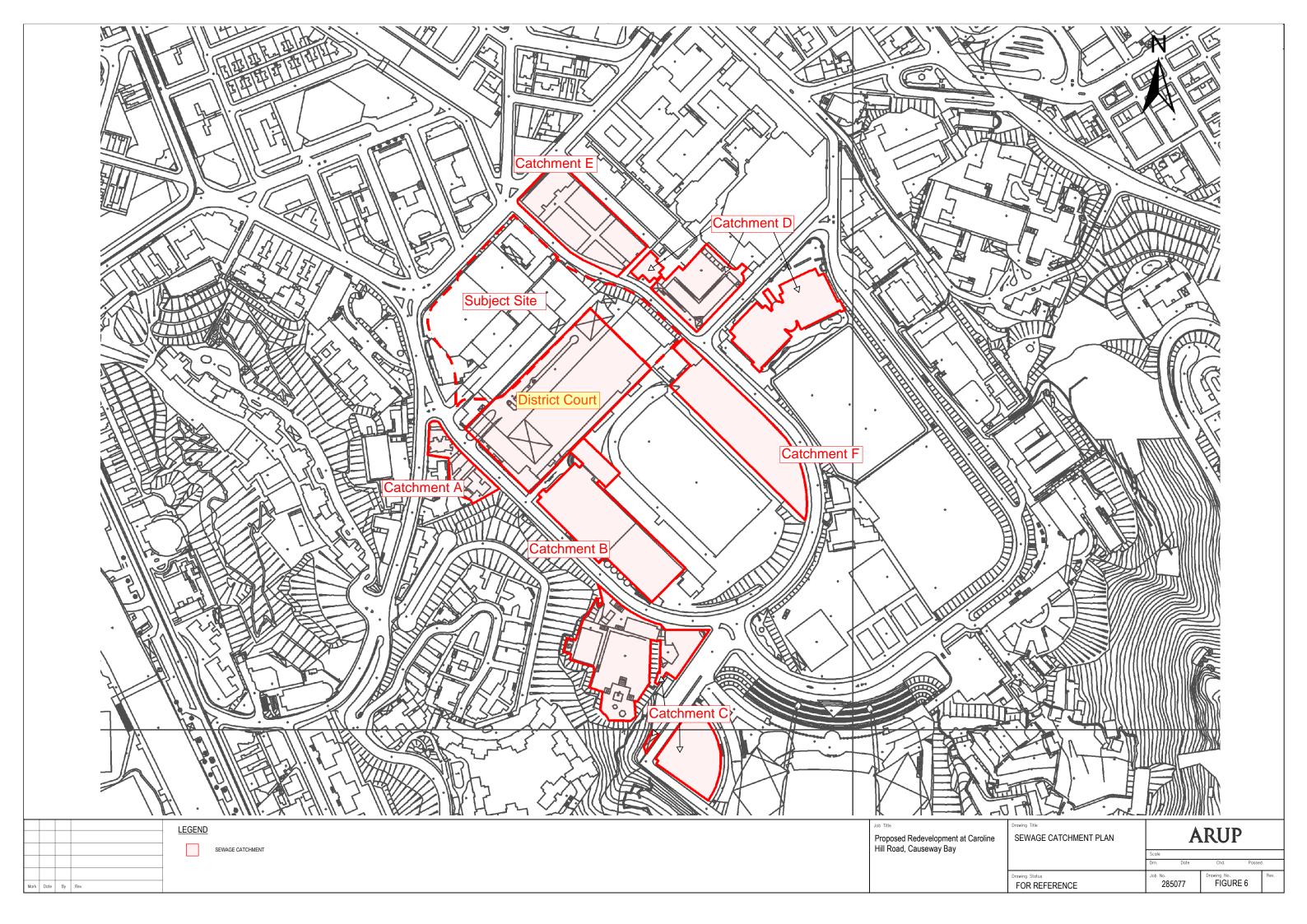


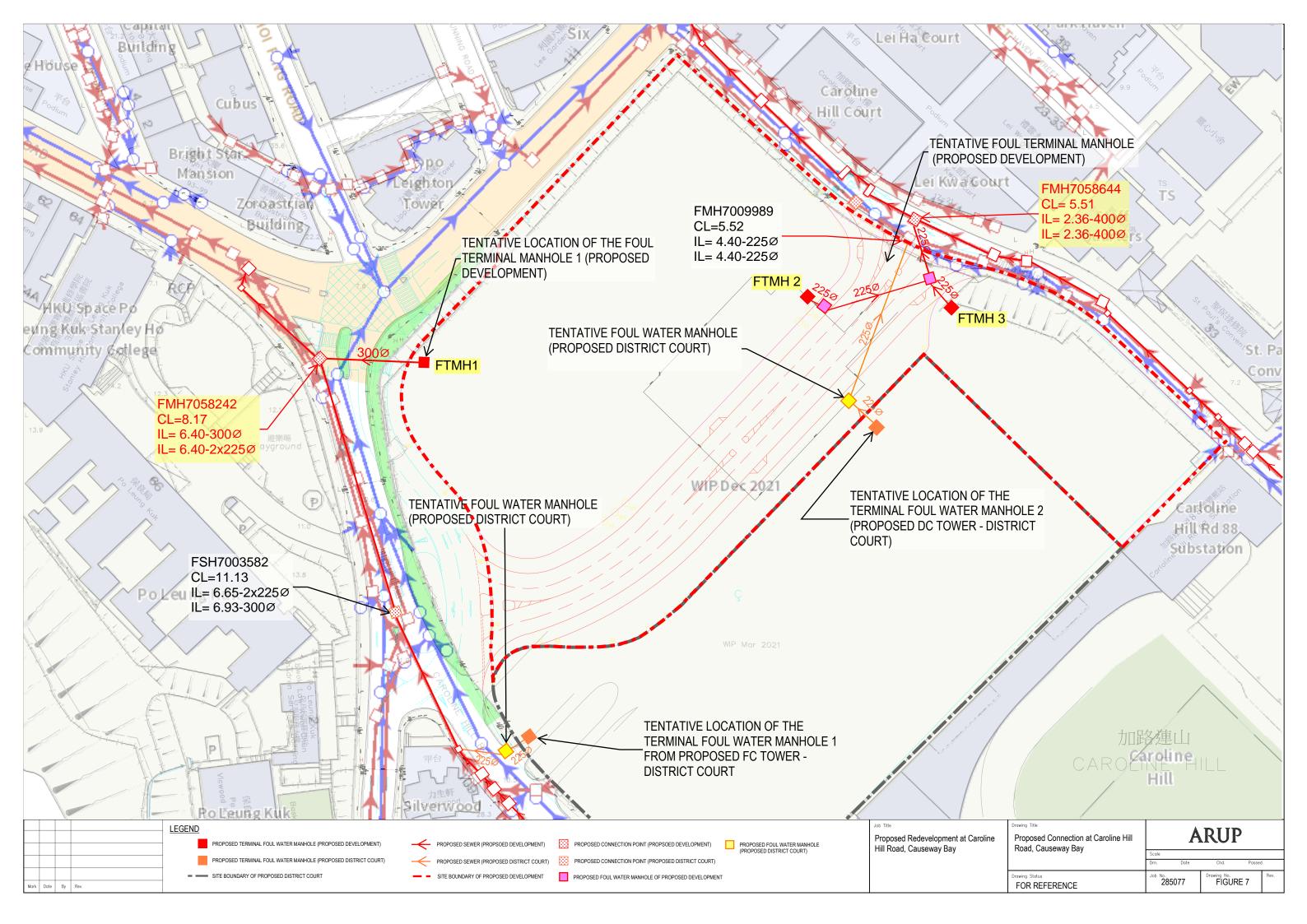












Appendix B

Calculation

| ARUP | Ove Arup & Partners Calculation Sheet | Job No. | 285077 | Sheet No. | 1 | Rev. | 5 | |
|-----------|---------------------------------------|---------|--------|-----------|----------|---------|----|--|
| Job Title | Caroline Hill Road, Causeway Bay | Made by | IP | Date | 16/01/24 | Checked | CC | |

TABLE B1

Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption: Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5 Global Unit Flow Factors as per Tables T-2 and T-3 Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

| Deve | Iopment Schedule | | |
|--------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------|-------------------------------------------------------------------|
| Sewaę | ge Flow Estimation for Caroline Hill Road - South | Estimation | Remark |
| Propo | sed Development | | |
| (S) | Subject SiteGFA (m²) for Office useWorker Density (No. of Worker per 100m²)No. of EmployeeUnit flow factor (m³/person/day) - J6 Financial, Insurance, Real Estate & Business Services | 85000 5.5 4675 0.08 | |
| | GFA (m ²) for F&B Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J10 Restaurant & Hotels | 10000 3.5 350 1.58 | |
| | GFA (m ²) for GIC Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services | 5000 3.3 165 0.28 | |
| Catabi | Total ADWF, (m ³ /day) 70% of Total ADWF, (m ³ /day) Total ADWF, (L/s) 70% of Total ADWF, (L/s) | 973.2 681.2 11.26 7.88 | New Development |
| Jatchi A1 | ment A Silverwood | | |
| | Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 81 219 0.27 59.05 0.68 | P _{CIF} = 1 included |
| A2 | 103 Caroline Hill Road (CHR) Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 8 22 0.270 5.83 0.07 | P _{CIF} = 1 included |
| A 3 | Caroline Garden Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 48 130 0.270 34.99 0.41 | P _{CIF} = 1 included |
| Jatchr 31 | ment B Bowling centre | | |
| | GFA (m ²) Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services ADWF, (m ³ /day) ADWF, (L/s) | 5704 3.3 188 0.28 52.71 0.61 | P _{CIF} = 1 included |
| B2 | Sport Complex GFA (m ²) Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services ADWF, (m ³ /day) ADWF, (L/s) | 8352 3.3 276 0.28 77.17 0.89 | P _{CIF} = 1 included |
| B3 | Sports Centre (50%) GFA (m ²) Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services ADWF, (m ³ /day) ADWF, (L/s) | 6351 3.3 210 0.28 29.34 0.34 | P _{CIF} = 1 included |
| ISTRIC | t Court District court - DC tower (connected to FSH7003584) ADWF, (m ³ /day) ADWF, (L/s) | 119.46 3.32 | According to approved technical feasibility statement from ArchSI |
| Propos | District court - FC tower (connected to FMH7009989) ADWF, (m³/day) ADWF, (L/s) sed Scenario Caroline Hill Road - South | 38.39 1.07 | According to approved technical feasibility statement from ArchSI |
| | Total ADWF (m ³ /day) Total ADWF (L/s) Contributing Population Global Peaking Factor Total Peak Flow (L/s) | 1060 12.27 3925 6.00 85.22 | |

