# **ARUP**

Appendix G Sewerage Impact Assessment



Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH)
Restrictions for Permitted/Proposed Commercial
Development, Public Transport Station and
Underground Vehicle Tunnel at Kai Tak Area 4C
Sites 4 and 5 and Adjoining Road Portion of Shing
King Street; and Minor Relaxation of PR and BH
Restrictions for Permitted Private Housing
Development with Proposed Eating Place, Shop and
Services and Social Welfare Facilities at Kai Tak
Area 3E Sites 1 and 2

Sewerage Impact Assessment Report

| March 2025

This report takes into account the particular instructions and requirements of our client. It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.

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| | March 2025 | Ove Arup & Partners HK Limited

# 1. Introduction

## 1.1 Project Background

1.1.1 Ove Arup & Partners Hong Kong Ltd. has been commissioned by Civil Engineering and Development Department to undertake the Sewerage Impact Assessment to support the Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2 under the approved Kai Tak Outline Zoning Plan No. S/K22/8 (Proposed Development). The location of the Proposed Development is shown in Figure 1-1. The project implementation year is 2030.

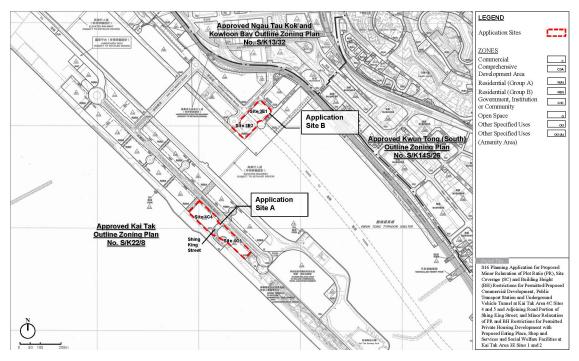


Figure 1-1 Study Area and Sites Locations

1.1.2 The aim of this Sewerage Impact Assessment (SIA) Study is to review and update the sewerage network arising from the minor relaxation of plot ratio, site coverage, and building height restrictions of the Proposed Development; to assess the impact of the Proposed Development on the existing sewerage network and to propose mitigation measures (if required).

# 2. Methodology and Design Criteria

#### 2.1 Overview

2.1.1 This chapter describes the methodology for assessing the sewerage network performance under both existing and proposed conditions.

## 2.2 Design Standards and Guidelines

- 2.2.1 The SIA has been prepared in accordance with the following design manuals and information have been adopted:
  - Drainage Services Department (DSD) Sewerage Manual (SM) Third Edition, January 2013;
  - DSD SM Corrigendum No. 1/2024;
  - Technical Circular No. 1/2022 "Handling of Abandoned Pipes under DSD's Purview", DSD.
  - Environmental Protection Department (EPD) Report No. EPD/TP 1/05 "Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning Version 1.0" (GESF);
  - Planning Department (PlanD) Commercial and Industrial Floor Space Utilization Survey (CIFSUS);
  - Sewerage Record Plans obtained from the GeoInfo<sup>1</sup> Map services of the Lands Department:
  - As built sewerage drawings from Kai Tak Development Stage 2 and Stage 3.

## 2.3 Design Criteria

2.3.1 The SM 2024 defines velocity requirements to prevent siltation and avoid sewer erosion as shown in **Table 2-1** below.

Table 2-1 Design Standards

Criteria	Conditions	Requirement
Minimum Velocity	DN<300mm (alternatives)	0.7 m/s
Minimum Gradient	,	1:DN
Minimum Velocity	nimum Velocity DN>300mm	
Maximum Velocity	No erosion protection	3 m/s
,	With erosion protection	6 m/s

<sup>1</sup> https://www.map.gov.hk/gm/

### 2.4 Method of Analysis

#### **Sewerage Network Performance**

2.4.1 The sewerage network performance has been assessed through Colebrook-White formula as per SM 2024 Section 5.2.

#### **Design Roughness**

2.4.2 The adopted roughness coefficients are consistent with SM Table 15; and are summarised in **Table 2-2**.

**Table 2-2 Adopted Roughness Coefficients** 

Sewerage Network Element	ork Description	Colebrook Roughness [mm]
	Slimed Clayware; velocity 0.75m/s	3.0
	Slimed Clayware; velocity 1.2m/s	0.6
Existing Sewerage Pipe	Slimed Ductile Iron; velocity 1.2m/s	0.3
	Slimed Concrete; velocity 0.75m/s	6.0
	Slimed Concrete; velocity 1.2m/s	3.0
Proposed Sewerage Pipe	es Ductile Iron	0.3

2.4.3 Ductile Iron roughness has been adopted consistent with DSD SM. In SM Table 5 for metal pipes, ductile iron is defined as having plastic lining (polyurethane). The closes material to polyurethane for slimed conditions is uPVC; therefore the roughness stated for uPVC has been adopted for ductile iron.

#### 2.5 Sewage Flows

#### **Population Density**

2.5.1 The CIFSUS conducted by the PlanD has been used to determine the worker density for various economic activities and planned usage types which are summarised in **Table 2-3**.

**Table 2-3 Adopted Worker Density** 

Economic Activity	GFA (m²) per Worker
Manufacturing	43.5
Retail Trade	28.6
Import/Export Trade	30.3
Business Services	18.2
Restaurants	19.6

#### **Unit Flow Factor – Commercial Flows**

2.5.2 The sewerage flows from commercial units are the are composed of flows due to employees and the associated commercial activities. The Unit Flow Factors (UFFs) for commercial sewage flows due to employed population of the proposed development and the existing sewerage catchment are shown in **Table 2-4** based on the Table T-2 of GESF.

**Table 2-4 Adopted Unit Flow Factor for Commercial Flows** 

Commercial Type	UFF (m³/employee/day)
Commercial Employee	0.080
Industrial Employee	0.080
<b>Commercial Activities</b>	1
J11 – Community, Social & Personal Services	0.200
J6 – Business Services	-
J4 – Wholesale & Retail	0.200
J10 – Restaurant and Hotels	1.500
Industrial Activities	1
J1 – Manufacturing – East Kowloon	0.450

#### **Catchment Inflow Factor**

2.5.3 The Catchment Inflow Factor (PCIF) indicate the net overall ingress of water or wastewater to the sewerage system. The values defined in GESF Table T-4 have been adopted, as summarised in **Table 2-5**.

**Table 2-5 Adopted Catchment Inflow Factor** 

Catchment	Catchment Inflow Factor
East Kowloon	1.10
Central Kowloon	1.00

#### **Peaking Factor**

2.5.4 Combination of flow variation due to diurnal and seasonal characteristics have been incorporated through the adoption of a peaking factor; the adopted peaking factor depend on the assessed equivalent population and have been adopted as per GESF Table T-5 inclusive of stormwater allowance.

# 3. Existing Conditions

#### 3.1 Overview

3.1.1 This chapter describes the existing sewerage network. The design flows are defined and the performance of the existing sewerage network in such conditions is assessed.

### 3.2 Existing Sewerage Infrastructure

3.2.1 The sites of the proposed developments are located on areas already served by existing infrastructure. Such sewerage infrastructure has been designed and provided to serve the sites under current development parameters. The existing sewerage network in the proximity of each site is described below.

#### Sites 3E1 and 3E2

- 3.2.2 The sites are flanked by existing sewerage network along Cheung Yip Street and Kai Hing Road, which collects also the inflows from Lam Chan Street, as shown in Drawing SEW\_EX\_NET\_3E.

  Both the sewer beneath Cheung Yip Street and Kai Hing Road discharge to the existing sewerage box culvert along Hoi Bun Road and finally Kwun Tong Preliminary Treatment Works.
- 3.2.3 Beneath Cheung Yip Street there is an existing 300mm diameter sewer with a dedicated tapping point for development of Sites 3E1 and 3E2 just upstream of existing manhole FMH4096818 as shown in **Appendix B**. The existing sewer proceeds along Cheung Yip Street progressively increasing in size, up to 600mm before the connection to the existing sewage box culvert.
- 3.2.4 Beneath Kai Hing Road there is an existing 225mm sewer which already serves the area of the proposed development. After receiving the flows from the existing sewer beneath Lam Chak Street, the sewer increases to 450mm diameter ultimately reaching 525mm diameter before the connection to the existing sewage box culvert. The sites' combined flows are then discharged to Kwun Tong Preliminary Treatment Works.

#### Sites 4C4 and 4C5

- 3.2.5 The sites are served by dedicated sewerage network constructed under Kai Tak Development as shown in Drawing SEW\_EX\_NET\_4C. Site 4C4 is served by a dedicated tapping point (FTH4011157) and 300mm diameter sewer pipe, upstream of existing manhole FMH4098424 as shown in **Appendix B**.
- 3.2.6 Site 4C5 is served by a dedicated tapping point (FTH4011160) and 300mm diameter sewer pipe, upstream of existing manhole FMH4098431 as shown in **Appendix B**.
- 3.2.7 Sewage flow from both sites is conveyed by the existing sewers to Shing Fung Road Sewage Pumping Station and pumped to the existing sewerage box culvert beneath Hoi Bun Road, finally discharging to Kwun Tong Preliminary Treatment Works.

## 3.3 Sewage Generation from the Existing Development

3.3.1 The existing sewerage infrastructure at the assessed sites, has been designed and constricted to cater for the current approved land use and development parameters. The estimated sewerage flows are provided in **Appendix C** and summarised in **Table 3-1**.

**Table 3-1 Sewage Flow Estimation for the Existing Conditions** 

Sites	Estimated ADWF (m <sup>3</sup> /day)
3E1	218.1
3E2	237.1
4C4	1,550.4
4C5	1,101.2

## 3.4 Performance of Existing Network

3.4.1 The existing sewerage network has generally sufficient capacity to discharge the existing flows as detailed in **Appendix C**. Limited sewerage network sections have been assessed without spare capacity as summarised in **Table 3-2**.

**Table 3-2 Existing Sewers Without Spare Capacity** 

From Manhole	To Manhole	Diameter [mm]	Capacity [l/s]	Flow [l/s]	Utilisation [%]	Serving Site
FMH4100328	FMH4061903	300	56	86	153%	3E2
FMH4061903	FMH4061905	300	34	86	256%	. 3E2
FMH4043143	FMH4043144	225	52	84	161%	3E1
FMH4043144	FMH4043145	225	51	84	164%	JE1

# 4. Proposed Conditions

#### 4.1 Overview

4.1.1 This chapter describes the proposed development, the generated sewage flows and the proposed network and its performance.

### 4.2 Sewage Generation from the Proposed Development

4.2.1 The proposed development will impact the sites as shown in **Table 4-1**. The proposed amendment will generate sewerage flows as summarised in **Table 4-2** and further detailed in **Appendix C**.

**Table 4-1 Proposed Development Parameters Compared to Existing Approved Conditions** 

			Baseline		Proposed			
Sites	Site area			GFA [m2]	Plot	GFA [m2]		
	2-12-02-00	OZP Zoning	Plot Ratio		Ratio	Domestic	Non- Domestic	
Site 3E1	7,064	R(B)2	4.5	31,788	7	44,098	3,532	
Site 3E2	7,686	R(B)2	4.5	34,587	/	51,777	3,843	
Site 4C4	10,694	C(7)	7.5	80,205		0	102,232	
Site 4C5	9,480	C(5)	6.0	56,880	8.16	0	79,334	
Site as Road	2,376	-	-	2,376	(about)	-	2,376	

4.2.2 The assumptions of Baseline development GFA and flats has been derived from existing planning information as detailed in **Appendix C** and **Appendix D**.

Table 4-2 Sewage Flow Estimation for the Proposed Development

Sites	ADWF (m³/day)				
Sites	Baseline	Proposed	Increase		
3E1	218.1	508.0	289.9		
3E2	237.1	590.6	353.5		
4C4	1,550.4	1,654.9	104.5		
4C5	1,101.2	1,301.2	200.0		

## 4.3 Proposed Development Sewerage System

4.3.1 The sites are already served by dedicated existing sewers. Under the proposed developments, the sites will discharge to the existing dedicated sewer network. A summary of the proposed discharge point for each site is shown in **Table 4-3**.

**Table 4-3 Proposed Sewage Discharge Locations** 

Sites	Upstream Manhole	Downstream Manhole	Sewer size (mm)	Upstream Invert Level (mPD)	Downstream Invert Level (mPD)	Capacity (L/s)
3E1	FMH4036432	FMH4043143	225	2.00	1.84	39.75
3E2	FMH4096818	FMH4100328	300	2.54	1.11	236.15
4C4	FTH4011157	FMH4098424	300	2.48	2.13	175.41
4C5	FTH4011160	FMH4098431	300	1.74	1.35	195.71

- 4.3.2 The existing sewer at the discharge location has sufficient capacity to convey the proposed flows as detailed in **Appendix C**.
- 4.3.3 The development terminal manhole will be located close to the lot boundary, exact location will be determined in detailed design.
- 4.3.4 The proposed development utilizes the existing sewer networks; no existing sewer pipe is proposed to be abandoned. In case future detailed design will require the abandoning of existing sewers, these should comply with DSD Technical Circular No. 1/2022 Handling of Abandoned Pipes under DSD's Purview.

# 5. Potential Sewerage Impacts and Mitigation Measures

#### 5.1 Overview

5.1.1 In this chapter the identified sewerage impacts and associated mitigation measures are discussed.

## 5.2 Impacts on Existing Sewerage Network

5.2.1 The proposed development will discharge to the existing network; the additional flows from the development may affect the existing network performance. The impacts for each site are detailed below and calculations are provided in **Appendix C**.

#### Sites 3E1 and 3E2

The proposed development will discharge to two discharge points: one at the existing sewer along Kai Hing Road (at manhole FMH4036432), and one the existing sewer along Cheung Yip Street (at manhole FMH4096818). The proposed flow split will alleviate the sewer along Kai Hing Road, increasing spare capacity. The existing utilisation rate of the first sewage pipe at Kai Hing Road downstream of Site 3E1 proposed discharge point, has utilisation rate close to 90%. The existing sewer is proposed to be upgraded to allow development flexibility, as shown in **Table 5-1**.

Table 5-1 Proposed Sewerage Upgrades at Sites 3E1

G.	Upstream	Downstream	Existing			Proposed		
Sites	Manhole	Manhole	US IL (mPD)	DS IL (mPD)	Capacity (L/s)	US IL (mPD)	DS IL (mPD)	Capacity (L/s)
3E1	FMH4036432	FMH4043143	2.00	1.84	39.75	2.05	1.80	57.22

5.2.3 The existing sewer along Cheung Yip Street will need to be upgraded due to existing insufficient capacity, as shown in **Table 5-2**.

Table 5-2 Proposed Sewerage Upgrades at Site 3E2

	Upstream	Downstream	Existing		Proposed	
Sites	Manhole	Manhole	Sewer size (mm)	Capacity (L/s)	Sewer size (mm)	Capacity (L/s)
3E2	FMH4100328	FMH4061903	300	56.26	525	328.65
	FMH4061903	FMH4061905	300	33.66	525	165.60

5.2.4 The existing sewer along Kai Hing Road will need to be upgraded due to existing insufficient capacity, as shown in **Table 5-3**. The adjacent redevelopment of Lot KNIL5813 approved under application no. A/422/27 "... has proposed to upgrade sewer size as mitigation measure." <sup>2</sup>. During detailed design, close coordination between the development under this application and at Lot KNIL5813 is recommended to ensure sewerage upgrading works will suit both developments and minimise disruption.

Table 5-3 Proposed Sewerage Upgrades at Site 3E1

	Upstream	Jostream Downstream		ting	Proposed	
Sites	Manhole	Manhole	Sewer size (mm)	Capacity (L/s)	Sewer size (mm)	Capacity (L/s)
3E1	FMH4043143	FMH4043144	225	51.89	300	120.30
	FMH4043144	FMH4043145	225	51.04	300	118.31

#### Sites 4C4 and 4C5

- 5.2.5 The proposed developments will discharge to the dedicated tapping points. The downstream network has sufficient capacity to convey the design flows.
- 5.2.6 The existing utilisation rate of the sewage pipe downstream of Site 4C4 proposed discharge point, has utilisation rate exceeding 90%. The existing sewer is proposed to be upgraded to allow development flexibility, as shown in **Table 5-4**; no sewerage upgrading work is required for Site 4C5.

Table 5-4 Proposed Sewerage Upgrades at Site 4C4

	Upstream Downstream		Exist	ting	Proposed	
Site	Manhole	Manhole	Sewer size (mm)	Capacity (L/s)	Sewer size (mm)	Capacity (L/s)
4C4	FMH4098424	FMH4098421	375	93.81	450	163.34

5.2.7 The existing sewers downstream of the propose discharge points of Sites 4C4 and 4C5, range from 750mm diameter to 900mm diameter. Their utilisation rate after the proposed developments is less than 80% as detailed in **Table 5-5**, the impacts are deemed acceptable.

Table 5-5 Additional Sewerage Utilisation Rate Downstream of Site 4C4 and 4C5

Site	Upstream Manhole	Downstream Manhole	Sewer size (mm)	Capacity (L/s)	Baseline Inflow (L/s)	Proposed Inflow (L/s)	Proposed Usage
	FMH4098421	FMH4098427	750	729.22	555	560	77%
4C4	FMH4098427	FMH4098429	750	707.51	555	560	79%
	FMH4098429	FSH4007922	750	888.82	616	621	70%

<sup>&</sup>lt;sup>2</sup> A K22 27 MainPaper(revised).pdf

Site	Upstream Manhole	Downstream Manhole	Sewer size (mm)	Capacity (L/s)	Baseline Inflow (L/s)	Proposed Inflow (L/s)	Proposed Usage
	FSH4007922	FMH4098438	900	1273.46	820	832	65%
4C5	FMH4098438	FMH4098439	900	1323.05	820	832	63%
	FMH4098439	FSH4007923	900	1328.58	984	996	75%

