Annex C

Updated Traffic Impact Assessment

Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application

Revised TIA Report

June 2024

CTA Consultants Limited志達顧問有限公司

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APPENDIX

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1. INTRODUCTION

1.1 Background

- 1.1.1 CTA Consultants Limited was commissioned as the traffic consultant to prepare a Traffic Impact Assessment (TIA) study for Redevelopment of The Salvation Army Lai King Home at Nos. 200-210 Lai King Hill Road (hereafter called "proposed development").
- 1.1.2 The site currently provides a total of 100 places of Day Activity Centre (DAC), 100 places of Hostel for Severely Mentally Handicapped Persons (HSMH), 20 places of Extended Care Programme (ECP) and 2 places of Residential Respite Service (RRS). No major renovation or refurbishment works have been carried out since last century, the entire building appears to be dilapidated. Water seepage and spalled concrete are easily found around the buildings.
- 1.1.3 It is planned to demolish the existing one 3-storey and two 4-storey main blocks, and redevelop into two 7-storey buildings (excluding LG/F) on the site. With the increase in floor areas, more facilities can be provided to serve the community after the redevelopment. The new buildings will provide a total of 178 places of DAC (including 20 places converted ECP) and 20 places of additional ECP, 178 places of Hostel for Severely Mentally Handicapped Persons (HSMH) (including 2 places (designated places) of Residential Respite Service (RRS)), 120 places of Integrated Vocational Rehabilitation Services Centre (IVRSC), 80 places of Hostel for Moderately Mentally Handicapped Persons (HMMH), 70 places of Care & Attention Home for Severely Disabled Persons (C&A/SD), and a multi-function hall.
- 1.1.4 The location of the proposed development is shown diagrammatically in **Figure 1.1**.



1.2 Study Objectives

- 1.2.1 The main objectives of this study are as follows:
 - To assess the existing traffic conditions in the vicinity of the proposed development;
 - To forecast traffic demands on the adjacent road network in the design year;
 - To estimate the likely traffic generated by the proposed development;
 - To assess the impacts of traffic generated by the proposed development on the adjacent road network; and
 - To recommend improvement measures, if necessary, to alleviate any traffic problems on the road network

2. THE PROPOSED DEVELOPMENT

2.1 Site Location

2.1.1 The proposed development is located at Nos. 200-210 Lai King Hill Road as shown in Figure 1.1. The site is the existing three 4-storey main blocks and it is proposed to redevelop into two 7-storey buildings (excluding LG/F).

2.2 **Proposed Development**

2.1.2 The development schedule for the proposed development is summarized in Table 2.1.

Type of Facilities	Existing Capacity	Proposed Capacity upon Redevelopment
Day Activity Centre ("DAC") Additional Extended Care Programme ("ECP")	100 ⁽¹⁾ 20	178 ⁽²⁾ 20
 Hostel for Severely Mentally Handicapped Persons "HSMH") Residential Respite Service ("RSS") 	$ \begin{array}{c} 100 \\ 2^{(3)} \end{array} $	$178 \\ 2^{(4)}$
Integrated Vocational Rehabilitation Services Centre ("IVRSC")	0	120
Hostel for Moderately Mentally Handicapped Persons "HMMH")	0	80
Care & Attention Home for Severely Disabled Persons ("C&A/SD")	0	70

Table 2.1	Development Parameters of the Proposed Develop	pment

Note:

(1) Includes 20 places of converted ECP.

(2) Includes reprovisioning of 100 places of DAC and 20 places of converted ECP.

(3) Refers to 2 places (casual vacancies) of RRS.

(4) Refers to 2 places (designated places) of RRS.

2.2.1 It is anticipated that the proposed development will be completed by 2029 tentatively. Therefore, design year 2032 (i.e. 3 years after the planned commencement year of the proposed development) is adopted for the Traffic Impact Assessment.

2.3 Vehicular Access

- 2.3.1 The existing vehicular access will be adopted for the proposed development. Location of the vehicular access is shown diagrammatically in **Figure 2.1 (Rev A)**. Swept path analysis demonstrates it is feasible to maneuver HGV in/out the proposed vehicular access is shown in **Figure SP-01 (Rev A)**.
- 2.3.2 The proposed routing for vehicles of proposed development is illustrated in Figure 2.2.