Notes: Employment density shall refer to Commercial and Industrial Floor Space Utilization Survey published by PlanD. Office = 5.5 employee per $100m^2$ of GFA Retails = 3.5 employee per $100m^2$ of GFA Community, Social & Personal Services = 3.3 employee per $100m^2$ of GFA

| ARUP | Ove Arup & Partners Calculation Sheet | Job No. | 285077 | Sheet No. | 2 | Rev. | 5 | |
|-----------|---------------------------------------|---------|--------|-----------|----------|---------|----|--|
| Job Title | Caroline Hill Road, Causeway Bay | Made by | IP | Date | 16/01/24 | Checked | CC | |

TABLE B1

Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption:

Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5 Global Unit Flow Factors as per Tables T-2 and T-3 Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

| Sewa | ge Flow Estimation for Caroline Hill Road - East | Estimation | Remark |
|-------|----------------------------------------------------------------------------------------------------------|------------|-------------------------------|
| Drong | and Development | | |
| | osed Development | | |
| S) | Subject Site GFA (m ²) for Office use | 85000 | |
| | | | |
| | Worker Density (No. of Worker per 100m ²) | 5.5 | |
| | No. of Employee | 4675 | |
| | Unit flow factor (m ³ /person/day) - J6 Financial, Insurance, Real Estate & Business Services | 0.08 | |
| | GFA (m ²) for Retail use | 10000 | |
| | Worker Density (No. of Worker per 100m ²) | 3.5 | |
| | No. of Employee | 350 | |
| | Unit flow factor (m ³ /person/day) - J10 Restaurant & Hotels | 1.58 | |
| | GFA (m ²) for GIC | 5000 | |
| | Worker Density (No. of Worker per 100m ²) | 3.3 | |
| | No. of Employee | 165 | |
| | | | |
| | Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services | 0.28 | |
| | Total ADWF, (m ³ /day) | 973.20 | |
| | 30% of Total ADWF, (m ³ /day) | 291.96 | |
| | Total ADWF, (L/s) | 11.26 | |
| | 30% of Total ADWF, (L/s) | 3.38 | New Development |
| | | | |
| 33 | Sports Centre (50%) | | |
| | GFA (m ²) | 6351 | |
| | Worker Density (No. of Worker per 100m ²) | 3.3 | |
| | No. of Employee | 210 | |
| | Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services | 0.28 | |
| | ADWF, (m ³ /day) | 29.34 | |
| | ADWF, (L/s) | 0.34 | P _{CIF} = 1 included |
| | iment C | | |
| :1 | Confucius Hall Secondary School | | |
| | Number of students | 360 | |
| | Number of staffs | 29 | |
| | Unit flow factor (m ³ /person/day) - students | 0.04 | |
| | Unit flow factor (m ³ /person/day) - staffs | 0.28 | |
| | ADWF, (m ³ /day) | 22.52 | |
| | ADWF, (L/s) | 0.26 | P _{CIF} = 1 included |
| 2 | So Kon Po Driving Test Centre | | |
| - | GFA (m ²) | 357 | |
| | Worker Density (No. of Worker per 100m ²) | 3.3 | |
| | | 12 | |
| | No. of Employee | | |
| | Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services | 0.28 | |
| | ADWF, (m ³ /day) | 3.30 | |
| | ADWF, (L/s) | 0.04 | P _{CIF} = 1 included |
| 3 | Olypmic House | | |
| | GFA (m ²) | 4343 | |
| | Worker Density (No. of Worker per 100m ²) | 3.3 | |
| | No. of Employee | 143 | |
| | Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services | 0.28 | |
| | ADWF, (m ³ /day) | 40.13 | |
| | | | |