# 6. Conclusion

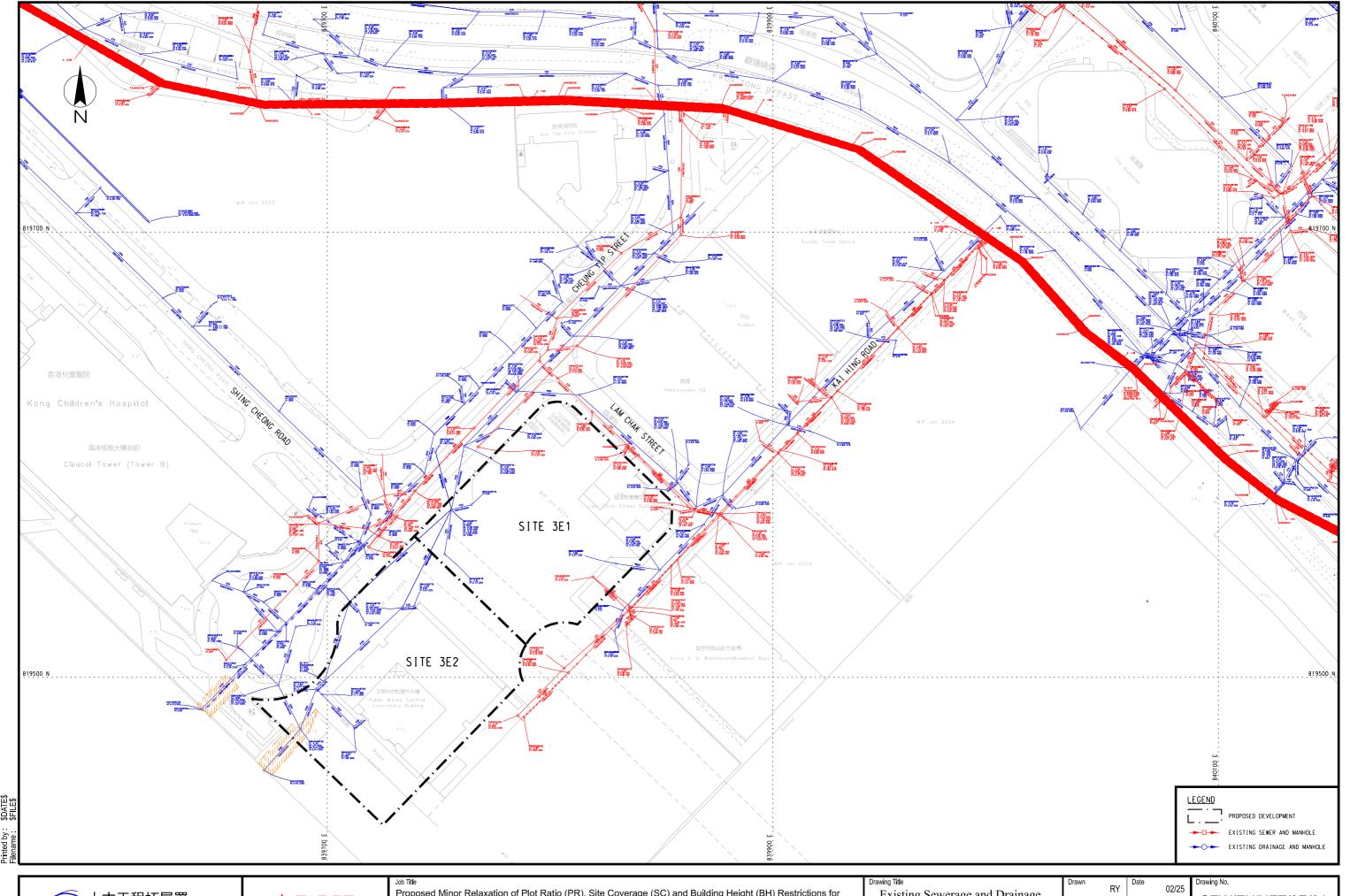
- Under the proposed developments at Sites 3E1, 3E2, 4C4 and 4C5, minor relaxation of Plot Ratio (PR) restriction is proposed. The sites of the proposed developments are located on areas already served by existing infrastructure. Such sewerage infrastructure has been designed and provided to serve the sites under current development parameters. This SIA has been conducted to identify the impacts of the proposed modified development parameters and propose mitigations measures, if found necessary.
- 6.1.2 The expected sewerage flows form the proposed development have been assessed and the impact on the performance of the existing sewerage network in the vicinity has been quantified.
- 6.1.3 The capacity of existing sewerage system is sufficient to convey the sewage flows from the proposed developments at 4C4, and 4C5. Hence, no upgrading works are required.
- 6.1.4 The existing sewer beneath Kai Hing Road will need to be upgraded to cater for the additional flows from development site 3E1. The proposed works upgrade the existing sewer from 225mm diameter to 300mm diameter between existing manhole FMH4043143 and existing manhole FMH4043145. Coordination with sewer upgrading works defined under the adjacent redevelopment of Lot no. NKIL5813 is recommended in detailed design.
- 6.1.5 The existing sewer beneath Cheung Yip Street will need to be upgraded to cater for the additional flows from development site 3E2. The proposed works upgrade the existing sewer from 300mm diameter to 525mm diameter between existing manhole FMH4100328 and existing manhole FMH4061905.
- 6.1.6 To support development flexibility, the upgrade of two sewers with utilisation rate close to 90% in the immediate vicinity of the sites has been proposed. Downstream of Site 3E1, the existing 225mm diameter sewer between manhole FMH4036432 and manhole FMH4043143 is to be reconstructed with steeper gradient to increase capacity. Downstream of Site 4C4, the existing 375mm diameter sewer between manhole FMH4098424 and manhole FMH4098421 is to be upgraded to 450mm diameter.
- 6.1.7 This SIA confirms the feasibility of the Proposed Developments in terms of its sewerage impacts.

# **Abbreviation List**

Abbreviation	Definition
ADWF	Average Dry Weather Flow
CIFSUS	Commercial and Industrial Floor Space Utilization Survey
DN	Nominal Diameter
DSD	Drainage Services Department
EPD	Environmental Protection Department
GESF	Guidelines for Estimating Sewage Flows
GFA	Domestic Gross Floor Area
OZP	Outline Zoning Plan
PCIF	Catchment Inflow Factor
PlanD	Planning Department
PPF	Person Per Flat
PR	Plot Ratio
SIA	Sewerage Impact Assessment
SM	Sewerage Manual
UFF	Unit Flow Factor

# Appendix A

# **Drawings**



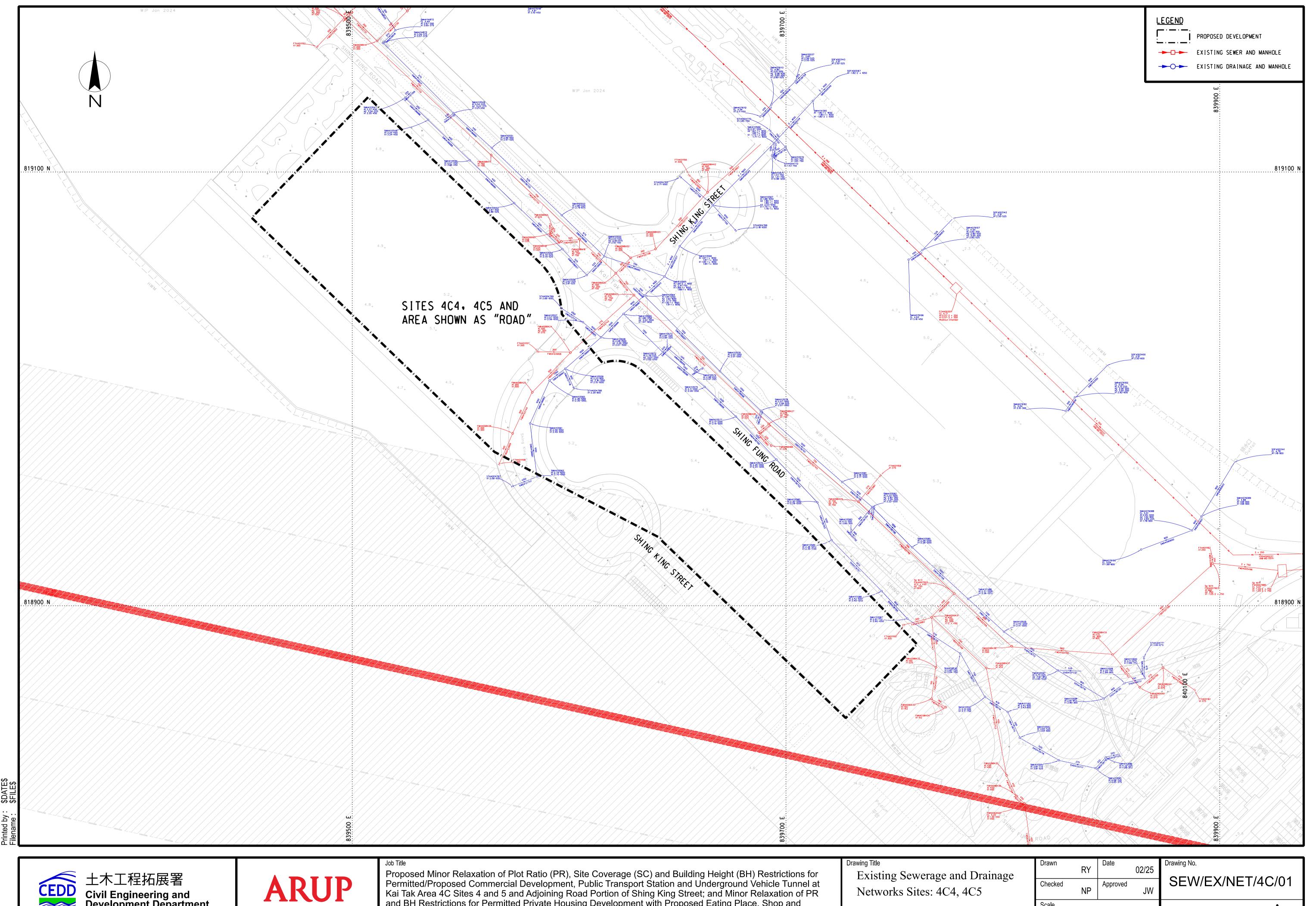
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Civil Engineering and
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Existing Sewerage and Drainage Networks Sites: 3E1, 3E2

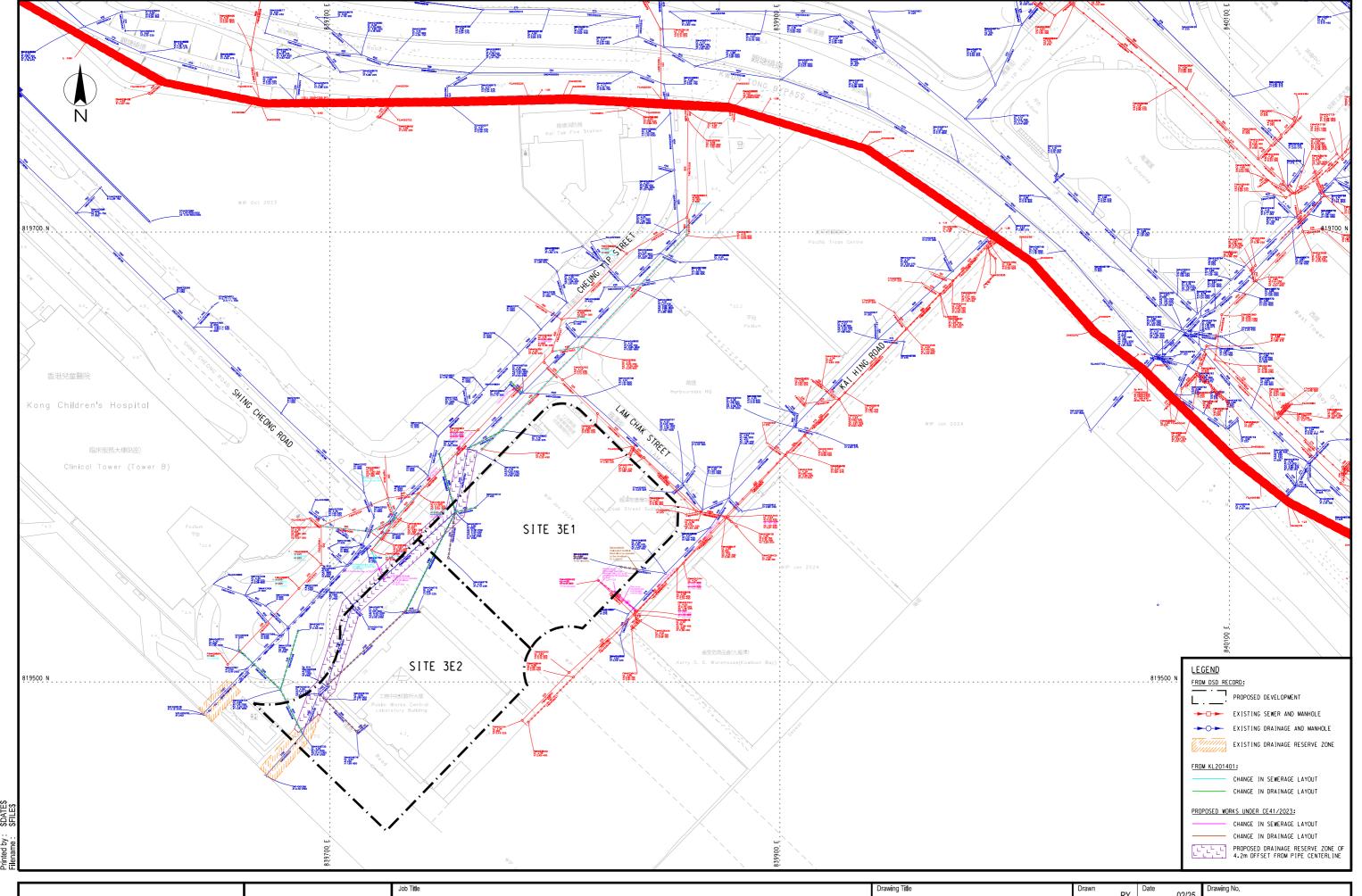
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CEDD Civil Engineering and Development Department

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

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Civil Engineering and Development Department

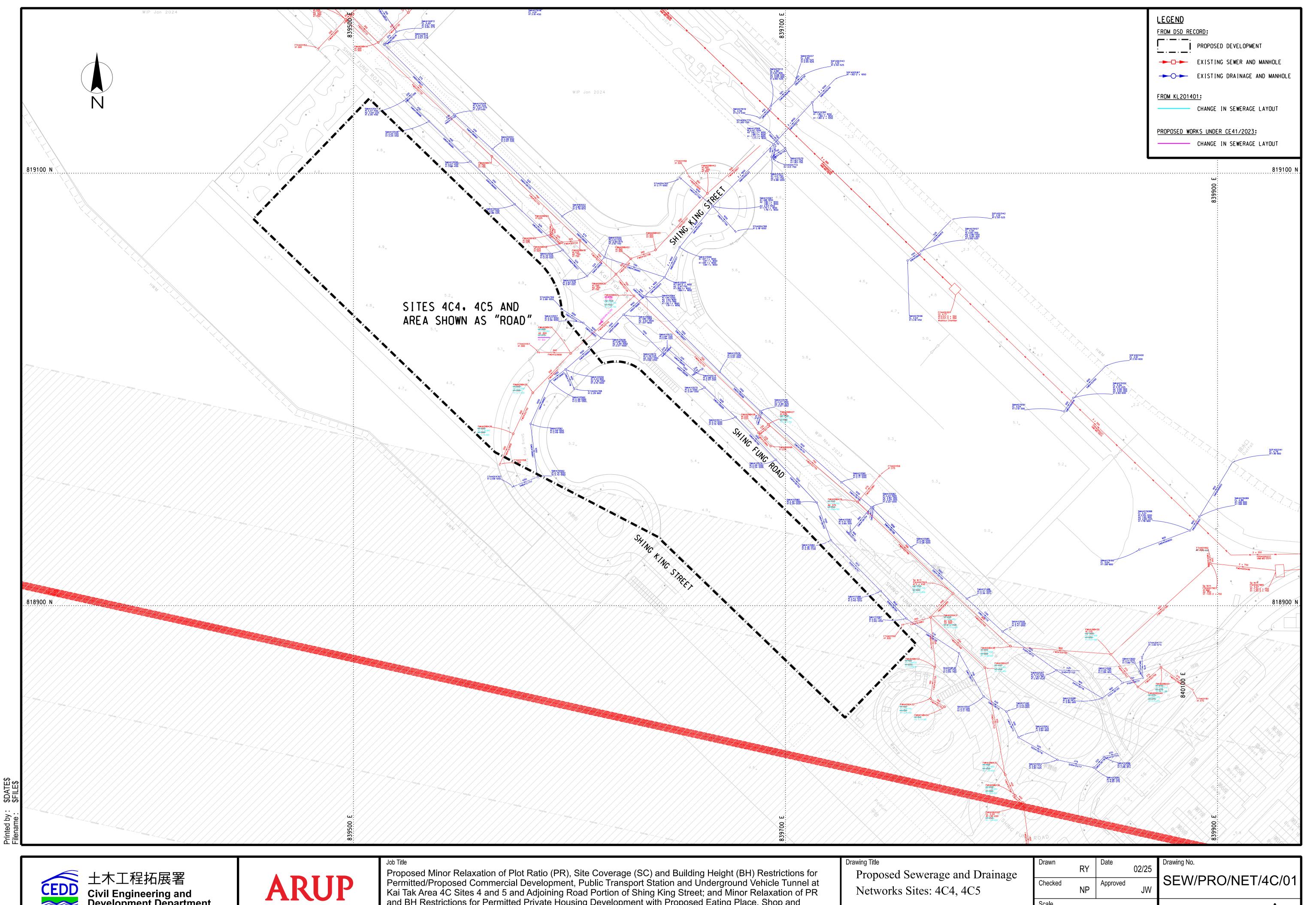
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Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Proposed Sewerage and Drainage Networks Sites: 3E1, 3E2 
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 RY
 Date
 02/25
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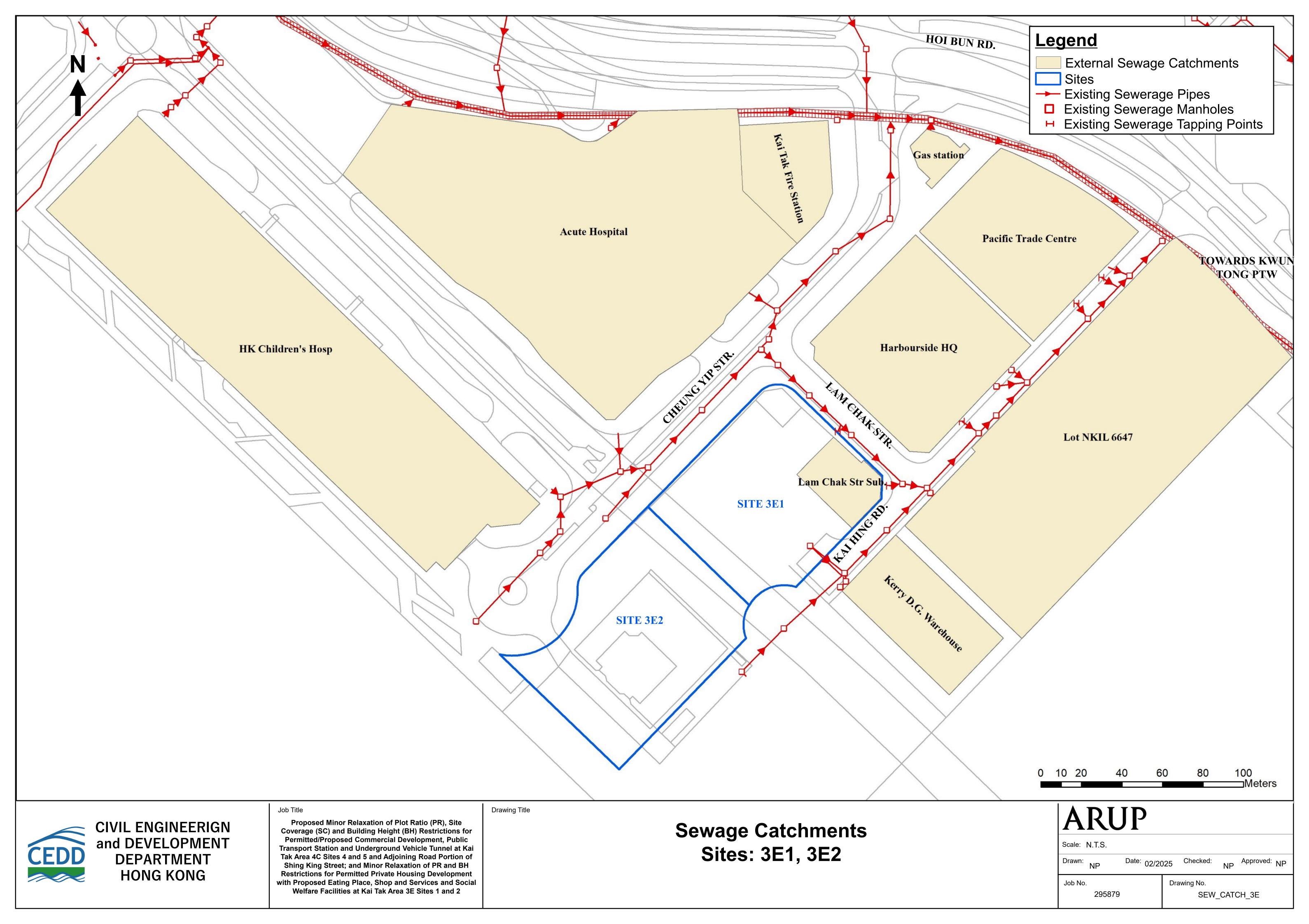
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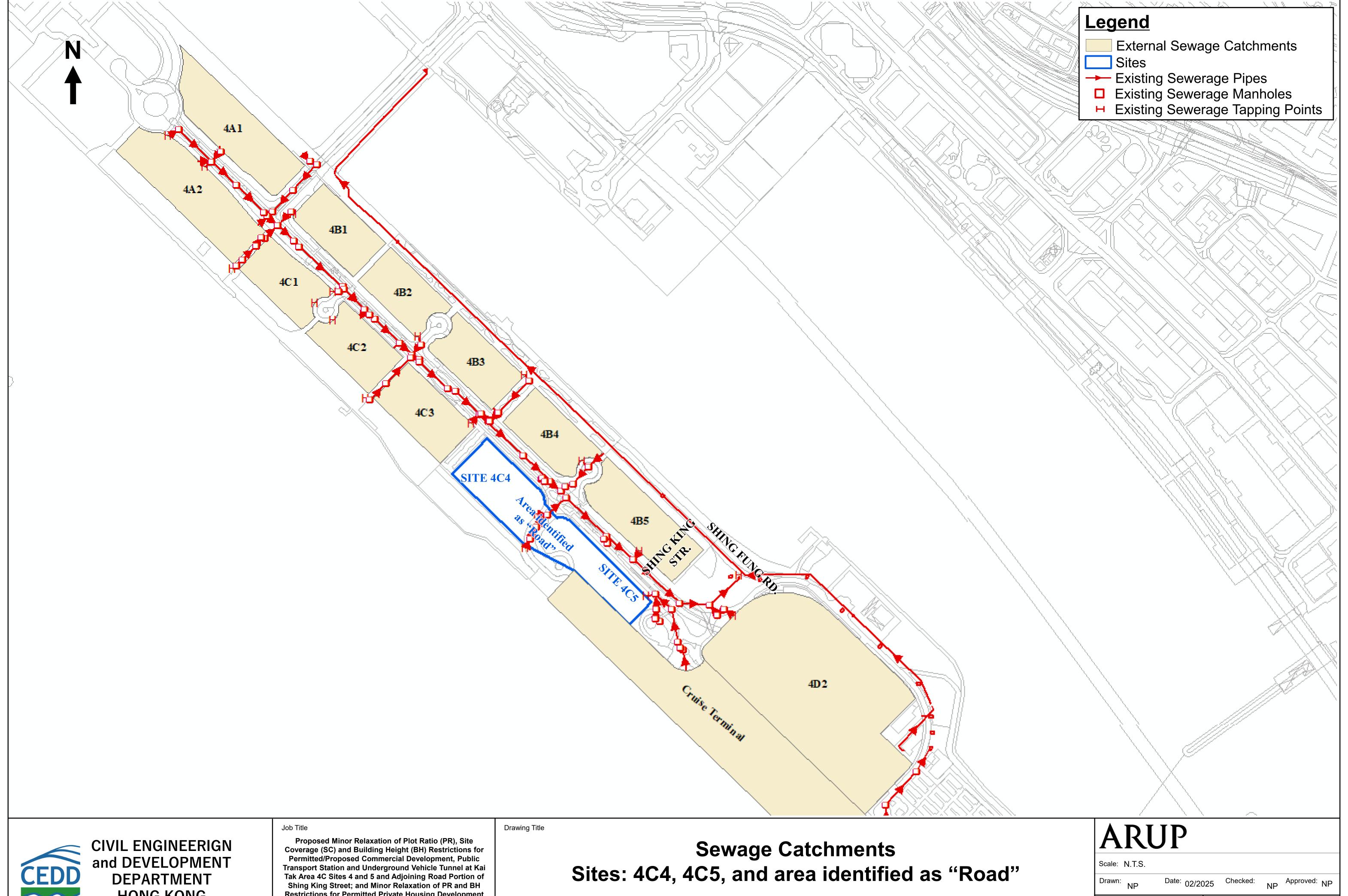