| ARUP | Ove Arup & Partners Calculation Sheet | Job No. | 285077 | Sheet No. | 2 | Rev. | 5 | |
|-----------|---------------------------------------|---------|--------|-----------|----------|---------|----|--|
| Job Title | Caroline Hill Road, Causeway Bay | Made by | IP | Date | 16/01/24 | Checked | CC | |

TABLE B1

Sewage Flow Estimation for Proposed Development

(Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning)

Design Assumption:

Global Peaking Factor, P (Including Stormwater Allowance) as per Table T-5 Global Unit Flow Factors as per Tables T-2 and T-3

Catchment Inflow Factor for Wan Chai (PCIF = 1.0) as per Table T-4

| Deve | elopment Schedule | | |
|-------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------|-------------------------------|
| | ge Flow Estimation for Caroline Hill Road - East | Estimation | Remark |
| Sewa | ge Flow Estimation for Caroline Hill Road - East | Estimation | Remark |
| | ment D | | |
| D1 | Disciplined Services Sports and Recreation Club GFA (m ²) Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services ADWF, (m ³ /day) ADWF, (L/s) | 10440 3.3 345 0.28 96.47 1.12 | P _{CIF} = 1 included |
| D2 | St. Paul Convent GFA (m ²) Worker Density (No. of Worker per 100m ²) No. of Employee Unit flow factor (m ³ /person/day) - J11 Community, Social & Personal Services ADWF, (m ³ /day) ADWF, (L/s) | 1528 3.3 50 0.28 14.12 0.16 | P _{CIF} = 1 included |
| D3 | Staff Quarters (D) Number of units Population Unit flow factor (m³/person/day) - Residential R2 ADWF, (m³/day) ADWF, (L/s) | 25 68 0.27 18.23 0.21 | P _{CIF} = 1 included |
| | ment E | | |
| E1 | Leishun Court Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 120 324 0.27 87.48 1.01 | P _{CIF} = 1 included |
| E2 | Caroline Hill Court Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 1146 3094 0.27 835.43 9.67 | P _{CIF} = 1 included |
| E3 | Lei Kwa Court Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 56 151 0.27 40.82 0.47 | P _{CIF} = 1 included |
| E4 | Lei Ha Court Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 120 324 0.27 87.48 1.01 | P _{CIF} = 1 included |
| E5 | Lei Wen Court Number of flats Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) | 146 394 0.27 106.43 1.23 | P _{CIF} = 1 included |
| Catch F1 | ment F Staff Quarters | | |
| | Staff Quarters Number of units Population Unit flow factor (m ³ /person/day) - Residential R2 ADWF, (m ³ /day) ADWF, (L/s) seed Scenario Caroline Hill Road - East | 35 95 0.27 25.52 0.30 | P _{CIF} = 1 included |
| . 1000 | Total ADWF (m3/day) Total ADWF (L/s) Contributing Population Global Peaking Factor Total Peak Flow (L/s) | 1655 19.16 6131 5.00 95.80 | |

Notes:

Employment density shall refer to Commercial and Industrial Floor Space Utilization Survey published by PlanD.

Office = $5.5 \text{ employee per } 100\text{m}^2 \text{ of GFA}$

Retails = $3.5 \text{ employee per } 100 \text{m}^2 \text{ of GFA}$

Community, Social & Personal Services = $3.3 \text{ employee per } 100\text{m}^2 \text{ of GFA}$

Table B2 - Capacity Performance of Existing Sewer

Notes:

(1) Calculate by Colebrook-White Equation

 $\overline{V} = -\sqrt{32gRS_f} \log \left[\frac{k_s}{14.8R} + \frac{1.255\nu}{R\sqrt{32gRS_f}}\right]$

where ks is roughness value

for clayware slimed sewers, ks equals 3mm v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81 m/s2

V is the velocity, D is the diameter of the sewer and S is the gradient of the sewer.

| Abbreviation: | | | | |
|---------------|--------------------------------------|---------|-------------------------|--------|
| UP_MAN | Upstream Manhole | CON_POP | Contributing Population | DN_GL |
| DN_MAN | Downstream Manhole | DIA | Diameter | UP_INV |
| ADWF | Average Dry Weather Flow | LEN | Length | DN_INV |
| ACC_ADWF | Accumulated Average Dry Weather Flow | UP_GL | Upstream Ground Level | VEL |