CEDD Civil Engineering and Development Department

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

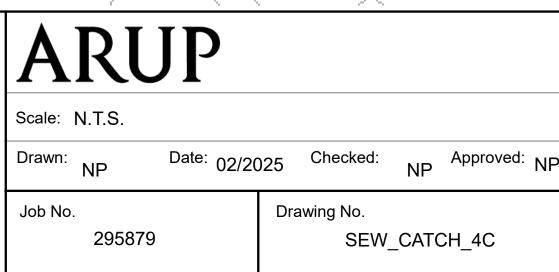
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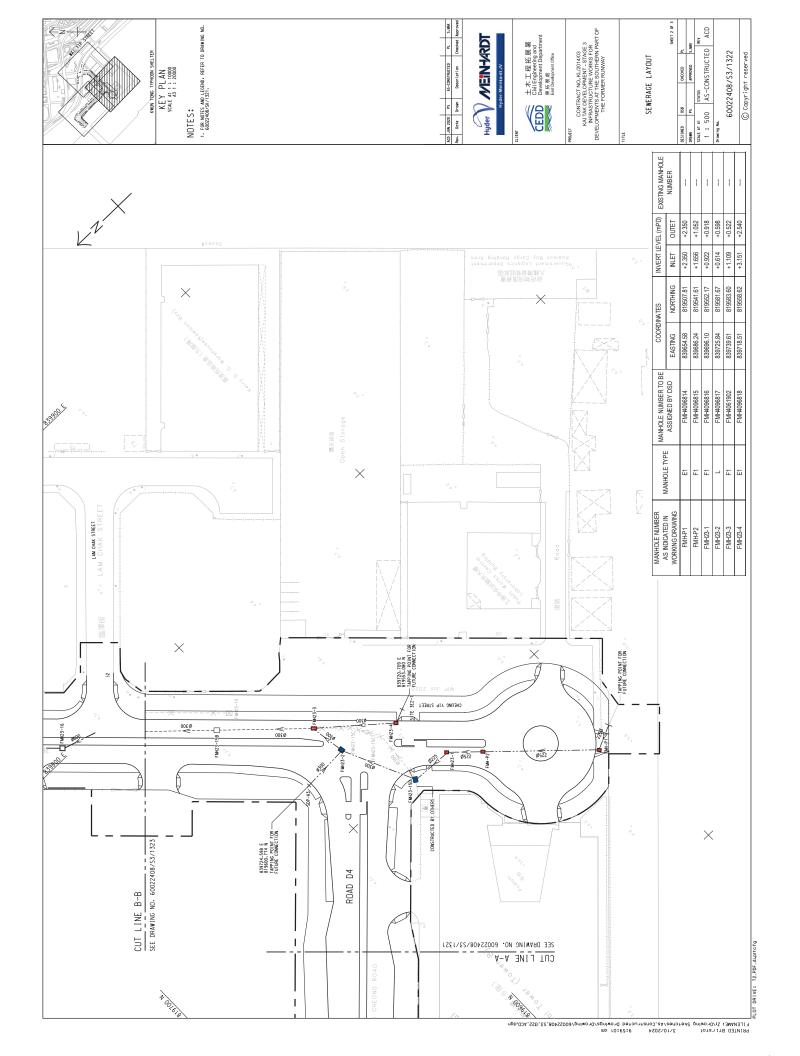
HONG KONG

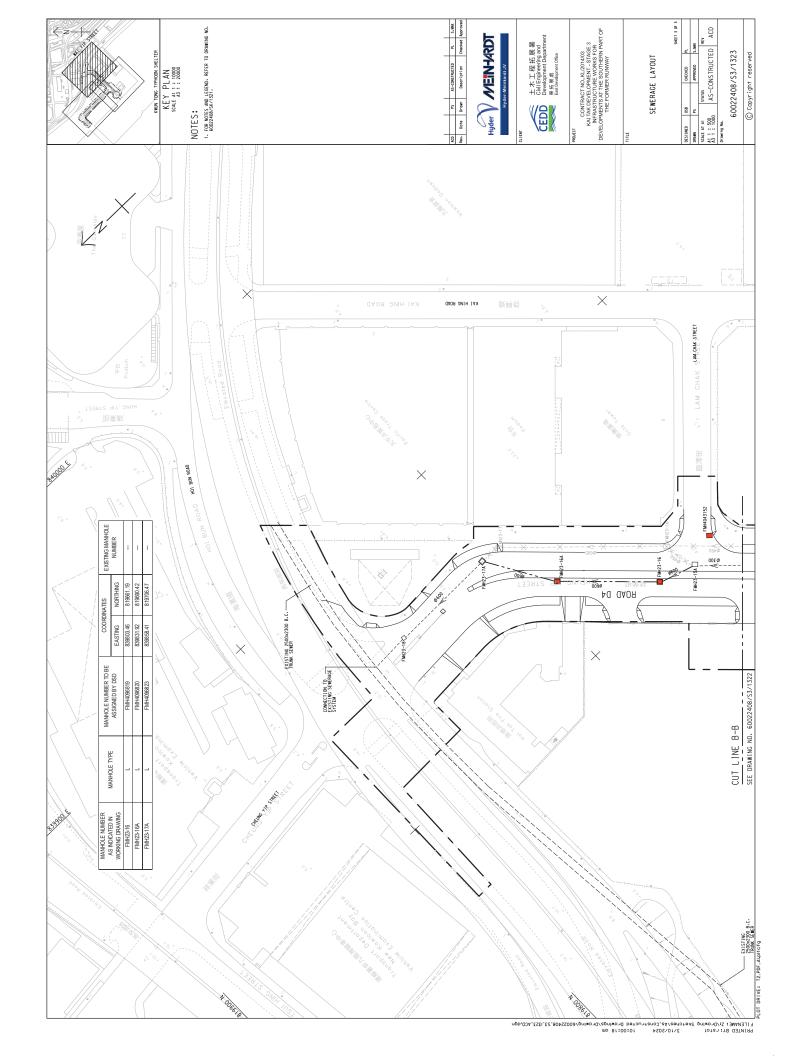
**Restrictions for Permitted Private Housing Development** with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

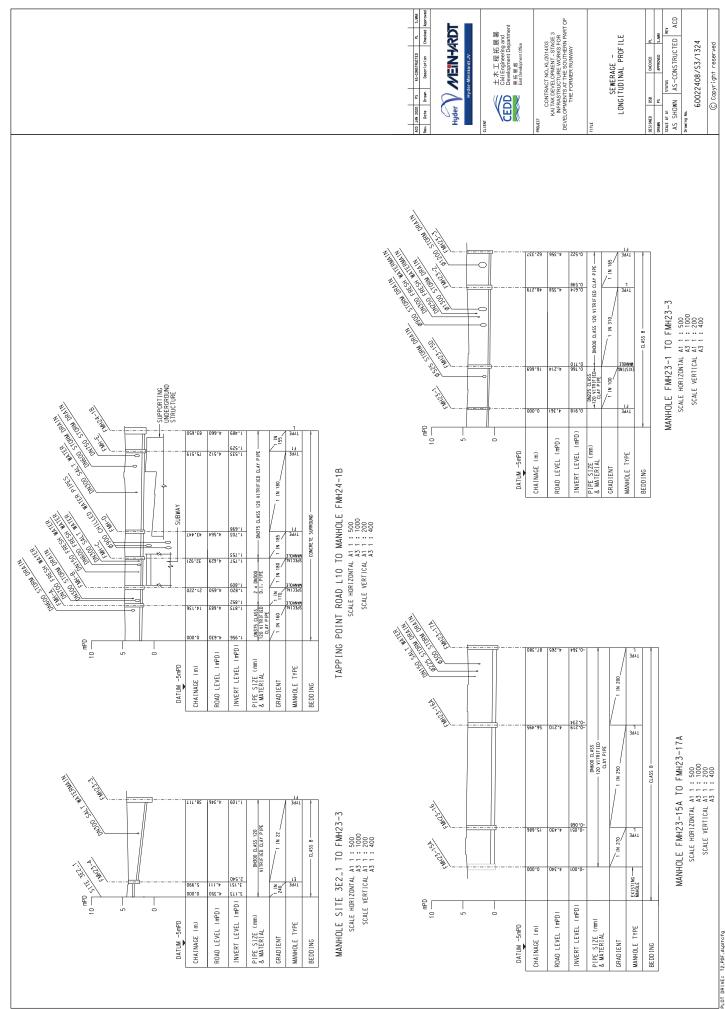


# Appendix B

## **As Built Information**







Marzoo   Par   Ac-CORTINGTED   N   S.WM
Hyder  CEDD  CHART  Hyder  CLINT  CLINT  CLINT  CLINT  CLINT  CEDD  CED  CEDD  CEDD

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FMH23-		
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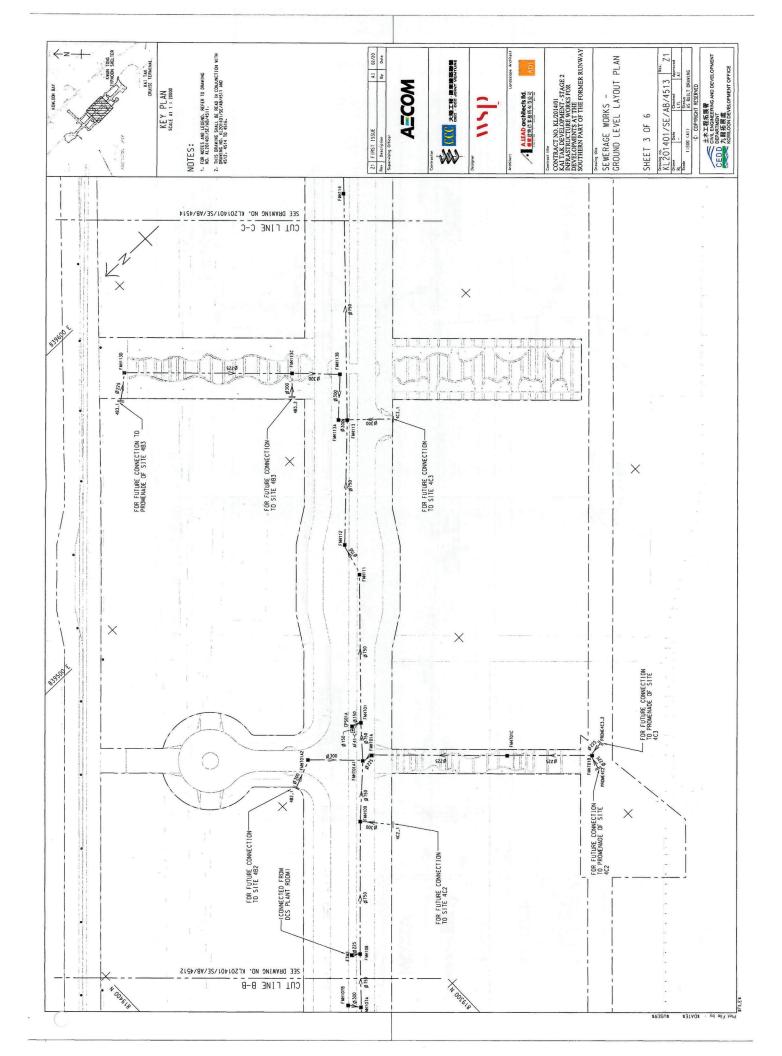
CONCRETE SURROUND

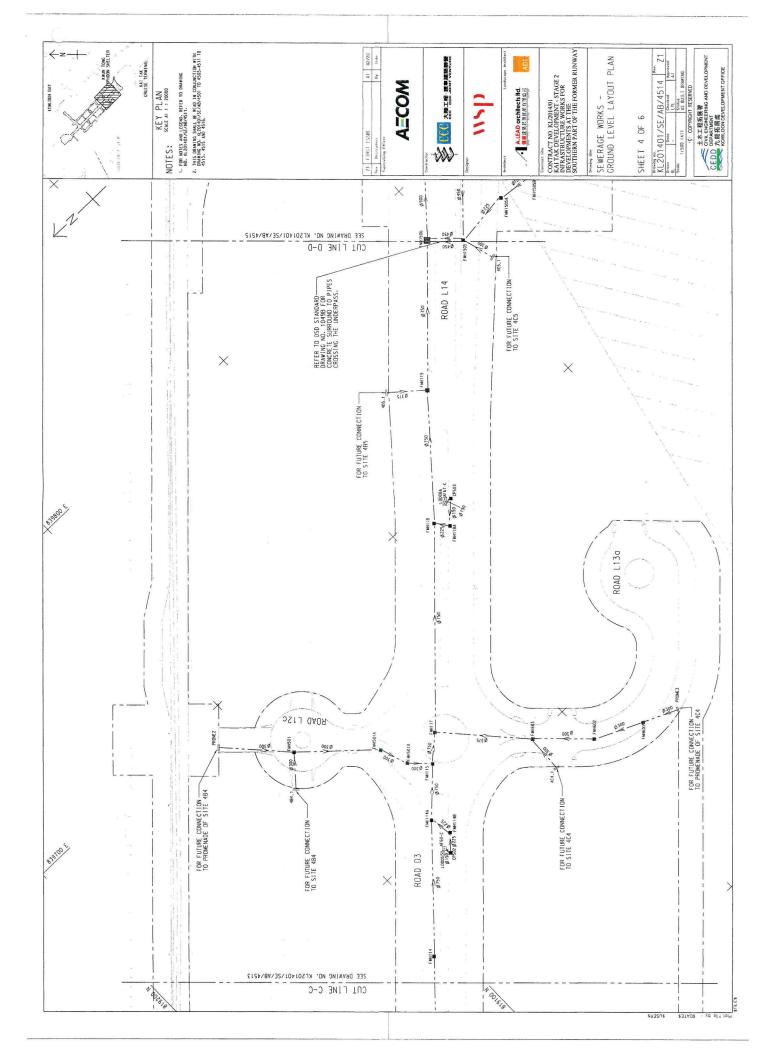
BEDDING

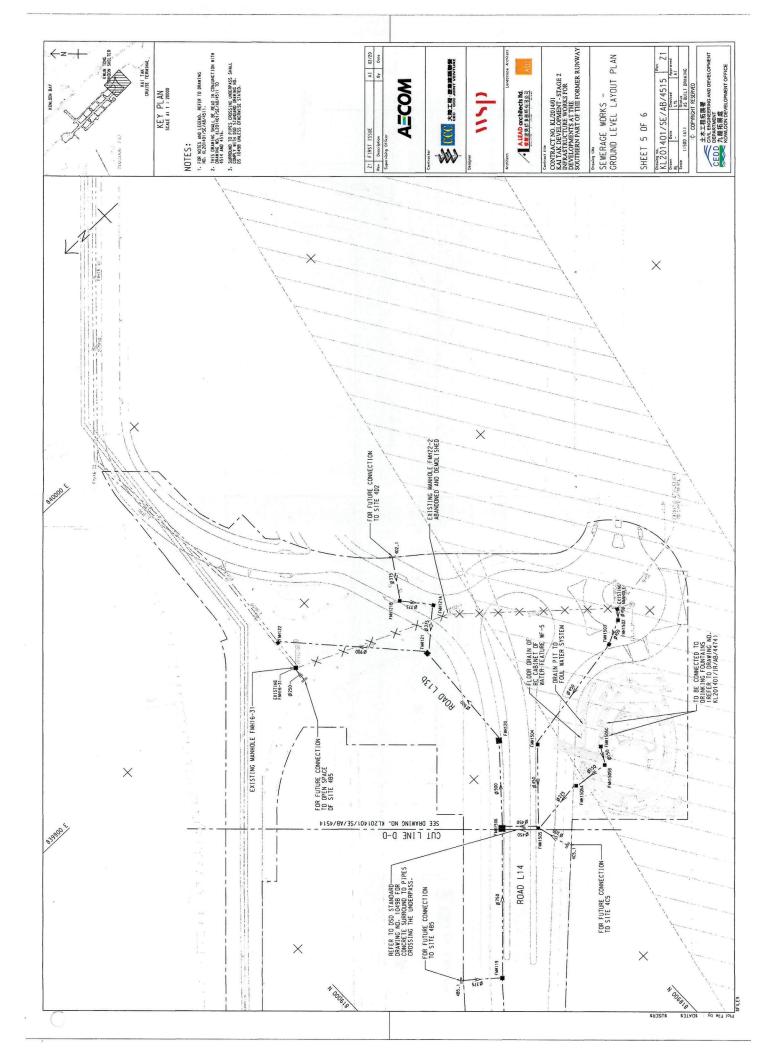
GRADIENT MANHOLE TYPE

ROAD LEVEL (mPD)
INVERT LEVEL (mPD)

DATUM −5mPD CHAINAGE (m) PIPE SIZE (mm) & MATERIAL







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4	Þ	1

PRONEALS	11 11 11	1 1 1	1 1 1	1 1 1 1	1 1 1						-	_	_		-	F	T	PDOME4C3**	+	POOMETS:	+	+	+	FMH501B FMH115	1		-		Н	FMH1505C FMH1505B		+		H1502 FMH1503	FMH1503 FMH1504	+	FMH1505 FMH1506		FTM1 FMH106		FTM2 FMH108	1	CPS01A FMH701	+		1	FMH114B FMH114A	CPS03 FMH118A	V	1	LDB01A FMH107			DS = Desilting Openity = Temp	SS2 =Sewerage Special Manhol														
	FMH	***	- WH	FMH	FMH	JWOOd	FROME	EME	EWH	1	FMH		PROME	FMH	FMH	FMH		Moda	TOWN TOWN	A C G G	LANGE	FMI				PROA	FMI	FME	FMI	FMH	FMH	IMI	FMH4	FMH	FMH	FMH	FMH		FT		FT		CPS	1	AFC	Ö	FMR	5	FMH		rpr		Notes:	DS = De	SS2 =Sev				, ,	_	-			,				_	
Mennok							DS				DS		DS	3					DS			DS	M		1	1	Si	1	DS											-										and the second																			
3/ 3/		V.C./ F	V.C. / F	V.C./F	V.C. / F	V.C. / F	CONC./M	CONC./M	CONC./M	CONC./M	CONC./M	CONC. / M	CONC./M	NO ON O		CONC./M	CONC./M	CONC./M	CONC./M	CONC./M	CONC./M	CONC./M	CONC./M	CONCIN	CONC. IN	CONC. IN	CONC./M	CONC./M	CONC./H	V.C./F		-	V.C./ F		V.C./F		V.C./F	V.C./F	V.C./ F	V.C./F	V.C./F		V.C./F	V.C./F	4101	V.C./F	V.C./F	VC/F	100.1	V.C./F		V.C./F	V.C./ F	V.C.J.F		V.C./ F	V.C.J.F		V.C./F		V.C./F	V.C./F	V.C.F	V.C./F			CP = Catchpit	Jown Pipe	
			B	8	B	8	<b>m</b>	8	m	8	8	8	8	9	9	9	8	8	9	В	æ	8	B	2	9 6	3 8	3	8	m	B			8		8		8	8	80	8	8		8	8		8	8	~	9	В	1	S	B	B		9	8		B		8	20	9	В			CP=0	DP = L	
	13	E	E1	EI	EI	5	=	=	=	=	H	н	E1	1	=	=	=	=	-	-	1	L/BD3	r	-	700	700	٦,	1	SS3	SS3		-	EI		EI		E1	EI	E1	EI	EI		EI	5		13	H	2	ā	FI		1	E	н		#	H		F1		SSI	FI	1	ı					
	211 10	EI	E1	EI	EI	<b>=</b>	5	=	=	н	н	H	1		3 :	=	=	н	H	-	I	7	L/BD3	-	,	7	282	1	7	PLUG			PLUG		PLUG		PLUG	E	<b>E</b>	E1	EI		PLUG	EI	01111	PLUG	E	PLIIG	2071	PLUG		PLUG	PLUG	EI		PLUG	PLUG		PLUG		PLUG	PLUG	FI	1					
Creary	310	483	458	404	832	275	1901	089	975	864	546	946	980	200	989	1060	427	916	642	129	121	42	329	217	710	977	157	237	234	45			426		310		01	255	197	969	878		102	293		68	113	181	101	55	!	17	7.3	25		94	44		43		33	2.4	25	12			_	3	
1	(4)	131	1.57	1.47	1.73	1.76	1.85	2.12	2.18	1.96	2.31	2.38	2.06	00.7	1.79	1.91	2.28	2.39	2.63	19.7	3.32	3.52	4.02	111	4.11	3.94	4.11	4.48	5.40	1.78			1.25		1.29		1.38	1.65	1.68	1.50	2.00		1.99	1.53	1	2.07	1.97	1 97	1.31	77.7		3.11	1.67	1.83		2.52	2.21		1.11		2.76	3.06	3.49	4.22			ole Type	ole Type	
(mLD)	3.019	3.845	3.835	3.622	3.238	3.225	3.301	3.303	3.290	3.191	3.133	3.083	3005	2.000	2.998	2.974	2.890	2.846	2.788	2.485	2.001	1.503	1.227	0000	7967	0.787	0.667	0.524	0.032	2.947			3.742		3.715		3.419	3.348	3.301	3.028	2.980		3.012	2.950		2.932	2.730	3 3 3 4	3.334	2.734		1.888	3 320	3.024		2.979	2.795		2.783		2.040	2 370	1.645	1.121			SSI =Sewerage Special Manhole Type I	ecial Man	
(m)	707 6	3.325	3.125	2.976	2.650	2.575	2.544	2.527	2.433	2.383	2.346	2.278	1366	167-7	2.227	2.168	2.111	2.042	1.896	1.309	1.076	0.483	0.221	177.0	0.045	-0.777	-0.365	-0.566	-1.119	2.685			3.326		3.300		3.052	3.009	2.730	2.660	2.640		2.664	2.556		2.504	2.250	3 070	6167	2.178		0.580	2 930	2.465		2.559	2.087		2.128		1.348	1 279	0.763	-0.749			verage Sp	verage Sp	
(mLD)	2000	3.470	3.235	3.122	2.658	2.645	2.575	2.544	2.527	2.433	2.383	2.333	3775	617.7	2.248	2.224	2.140	2.096	2.038	1.735	1.251	0.753	0.477		717.0	0.037	-0.233	-0.376	898.0-	2.722		ı	3.377		3.340		3.119	3.048	3.001	2.730	2.660		2.712	2.650		2.632	2.430	3.034	9:034	2.434	1	1.513	3 029	2.724		2.679	2 495		2.483		1.740	1 005	1,770	0.746			SSI =Sev	3S3 =Set	
+	-	5.150	+-			+	5.178	-	$\rightarrow$	5.140	5.440	+-	+	+	+	-+	-	5.240		5.093	-	+	+-	+	+	+	-	5.000	5.427	4.730	-	T	5.000	-	2.000	-	4.800	5.000	4.980		4.960	-	$\rightarrow$	4.480	+	2.000	_	+	3.300	5.000	-	2.000	2 000	+	-	5.500	2 000		2.000	$\rightarrow$	4.800	5475	-	+			<b>9</b> 1	3	
(mm)		375	1		-	009		+	$\dashv$	-	750	+	+	+	750	+		-			+	750	+	+	+	+	+	006	006	225			375		375		300	300	300			-	300		$\pm$	300	-	300	+	300	+	375	300	1		300	300	+	300		300	+	+	375	1				
_		70.035	50.402	950.65	6.659	19.234	32.887	11.553	91.623	43.175	20.209	52.031	22.630	73.550	14.387	59,355	12.393	49.454	91.192	55.075	217.112	11.259	84 184	101.104	52.941	59.136	33.913	44.965	28.665	1.668			21.742		12.403		0.684	9.953	53.293	44.552	11.558		4.908	27.563		11.402	20.297	2200	9.976	14.114		15.930	2362	6.597		11.337	21871		15.429		13.028	17.465	12 506	17.727			ary Plug	Type 2	
No.		FMH102			Н	-	FMH106AA	-	7	FMH107A			1.	+	+	1	FMH112	FMH113		_		+	t	1	+			FMH121	FMH122	FMH16-1			FMH102		FMH102		FMH201A	FMH201	FMH202	-				FMH105		FMH701A2		20000	FMHIISC	FMH501		FMH119	CMUADI	FMH107		FMH109	EMHIII		FMH603		FMH1505	EMUINIO			1		effing Tempor	ecial Manhole	
No	100	Metro Park	FMH102	FMH103	FMH103A	FMH104			FMH106	FMH107	FMH107A	FMH108	t	t	FMH701A1	FMH701	FMH111	FMH112	FMHII3	FMH114	FMH114A	PMH115	FMHIII	FMHIII	FMHII8	FMH119	FMH1506	FMH120	FMH121	FMH122 PULG			4AI_1**		4A2 1**		PROMEI**	FMH201A	FMH201	FMH202	FMH104A		4B1_1**	FMH301	$\neg$	4B2 1**	$\neg$	200 100	483 2**	4B4_1**		4B5 1**	101 107	FMH401		4C2_1**	463 100	1	4C4_1**		4C5 1**	400 100	FMH121B	FMH121A		Notes:	DS = Desilting Operfing Temporary Plug	SS2 =Sewerage Sp	