| UP_MAN | Upstream Manhole | | CO | ON_POP | 0 1 | DN_GL | Downstream G | | CAP | Peak Pipe Cap | , | 4 | | | | | | | | | | | |
|----------------------------|---------------------------|-----------------------------------------|-------------------|----------------------------|--------------------------------------------------------------------------------|---------------------------------------------------------------------|-----------------------------------------|----------|-------------------|---------------|---------|------------|----------------|------------------|-----------------|-----------------|----------------|-----------|---------------------------|-------------------------------------|--------|--------------|-----------------------|
| DN_MAN | Downstream Manhole | | DIA | A | Diameter | UP_INV | Upstream Inve | | F/C | Peak Flow/Cap | bacity | J | | | | | | | | | | | |
| ADWF | Average Dry Weather Flow | | | | Length | | Downstream Ir | | 4 | | | | | | | | | | | | | | |
| ACC_ADWF | Accumulated Average Dry V | Veather Flow | UP. | P_GL | Upstream Ground Level | VEL | Peak Pipe Vel | ocity |] | | | | | | | | | | | | | | |
| Proposed Development | | | | | | | | | | | | | | | | | | | | | | | |
| | nhole | | | | | | 1 | 1 | 1 | | | | | | | Eviat | ing Dine Derem | otor | | | | | 1 |
| UP_MAN | DN_MAN | - | | F | FROM SITE | | CON_POP | PEAKING | ACC_ADWF | Dook Flow | | | | | | | ing Pipe Param | | | | CAD | | Adaguata |
| | DN_WAN | Catchment | | | Description | | | FACTOR | ACC_ADWF (L/s) | Peak Flow | DIA (D) | LEN (m) | UP_GL (mPD) | DN_GL (mPD) | UP_INV (mPD) | DN_INV (mPD) | Gradient | VEL (m/s) | AREA (m ²) | REDUCTION AREA (m ²) | CAP | F/C (%) | Adequate Capacity? |
| INO. | INU. | Catchinent | | | Description | | | | (Ľ/5) | (L/S) | (11111) | (11) | (IIIFD) | (IIIPD) | (IIIPD) | (IIIPD) | (3) | (11/5) | (111) | | (L/S) | (76) | Capacity? |
| Caroline Hill Road - South | | | | | | | | | | | | | | | | | | | | | | , | |
| FMH7019739 | FMH7019738 | B1, 50%B3 | | | Bowling centre, Sports Cent | tre (50%) | 304 | 0 | 0.95 | 7.60 | 150 | 44.5 | 19.69 | 17.71 | 18.88 | 16.82 | 22 | 1.67 | 0.0177 | 0.0159 | 26.58 | 28.6% | YES |
| FMH7019738 | FMH7019738 | B1, 50%B3 B1, 50%B3 | | | Bowling centre, Sports Cent | | 304 | 0 | 0.95 | 7.60 | 150 | 44.5 | 19.09 | 16.77 | 16.82 | 15.85 | 22 | 1.93 | 0.0177 | 0.0159 | 30.70 | 20.0% | YES |
| | | B1, 50%B3 B1. 50%B3, B2 | | | Bowling centre, Sports Centre (50% | | | 0 | | | | | | 15.48 | 10.02 | | 10 | | | | 25.58 | | |
| FMH7019737 | FMH7019719 | B1, 50%B3, B2 B1. 50%B3, B2 | | | Bowling centre, Sports Centre (50% | | 590 | 8 | 1.84 | 14.74 | 150 | 29.6 | 16.77 | | 15.85 | 14.58 | 23 18 | 1.61 | 0.0177 | 0.0159 | _0.00 | 57.6% | YES |
| FMH7019719 | FMH7019793 |)) | | | e 1 (| | 590 | 8 | 1.84 | 14.74 | 150 | 12.8 | 15.48 | 14.80 | 14.58 | 13.86 | 18 | 1.84 | 0.0177 | 0.0159 | 29.29 | 50.3% | YES |
| FMH7019793 | FMH7019713 | B1, 50%B3, B2, A3 | | 0 | centre, Sports Centre (50%), Sport (centre, Sports Centre (50%), Sport (| | 719 | 8 | 2.25 | 17.98 | 150 | 10.1 | 14.80 | 14.42 | 13.86 | 13.47 | 26 | 1.53 | 0.0177 | 0.0159 | 24.26 | 74.1% | YES |
| FMH7019713 | FMH7047620 | B1, 50%B3, B2, A3 | | 0 | | | 719 | 8 | 2.25 | 17.98 | 150 | 17.0 | 14.42 | 13.46 | 12.32 | 11.91 | 41 | 1.21 | 0.0177 | 0.0159 | 19.27 | 93.3% | YES |
| FMH7047620 | FMH7019711 | B1, 50%B3, B2, A3, A2 | | • | | aroline Garden, 103 Caroline Hill Road | 741 | 8 | 2.32 | 18.52 | 150 | 4.0 | 13.46 | 13.23 | 11.91 | 11.80 | 38 | 1.26 | 0.0177 | 0.0159 | 20.03 | 92.5% | YES |
| FMH7019711 | FMH7047621 | B1, 50%B3, B2, A3, A2 | | | | aroline Garden, 103 Caroline Hill Road | 741 | 8 | 2.3 | 18.52 | 150 | 4.7 | 13.23 | 13.23 | 11.80 | 11.64 | 30 | 1.42 | 0.0177 | 0.0159 | 22.54 | 82.2% | YES |
| FMH7047621 | FSH7003584 | B1, 50%B3, B2, A3, A2 | | Bowling centre, Spo | orts Centre (50%), Sport Complex, Ca | aroline Garden, 103 Caroline Hill Road | 741 | 8 | 2.3 | 18.52 | 150 | 8.9 | 13.23 | 12.54 | 11.64 | 11.28 | 24 | 1.57 | 0.0177 | 0.0159 | 24.95 | 74.2% | YES |
| FSH7003584 | FSH7003582 | B1, 50%B3, B2, A3, A2, A1 | Bowling centr | tre, Sports Centre (50%), | Sport Complex, Caroline Garden, 103 | 3 Caroline Hill Road, Silverwood, District Court DC tower | 2,022 | 6 | 6.3 | 37.91 | 300 | 38.8 | 12.54 | 11 13 | 8.61 | 6.93 | 22 | 2.59 | 0.0707 | 0.0636 | 164.77 | 23.0% | YES |
| F317003364 | F3117003362 | B1, 50%B5, B2, A5, A2, A1 | | | | | 2,022 | 0 | 0.3 | 37.91 | 300 | 30.0 | 12.34 | 11.15 | 0.01 | 0.95 | 23 | 2.59 | 0.0707 | 0.0030 | 104.77 | 23.0 % | 163 |
| FSH7003582 | FMH7058242 | B1, 50%B3, B2, A3, A2, A1, G | Bowling cent | ntre, Sports Centre (50%), | Sport Complex, Caroline Garden, 103 | 3 Caroline Hill Road, Silverwood, District Court DC tower | 2,022 | 6 | 6.3 | 37.91 | 2x225 | 67.4 | 11.13 | 4.44 | 6.65 | 6.40 | 269 | 0.62 | 0.0398 | 0.0358 | 44.55 | 85.1% | YES |
| | | B1, 50%B3, B2, A3, A2, A1, G, 70%S | Bowling centr | tre, Sports Centre (50%), | Sport Complex, Caroline Garden, 103 | Caroline Hill Road, Silverwood, District Court DC tower, | | | | | | | | | | | | | | | | (t | |
| FMH7058242 | FSH7003581 | B1, 50%B3, B2, A3, A2, A1, G, 70%S | | | Proposed development | | 4,545 | 6 | 14.2 | 85.22 | 300 | 13.1 | 4.44 | 4.44 | 6.40 | 5.98 | 31 | 2.23 | 0.0707 | 0.0636 | 141.76 | 60.1% | YES |
| | E0117000500 | B1, 50%B3, B2, A3, A2, A1, G, 70%S | Bowling centr | tre, Sports Centre (50%), | | Caroline Hill Road, Silverwood, District Court DC tower, | 4 5 4 5 | | | 05.00 | 000 | | | | | - - | 05 | 4.54 | 0.0707 | 0.0000 | 07 70 | | |
| FSH7003581 | FSH7003580 | | Rowling contr | tre Sporte Contro (E00/) | Proposed development | (70%) B Caroline Hill Road, Silverwood, District Court DC tower, | 4,545 | 6 | 14.2 | 85.22 | 300 | 14.4 | 4.44 | 4.44 | 5.98 | 5.76 | 65 | 1.54 | 0.0707 | 0.0636 | 97.79 | 87.1% | YES |
| FSH7003580 | FMH7009549 | B1, 50%B3, B2, A3, A2, A1, G, 70%S | | | Proposed development | | 4,545 | 6 | 14.2 | 85.22 | 300 | 5.6 | 4.44 | 4.44 | 5.76 | 5.61 | 37 | 2.05 | 0.0707 | 0.0636 | 130.14 | 65.5% | YES |
| | | 1 | | | | | | | 17.2 | 00.22 | | 0.0 | - 7.77 | 7.77 | 0.10 | 0.01 | 51 | 2.00 | 0.0101 | 0.0000 | 100.14 | 00.070 | |
| | | | | | | | | | | | | | | | | | | | | | | I I | <u> </u> |
| FTMH 1 | FMH7058242 | 70%S | | | Proposed development | (70%) | 2,523 | 6 | 7.9 | 47.31 | 300 | 3.0 | 8.00 | 4.44 | 6.43 | 6.40 | 100 | 1.24 | 0.0707 | 0.0636 | 79.08 | 59.8% | YES |
| | | | | | · · · · · · · · · · · · · · · · · · · | | | | | | | | | | | | | | | | | | |
| Caroline Hill Road - East | | | | | | | | | | | | | | | | | | | | | | | _ |
| FMH7019744 | FMH7019743 | C1, C2, C3, 50%B3 | | Confucius Hall Secondar | ry School, So Kon Po Driving Test Ce | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 1.1 | 8.82 | 225 | 37.5 | 13.37 | 11.84 | 10.78 | 10.55 | 163 | 0.80 | 0.0398 | 0.0358 | 28.66 | 30.8% | YES |
| FMH7019743 | FMH7019742 | C1, C2, C3, 50%B3 | | Confucius Hall Secondar | ry School, So Kon Po Driving Test Ce | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 1.1 | 8.82 | 225 | 33.4 | 11.84 | 10.48 | 10.55 | 9.20 | 25 | 2.06 | 0.0398 | 0.0358 | 73.75 | 12.0% | YES |
| FMH7019742 | FMH7019726 | C1, C2, C3, 50%B3 | | | | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 11 | 8.82 | 225 | 33.4 | 10.48 | 9.20 | 9.20 | 7.89 | 25 | 2.03 | 0.0398 | 0.0358 | 72.65 | 12.1% | YES |
| FMH7019726 | FMH7019725 | C1, C2, C3, 50%B3 | | | | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 11 | 8.82 | 225 | 28.3 | 9.20 | 8.17 | 7.89 | 6.68 | 23 | 2.12 | 0.0398 | 0.0358 | 75.86 | 11.6% | YES |
| FMH7019725 | FMH7019724 | C1, C2, C3, 50%B3 | | | | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 1 1 | 8.82 | 225 | 41.5 | 8.17 | 7.45 | 6.68 | 6.16 | 80 | 1.15 | 0.0398 | 0.0358 | 41.02 | 21.5% | YES |
| FMH7019724 | FMH7019720 | C1, C2, C3, 50%B3 | | | | entre, Olypmic House, Sports Centre (50%) | 353 | 8 | 1.1 | 8.82 | 225 | 38.1 | 7.45 | 7.03 | 6.16 | 5.72 | 87 | 1.10 | 0.0398 | 0.0358 | 39.37 | 22.4% | YES |
| FMH7019720 | FMH7009996 | C1, C2, C3, 50%B3, F | | | | ypmic House, Sports Centre (50%), Staff Quarters | 447 | 8 | 1.4 | 11.19 | 225 | 40.6 | 7.03 | 6.57 | 5.72 | 5.00 | 56 | 1.36 | 0.0398 | 0.0358 | 48.82 | 22.9% | YES |