12   12   12   12   12   12   12   12	12.5   1.04   1.04   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05   1.04   1.05	orc	DS MILVOTG		Trial Dia	-	US B.		CoverLevel	Cover	Gradleat	US Manhok Type	DS Manhole Type	Bedding	Pipe	Remark for US	1. FOR GENERAL NOTES AND LEGEND SHALL REFER TO DRAWING NO. KL201401/28/A8/A821. 2. THE MANHOLE TYPE IN COMPULIE CHAIL REFER TO
12   120	12   120	+	No.	(B)	+	+	(mPD)	(mPD)	(mPD)	(m)	(1 ln x)	01111				Inhole	DOWNSTREAM MANHOLE.
12   12   12   12   13   13   14   15   15   15   15   15   15   15	12   12   12   12   12   13   13   13	+	Acolh	7.154	+	+	3.340	3,335	1/66	1,03	130	PLUG.	1	9 9	A.C.		3. CONTENT IN THE REMARK SHALL REFER TO
15   15   15   15   15   15   15   15	1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	+	geniu		+	+	2.002	3.000	3.110	1.10	167	10	1.0	9 8	VC/E		DOMINSTREAM MANHULE.
12   12   12   12   12   13   13   13	15   15   15   15   15   15   15   15	t	Tion of	+	Ť	-	2007	2000	3.133	00	1,66	1	1.3		VC/E		4. THE GROUND LEVEL OF MANHOLE SHALL BE VEHIFT
12   12   12   12   12   13   13   13	1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	t	HIOSD	-	+	-	100	7.000	201.0	7.00	201	4	1	1	4 10 10	-	
12.5   5.300   5.251   5.210   5.476   5.187   5.40   5.187   5.40   5.210	12.5   4.500   1.251   1.210   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240   1.271   1.240	+	TH 105	27.762	+	+	2.872	2.631	3.097	1.92	cii	13	5	a	V.C./ F		
12   12   12   12   12   13   13   13	125   5.500   3.155   3.175   3.476   3.187	- 1	1	+	+	+		1							1		
215 4.96 2.197 2.1	215 4.96 2.19 2.29 2.19 2.19 2.19 2.19 2.19 2.19	- 1	H113D	-	$\dashv$	+	3.251	3.210	3.476	1.82	569	PLUG	EI	9	V.C./F		
15   15   15   15   15   15   15   15	10   5.45   2.19   2.85   2.19   2.18   2.		H113C	66.620		-	3.162	2.976	3.387	1,57	358	EI	EI	8	V.C./F		
15.   15.	State   2.455   2.455   2.455   3.155   2.156   3.155   2.156   3.155   2.156   3.155   2.156   3.155   2.156   3.155   2.156   2.15	_	HII3B	18,302	-	-	2.973	2.889	3.273	2.18	218	EI	E1	8	V.C./F		
128   5.366   2.481   2.460   2.491   2.18   9   EL   H   B   V.C.P     128   4.290   2.581   2.580   2.491   2.39   2.291   2.291   2.291   2.291   2.291   2.292	13   13   13   13   13   13   13   13	-	HII3A	_	_	-	-	2.842	3.179	2.02	463	EI	EI	8	V.C./F		
15.5   5.96   1.561   1.561   1.562   1.566   1.460   1.261   1.562   1.566	125   5.00   1.581   1.281   1.281   1.281   1.282	t	111113	-	+	-	⊢	2.602	3.125	2.18	8	EI	=	B	V.C./F		
125   4.000   1.55   1.55   1.586   1.40   1.50	125   4.96   1.264   1.265   1.266   1.269   1.265   1.267	t		_	+	+											
12   4.95   2.50   2.	12   1.590   1.294   1.295		0.0231	+	+	+-	1000	1613	3 000	9.40	30	DILLIG	1.2	g	VCIE		
125   4700   1250   1250   1250   1251   125	125   4790   1290   4200   4212   241		dio/a	-	+	+	1000	2010	000.7	4.47	300	200		9	2/0/2		
125   4700   1259   1260   1261   1261   1261   1262   1	200 5.300 1.357 1.200 1.267 2.20 5.6 PLUG EI B VC_F F 200 5.300 1.555 1.258 2.351 2.276 2.29 5.6 PLUG EI B VC_F F 200 5.300 1.555 1.288 2.365 2.25 1.29 1.36 1.35 1.37 1.20 1.36 1.36 1.36 1.36 1.36 1.36 1.36 1.36	†	H/01C	-	+	+	7.500	604.7	16/.7	77.7	276	17	17	3 8	V.C. F		
256   1.250   1.250   2.550   2.51   2.75   2.55   2.50   2.51   2.75   2.50   2.50   2.51   2.75   2.50   2.50   2.50   2.51   2.75   2.50   2.50   2.50   2.51   2.75   2.50   2.50   2.51   2.75   2.50   2.50   2.51   2.75   2.50   2.50   2.51   2.75   2.50   2.50   2.51   2.75   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50   2.50   2.51   2.50	120   1.250   1.250   1.250   1.251   1.275   1.250   1.251   1.275   1.250	1	H701A	+	+		-	2.290	779.7	87.7	cac	2	3	3	4.7.4		
200 5.200 1.755 1.880 2.055 2.541 3.796 2.59 5.6 PLUG EI B V.C.JF	200 5.300 1.259 1.281 2.796 1.250 1.	T	H701A1		$\neg$		-	2.250	2.515	2.27	79	E1	=	S	V.C./F	T	
200   5.300   1.256   1.264   1.27   1.264   1.27	200   5.300   1.256				+	$\rightarrow$											
100 5.000 1.755 1.589 2.055 2.95 19.7 PLUG E1 B V.C./F   B V.C./F	100 5.000 1.755 1.589 1.680 1.295 1.92 PLUG EI B V.C./F  100 5.210 1.260 1.250 1.260 1.800 1.250 1.600 3.60 1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.2	+	H701B	3.634	$\rightarrow$	$\rightarrow$	2.596	2.531	2.796	2.50	99	PLUG	EI	В	V.C./ F		
100 6.200 1.755 1.862 7.867 1.87	100 6.200 1.755 1.856 7	$\forall$		-	-	$\neg$	-						-			1	
300 5.210 1.464 1.137 1.744 3.51 1.52 FT FT B V.C.P F  300 5.210 1.464 1.137 1.744 3.51 1.52 FT L B V.C.P F  300 5.210 1.464 1.137 1.744 3.51 1.52 FT L B V.C.P F  300 5.210 1.265 1.265 1.265 2.265 2.91 FT L B V.C.P F  300 6.200 1.215 1.127 2.141 2.240 2.91 FT L L B V.C.P F  316 5.310 1.315 1.473 2.415 2.240 2.91 FT L L B V.C.P F  450 5.300 1.215 1.137 1.471 2.137 2.13 1.25  2.91 FT L L B V.C.P F  451 5.300 1.215 1.137 1.415 1.217 2.115 1.21  2.91 1.1 SST SSI B V.C.P F  452 5.300 1.215 1.136 1.353 1.135 1	1900 5.210 1.550 1.66 1.860 1.550 1.670 1.560 1.670 1.570 1.670 1.570 1.670 1.570 1.670 1.570 1.670 1		TH501	_	$\dashv$	-	$\dashv$	1.588	2.053	2.95	192	PLUG	FI	9	V.C./F		
190 5.210 1.444 1.157 1.794 3.451 162 FT E B V.C.F F F B V.C.F F F B V.C.F F F F F B V.C.F F F B V.C.F F F F B V.C.F F F B V.C.F F F B V.C.F F F B V.C.F F	190 6.210 1.444 1.137 1.794 3.451 1.62 FT FT B V.C.FF  190 6.210 1.244 1.245 1.245 3.459 1.25 1.24		H501A	_		_	1.580	1,406	1.880	3.14	961	F1	FI	B	V.C./F		
250   2.10   1.356   1.456   2.56   2.66   2.59   PUIC   E1   B   V.C.JF     250   2.200   2.256   2.46   2.50   2.51   2.50   2.51   E1   B   V.C.JF     250   2.500   1.15   1.47   2.41   2.50   2.51   E1   E1   E   V.C.JF     251   2.10   2.11   1.47   2.41   2.50   2.51   E1   E1   E   V.C.JF     252   2.10   1.45   2.45   2.45   2.51   2.51   E1   E1   E   V.C.JF     253   2.10   1.45   2.45   2.45   2.51   2.51   E1   E1   E   V.C.JF     254   2.40   1.25   1.25   1.25   2.51   E1   E1   E1   E1   E1   E1   E1     255   2.40   1.25   1.25   2.41   2.51   2.51   E1   E1   E1   E1   E1   E1     255   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     255   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     255   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     256   2.50   2.51   2.51   2.51   2.51   2.51   2.51   2.51   2.51     257   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     258   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     258   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     258   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     258   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     259   2.41   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     250   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     250   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     250   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40   2.40     250   2.40	250   1.216   1.265   1.265   1.265   2.266   2.2	-	H501B				_	1.337	1.704	3.51	162	E	E	8	V.C./ F		
150   1210   1	200   2.10   2	t	21111	-	+		-	1 363	9691	3 68	133	-	-	2	VC/F		
250   2.226   2.126   2.126   2.256	250   2.256	t		+	+		+-										
150   4.590   2.115   2.145	150   4.590   2.110   2.115   2.482   2.50   2.51   2.115   2.482   2.52   2.51   2.	H	TH601	14.742	+	+-	2.236	2.186	2.536	2.66	295	PLUG	EI	9	V.C./F		
156   1,10   1	156   1.10   1.15   1.15   2	t	COVER S	10 707	+	-	2 183	3116	2 483	2.50	102	-	FI	~	VCIE		
156 5.130 3.165 3.097 3.15 1.12 2.00	150   1.10   1.14   1.15   1	t	1000		+	+-		1 670		050	22	Ğ	1.0	9	3/3/		
155   51.00   1.540   1.540   2.144   2.91   1.1   1	155   5,100   1,201	t	COOL	-	$^{+}$		+		-	1	2				1000		
156   5130   5146   5140   5	156   5130   5146   3497   3416   1427   390   E1   E1   B   uPVC     215   5130   1439   1436   1431   1431   315   142   139   142   1431	†	(HIII)	-	+		-	.543	2.045	3.02	167		LIBDS	9	V.C.7 F		
156   51.00   3.055   3.145   3.155	156   51.00   3.016   3.018	1	1	1	+	-+	1	1									
126   51.00   1.00	126   51.00   1.39   1.156   1.151   1.25		H1505B	+	+	$\rightarrow$	3.165	3.092	3315	1.82	90	EI	3	9	uPVC		
255 5.000 1.89 1.36 2.114 2.91 41 SST SSI B V.C.F   P. C.	255   5,000   1,395   1,365   1,114   2,91   41   5,87   581   B   V.C.F   1,114   1,114   1,115   1		H1505A	_		-	3.035	1.883	3.185	1.95	12	EI	SS7	8	uPVC		
See   1.20   1	\$450 5.200 1.229 1.236 1.259 2.25 2.25 EXISTING L CS V.C.PF  \$450 5.500 1.229 1.236 1.239 3.24 3.05 4.00 L L CS V.C.PF  \$2 4.501 1.22 0.385 1.299 3.48 494 L L L CS V.C.PF  \$2 4.50 6.370 0.324 1.299 3.58 2.99 5.51 5.51 5.50 CS V.C.PF  \$2 5.500 1.32 0.385 1.299 3.58 2.99 5.51 5.51 5.50 CS V.C.PF  \$2 5.500 0.325 1.299 0.285 1.299 8 EXISTING H B V.C.PF  \$2 5.500 5.500 2.266 3.262 2.260 2.26 1.21 5.10 E. FI B UPVC  \$2 5.500 5.322 3.404 3.27 3.21 3.17 E. FI B UPVC  \$2 5.500 2.252 2.253 2.253 2.254 3.50 0.25 1.29  8 FI P P P V.C.PF  \$2 5.500 5.322 3.405 3.207 3.20 1.20 1.20 E. FI B UPVC  \$2 5.500 5.322 3.405 3.207 3.20 1.20 1.20 FI B UPVC  \$2 5.500 5.322 3.405 3.207 3.20 1.20 FI P P V.C.PF  \$2 5.500 5.322 3.405 3.207 3.20 1.20 FI P P P V.C.PF  \$2 5.500 5.300 5.300 5.300 0.205 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 5.300 0.205 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 5.300 0.205 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 5.300 0.205 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 5.300 0.205 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 0.205 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 0.205 1.20 1.20 1.20 1.20 FI P P UPVC  \$2 5.500 5.300 5.300 0.205 1.20 1.20 1.20 1.20 FI P UPVC  \$2 5.500 5.300 0.205 1.20 1.20 1.20 1.20 1.20 1.20 FI P UPVC  \$2 5.500 5.300 0.205 1.20 1.20 1.20 1.20 1.20 1.20 FI P UPVC  \$2 5.500 5.300 0.205 1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.20		H1505	-			_	1,356	2.114	1.91	41	SS7	SSI	8	V.C./F		
1.00   1.29   1.286   1.29   1.286   1.29   1.286   1.29   1.286   1.29   1.286   1.29   1.286   1.29   1.286   1.29   1.28   1.29   1.28   1.29	1.5   1.2	t		-	1		-										
12   12   12   12   12   12   12   12	12   12   12   12   12   12   12   12	+		-	+		000	1 104	1 740	3.16	T	EVICTING	_	2	AC.		
14   15   15   15   15   15   15   15	146   125	+	700	4	+		1	200		-	Т			3	4101		
469 6.500 1.122 0.186 1.468 1.485 4.94 L. L. B. V.C.P.F   Phys. Decorporation of the control of	446 6, 5,00 1,122 0,186 1,185 4,94 L L B V.C.P F 1   1   1   1   1   1   1   1   1   1	1	H1503	+	+	+	1.275	1.253	1.725	3.93	400	1	1	3	V.C.) F	1	
1.456   4.700   0.181   1.127   0.185   1.127   0.185   1.127   0.185   1.127   0.185   1.127   0.185   1.127   0.185   1.127   0.185   0.185   1.127   0.185   0.18	1466   4,780   0,132   1,139   3,45   2,15   5,51		H1504	48.937	_	-	1.235	1.136	1.685	3.85	464	7	7	8	V.C./F		Elber reede
224 64.70 6.749 0.281 1.199 3.58 29 SS1 SS2 CS V.C.F DS.CS  225 6.377 4.111 3.733 4.336 1.199 8 EXISTING H B V.C.F   226 6.376 2.322 3.444 3.377 2.31 177 EI H B UPVC  227 5.440 3.269 4.580 0.22 1.10 CP EI B UPVC  228 5.40 1.592 1.269 4.580 0.21 1 CP EI B UPVC  228 5.40 1.592 1.269 4.580 1.261 1.2 EI FI B V.C.F   229 5.40 1.502 2.243 3.201 3.4 FI FI B UPVC  220 5.00 4.30 1.205 2.40 1.205 2.40 1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.2	214 64 770 6.719 6.218 1.199 3.58 29 SS1 SS2 CS V.C.F DS.CS  215 6.217 4.111 3.733 4.236 1.199 8 ENSTING H B V.C.F F  216 6.617 3.020 2.606 3.266 3.218 8 ENSTING H B V.C.F F  217 5.440 3.020 2.606 3.252 3.421 2.18 8 ENSTING H B V.C.F F  218 5.400 4.310 2.458 3.831 3.77 2.11 1.7 E1 H B UPVC  219 5.400 4.310 2.458 3.831 3.75 2.11 1.7 E1 H B UPVC  210 5.400 1.205 1.205 2.201 2.80 0.42 1 75 E1 F1 B UPVC  211 5.400 1.205 1.205 2.201 2.80 0.42 1 75 E1 F1 B UPVC  212 5.400 1.205 1.205 2.201 2.80 0.42 1 75 E1 F1 L B V.C.F F  213 5.400 1.205 1.205 2.201 2.30 1 7 F1 F1 B UPVC  214 6.205 3.402 1.203 3.401 2.30 1 7 F1 F1 B UPVC  215 5.400 1.205 1.205 1.201 2.30 1 7 F1 F1 B UPVC  216 5.500 3.402 1.203 3.401 2.30 1 7 F1 F1 B UPVC  217 6.205 3.402 1.203 3.401 2.30 1 7 F1 F1 B UPVC  218 5.400 1.205 1.20		H1505	-		_	1.122	986.0	1.572	3.45	235	7	SSI	S	V.C./ F		
125   6.377   4.111   3.733   4.236   3.189   8   EXISTING   H   B   V.C./F     126   5.662   3.222   3.404   3.277   2.31   1.7   E1   H   B   V.C./F     127   5.440   2.592   2.592   2.590   2.541   2.592   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.592   2.590   2.590   2.592   2.590   2.5	125   6.317   4.111   3.133   4.236   3.246		9051H	13.624 2	2 x 450	_	0.749	0.283	1.199	3.58	53	SSI	SSZ	S		S, CS	on a continue
255 6477 4.111 3.733 4.336 1.99 8 EXISTING H B V.C.F   Connection	225 6.377 4.111 3.733 4.336 1.199 8 EXISTING H B V.C.F																Supervising Uniced
125   5.440   3.040   2.056   3.266   2.116   6   EXISTING   H   B   VCLF     150   5.640   3.272   3.441   3.277   2.11   17   E1   H   B   UPVC   DP     225   5.490   2.592   2.287   2.890   2.217   3.19   B   E1   H   B   VCLF     225   5.490   2.292   2.292   2.290   2.240   2.200   2.17   E1   E1   B   VCLF     225   5.190   1.697   2.245   2.240	126   5.642   3.040   2.056   3.156   2.118   8   EXISTING   H   B   V/C/F     150   5.642   3.222   3.043   3.372   2.131   17   E1   H   B   uPVC   DP     200   5.000   4.390   2.494   2.820   2.64   175   E1   E1   B   V/C/F     215   5.040   2.595   2.450   2.450   2.60   17   E1   E1   E1   B   V/C/F     215   5.050   2.225   1.935   2.450   2.60   17   E1   E1   E1   B   V/C/F     216   5.050   3.462   2.439   3.612   2.34   4   CP   H   B   uPVC     217   5.190   2.235   2.439   3.612   2.34   4   CP   H   B   uPVC     218   5.190   2.235   2.439   3.612   2.34   4   CP   H   B   uPVC     219   5.950   3.462   2.439   3.612   2.34   4   CP   H   B   uPVC     210   5.950   3.462   5.641   Manhel Typa   TP   TP   TP   TP   TP   TP   TP   T		1H106	_		-	4.111	3,733	4,336	1,99		EXISTING	H	8	V.C.J.F		-
150   5.440   3.040   2.656   3.125   3.041   3.775   2.11   17   E1   H   B   V.C.F     150   5.662   3.222   3.043   3.77   2.11   17   E1   H   B   uPVC     210   5.090   4.380   2.459   2.651   2.65	150   5.640   3.040   2.694   3.265   2.18   8   EXISTING   H   B   V.C.F     150   5.642   3.242   3.441   3.77   2.31   17   E1   H   D   uPVC     200   5.000   4.380   2.642   4.580   0.42   1   CF   E1   B   V.C.F     215   5.440   2.595   2.242   2.820   2.61   15   E1   E1   B   V.C.F     215   5.490   2.235   2.490   2.21   3.19   81   F   F   B   V.C.F     215   5.310   1.667   0.326   1.292   2.430   2.43																VICUITY
150   5.687   3.121   3.641   3.177   2.13   1.17   E.   H   B   uPVC   DP	159   5.687   3.222   3.840   3.972   2.31   17   E1   H   B   uPVC   DP   Contractor		1H108	2.602	-	-	3.040	2.696	3.265	2.18		EXISTING	H	В	V.C./ F		
159   5.647   3.342   3.342   3.342   3.372   2.31   17   E1   H   B   uPVC   DP   Controver   220   5.640   5.251	159   5.687   3.222   3.441   3.577   2.31   17   E1   H   B   uPVC     220   5.000   4.380   2.649   4.580   0.42   1   CP   E1   B   V.C.P   P     221   5.440   1.592   1.593   2.141   3.19   18   F1   B   V.C.P   P     222   5.440   1.593   1.593   2.241   3.19   18   F1   F1   B   V.C.P   P     223   5.440   1.593   2.433   3.24	H			-	-											
200   5.00   4.380   5.459   5.450	200   5.000   4.340   2.649   4.580   0.42   1   CP   E1   B   UPVC   DP   CE   CE   CE   CE   CE   CE   CE   C		1H701	-	-		3.222	3.043	3,372	2,31	17	E1	H	В	uPVC		Contractor
225 6 5.00 d. 2.40 d.	225   5.000   2.595   2.544   4.580   0.427   1.00   2.595   2.544   2.590   2.545																
225   5440   2585   252   280   2461   75   E   F   B   VC.F     225   5440   2592   1592   2217   219   2	225   5.459   2.555   2.524   2.529   2.61   75   E   F   B   V.C.F		PS02	1.944	-	-	4380	2.649	4,580	0.42	-	CP	E1	B	uPVC	DP	
225 5.406 1.992 1.899 2.217 3.19 81 F1 1 B V.C.F Property 1.209 2.209 2.209 2.209 2.209 2.20 1.7 F1 F1 B UPVC Property 1.209 2.429 2.429 2.439 2.449 2.40 2.40 1.7 F1 F1 B UPVC Property 1.209 2.429 2.429 2.439 2.449 2.44 4 CP H B UPVC Property 1.209 2.429 2.4	225 5.10 1.592 1.698 2.217 3.19 81 FI 1 B V.C.F Professor 2.22 5.130 1.667 0.226 1.892 3.24 5 FI L B V.C.F R.		HIIAR	-	-	-	2 595	2.524	2.820	2.61	75	E1	FI	8	V.C./F		
Decision	125   5.050   2.225   1.675   2.450	T	H114A	+-	+	+-	1.992	1.898	2.217	3.19	81	FI	-	8	V.C./F		
225   5369   2225   1,675   2459   2450   1,75	155   5,050   1,275   1,675   2,450   1,207   1,507	1			T	+				The same of the sa							Neglect
150 5.950 3.462 2.423 3.612 2.34 4 CP H B uPVC   Notice     SSI = Severage Special Manhole Type   CP = Catchpit   CP = Catch	150   5,950   3,462   2,428   3,612   2,34   4   CP   H   B   UPVC	T	W118A	1	-	+	1225	1.675	2,450	2,60	17	E	F	8	uPVC		Designer
150   5,950   3,462   2,428   3,612   2,34   4   CP   H   B   uPVC   Montest	150   5,950   3,462   2,428   3,512   2,34   4   CP   H   B   uPVC	-	HIIB	-	-		1.667	0.826	1.892	3.24	8	FI	1	8	V.C./F		
150   5,050   3,462   2,428   3,612   2,34   4   CP   H   B   uPVC   Wesherd	150   5.959   3.462   2.434   3.612   2.34   4   CP   H   B   uPVC	t			_												
SSI "Severage Special Manhale Type I CP = Carctpit	National   Alexandrian   Ale	t	TH107	-	-	+	3.462	2.428	3.612	2,34	*	CP	H	B	uPVC		
SSI -Sewerage Special Manhole Type I CP - Catchpit - CP - CA	SSI "Sewerage Special Manhale Type I DP = Down Pipe SS3 "Sewerage Special Manhale Type I DP = Down Pipe																
SSI "Severage Special Manhole Type I CP = Carchpit (Type I Defined Type I Define	SSI "Severage Special Manhole Type 1 CP = Catebpit SS3 "Sewerage Special Manhole Type 3 DP = Down Ppe										1					T	THE THE PERSON NAMED IN
SSI =Sewerage Special Manhole Type I CP = Catchpit	SSI -Sewerage Special Manhote Type 1 CP = Catchpit SS3 -Sewerage Special Manhote Type 3 DP = Down Pipe					Ì										T	
	SS3 = Sewerage Special Manhole Type 3 DP = Down Pipe	Desilting Opening	= Tempor	rary Plug		S	SS1 =Sew	erage Spe-	cial Manh	ole Type I				CP = C	tchpit		

Confrost line
CONTRACT NO. KLZ014/01
KAT TAK DEVELOPMENT-STAGE 2
KAT TAK DEVELOPMENT-STAGE 2
DEVELOPMENTS AT THE
SOUTHERN PART OF THE FORMER RUNWAY

SEWERAGE WORKS -MANHOLE SCHEDULE

| KL201401/SE/AB/4522 | Inter-| Riceas | Pole | Cuccase | Al Pole | September | September

上木工程和展響 COUL ENGINEERING AND DEVELOPMENT CEDD DEVERTIMENT TAN TAN TAN TAN TAN TO SEVELOPMENT OFFICE

# Appendix C

## **Detailed Sewerage Impact Assessment Calculations**



Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

Sheet No.

NP

Date

Rev.

Checked

23/01/2025

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Estimation of Sewage Flows Estimation from Catchments Not Affected by the Proposed Development

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

# **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

2. Planning Department CIFSUS.

3. BS EN 12056-2:2000 Gravity drainage systems inside buildings

External I	Discharge - Near Sites 3E1 and 3E	22	-	
EAUTHALL	Discharge - Mear Siles JET and JE	.=	_	
	vage Flow Estimates	Estimation	Unit	Remark
E_1 Kai Type	i Tak Fire Station	Social Services	-	
Tota	tal GFA	6,450	$m^2$	Building footprint on 5 floors
	sumed GFA per Employee uployee Population	30.3 213	m <sup>2</sup> /Person Persons	Table 8 CIFSUS - Social Services
Unit	it flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & Community, Social & Personal Services
Cate	chment Inflow Factors(PCIF)	1.1	-	Table T-4 of GESF - East Kowloon
ADV	, wr	65.60	m <sup>3</sup> /day	
	tal building ADWF	65.60	m <sup>3</sup> /day	
Disc	chages to: FMH4096823	65.60	m <sup>3</sup> /day	assumed
Ear		22.00		
E_2 Paci Type	cific Trade Centre	Manufacturing	-	
Tota	tal GFA	70,856	1	Building footprint time 17 floors. Source: geoinfo and https://property.jll.com.hk/en/industrial-lease/hong-kong/kowloon-bay/pacific-trade-centre-hk-p-1933
	sumed GFA per Employee uployee Population	43.5 1,630	m <sup>2</sup> /Person Persons	Table 8 CIFSUS - Manufacturing
Unit	it flow factor (UFF)	0.53	m³/person/day	Table T-3: Empolyee & J1 Manufacturing - East Kowloon
Cate ADV	chment Inflow Factors(PCIF)	1.1 950.29	- m <sup>3</sup> /day	Table T-4 of GESF - East Kowloon
	tal building ADWF chages to:	950.29		
	FMH4043148 FMH4043149	475.15 475.15		Assumed 50% flow split between the two discharge locations Assumed 50% flow split between the two discharge locations
		4/3.13		Assumed 5070 now spirt between the two discharge locations
E_3 Har	rbourside HQ	Business Services	-	
	tal GFA	63,080		Source: https://office.propwiser.com.hk/en/Building/kowloon-bay/harbourside-hq/316
	sumed GFA per Employee	18.2 3.470	m <sup>2</sup> /Person Persons	Table 8 CIFSUS - Business Services
_	ployee Population it flow factor (UFF)	3,470 0.08	Persons m³/person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services
Cate	chment Inflow Factors(PCIF)	1.1	-	Table T-4 of GESF - East Kowloon
ADV	DWF	42.81	m <sup>3</sup> /day	
	tal building ADWF	42.81		
Disc	chages to: FMH4043146	42.81		
E 4 Ker	rry D.G. Warehouse			Source: https://www.tpb.gov.hk/en/papers/MPC/K/A_K22_27/A_K22_27_MainPaper(revised).pdf
Туре	pe	Business Services	-	
	tal GFA sumed GFA per Employee	26,917 18.2	m <sup>2</sup> m <sup>2</sup> /Person	Assumed 55% office Table 8 CIFSUS - Business Services
Emp	ployee Population	1,481	Persons	The state of the s
	it flow factor (UFF) schment Inflow Factors(PCIF)	0.08 1.1	m³/person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services Table T-4 of GESF - East Kowloon
ADV	· · · · · · · · · · · · · · · · · · ·	130.33	m <sup>3</sup> /day	TWOIL TO GLOT LAST INVITABLE TO THE STATE OF
		Retail Trade	_	
Type Tota	tal GFA	17,129	$m^2$	Assumed 35% retail trade
	sumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
_	ployee Population it flow factor (UFF)	600 0.28	Persons m³/person/day	Table T-2: Empolyee & J4 Retail
Cate	chment Inflow Factors(PCIF)	1.1	-	Table T-4 of GESF - East Kowloon
ADV	OWF	184.80	m <sup>3</sup> /day	
Туре		Restaurants	- 2	
	tal GFA sumed GFA per Employee	4,894 19.6	m <sup>2</sup> /Person	Assumed 10% restaurants  Table 8 CIFSUS - Restaurants
Emp	ployee Population	250	Persons	
	it flow factor (UFF) schment Inflow Factors(PCIF)	1.58 1.1	m³/person/day	Table T-2: Empolyee-Customers & J10 Restaurants  Table T-4 of GESF - East Kowloon
ADV	, , ,	434.50	m <sup>3</sup> /day	
Tree	tal building ADWE	740.62	m <sup>3</sup> /day	
	tal building ADWF schages to:	749.63	ını /day	
	FMH4043143	749.63	m³/day	
E_5 <b>NKI</b>	XIL 6647			
Тур		Residential 1,782	- flats	Source: https://www.tpb.gov.hk/tc/plan_application/Attachment/20210803/s16fi_A_K22_31_4_gist.pdf
Avei	erage household	2.7	Persons/flat	Census, Household Characteristics of Population in Kowloon City District, 2021
_	oulation it flow factor (UFF)	4,812 0.27	Persons m³/person/day	Table T-1 of GESF - Residential R2
Catc	chment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADV	OWF	1,299.24	m <sup>3</sup> /day	
Туре		Social Services	-	Club house
	tal GFA	5,500	$m^2$ $m^2/Person$	assumed Table 8 CIESUS - Social Services
	sumed GFA per Employee uployee Population	30.3 182	Persons	Table 8 CIFSUS - Social Services
Unit	it flow factor (UFF)	0.28	m³/person/day	Table T-2: Empolyee & Community, Social & Personal Services
Cate ADV	chment Inflow Factors(PCIF)  WF	1.0 50.96	- m <sup>3</sup> /day	GESF section 10 - Not applicable for new developments.
	tal building ADWF chages to:	1,350.20	m <sup>3</sup> /day	
	FMH4043145	450.07	m³/day	Assumed split in 3 terminal manholes
	FMH4043147 FMH4043149	450.07 450.07	m <sup>3</sup> /day m <sup>3</sup> /day	Assumed split in 3 terminal manholes Assumed split in 3 terminal manholes
		<b>⊣</b> JU.U/	iii / du y	2 totalines opite in 2 terminal maintones
E_6 Gas		Retail Trade	_	
	tal GFA	106	$m^2$	Source: plan form geoinfo map
	sumed GFA per Employee uployee Population	28.6 4	m <sup>2</sup> /Person Persons	Table 8 CIFSUS - Retail Trade
Unit	it flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catc	chment Inflow Factors(PCIF)	1.1	-	Table T-4 of GESF - East Kowloon
ADV	PWF	1.23	m <sup>3</sup> /day	
	tal building ADWF	1.23	m <sup>3</sup> /day	
I Disc	chages to:	1.23	m <sup>3</sup> /day	EXTERNAL (on box culvert)
Disc	FMH4036900	1 / 3	1111 / 522 **	The state of the s
Disc	FMH4036900	1.23	in raay	

<b>Localised Sewage Flow Estimates</b>	Estimation	Unit	Remark
EL_1 Hong Kong Children's Hospital Total building Localised Flow Dischages to:	36.24	l/s	information provided by development



Ove Arup & Partners Calculation Sheet

Sheet No.

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of

Sheet No.