| 110117013720 | 1 1 1 1 1 0 0 3 3 3 0 | 01, 02, 03, 30 /803, 1 | | | - | ypmic House, Sports Centre (50%), Staff Quarters, | | 0 | 1.7 | 11.13 | 225 | 40.0 | 1.00 | 0.57 | 5.12 | 5.00 | 50 | 1.50 | 0.0330 | 0.0350 | 40.02 | 22.370 | |
| FMH7009996 | FMH7058640 | C1, C2, C3, 50%B3, F, D1, D2 | Contact | | blined Services Sports and Recreation | | 857 | 0 | 2.68 | 21.42 | 400 | 2.1 | 6.57 | 6.58 | F 00 | F 00 | 2067 | 0.22 | 0 1057 | 0.1131 | 27.20 | F7 69/ | VES |
| FIMIT/009996 | FIMIT/058640 | | Contus | | • | | 100 | 0 | 2.00 | 21.43 | 400 | 3.1 | 0.07 | 00.0 | 5.00 | 5.00 | 2007 | 0.33 | 0.1257 | 0.1131 | 37.20 | 57.6% | YES |
| | 50117000500 | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | | bl, So Kon Po Driving Test Centre, Or blined Services Sports and Recreation | ypmic House, Sports Centre (50%), Staff Quarters, | 0.57 | | 0.00 | | 400 | 10.1 | 0.50 | 0.40 | 5.00 | | | 4.00 | 0.4057 | 0.4404 | 04445 | 1 40.000 | |
| FMH7058640 | FSH7003590 | | | | 1 | | 857 | 8 | 2.68 | 21.43 | 400 | 10.1 | 6.58 | 6.42 | 5.00 | 4.84 | 63 | 1.90 | 0.1257 | 0.1131 | 214.45 | 10.0% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | • | . . | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FSH7003590 | FSH7003589 | | | , | blined Services Sports and Recreation | - | 857 | 8 | 2.68 | 21.43 | 400 | 9.9 | 6.42 | 6.34 | 4.84 | 4.72 | 82 | 1.66 | 0.1257 | 0.1131 | 187.54 | 11.4% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | | | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FSH7003589 | FSH7003588 | | | | blined Services Sports and Recreation | | 857 | 8 | 2.68 | 21.43 | 400 | 28.4 | 6.34 | 6.03 | 4.72 | 4.35 | 77 | 1.72 | 0.1257 | 0.1131 | 194.44 | 11.0% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | | | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FSH7003588 | FMH7058641 | - , - , - , - , , , , , | | • | blined Services Sports and Recreation | | 857 | 8 | 2.68 | 21.43 | 400 | 3.3 | 6.03 | 5.94 | 4.35 | 3.60 | 4 | 7.19 | 0.1257 | 0.1131 | 813.22 | 2.6% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | | | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FMH7058641 | FMH7058642 | | | • | plined Services Sports and Recreation | | 857 | 8 | 2.68 | 21.43 | 400 | 21.9 | 5.94 | 5.70 | 3.60 | 3.37 | 95 | 1.54 | 0.1257 | 0.1131 | 174.54 | 12.3% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | | | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FMH7058642 | FMH7058643 | 01, 02, 00, 30, 003, 1, 01, 02 | | Discip | olined Services Sports and Recreation | n Club, St. Paul Convent | 857 | 8 | 2.68 | 21.43 | 400 | 8.4 | 5.70 | 5.70 | 2.60 | 2.53 | 120 | 1.37 | 0.1257 | 0.1131 | 155.44 | 13.8% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2 | Confuci | cius Hall Secondary Schoo | ol, So Kon Po Driving Test Centre, Ol | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | 1 | |
| FMH7058643 | FMH7058644 | C1, C2, C3, 30%B3, F, D1, D2 | | Discip | olined Services Sports and Recreation | n Club, St. Paul Convent | 857 | 8 | 2.68 | 21.43 | 400 | 22.4 | 5.70 | 5.51 | 2.53 | 2.36 | 134 | 1.30 | 0.1257 | 0.1131 | 147.01 | 14.6% | YES |
| | | | Confuci | cius Hall Secondary Schoo | ol, So Kon Po Driving Test Centre, Ol | ypmic House, Sports Centre (50%), Staff Quarters, | | | | | | | | | | | | | | | | ·+ | (] |
| FMH7058644 | FSH7003587 | C1, C2, C3, 50%B3, F, D1, D2, 30%S | | | | Convent, Proposed development (30%) | 1,938 | 6 | 6.06 | 36.34 | 400 | 13.4 | 5.51 | 5.39 | 2.36 | 2.25 | 119 | 1.38 | 0.1257 | 0.1131 | 156.37 | 23.2% | YES |
| | | | Confuci | cius Hall Secondary Schoo | ol, So Kon Po Driving Test Centre. Ol | ypmic House, Sports Centre (50%), Staff Quarters, | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | | | | - | | | | | | | _ | | | - | _ | ,] |
| FSH7003587 | FSH7003586 | C1, C2, C3, 50%B3, F, D1, D2, 30%S | | | | Convent, Proposed development (30%) | 1,938 | 6 | 6.06 | 36.34 | 400 | 7.8 | 4.53 | 5.27 | 2.25 | 2.20 | 156 | 1.21 | 0.1257 | 0.1131 | 136.29 | 26.7% | YES |
| | | | Confuci | cius Hall Secondary Schoo | ol, So Kon Po Drivina Test Centre. Ob | ypmic House, Sports Centre (50%), Staff Quarters, | 1,500 | 1 | | | | | | | | | | | | | | _ | |
| FSH7003586 | FMH7058647 | C1, C2, C3, 50%B3, F, D1, D2, 30%S | | | | Convent, Proposed development (30%) | 1,938 | 6 | 6.06 | 36.34 | 400 | 28.2 | 5.27 | 4.97 | 2.20 | 2.14 | 470 | 0.69 | 0.1257 | 0.1131 | 78.38 | 46.4% | YES |
| | | | | • | • | | ., | 1 Ŭ | | 0.01 | | | 5.2. | | | | | 0.00 | | 501 | . 0.00 | | |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, | | | | House, Sports Centre (50%), Staff Quarters, Disciplined | | | | | | | | | | | | | | | | 1 1 | í I |
| FMH7058647 | FSH7003585 | 50%E2, 50%E3 | Services Sports | rts and Recreation Club, S | a. Paul Convent, Proposed developm | ent (30%), Caroline Hill Court (50%), Lei Kwa Court (50%) | 3,561 | 6 | 11.13 | 66.77 | 400 | 21.1 | 4.97 | 4.72 | 2.14 | 2.09 | 422 | 0.73 | 0.1257 | 0.1131 | 82.73 | 80.7% | YES |
| | | 1 | Confuciue Ha | all Secondary School So | Kon Po Driving Test Centre Olypmic | House, Sports Centre (50%), Staff Quarters, Disciplined | | 1 | | | | | | | | | | | | 5 | | | |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, E2, | | | | nt (30%), Caroline Hill Court, Lei Kwa Court, Lei Wen Court, | | | | | | | | | | | | | | | | 1 1 | í I |
| FSH7003585 | FMH7058340 | E3, E5, E4, 50%E1 | | | Lei Ha Court, Leishun Cou | | 6,064 | 5 | 18.95 | 94.75 | 500 | 10.9 | 4.72 | 4.69 | 2.09 | 2.07 | 545 | 0.75 | 0.1963 | 0.1767 | 131.76 | 71.9% | YES |
| 1011/00000 | | | Confusius Li- | all Secondary Sahaal Co | • | House, Sports Centre (50%), Staff Quarters, Disciplined | 0,004 | | 10.35 | 34.73 | | 10.3 | 7.12 | т.0 3 | 2.03 | 2.01 | 545 | 0.13 | 0.1303 | 0.1707 | 101.70 | 11.370 | |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, E2, | | | | nt (30%), Caroline Hill Court, Lei Kwa Court, Lei Wen Court, | | | | | | | | | | | | | | | | 1 | |
| FMH7058340 | FSH7004660 | E3, E5, E4, 50%E1 | | | Lei Ha Court, Leishun Cou | | 6,064 | 5 | 18.95 | 94.75 | 500 | 34.1 | 4.69 | 4.19 | 2.07 | 2.00 | 487 | 0.79 | 0.1963 | 0.1767 | 139.40 | 68.0% | VES |
| FINH7058340 | FSH7004060 | | | | | | 6,064 | 5 | 18.95 | 94.75 | 500 | 34.1 | 4.69 | 4.19 | 2.07 | 2.00 | 487 | 0.79 | 0.1963 | 0.1767 | 139.40 | 68.0% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, E2, | | | | House, Sports Centre (50%), Staff Quarters, Disciplined | | | | | | | | | | | | | | | | 1 | |
| | | E3, E5, E4, 50%E1 | Services Sports | s and Recreation Club, St. | Lei Ha Court, Leishun Cou | nt (30%), Caroline Hill Court, Lei Kwa Court, Lei Wen Court, | | _ | | | | | | | | | | | | | | 1 | |
| FSH7004660 | FSH7004661 | + | | | • | | 6,064 | 5 | 18.95 | 94.75 | 500 | 24.1 | 4.19 | 4.09 | 2.00 | 1.76 | 100 | 1.74 | 0.1963 | 0.1767 | 307.67 | 30.8% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, E2, | | | | House, Sports Centre (50%), Staff Quarters, Disciplined | | | | | | | | | | | | | | | | , I | í I |
| | | E3, E5, E4, 50%E1, D3 | Services Sports | | Paul Convent, Proposed developmen Lei Ha Court, Leishun Court (50%), | nt (30%), Caroline Hill Court, Lei Kwa Court, Lei Wen Court, | | | | | | | | | | | | | | _ | | 1 - 1 | () |
| FSH7004661 | FSH7004662 | | ļ | | | | 6,131 | 5 | 19.16 | 95.80 | 500 | 34.5 | 4.09 | 4.09 | 1.76 | 1.40 | 96 | 1.78 | 0.1963 | 0.1767 | 314.95 | 30.4% | YES |
| | | C1, C2, C3, 50%B3, F, D1, D2, 30%S, E2, | | | | House, Sports Centre (50%), Staff Quarters, Disciplined | | | | | | | | | | | | | | | | 1 1 | í I |
| | | E3, E5, E4, 50%E1, D3 | ' Services Sports | | · · · · · | nt (30%), Caroline Hill Court, Lei Kwa Court, Lei Wen Court, | | | | | | | | | | | | | | | | 1 1 | í I |
| FSH7004662 | FMH7010025 | , | | | Lei Ha Court, Leishun Court (50%), | Stan Quarters (D) | 6,131 | 5 | 19.16 | 95.80 | 500 | 6.0 | 4.09 | 4.08 | 1.40 | 1.39 | 600 | 0.71 | 0.1963 | 0.1767 | 125.55 | 76.3% | YES |
| | | | | | | | | | | | | | | | | | | | | | | 1 1 | í I |
| | | | | | | | | | | | | | | | | | | | | | | , I | í I |
| FTMH 2 | FMH7058644 | 30%S | | | Dronocod development | (30%) | 1.004 | ^ | 2.4 | 20.00 | 205 | 2.0 | 0.00 | E 54 | 0.00 | 0.00 | 100 | 1.00 | 0.0202 | 0.0250 | 26.00 | EE 00/ | |
| | FIVIER / US0644 | 30%3 | | | Proposed development | 00/0) | 1,081 | 0 | 3.4 | 20.28 | 225 | 3.0 | 8.00 | 5.51 | 2.39 | 2.36 | 100 | 1.02 | 0.0398 | 0.0358 | 36.63 | 55.3% | YES |
| | | | | | | | | | | | | | | | | | | | | | | | |