Rev. 0

Othecked NP

Estimation of Sewage Flows Estimation from Catchments Not Affected by the Proposed Development

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

# **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

2. Planning Department CIFSUS.

3. BS EN 12056-2:2000 Gravity drainage systems inside buildings

DWF Sewage Flow Estimates	Estimation	Unit	Remark
FMH4100328	36.24	1/s	information provided by development
EL_2 Acute Hospital			New Acute Hospital at Kai Tak (under construction)
Design sewerage flows	100.00	1/s	information provided by development
Total building Localised Flow	100.00	1/s	information provided by development
Dischages to:			
FMH4096819	50.00		50% flow split between the two discharge locations (information provided by development)
FMH4100328	50.00		50% flow split between the two discharge locations (information provided by development)
EL_2 NKIL 6647 Swimming pool			
Pool Area	225	$m^2$	assumed
Pool Depth	1.20	m	assumed
Pool Water Volume	270.00	$m^3$	
Turnover Period	6.00	hours	Assumed outdoor pool (6 hours for open air pools, 4 hours for indoor pools). Source Cap. 132CA Swimming Pool Regulation, Section 6 paragraph j
Recirculation Flow	45.00	$m^3/h$	
Filter flow rate	30.00	$m^3/m^2/h$	Source: Medium Rate Sand Filter, from Table 5.1, WHO, Guidelines for Safe Recreational Water Environments, Volume 2, Swimming Pools and Similar Environment
filter area	1.50	$m^2$	
backwash unit flow	37.00	$m^3/m^2/h$	Source: Medium Rate Sand Filter, from Table 5.1, WHO, Guidelines for Safe Recreational Water Environments, Volume 2, Swimming Pools and Similar Environment
	55.50	$m^3/h$	Source: Medium Tune sund Theer, from Tune 371, W116, Guidelines for suic Recreational Water Environments, Volume 2, 5 winning Tools and Similar Environments
backwash flow	15.42	1/s	
Design sewerage flows	15.42	1/s	
Total building Localised Flow	15.42	1/s	
Dischages to:			
FMH4043145	15.42		assumed at the most upstream manhole



Ove Arup & Partners Calculation Sheet

Sheet No.

Rev. 0

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Made by

NP

Date

23/01/2025

Checked

NP

**Estimation of Sewage Flows Estimation for Existing Development** 

### **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

2. Planning Department CIFSUS.

DWF Sewage Flow Estimates	Estimation	Unit	Remark
B 1 Site 3E1	Estimation	Ont	Acmark
Type	Residential	-	Residential R(B)2 from OZP
Total GFA	31,815	$m^2$	From Maximum Plot Ratio stated in OZP 4.5 of Net Site Area of around 7,070 m2.
Assumed GFA per flat	75.0	m <sup>2</sup> /flats	Kai Tak Planning Report; https://www.tpb.gov.hk/en/resources/general_papers.html?category=&year=2022&month=#content
Number of flats	425	flats	
Average household	2.7	Persons/flat	Census, Household Characteristics of Population in Kowloon City District, 2021
Population	1,148	Persons	
Unit flow factor (UFF)	0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	218.12	m <sup>3</sup> /day	
Total building ADWF	218.12	m <sup>3</sup> /day	
Dischages to:			
FMH4036432	218.12	m <sup>3</sup> /day	
B 2 Site 3E2			
Type	Residential	-	Residential R(B)2 from OZP
Total GFA	34,605	$m^2$	From Maximum Plot Ratio stated in OZP 4.5 of Net Site Area of around 7,690 m2.
Assumed GFA per flat	75.0	m <sup>2</sup> /flats	Kai Tak Planning Report; https://www.tpb.gov.hk/en/resources/general_papers.html?category=&year=2022&month=#content
Number of flats	462	flats	
Average household	2.7	Persons/flat	Census, Household Characteristics of Population in Kowloon City District, 2021
Employee Population	1,248	Persons	
Unit flow factor (UFF)	0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	237.12	m <sup>3</sup> /day	
Total building ADWF	237.12	m <sup>3</sup> /day	
Dischages to:	237.12	III / day	
FMH4043141	237.12	m <sup>3</sup> /day	
1 1/11/1/17/17/17/1	237.12	III / duy	

Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Job No. Sheet No. Date 23/01/2025 NP Made by Checked

Table - Capacity Performance of Existing Sewer

Notes: (1) Calculate by Colebrook-White Equation

Job Title

 $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

		Roughness	
_		ks (mm)	
Pipe Materal	V 0.75m/s	V 1.2m/s	0.75 < V < 1.2
VC	3.0	0.6	interpolated
PC	6.0	3.0	interpolated
PE	1.5	0.3	interpolated

V is the velocity, D is the diameter of the sewer and S is the gradient of the sewer. Assumed data

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2

Existing Netw	ork - Sites 3E	1 and 3E2																	
Man	nhole			DEAKING								Existing Pi	oe Parameter						New Sewer
UP_MAN	DN_MAN	Catchment inflow	CON_POP	PEAKING FACTOR	ACC_ADWF (m <sup>3</sup> /d)	Peak Flow	DIA (D)	LEN	UP_GL (mPD)	DN_GL (mPD)	UP_INV (mPD)	DN_INV (mPD)	Gradient	Pipe Materal	VEL	CAP	F/C (%)	Sufficient	Adequate
No.	No.				(111 /4)	(L/s)	(mm)	(m)	(MPD)	(MPD)	(MPD)	(MPD)	(S)	ks (mm)	(m/s)	(L/s)	(%)	Capacity?	Velocity?
FMH4100328	FMH4061903	EL_1, EL_2	0	8	0.00	86.24	300	38.0	4.36	4.17	0.52	0.37	250	VC, interpolated, ks=2.8 mm	0.80	56.26	153.3%	NO	Existing
FMH4061903	FMH4061905		0	8	0.00	86.24	300	47.3	4.17	4.15	0.37	0.30	676	VC, V<=0.75, ks=3 mm	0.48	33.66	256.2%	NO	Existing
FMH4061905	FMH4096819		0	8	0.00	86.24	600	13.90	4.15	4.43	0.00	-0.05	278	VC, V>=1.2, ks=0.6 mm	1.45	410.85	21.0%	YES	Existing
FMH4096819	FMH4096820	EL_2	0	8	0.00	136.24	600	40.4	4.43	4.21	-0.07	-0.22	267	VC, V>=1.2, ks=0.6 mm	1.48	419.14	32.5%	YES	Existing
FMH4096820	FMH4096823		0	8	0.00	136.24	600	30.6	4.21	4.27	-0.23	-0.36	235	VC, V>=1.2, ks=0.6 mm	1.58	447.06	30.5%	YES	Existing
FMH4096823	FMH4061908	E_1	243	8	65.60	142.31	600	42.9	4.27	4.59	-0.60	-0.93	130	PC, V>=1.2, ks=3 mm	1.72	487.14	29.2%	YES	Existing
FMH4061908	Box Culvert		243	8	65.60	142.31	600	3.90	4.59	4.59	-1.02	-1.03	390	PC, interpolated, ks=4.8 mm	0.92	260.74	54.6%	YES	Existing
FMH4096818	FMH4100328		0	8	0.00	0.00	300	31.9	4.1	4.36	2.54	1.11	22	VC, V>=1.2, ks=0.6 mm	3.34	236.15	0.0%	YES	Existing
FMH4043141	FMH4043142	B_2	878	8	237.12	21.96	225	29.1	4.34	3.83	2.43	2.15	104	VC, V>=1.2, ks=0.6 mm	1.28	50.89	43.1%	YES	Existing
FMH4043142	FMH4043143		878	8	237.12	21.96	225	39.8	3.83	4.05	2.15	1.76	102	VC, V>=1.2, ks=0.6 mm	1.29	51.36	42.7%	YES	Existing
FMH4043143	FMH4043144	E_4	4,462	6	1204.87	83.67	225	29.0	4.05	4.24	1.76	1.47	100	VC, V>=1.2, ks=0.6 mm	1.31	51.89	161.3%	NO	Existing
FMH4043144	FMH4043145		4,462	6	1204.87	83.67	225	27.9	4.24	4.53	1.47	1.20	103	VC, V>=1.2, ks=0.6 mm	1.28	51.04	163.9%	NO	Existing
FMH4043145	FMH4043146	E_5 EL_2 BL_5	6,129	5	1654.93	111.19	450	36.4	4.53	4.39	0.97	0.83	260	VC, V>=1.2, ks=0.6 mm	1.25	199.40	55.8%	YES	Existing
FMH4043146	FMH4043203	E_3	6,288	5	1697.75	113.67	525	11.5	4.39	4.31	0.73	0.69	288	VC, V>=1.2, ks=0.6 mm	1.31	284.35	40.0%	YES	Existing
FMH4043203	FMH4043147		6,288	5	1697.75	113.67	525	21.6	4.91	4.23	0.69	0.63	360	VC, interpolated, ks=1.3 mm	1.06	230.36	49.3%	YES	Existing
FMH4043147	FMH4043148		7,955	5	2147.81	139.71	525	42.6	4.23	4.20	0.63	0.49	304	VC, V>=1.2, ks=0.6 mm	1.28	276.31	50.6%	YES	Existing
FMH4043148	FMH4043149	E_2	9,715	5	2622.96	167.21	525	28.8	4.20	4.25	0.49	0.40	320	VC, V>=1.2, ks=0.6 mm	1.24	269.36	62.1%	YES	Existing
FMH4043149	FMH4043150	E_2	13,141	4	3548.17	179.68	525	22.7	4.25	3.96	0.40	0.32	284	VC, V>=1.2, ks=0.6 mm	1.32	286.24	62.8%	YES	Existing
FMH4043150	Box Culvert		13,141	4	3548.17	179.68	525	1.4	3.96	3.96	0.32	0.00	4	VC, V>=1.2, ks=0.6 mm	10.76	2327.43	7.7%	YES	Existing
FMH4036432	FMH4043143	B_1	808	8	218.12	20.20	225	20.6	4.05	4.05	2.00	1.84	127	VC, interpolated, ks=1.7 mm	1.00	39.75	50.8%	YES	Existing

Abbreviation: UP\_MAN

Upstream Manhole DN\_MAN Downstream Manhole  $\overline{\mathsf{ADWF}}$ Average Dry Weather Flow ACC\_ADWF CON\_POP Accumulated Average Dry Contributing Population DIA LEN Diameter Length UP\_GL DN\_GL UP\_INV DN\_INV **Upstream Ground Level** . Downstream Ground Level Upstream Invert Level

Downstream Invert Level Peak Pipe Velocity Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe

VEL
PE
CAP
F/C
VC
PC Peak Pipe Capacity Peak Flow/Capacity Vitrified Clay Pipe Precast Concrete Pipe



Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Proposed C

**Estimation of Sewage Flows Estimation for Proposed Development** 

## **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

2. Planning Department CIFSUS.

Cowago Flow Estimates	Estimation	Unit	Damaule
F Sewage Flow Estimates Site 3E1	Estimation	Unit	Remark
Type	Residential		
Number of flats	888	flats	
Average household	2.7	Persons/flat	Census, Household Characteristics of Population in Kowloon City District, 2021
Population	2,398	Persons	
Unit flow factor (UFF)	0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	455.62	m <sup>3</sup> /day	
m			
Type	Retail Trade	-	
Total GFA	2,405	m <sup>2</sup>	FROM PRELIM LAYOUT
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	85	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	23.80	m <sup>3</sup> /day	
Type	Social Services	-	Club house and social and welfare facilities
Total GFA	3,087	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee	30.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Social Services
Employee Population	102	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & Community, Social & Personal Services
Catchment Inflow Factors(PCIF)	1.0	in /person/day	GESF section 10 - Not applicable for new developments.
, ,		m <sup>3</sup> /day	GESI* section 10 - Not applicable for new developments.
ADWF	28.56	m /day	
T - 11 31 - 1 DWD	507.00	3,1	
Total building ADWF	507.98	m <sup>3</sup> /day	
Dischages to:		3	
TMH_3E1	507.98	m <sup>3</sup> /day	
2 64, 252		+	
2 Site 3E2	Residential		
Type Number of flats	1,040	flats	
Average household	2.7	Persons/flat	Census, Household Characteristics of Population in Kowloon City District, 2021
Employee Population	2,808	Persons	Census, Household Characteristics of Fopulation in Kowloon City District, 2021
	0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1
Unit flow factor (UFF) Catchment Inflow Factors(PCIF)	1.0	iii /person/day	GESF section 10 - Not applicable for new developments.
		3/1	GESF section 10 - Not applicable for new developments.
ADWF	533.52	m <sup>3</sup> /day	
Type	Potail Trade		
Type	Retail Trade	2	
Total GFA	3,843	m <sup>-</sup>	FROM PRELIM LAYOUT
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	135	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	[- <u>.</u>	GESF section 10 - Not applicable for new developments.
ADWF	37.80	m <sup>3</sup> /day	
		1	
Type	Social Services	J-,	Club house and social
Total GFA	2,065	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee	30.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Social Services
Employee Population	69	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & Community, Social & Personal Services
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	19.32	m <sup>3</sup> /day	
	17.52		
Total building ADWF	590.64	m <sup>3</sup> /day	
Dischages to:	370.04	in , day	
TMH_3E2	590.64	m <sup>3</sup> /day	
11/111_3152	370.04	III /day	
S ADWE	1 000 (2	m³/day	
T ADWF	1,098.62	<sub>[III /</sub> uay	

Job Title

Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Job No. Sheet No. Date 26/02/2025

Made by NP

Table - Capacity Performance of Existing and Proposed Sewer under Development Scenario

Notes: (1) Calculate by Colebrook-White Equation  $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

		Roughness	
		ks (mm)	
Pipe Materal	V 0.75m/s	V 1.2m/s	0.75 < V < 1.2
VC	3.0	0.6	interpolated
PC	6.0	3.0	interpolated
PE	1.5	0.3	interpolated

V is the velocity, D is the diameter of the sewer and S is the gradient of the sewer. Assumed data

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2

Mar	nhole				PEAKING								Existing Pi	ipe Parameter						New Se
UP_MAN No.	DN_MAN No.	Existing / New / Upgraded	Catchment inflow	CON_POP	FACTOR	ACC_ADWF (m³/d)	Peak Flow (L/s)	DIA (D) (mm)	LEN (m)	UP_GL (mPD)	DN_GL (mPD)	UP_INV (mPD)	DN_INV (mPD)	Gradient (S)	Pipe Materal ks (mm)	VEL (m/s)	CAP (L/s)	F/C (%)	Sufficient Capacity?	Adequ Veloci
FMH4100328	FMH4061903		EL_1, EL_2	2,188	6	590.64	127.26	300	38.0	4.36	4.17	0.52	0.37	250	VC, interpolated, ks=2.8 mm	0.80	56.26	226.2%	NO	Exis
FMH4061903	FMH4061905			2,188	6	590.64	127.26	300	47.3	4.17	4.15	0.37	0.30	676	VC, V<=0.75, ks=3 mm	0.48	33.66	378.1%	NO	Exis
FMH4061905	FMH4096819			2,188	6	590.64	127.26	600	13.90	4.15	4.43	0.00	-0.05	278	VC, V>=1.2, ks=0.6 mm	1.45	410.85	31.0%	YES	Exis
FMH4096819	FMH4096820		EL_2	2,188	6	590.64	177.26	600	40.4	4.43	4.21	-0.07	-0.22	267	VC, V>=1.2, ks=0.6 mm	1.48	419.14	42.3%	YES	Exis
FMH4096820	FMH4096823			2,188	6	590.64	177.26	600	30.6	4.21	4.27	-0.23	-0.36	235	VC, V>=1.2, ks=0.6 mm	1.58	447.06	39.6%	YES	Exis
FMH4096823	FMH4061908		E_1	2,431	6	656.24	181.81	600	42.9	4.27	4.59	-0.60	-0.93	130	PC, V>=1.2, ks=3 mm	1.72	487.14	37.3%	YES	Exis
FMH4061908	Box Culvert			2,431	6	656.24	181.81	600	3.90	4.59	4.59	-1.02	-1.03	390	PC, interpolated, ks=4.8 mm	0.92	260.74	69.7%	YES	Exis
TMH_3E2	Exist_tapping	NEW	P_2	2,188	5	590.64	34.18	300	7.7	4.35	4.35	3.21	3.17	208	PE, interpolated, ks=0.6 mm	1.08	76.30	44.8%	YES	Y
Exist_tapping	FMH4096818			2,188	6	590.64	41.02	300	6.0	4.4	4.11	3.17	3.15	272	VC, interpolated, ks=3 mm	0.75	53.19	77.1%	YES	Exi
FMH4096818	FMH4100328			2,188	6	590.64	41.02	300	31.9	4.1	4.36	2.54	1.11	22	VC, V>=1.2, ks=0.6 mm	3.34	236.15	17.4%	YES	Ex
FMH4043141	FMH4043142			0	8	0.00	0.00	225	29.1	4.34	3.83	2.43	2.15	104	VC, V>=1.2, ks=0.6 mm	1.28	50.89	0.0%	YES	Exi
FMH4043142	FMH4043143			0	8	0.00	0.00	225	39.8	3.83	4.05	2.15	1.76	102	VC, V>=1.2, ks=0.6 mm	1.29	51.36	0.0%	YES	Exi
FMH4043143	FMH4043144		E_4	4,658	6	1257.61	87.33	225	29.0	4.05	4.24	1.76	1.47	100	VC, V>=1.2, ks=0.6 mm	1.31	51.89	168.3%	NO	Exi
FMH4043144	FMH4043145			4,658	6	1257.61	87.33	225	27.9	4.24	4.53	1.47	1.20	103	VC, V>=1.2, ks=0.6 mm	1.28	51.04	171.1%	NO	Ex
FMH4043145	FMH4043146		E_5 EL_2	6,325	5	1707.67	114.24	450	36.4	4.53	4.39	0.97	0.83	260	VC, V>=1.2, ks=0.6 mm	1.25	199.40	57.3%	YES	Ex
FMH4043146	FMH4043203		E_3	6,483	5	1750.49	116.72	525	11.5	4.39	4.31	0.73	0.69	288	VC, V>=1.2,	1.31	284.35	41.0%	YES	Exi
FMH4043203	FMH4043147			6,483	5	1750.49	116.72	525	21.6	4.91	4.23	0.69	0.63	360	VC, interpolated,	1.06	230.36	50.7%	YES	Exi
FMH4043147	FMH4043148			8,150	5	2200.55	142.76	525	42.6	4.23	4.20	0.63	0.49	304	ks=1.3 mm VC, V>=1.2,	1.28	276.31	51.7%	YES	Ex
FMH4043148	FMH4043149		E_2	9,910	5	2675.70	170.26	525	28.8	4.20	4.25	0.49	0.40	320	ks=0.6 mm VC, V>=1.2,	1.24	269.36	63.2%	YES	Exi
FMH4043149	FMH4043150		E_2	13,337	4	3600.91	182.13	525	22.7	4.25	3.96	0.40	0.32	284	ks=0.6 mm VC, V>=1.2,	1.32	286.24	63.6%	YES	Exi
FMH4043150	Box Culvert		<u></u>	13,337	4	3600.91	182.13	525	1.4	3.96	3.96	0.40	0.00		ks=0.6 mm VC, V>=1.2,	10.76	2327.43	7.8%	YES	Ex
IVII I+U43 I3U	Box Cuivert			13,337	4	3000.91	102.13	320	1.4	3.80	3.90	0.32	0.00	4	ks=0.6 mm	10.70	2321.43	1.070	163	EX
TMH_3E1	FMH4043143		P_1	1,881	6	507.98	35.28	225	12.3	4.05	4.05	1.93	1.84	127	VC, interpolated, ks=1.7 mm	1.00	39.80	88.6%	YES	Ex
		1																		

Abbreviation: UP\_MAN Upstream Manhole DN\_MAN Downstream Manhole ADWF Average Dry Weather Flow ACC\_ADWF Accumulated Average Dry CON\_POP DIA Contributing Population Diameter LEN Length UP\_GL Upstream Ground Level DN\_GL Downstream Ground Level UP\_INV Upstream Invert Level DN\_INV Downstream Invert Level Peak Pipe Velocity

VEL PE CAP F/C Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe Peak Pipe Capacity

Peak Flow/Capacity Vitrified Clay Pipe Precast Concrete Pipe VC PC

Ove Arup & Partners Calculation Sheet

Job No. Sheet No. Date 26/02/2025 Rev. Made by Checked NP

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Job Title Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Table - Capacity Performance of Existing and Proposed Sewer under Development Scenario - Mitigated  $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ Notes:

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2  $\overline{V}$  is the velocity, D is the diameter of the sewer and S is the gradient of the sewer.

Assumed data

(1) Calculate by Colebrook-White Equation

**Existing and Proposed Network - Sites 3E1 and 3E2** 

Ma	anhole			DEVINO								Existing P	ipe Parameter						New Sewer	Utilisation Ra	ate Comparison
UP_MAN	DN_MAN	Catchment inflow	CON_POP	PEAKING FACTOR	ACC_ADWF (m³/d)	Peak Flow	DIA (D)	LEN	UP_GL	DN_GL	UP_INV	DN_INV	Gradient	Pipe Materal	VEL	CAP	F/C	Sufficient	Adequate	Existing F/C	Proposed F/C
No.	No.				(m³/d)	(L/s)	(mm)	(m)	(mPD)	(mPD)	(mPD)	(mPD)	(S)	ks (mm)	(m/s)	(L/s)	(%)	Capacity?	Velocity?	(%)	(%)
FMH4100328	FMH4061903	EL_1, EL_2	2,188	6	590.64	127.26	525	38.0	4.36	4.17	0.52	0.37	250	PE, V>=1.2, ks=0.3 mm	1.52	328.65	38.7%	YES	Existing	153%	39%
FMH4061903	FMH4061905		2,188	6	590.64	127.26	525	47.3	4.17	4.15	0.37	0.30	676	PE, interpolated,	0.77	165.60	76.8%	YES	Existing	256%	77%
			<u> </u>	-										ks=1.5 mm VC, V>=1.2,							
FMH4061905	FMH4096819		2,188	6	590.64	127.26	600	13.90	4.15	4.43	0.00	-0.05	278	ks=0.6 mm	1.45	410.85	31.0%	YES	Existing	21%	31%
FMH4096819	FMH4096820	EL_2	2,188	6	590.64	177.26	600	40.4	4.43	4.21	-0.07	-0.22	267	VC, V>=1.2, ks=0.6 mm	1.48	419.14	42.3%	YES	Existing	33%	42%
	+													VC, V>=1.2,							
FMH4096820	FMH4096823		2,188	6	590.64	177.26	600	30.6	4.21	4.27	-0.23	-0.36	235	ks=0.6 mm	1.58	447.06	39.6%	YES	Existing	30%	40%
FMH4096823	FMH4061908	E_1	2,431	6	656.24	181.81	600	42.9	4.27	4.59	-0.60	-0.93	130	PC, V>=1.2,	1.72	487.14	37.3%	YES	Existing	29%	37%
	-		_,											ks=3 mm							
FMH4061908	Box Culvert		2,431	6	656.24	181.81	600	3.90	4.59	4.59	-1.02	-1.03	390	PC, interpolated, ks=4.8 mm	0.92	260.74	69.7%	YES	Existing	55%	70%
TMH_3E2	Exist_tapping	P_2	2,188	5	590.64	34.18	300	7.7	4.35	4.35	3.21	3.17	208	PE, interpolated, ks=0.6 mm	1.08	76.30	44.8%	YES	YES	0%	45%
				_										VC, interpolated,						201	
Exist_tapping	FMH4096818		2,188	6	590.64	41.02	300	6.0	4.4	4.11	3.17	3.15	272	ks=3 mm	0.75	53.19	77.1%	YES	Existing	0%	77%
FMH4096818	FMH4100328		2,188	6	590.64	41.02	300	31.9	4.1	4.36	2.54	1.11	22	VC, V>=1.2,	3.34	236.15	17.4%	YES	Existing	43%	17%
														ks=0.6 mm							
FMH4043141	FMH4043142		0	8	0.00	0.00	225	29.1	4.34	3.83	2.43	2.15	104	VC, V>=1.2,	1.28	50.89	0.0%	YES	Existing	43%	0%
														ks=0.6 mm							
FMH4043142	FMH4043143		0	8	0.00	0.00	225	39.8	3.83	4.05	2.15	1.76	102	VC, V>=1.2, ks=0.6 mm	1.29	51.36	0.0%	YES	Existing	43%	0%
FMH4043143	FMH4043144	E 4	4,658	6	1257.61	87.33	300	29.0	4.05	4.24	1.76	1.47	100	PE, V>=1.2,	1.70	120.30	72.6%	YES	Existing	161%	73%
F IVII 14043 143	F1VII 14043 144	E_4	4,030	0	1237.01	67.33	300	29.0	4.05	4.24	1.70	1.47	100	ks=0.3 mm	1.70	120.30	72.070	TES	Existing	10176	7376
FMH4043144	FMH4043145		4,658	6	1257.61	87.33	300	27.9	4.24	4.53	1.47	1.20	103	PE, V>=1.2, ks=0.3 mm	1.67	118.31	73.8%	YES	Existing	164%	74%
FMH4043145	FMH4043146	E_5 EL_2	6,325	5	1707.67	114.24	450	36.4	4.53	4.39	0.97	0.83	260	VC, V>=1.2, ks=0.6 mm	1.25	199.40	57.3%	YES	Existing	56%	57%
														VC, V>=1.2,							
FMH4043146	FMH4043203	E_3	6,483	5	1750.49	116.72	525	11.5	4.39	4.31	0.73	0.69	288	ks=0.6 mm	1.31	284.35	41.0%	YES	Existing	40%	41%
FMH4043203	FMH4043147		6,483	5	1750.49	116.72	525	21.6	4.91	4.23	0.69	0.63	360	VC, interpolated,	1.06	230.36	50.7%	YES	Existing	49%	51%
			<u> </u>											ks=1.3 mm							
FMH4043147	FMH4043148		8,150	5	2200.55	142.76	525	42.6	4.23	4.20	0.63	0.49	304	VC, V>=1.2, ks=0.6 mm	1.28	276.31	51.7%	YES	Existing	51%	52%
FMH4043148	FMH4043149	E 2	9,910	5	2675.70	170.26	525	28.8	4.20	4.25	0.49	0.40	320	VC, V>=1.2,	1.24	269.36	63.2%	YES	Existing	620/	63%
FIVIП4043146	FIVITH4043149	E_2	9,910	5	2075.70	170.20	525	20.0	4.20	4.25	0.49	0.40	320	ks=0.6 mm	1.24	209.30	03.2%	TES	Existing	62%	63%
FMH4043149	FMH4043150	E_2	13,337	4	3600.91	182.13	525	22.7	4.25	3.96	0.40	0.32	284	VC, V>=1.2, ks=0.6 mm	1.32	286.24	63.6%	YES	Existing	63%	64%
														VC, V>=1.2,							
FMH4043150	Box Culvert		13,337	4	3600.91	182.13	525	1.4	3.96	3.96	0.32	0.00	4	ks=0.6 mm	10.76	2327.43	7.8%	YES	Existing	8%	8%
														DE 1/5-4.0							
TMH_3E1	FMH4043143	P_1	1,881	5	507.98	29.40	225	12.3	4.05	4.05	2.05	1.80	49	PE, V>=1.2, ks=0.3 mm	2.03	80.83	36.4%	YES	YES	51%	36%
			0	8	0.00	0.00															

Roughness ks (mm)

0.3

0.6

3.0

V 1.2m/s 0.75 < V < 1.2

interpolated

interpolated

interpolated

Pipe Materal

PE

VC

PC

V 0.75m/s

1.5

3.0

6.0

Abbreviation: UP\_MAN Upstream Manhole DN\_MAN Downstream Manhole ADWF Average Dry Weather Flow ACC\_ADWF Accumulated Average Dry CON\_POP Contributing Population DIA LEN Diameter Length UP\_GL Upstream Ground Level DN\_GL Downstream Ground Level UP\_INV Upstream Invert Level

DN\_INV Downstream Invert Level VEL Peak Pipe Velocity Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe PE

CAP F/C Peak Pipe Capacity Peak Flow/Capacity VC PC Vitrified Clay Pipe Precast Concrete Pipe



Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

Sheet No.

Rev. 0

NP

Date

23/01/2025

Checked NP

Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

Estimation of Sewage Flows Estimation from Catchments Not Affected by the Proposed Development

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

## Design Code

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

2. Planning Department CIFSUS.

External Discharge - Near Sites 4C4 and	4C5	<u> </u>			
		1			
E_A Upstream Developments Type	Residential	_			
Number of flats	7,985	flats	Catchments: 4A1 (2138), 4A2 (2060), 4B1 (1590), 4B2 (1059), 4C1 (582), 4C1 (5		
Average household	2.7	Persons/flat Persons	Census, Household Characteristics of Population in Kowloon City District, 20	21	
Population Unit flow factor (UFF)	21,560 0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1, PlanD classification R2 obtained from C	7P plot ratio >6	
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.	zr piotrano > 0	
ADWF	4,096.40	m <sup>3</sup> /day			
T	D 11 411		G . 1	Plot	Source (Victoria Marie Control of the Control of th
Type Number of flats	Residential 2,963	flats	Catchments: 4B3 (1219), 4B4 (1305), 4C3 (439).	4C3 4C2	https://richitt.com/%E5%A4%A9%E7%92%BD%C2%B7%E6%B5%B7/ https://richitt.com/%E5%A4%A9%E7%80%A7/
Average household	2.7	Persons/flat	Census, Kowloon City District, 2021	4C1	https://richitt.com/%E6%BE%90%E7%92%9F/
Population	8,001	Persons		4A2	https://richitt.com/kaitakarea4asite2development/
Unit flow factor (UFF)	0.27	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R2, from OZP plot ratio <6	4B3	https://www.info.gov.hk/gia/general/201811/07/P2018110700761.htm
Catchment Inflow Factors(PCIF) ADWF	1.0 2,160.27	- m <sup>3</sup> /day	GESF section 10 - Not applicable for new developments.	4B2	https://www.landsd.gov.hk/doc/en/consent/monthly/t1_2106.pdf
ADWF	2,100.27	III /day		4B1 4A1	https://www.doublecoast1.hk/api/data/911e6c92/NKIL6576_Phase1_20241104.pdf https://richitt.com/%E5%95%9F%E5%BE%B7%E6%B5%B7%E7%81%A31/
Type	Retail Trade	-		4B4	https://richitt.com/%E6%9F%8F%E8%94%9A%E6%A3%AE/
Total GFA	5,666	$m^2$	Catchments: 4C1 (92m2), 4C2 (2787m2), 4C3 (2787m2).		
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade		
Employee Population	199	Persons			
Unit flow factor (UFF) Catchment Inflow Factors(PCIF)	0.28 1.0	m³/person/day	Table T-2: Empolyee & J4 Retail GESF section 10 - Not applicable for new developments.		
ADWF	55.72	m <sup>3</sup> /day	GEST Section to - Not applicable for new developments.		
	33.72				
Type	Social Services	-	Club house		
Total GFA	26,199	$m^2$	Catchments: 4A1 (2088)), 4A2 (2500), 4B1 (7340), 4B2 (3784), 4B3 (2861),	B4 (905), 4C1 (2540), 4C2 (3	3161), 4C3 (3520).
Assumed GFA per Employee	30.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Social Services		
Employee Population Unit flow factor (UFF)	865 0.28	Persons m³/person/day	Table T-2: Empolyee & Community, Social & Personal Services		
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.		
ADWF	242.20	m <sup>3</sup> /day			
Total building ADWF	6,554.59	m <sup>3</sup> /day			
Dischages to: FMH4098421	6,554.59	m <sup>3</sup> /day			
1 1411 1 T U 7 O T Z I	0,334.33	m /day			
E_B Development 4B5					
Type Number of flats	Residential 2,557	- flats	From the note has ad on an eletantic and site 4D4 as a forestment.		
Average household	2,337	Persons/flat	From pro-rata based on on plot ratio and site 4B4 no. of apartments  Census, Household Characteristics of Population in Kowloon City District, 20	21	
Population	6,904	Persons	consus, reasonous characteristics of reputation in reconson city sisterio, 20		
Unit flow factor (UFF)	0.19	m <sup>3</sup> /person/day	Table T-1 of GESF - Residential R1, PlanD classification R2 obtained from C	ZP plot ratio =7.5	
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.		
ADWF	1,311.76	m <sup>3</sup> /day			
Type	Social Services	_	Club house		
Total GFA	1,300	$m^2$	From pro-rata based on on plot ratio and site 4B4 club house size		
Assumed GFA per Employee	30.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Social Services		
Employee Population	43	Persons			
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & Community, Social & Personal Services		
Catchment Inflow Factors(PCIF) ADWF	1.0 12.04	m <sup>3</sup> /day	GESF section 10 - Not applicable for new developments.		
ΔD M1.	12.04	m /day			
Total building ADWF	1,323.80	m <sup>3</sup> /day			
Dischages to:					
FMH4098429	1,323.80	m <sup>3</sup> /day			
E C Cruise Terminus		+			
ADWF	3,121.00	m <sup>3</sup> /day	Catchments: Cruise Terminus		
Total building ADWF	3,121.00	m <sup>3</sup> /day			
Dischages to:	2.121.22	3/1			
FMH4098431	3,121.00	m <sup>3</sup> /day	From KL/2014/01 Kai Tak Development-Stage 2 Infrastructure works for Development	elopments at the Southern Par	rt of the Former Runway, Design for Sewerage Design
E_D Downstream Developments					
ADWF	4,200.00	m <sup>3</sup> /day	Catchments: 4D2: 4D3		
Total building ADWF	4,200.00	m <sup>3</sup> /day			
Dischages to: FMH4098439	4,200.00	m <sup>3</sup> /day	From KL/2014/01 Kai Tak Development-Stage 2 Infrastructure works for De	relanments at the Southern De-	rt of the Former Runway, Decign for Sewerage Decign
1 <sup>-</sup> 1411 1 <sup>+</sup> U70 <sup>+</sup> J7	7,200.00	iii /day	1 10111 KL/2014/01 Kai Tak Development-Stage 2 infrastructure works for De	cropments at the Southern Par	it of the Former Kunway, Design for Sewerage Design
<b>.</b>	· T	· 	· T		
ocalised Sewage Flow Estimates	Estimation	Unit			Remark
EL 3 Upstream Swimming pools	+	+			
Unit backwash flow	20	l/s/development	assumed		
Design sewerage flows	180.00	1/s	Catchments: 4A1, 4A2, 4B1, 4B2, 4B3, 4B4, 4C1, 4C2, 4C3.		
Total building Localised Flow	180.00	1/s			
Dischages to:	100.00				
•		1	I .		
FMH4098421	180.00				
	180.00 331.66	l/s			



Ove Arup & Partners Calculation Sheet Job No. Sheet No. Rev. 0 Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Made by NP NP Date 23/01/2025 Checked Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of

**Estimation of Sewage Flows Estimation for Existing Development** 

Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place,

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

### **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

2. Planning Department CIFSUS.

**TOT** Localised Flow

0.00

l/s

3. BS EN 12056-2:2000 Gravity drainage systems inside buildings

xisting Discharge - Sites 4C4 and 4C5			
OWF Sewage Flow Estimates	Estimation	Unit	Remark
3_3 Site 4C4			
Type	Hotels	-	
Total GFA	24,062	$m^2$	From Maximum Plot Ratio stated in OZP 7.5 of Net Site Area of around 10,700 m2 (30% as Hotel).
Assumed GFA per Employee	31.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Hotels
Employee Population	770	Persons	
Unit flow factor (UFF)	1.58	m <sup>3</sup> /person/day	Table T-2: Empolyee-Customers & J10 Hotels
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	1,216.60	m <sup>3</sup> /day	
Туре	Retail Trade	-	
Total GFA	16,041	$m^2$	From Maximum Plot Ratio stated in OZP 7.5 of Net Site Area of around 10,700 m2 (20% as Retail).
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	562	Persons	Table 6 Ch SUS - Retail Hade
		2	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	3.4	GESF section 10 - Not applicable for new developments.
ADWF	157.36	m <sup>3</sup> /day	
Type	Business Services	-	Office
Total GFA	40,103	$m^2$	From Maximum Plot Ratio stated in OZP 7.5 of Net Site Area of around 10,700 m2 (50% as Office).
Assumed GFA per Employee	18.2	m <sup>2</sup> /Person	Table 8 CIFSUS - Business Services
Employee Population	2,206	Persons	
Unit flow factor (UFF)	0.08	m <sup>3</sup> /person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	176.48	m <sup>3</sup> /day	ODST SECTION TO THE WAR DESCRIPTION OF THE WA
Total building ADWF Dischages to:	1,550.44	m <sup>3</sup> /day	
FTH4011157	1 550 44	m <sup>3</sup> /day	
F1H401113/	1,550.44	III /day	
_4 Site 4C5			
Туре	Hotels	-	
Total GFA	17,064	$m^2$	From Maximum Plot Ratio stated in OZP 6.0 of Net Site Area of around 9,490 m2 (30% as Hotel).
Assumed GFA per Employee	31.3	m <sup>2</sup> /Person	Table 8 CIFSUS - Hotels
Employee Population	547	Persons	
Unit flow factor (UFF)	1.58	m³/person/day	Table T-2: Empolyee-Customers & J10 Hotels
Catchment Inflow Factors(PCIF)	1.0	_	GESF section 10 - Not applicable for new developments.
ADWF	864.26	m <sup>3</sup> /day	
T	D 4 11 T 1		
Туре	Retail Trade	-	
Total GFA	11,376	m²	From Maximum Plot Ratio stated in OZP 6.0 of Net Site Area of around 9,490 m2 (20% as Retail).
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	399	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	111.72	m <sup>3</sup> /day	
Type	Business Services	-	Office
Total GFA	28,440	$m^2$	From Maximum Plot Ratio stated in OZP 6.0 of Net Site Area of around 9,490 m2 (50% as Retail).
Assumed GFA per Employee	18.2	m <sup>2</sup> /Person	Table 8 CIFSUS - Business Services
Employee Population	1,565	Persons	
Unit flow factor (UFF)	0.08	m <sup>3</sup> /person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	125.20	m <sup>3</sup> /day	The state of the s
Total building ADWF	1,101.18	m <sup>3</sup> /day	
Dischages to: FTH4011160	1,101.18	m <sup>3</sup> /day	
1 111 101 1100	1,101.10		
T ADWF	2,651.62	m <sup>3</sup> /day	
calised Sewage Flow Estimates	Estimation	Unit	Remark
L_5 Lam Chak Street Substation			
Emergency discharge	0.00	1/s	Assumed no discharge during normal operations
T-4-1111 1 1 1 T	0.00	1/-	
Total building Localised Flow Dischages to:	0.00	1/s	
FMH4043145	0.00	1/s	Transformer buildign sewer discharge is related to leakage or emergencies, assumed zero or neglegible during normal operations

Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Job No. Sheet No. Date 23/01/2025 NP Made by Checked

Table - Capacity Performance of Existing Sewer

Notes: (1) Calculate by Colebrook-White Equation

Job Title

 $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

		Roughness	
_		ks (mm)	
Pipe Materal	V 0.75m/s	V 1.2m/s	0.75 < V < 1.2
VC	3.0	0.6	interpolated
PC	6.0	3.0	interpolated
PE	1.5	0.3	interpolated

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2 V is the velocity, D is the diameter of the sewer and S is the gradient of the sewer. Assumed data