Downstream Ground Level CAP

Peak Pipe Capacity

| B | a | y |
|---|---|---|
| | | |

| Job No. | 285077 | Sheet No. | 4 | Rev. | 5 |
|---------|--------|-----------|----------|---------|----|
| Made by | IP | Date | 16/01/24 | Checked | CC |

Appendix C

Preliminary Design of sewerage system from District Court Complex at Caroline Hill Road, ArchSD

| Fower DC | Population density (approx.) 2172 | Daily Sewage discharged capacity/ head (liter) 5 | Flow Period in a day (hour) | DWF (m3/day) 10 119.46 | DWF (lit/sec) 3.32 | Peak Factor | Peak Discharge Flow Rate (lit/sec) 6 19.91 |
|----------------------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------|---------------------------------------------------------------------------------------------------------------------------|--------------------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------|-----------------------------------------------------|
| Tower FC | 698 | | | 10 38.39 | 117-87-00- | | 6 6.40 |
| | | | | 8 | | Total | 26.31 |
| | 8 | | | tur | mm | 3 | |
| ly Manning F | | 2 | | | | | |
| $V = \prod_{i=1}^{HM}$ | $\frac{D^{3/3} \times S^{1/2}}{2}$ | | | | | | <i>P</i> . |
| (| n) | | | | | | |
| Where: | | | | | | | |
| v | = mean veloc | miles (ma da) | | | | | |
| S | = slope of the | city (m/s) total energy line | | | | | |
| n | = Manning s r | rougnness coettici | ent | | | | |
| | (For vinned | l Clay pipe: n = 0.0 | 12 value) | | | | |
| MD = | Hydraulic Mean | Depth (HMD = 0.1 | 25 x d @ 1/2 bore c | anacity | | | |
| | | | | apacity | | | |
| ۹ = | Sectional Area | (0.393 x d ² @ 1/2 b | ore capacity) | • | | | |
| | | | ore capacity | | | | |
| r Ductile li | on Pipe in Tower DC : In | ternal Diameter - 225r | | | | | |
| in obtaine in | HMD | 0.0562 | | | | | |
| | Pipe size | | 5 m | | | | |
| ×. | Slope of the total | | | | | | |
| | energy line | 0.03 | 1 | | | | |
| | | | | | | | |
| | Manning's roughness | | | | | | |
| | coefficient, n V= | | 2 (for Ductile Iron pipe) | | | | |
| | V- | 1.22: | 3 m/s | | | | |
| | Q=VxA | | | | | | |
| | where | | | | | | |
| | Q = discharge flow rate | capacity (I/s) | | | | | |
| | A = area of pipe work (| | | | | | |
| | 24.31 | | | | | | |
| | the calculation (1/2 bore | e capacity) | | | | | |
| 4.31 l/s > 19 | m Ductile Iron pipe with | gradient 1:100 shall er | tor the site discharge 1/ | have enneity at | 1 332m /s valasity | | N N |
| | in outlie non pipe with | gradient 1.100 Shan ca | iter the site discharge 1/ | z bore capacity at | 1.225m/5 velocity. | | |
| | | | | | | | |
| - Durabile Is | on Pipe in Tower FC : Int | ernal Diameter = 225m | m · | For Ductile Iron | n Pipe in Tower FC : Interna | l Diameter = 2 | .00mm |
| or Ductile in | HMD | | | | HMD | 0.0 | |
| or Ductile in | | 0.05625 | | | | | |
| r Ductile in | Pipe size | | 5 i m | | Pipe size | 20 | <mark>0</mark> m |
| r Ductie ir | Pipe size Slope of the total | 225 | <mark>i</mark> m | | Pipe size Slope of the total energy | | |
| r Ductile in | Pipe size | | <mark>i</mark> m | | Pipe size | 20 0.0 | |
| r Ductie in | Pipe size Slope of the total energy line | 225 | <mark>i</mark> m | | Pipe size Slope of the total energy line | | |
| or Ductile in | Pipe size Slope of the total energy line Manning's roughness | 225 0.01 | <mark>im</mark> L . | | Pipe size Slope of the total energy line Manning's roughness | 0.0 | 1. |
| r Ductile ir | Pipe size Slope of the total energy line | 225 0.01 0.012 | <mark>i</mark> m | | Pipe size Slope of the total energy line | 0.0 | 1 · 2 (for Ductile Iron pij |
| r Ductile ir | Pipe size Slope of the total energy line Manning's roughness coefficient, n | 225 0.01 0.012 | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line Manning's roughness coefficient, n | 0.0 | 1. |
| r Ductile ir | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A | 225 0.01 0.012 | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line Manning's roughness coefficient, n | 0.0 | 1 · 2 (for Ductile Iron pi |
| r Ductile ir | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where | 225 0.01 0.017 1.225 | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where | 0.01 0.011 1.13 | 1 · 2 (for Ductile Iron pi |
| or Ductile in | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate | 225 0.01 0.012 1.225 capacity (l/s) | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap | 0.01 0.01 1.13 pacity (I/s) | 1 · 2 (for Ductile Iron pi |
| or Ductile in | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r | 225 0.01 0.012 1.225 capacity (l/s) m2) | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate caj A = area of pipe work (m2) | 0.01 0.01 1.13 pacity (l/s) | 1 · 2 (for Ductile Iron pij |
| | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 | 225 0.01 0.012 1.225 capacity (I/s) m2) I/s | <mark>i</mark> m L 2 (for Ductile Iron pipe) | According to th | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate caj A = area of pipe work (m2) 17.76 | 0.01 1.13 pacity (l/s) l/s | 1 · 2 (for Ductile Iron pij |
| cording to | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore | 225 0.01 0.012 1.225 capacity (I/s) m2) I/s | <mark>i</mark> m L 2 (for Ductile Iron pipe) | | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate caj A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap | 0.01 1.13 pacity (l/s) l/s | 1 · 2 (for Ductile Iron pi |
| cording to | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 0 l/s | 22: 0.01 0.017 1.225 capacity (I/s) m2) I/s e capacity) | s m L 2 (for Ductile Iron pipe) 3 m/s | 17.76l/s >6.40 | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate caj A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap 1/s | 0.01 1.13 pacity (l/s) l/s | 1 · 2 (for Ductile Iron pi |
| coording to .311/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 0 l/s | 225 0.01 0.017 1.225 capacity (I/s) m2) I/s e capacity) ht 1:100 shall cater the | im L 2 (for Ductile Iron pipe) 8 m/s site discharge 1/2 bore o | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap l/s /s velocity. | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pi |
| coording to .311/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 0 l/s ile Iron pipe with gradien on to DSD manhole shall | 225 0.01 0.017 1.225 capacity (I/s) m2) I/s e capacity) ht 1:100 shall cater the | im L 2 (for Ductile Iron pipe) 8 m/s site discharge 1/2 bore o | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate caj A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap 1/s | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pi |
| cording to 1.31/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 0 l/s le Iron pipe with gradien on to DSD manhole shall | 225 0.01 0.012 1.225 capacity (I/s) m2) I/s a capacity) ht 1:100 shall cater the be 225mm at least. Th | im (for Ductile Iron pipe) m/s site discharge 1/2 bore o us, 225mm discharge pij 1:40 1:70 | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap l/s /s velocity. | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pi |
| cording to 1.31/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore O l/s le Iron pipe with gradien on to DSD manhole shall | 225 0.01 0.017 1.225 capacity (I/s) m2) I/s capacity) tt 1:100 shall cater the <u>be 225mm at least. Th</u> 100mm 150mm | im (for Ductile Iron pipe) m/s site discharge 1/2 bore o us, 225mm discharge pij 1:40 1:70 | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap l/s /s velocity. | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pij |
| cording to 1.31/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 0 l/s ile Iron pipe with gradier on to DSD manhole shall | 22: 0.01 0.012 1.223 capacity (I/s) m2) I/s capacity) t 1:100 shall cater the be 225mm at least. Th 100mm 150mm 200mm 225mm | im (for Ductile Iron pipe) m/s site discharge 1/2 bore ous, 225mm discharge pij 1:40 1:70 1:70 1:100 | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap l/s /s velocity. | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pi |
| cording to 1.31/s >6.4 | Pipe size Slope of the total energy line Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate A = area of pipe work (r 24.31 the calculation (1/2 bore 01/s ile Iron pipe with gradier on to DSD manhole shall | 225 0.01 0.017 1.225 capacity (I/s) m2) I/s capacity) tt 1:100 shall cater the <u>be 225mm at least. Th</u> 100mm 150mm | im (for Ductile Iron pipe) m/s site discharge 1/2 bore o us, 225mm discharge pij 1:40 1:70 | 17.76l/s >6.40 apacity at 1.131m/ | Pipe size Slope of the total energy line . Manning's roughness coefficient, n V= Q= V x A where Q = discharge flow rate cap A = area of pipe work (m2) 17.76 the calculation (1/2 bore cap l/s /s velocity. | 0.01 0.01 1.13 pacity (I/s) I/s pacity) | 1 · 2 (for Ductile Iron pi |

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