Mar	nhole			DEALING								Existing Pi	pe Parameter						New Sewe
UP_MAN	DN_MAN	Catchment inflow	CON_POP	PEAKING FACTOR	ACC_ADWF	Peak Flow	DIA (D)	LEN	UP_GL	DN_GL	UP_INV	DN_INV	Gradient	Pipe Materal	VEL	CAP	F/C	Sufficient	Adequate
No.	No.				(m³/d)	(L/s)	(mm)	(m)	(mPD)	(mPD)	(mPD)	(mPD)	(S)	ks (mm)	(m/s)	(L/s)	(%)	Capacity?	Velocity?
FTH4011157	FMH4098424	B_3	5,742	5	1550.44	89.72	300	14.3	5.00	5.06	2.48	2.13	40	PE, V>=1.2, ks=0.3 mm	2.70	190.66	47.1%	YES	Existing
FMH4098424	FMH4098421		5,742	5	1550.44	89.72	375	38.8	5.06	5.25	1.67	1.54	306	PE, interpolated, ks=0.9 mm	0.98	108.45	82.7%	YES	Existing
FMH4098421	FMH4098427	E_A EL_3	30,019	4	8105.03	555.23	750	84.0	5.25	5.07	0.48	0.22	328	PE, V>=1.2, ks=0.3 mm	1.65	729.22	76.1%	YES	Existing
FMH4098427	FMH4098429		30,019	4	8105.03	555.23	750	54.0	4.95	4.82	0.20	0.05	348	PE, V>=1.2, ks=0.3 mm	1.60	707.51	78.5%	YES	Existing
FMH4098429	FSH4007922		34,922	4	9428.83	616.52	750	60.4	4.82	4.82	0.04	-0.23	222	PE, V>=1.2, ks=0.3 mm	2.01	888.82	69.4%	YES	Existing
FTH4011160	FMH4098431	B_4	4,078	6	1101.18	76.47	300	12.7	4.80	4.78	1.74	1.35	32	PE, V>=1.2, ks=0.3 mm	3.01	212.82	35.9%	YES	Existing
FMH4098431	FSH4007922	E_C	15,638	4	4222.18	195.47	450	14.0	4.78	4.78	0.75	0.28	30	PE, V>=1.2, ks=0.3 mm	4.03	640.61	30.5%	YES	Existin
FSH4007922	FMH4098438		50,559	4	13651.01	820.33	900	35.3	4.82	4.91	-0.23	-0.36	280	PE, V>=1.2, ks=0.3 mm	2.00	1273.46	64.4%	YES	Existing
FMH4098438	FMH4098439		50,559	4	13651.01	820.33	900	46.0	4.91	5.11	-0.38	-0.56	260	PE, V>=1.2, ks=0.3 mm	2.08	1323.05	62.0%	YES	Existing
FMH4098439	FSH4007923		66,115	4	17851.01	984.32	900	61.8	5.11	5.11	-0.88	-1.12	258	PE, V>=1.2, ks=0.3 mm	2.09	1328.58	74.1%	YES	Existin

UP\_MAN

Abbreviation: Upstream Manhole DN\_MAN Downstream Manhole Average Dry Weather Flow Accumulated Average Dry  $\overline{\mathsf{ADWF}}$ ADWF ACC\_ADWF CON\_POP DIA LEN UP\_GL Contributing Population Diameter Length Upstream Ground Level

DN\_GL UP\_INV Downstream Ground Level Upstream Invert Level DN\_INV Downstream Invert Level VEL Peak Pipe Velocity

PE Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe CAP Peak Pipe Capacity

F/C Peak Flow/Capacity VC Vitrified Clay Pipe PC Precast Concrete Pipe



Ove Arup & Partners Calculation Sheet Job No. Sheet No. Rev. 0 Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Made by Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, NP 26/02/2025 NP Date Checked

## **Estimation of Sewage Flows Estimation for Proposed Development**

## **Design Code**

1. Based on EPD Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning.

Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

2. Planning Department CIFSUS.

existing Discharge - Sites 4C4 and 4C5			
DWF Sewage Flow Estimates P_3 Site 4C4	Estimation	Unit	Remark
Type	Hotels	_	
Total GFA	24,522	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee		m <sup>2</sup> /Person	Table 8 CIFSUS - Hotels
Employee Population	785	Persons	Table 6 CH 503 - Hotels
Unit flow factor (UFF)	1.58	m <sup>3</sup> /person/day	Table T-2: Empolyee-Customers & J10 Hotels
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	1,240.30	m <sup>3</sup> /day	
	1,2 10.00		
Туре	Retail Trade	-	
Total GFA	20,086	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	704	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	197.12	m <sup>3</sup> /day	
_			
Type	Business Services	-	Office
Total GFA	49,422	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee	18.2	m <sup>2</sup> /Person	Table 8 CIFSUS - Business Services
Employee Population	2,719	Persons	
Unit flow factor (UFF)	0.08	m <sup>3</sup> /person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	217.52	m <sup>3</sup> /day	
Total building ADWF Dischages to:	1,654.94	m <sup>3</sup> /day	
FTH4011157	1,654.94	m <sup>3</sup> /day	
P_4 Site 4C5			
Type	Hotels	-	
Total GFA	20,864	$\mathbf{m}^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee		m <sup>2</sup> /Person	Table 8 CIFSUS - Hotels
Employee Population	668	Persons	
Unit flow factor (UFF)		m <sup>3</sup> /person/day	Table T-2: Empolyee-Customers & J10 Hotels
Catchment Inflow Factors(PCIF)	1.0	- 3	GESF section 10 - Not applicable for new developments.
ADWF	1,055.44	m <sup>3</sup> /day	
Type	Retail Trade	-	
Total GFA	8,380	$m^2$	FROM PRELIM LAYOUT
Assumed GFA per Employee	28.6	m <sup>2</sup> /Person	Table 8 CIFSUS - Retail Trade
Employee Population	294	Persons	
Unit flow factor (UFF)	0.28	m <sup>3</sup> /person/day	Table T-2: Empolyee & J4 Retail
Catchment Inflow Factors(PCIF)	1.0	3	GESF section 10 - Not applicable for new developments.
ADWF	82.32	m <sup>3</sup> /day	
Type	Business Services		Office
Total GFA	37,128	$m^2$	FROM PRELIM LAYOUT
		m m <sup>2</sup> /Person	
Assumed GFA per Employee Employee Population	18.2 2,043	m /Person Persons	Table 8 CIFSUS - Business Services
Unit flow factor (UFF)	0.08	m <sup>3</sup> /person/day	Table T-2: Empolyee & J6 Finance, Insurance, Real Estate & Business Services
Catchment Inflow Factors(PCIF)	1.0	-	GESF section 10 - Not applicable for new developments.
ADWF	163.44	m <sup>3</sup> /day	,
Total building ADWF	1,301.20	m <sup>3</sup> /day	
Dischages to:			
FTH4011160	1,301.20	m <sup>3</sup> /day	
	1	m <sup>3</sup> /day	

Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Job Title Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Job No. Sheet No. Date 26/02/2025 Made by Checked

NP

Table - Capacity Performance of Existing and Proposed Sewer under Development Scenario

Notes: (1) Calculate by Colebrook-White Equation  $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

	Roughness										
Pipe Materal	V 0.75m/s	V 1.2m/s	0.75 < V < 1.2								
VC	3.0	0.6	interpolated								
PC	6.0	3.0	interpolated								
PE	1.5	0.3	interpolated								

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2  $\underline{V}$  is the velocity,  $\underline{D}$  is the diameter of the sewer and  $\underline{S}$  is the gradient of the sewer. Assumed data

Existing and Proposed Network - Sites 4C4 and 4C5

Mar		PEAKING	,		Existing Pipe Parameter											New Sewer			
UP_MAN No.	DN_MAN No.	Catchment inflow	CON_POP	FACTOR	ACC_ADWF (m³/d)	Peak Flow (L/s)	DIA (D) (mm)	LEN (m)	UP_GL (mPD)	DN_GL (mPD)	UP_INV (mPD)	DN_INV (mPD)	Gradient (S)	Pipe Materal ks (mm)	VEL (m/s)	CAP (L/s)	F/C (%)	Sufficient Capacity?	Adequate Velocity?
FTH4011157	FMH4098424	P_3	6,129	5	1654.94	95.77	300	14.3	5.00	5.06	2.48	2.13	40	PE, V>=1.2, ks=0.3 mm	2.70	190.66	50.2%	YES	Existing
FMH4098424	FMH4098421		6,129	5	1654.94	95.77	375	38.8	5.06	5.25	1.67	1.54	306	PE, interpolated, ks=0.9 mm	0.98	108.45	88.3%	YES	Existing
FMH4098421	FMH4098427	E_A EL_3	30,406	4	8209.53	560.07	750	84.0	5.25	4.95	0.48	0.22	328	PE, V>=1.2, ks=0.3 mm	1.65	729.22	76.8%	YES	Existing
FMH4098427	FMH4098429		30,406	4	8209.53	560.07	750	54.0	4.95	4.82	0.20	0.05	348	PE, V>=1.2, ks=0.3 mm	1.60	707.51	79.2%	YES	Existin
FMH4098429	FSH4007922		35,309	4	9533.33	621.36	750	60.4	4.82	4.82	0.04	-0.23	222	PE, V>=1.2, ks=0.3 mm	2.01	888.82	69.9%	YES	Existir
FTH4011160	FMH4098431	P_4	4,819	6	1301.20	90.36	300	12.7	4.80	4.78	1.74	1.35	32	PE, V>=1.2, ks=0.3 mm	3.01	212.82	42.5%	YES	Existin
FMH4098431	FSH4007922	E_C	16,379	4	4422.20	204.73	450	14.0	4.78	4.82	0.75	0.28	30	PE, V>=1.2, ks=0.3 mm	4.03	640.61	32.0%	YES	Existin
FSH4007922	FMH4098438		51,687	4	13955.53	832.45	900	35.3	4.82	4.91	-0.23	-0.36	280	PE, V>=1.2, ks=0.3 mm	2.00	1273.46	65.4%	YES	Existin
FMH4098438	FMH4098439		51,687	4	13955.53	832.45	900	46.0	4.91	5.11	-0.38	-0.56	260	PE, V>=1.2, ks=0.3 mm	2.08	1323.05	62.9%	YES	Existin
FMH4098439	FSH4007923		67,243	4	18155.53	995.96	900	61.8	5.11	5.11	-0.88	-1.12	258	PE, V>=1.2, ks=0.3 mm	2.09	1328.58	75.0%	YES	Existin
			0	8	0.00	0.00													
																			$\vdash$

Abbreviation:

UP MAN Upstream Manhole DN\_MAN Downstream Manhole ADWF Average Dry Weather Flow ACC\_ADWF Accumulated Average Dry CON\_POP Contributing Population DIA Diameter LEN Length

UP\_GL Upstream Ground Level DN\_GL Downstream Ground Level UP\_INV **Upstream Invert Level** DN\_INV Downstream Invert Level

VEL Peak Pipe Velocity PE Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe

CAP Peak Pipe Capacity F/C Peak Flow/Capacity Vitrified Clay Pipe
Precast Concrete Pipe VC PC

Job Title

Ove Arup & Partners Calculation Sheet

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2

Job No. Sheet No. Date 26/02/2025 Rev. Made by NP Checked NP

Table - Capacity Performance of Existing and Proposed Sewer under Development Scenario - Mitigated

Notes: (1) Calculate by Colebrook-White Equation

 $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

Roughness ks (mm) Pipe Materal V 0.75m/s V 1.2m/s 0.75 < V < 1.2 interpolated PΕ 1.5 0.3 VC 3.0 0.6 interpolated PC 6.0 3.0 interpolated

v is kinematic viscosity of fluid = 1.14 x 10-6 m2/s and g is the gravity = 9.81m/s2 V is the velocity, D is the diameter of the sewer and S is the gradient of the sewer.

#### Assumed data Proposed Improvements

d Notwork Sites 101 and 105

Manhole				DEALGNIO			Existing Pipe Parameter										New Sewer	Utilisation Rate Compariso			
UP_MAN	DN_MAN	Catchment inflow	CON_POP	PEAKING FACTOR	ACC_ADWF (m³/d)	Peak Flow	DIA (D)	LEN (m)	UP_GL	DN_GL	UP_INV	DN_INV	Gradient	Pipe Materal	VEL	CAP	F/C	Sufficient	Adequate		Proposed I
No.	No.	1 T	I		(III /u)	(L/s)	(mm)	(m)	(mPD)	(mPD)	(mPD)	(mPD)	(S)	ks (mm)	(m/s)	(L/s)	(%)	Capacity?	Velocity?	(%)	(%)
FTH4011157	FMH4098424	P_3	6,129	5	1654.94	95.77	300	14.3	5.00	5.06	2.48	2.13	40	PE, V>=1.2, ks=0.3 mm	2.70	190.66	50.2%	YES	Existing	47%	50%
FMH4098424	FMH4098421		6,129	5	1654.94	95.77	450	38.8	5.06	5.25	1.67	1.54	306	PE, V>=1.2, ks=0.3 mm	1.25	197.94	48.4%	YES	Existing	83%	48%
FMH4098421	FMH4098427	E_A EL_3	30,406	4	8209.53	560.07	750	84.0	5.25	5.07	0.48	0.22	328	PE, V>=1.2, ks=0.3 mm	1.65	729.22	76.8%	YES	Existing	76%	77%
FMH4098427	FMH4098429		30,406	4	8209.53	560.07	750	54.0	4.95	4.82	0.20	0.05	348	PE, V>=1.2, ks=0.3 mm	1.60	707.51	79.2%	YES	Existing	78%	79%
FMH4098429	FSH4007922		35,309	4	9533.33	621.36	750	60.4	4.82	4.82	0.04	-0.23	222	PE, V>=1.2, ks=0.3 mm	2.01	888.82	69.9%	YES	Existing	69%	70%
FTH4011160	FMH4098431	P_4	4,819	6	1301.20	90.36	300	12.7	4.80	4.78	1.74	1.35	32	PE, V>=1.2, ks=0.3 mm	3.01	212.82	42.5%	YES	Existing	36%	42%
FMH4098431	FSH4007922	E_C	16,379	4	4422.20	204.73	450	14.0	4.78	4.78	0.75	0.28	30	PE, V>=1.2, ks=0.3 mm	4.03	640.61	32.0%	YES	Existing	31%	32%
FSH4007922	FMH4098438		51,687	4	13955.53	832.45	900	35.3	4.82	4.91	-0.23	-0.36	280	PE, V>=1.2, ks=0.3 mm	2.00	1273.46	65.4%	YES	Existing	64%	65%
FMH4098438	FMH4098439		51,687	4	13955.53	832.45	900	46.0	4.91	5.11	-0.38	-0.56	260	PE, V>=1.2, ks=0.3 mm	2.08	1323.05	62.9%	YES	Existing	62%	63%
FMH4098439	FSH4007923		67,243	4	18155.53	995.96	900	61.8	5.11	5.11	-0.88	-1.12	258	PE, V>=1.2, ks=0.3 mm	2.09	1328.58	75.0%	YES	Existing	74%	75%
			0	8	0.00	0.00															
									<u> </u>												

UP\_MAN

Abbreviation: Upstream Manhole DN\_MAN Downstream Manhole ADWF Average Dry Weather Flow ACC\_ADWF CON\_POP DIA Accumulated Average Dry Contributing Population Diameter LEN Length

UP\_GL Upstream Ground Level DN\_GL Downstream Ground Level UP\_INV Upstream Invert Level DN\_INV Downstream Invert Level

VEL Peak Pipe Velocity Polyethylene PE100 Pipe or Concrete UPVC/HDPE lined Pipe PΕ

CAP F/C VC PC Peak Pipe Capacity Peak Flow/Capacity
Vitrified Clay Pipe
Precast Concrete Pipe

# Appendix D

#### **Baseline Conditions**

Proposed Minor Relaxation of Plot Ratio (PR), Site Coverage (SC) and Building Height (BH) Restrictions for Permitted/Proposed Commercial Development, Public Transport Station and Underground Vehicle Tunnel at Kai Tak Area 4C Sites 4 and 5 and Adjoining Road Portion of Shing King Street; and Minor Relaxation of PR and BH Restrictions for Permitted Private Housing Development with Proposed Eating Place, Shop and Services and Social Welfare Facilities at Kai Tak Area 3E Sites 1 and 2



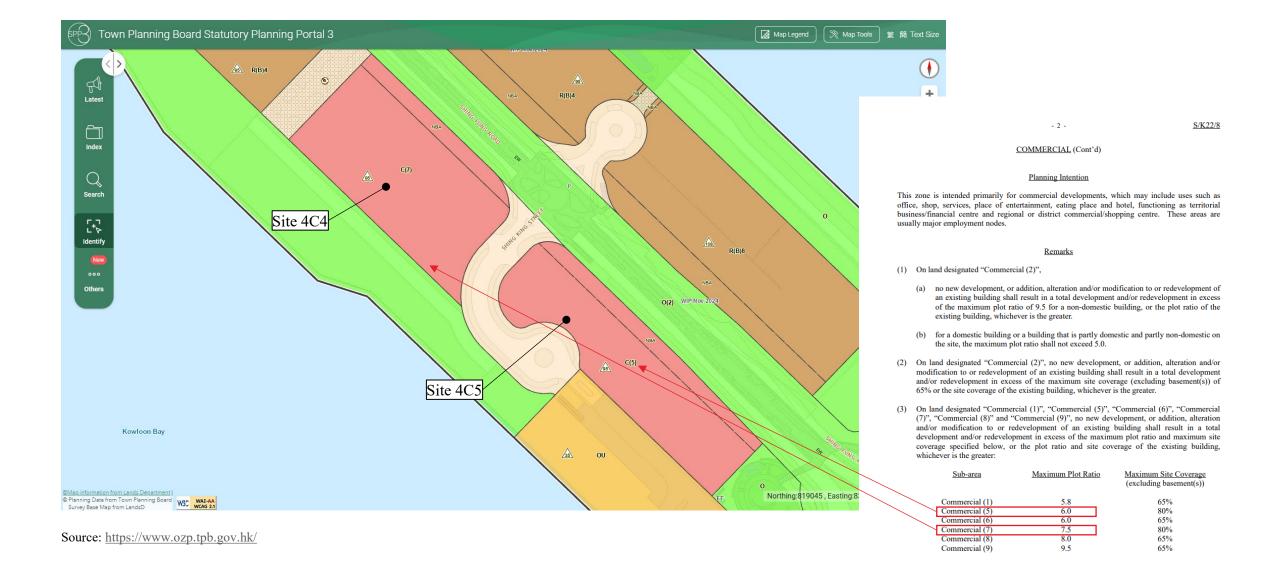
# Baseline: OZP conditions

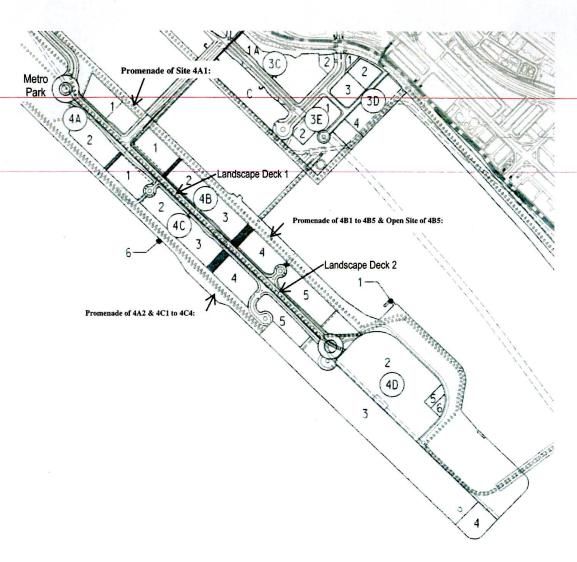


Source: https://www.ozp.tpb.gov.hk/



# Baseline: OZP conditions





Sewage Demand Layout

Catchment Area	m3/d	l/sec
4A1	1200.00	13.89
4A2	1525.00	17.65
4B1	860.00	9.95
4B2	830.00	9.61
4B3	910.00	10.53
4B4	910.00	10.53
4B5	2078.20	24.05
4C1	1093.00	12.65
4C2	1311.00	15.17
4C3	1520.00	17.59
4C4	1460.00	16.90
4C5	1030.00	11.92
4D2	4040.00	46.76
4D3	160.00	1.85
4D4	20.00	0.23
Cruise	3121.00	36.12
Prom of Site 4A1	63.59	0.74
Prom of Site 4A2	40	0.46
Prom of Site 4C3	40	0.46
Prom of Site 4B3	40	0.46
Prom of Site 4B4	40	0.46
Prom of Site 4B5	40	0.46
Prom of Site 4B1 to 4B2 &		
Open Site of 4B5	254.37	2.94
Prom of Site 4C1, 4C2 &		,
4C4	254.37	2.94
Metro Park	1694.35	19.61