2.4 Internal Transport Facilities Provision

2.4.1 There is no relevant requirements stipulated in the latest Hong Kong Planning Standards and Guidelines (HKPSG) published by Planning Department for "DAC", "HSMH", "ECP", "RRS", ""IVRSC", "HMMH" or "C&A/SD". Comments from SWD on the proposed parking provisions have been and are summarized in Table 2.2.

Parking Re	quirement	Loading/	Unloading Requ	irement
Private Car Parking Space 5m x 2.5m	Light Bus Parking Space 8m x 3m	LGV 7m x 3.5m	M/HGV 11m x 3.5m	Ambulance 7.5m x 3.5m
5 ⁽¹⁾	6 ⁽²⁾	1 ⁽²⁾	1	1 ⁽²⁾

Table 2.2Proposed Parking Provision

Notes:

(1) Including 1 accessible car parking space for 1-50 car parking spaces.

(2) As per SWD comments dated 11 October 2023,

Comment (A)(I)(6)(i): 4 parking spaces measuring 8mL* 3mW*3.3mH for the 4 light buses for the 178-p HSMH; 1 parking space measuring 7mL*3.5mW*3.6H for a 5.5 ton goods vehicle for the 120-p IVRSC and a parking space measuring 8mL*3mW*3.3mH for a light bus for C&A/SD are required.

Comment (A)(I)(6)(ii): A shared loading / unloading area for the private light buses and ambulance for DAC cum HSMH and C&A/SD, a shared loading /unloading area or lay-by for ambulance for HMMH, a shared loading /unloading area for 5.5 ton goods vehicle of IVRSC are required.

2.4.2 Swept path analysis demonstrates it is feasible to maneuver for the proposed critical parking spaces are shown in **Figure SP-01** (**Rev A**) to **Figure SP-03** (**Rev A**).

2.5 Public Transport Services in the Vicinity

2.5.1 Numerous road-based public transport services are provided in vicinity of the proposed development. Details of the current services of franchised buses and GMB routes within 500 meters catchment area are listed in **Table 2.3**.

Service	Service Route Origin-Destination			
	269M	Tin Yan Estate - Cho Yiu	12-25	
	30	Tsuen Wan (Allway Gardens) - Cheung Sha Wan	25-30	
	32H Cheung Shan - Lai Chi Kok		30-60	
	42	Tsing Yi (Cheung Hong Estate) - Shun Lee	15-20	
Franchised	45	Kwai Chung (Lai Yiu Estate) - Kowloon City Ferry	25-30	
Buses	46	Kwai Chung (Lai Yiu Estate) - Jordan (West Kowloon Station)	25-30	
	46X	Hin Keng - Mei Foo	5-15	
	N241	Tsing Yi (Cheung Wang Estate) - Hung Hom Station	25-30 (00:15 to 06:00 only)	
	46M	Lai Kong Street - Lai King Station (Circular)	5-15	
	47M	Wonderland Villas - Lai King Station (Circular)	8-15	
	87K	Hoi Kwai Road Public Transport Interchange - Kwai Fong Station	6-10	
	90M	Mei Foo Station - Lai King Headland (Circular)	4-6	
	91A	Kwai Fong Station - Lai Kong Street	9-15	
	92M	Mei Foo Station - Wah Yuen Chuen (Circular)	5-10	
	93	Wah Yuen Chuen - Tsuen Wan (Ham Tin Street)	6-15	
GMB	93A	Wonderland Villas - Tsuen Wan (Ham Tin Street)	15-25	
	313	Princess Margaret Hospital - Tsuen Wan (Tso Kung Street)	6-11	
	405	Lai King South - Cheung Hang (Circular)	15-20	
	407	Cheung Wang - Princess Margaret Hospital	4-10	
	407A	Princess Margaret Hospital - Kwai Fong Station (Hing Ning Road)	7-13	
	411	Kwai Chung (Lai Kong Street) - Sham Shui Po (Un Chau Street) (Circular)	8-15	
	413	Princess Margaret Hospital - Tsing Yi Public Pier	10-20	

Table 2.3	Road-Based Public Transport Services in the Vicinity
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^{2.5.2} It is revealed that the proposed development is well-served by the comprehensive public transport services in the vicinity.

3. THE EXISTING TRAFFIC CONDITIONS

3.1 Critical Junctions

3.1.1 As shown in Figure 3.1 (Rev B), 5 junctions were identified to be critical for assessment of traffic impact due to the proposed development. They are listed in below Table 3.1 and their existing junction layout arrangements are shown in Figure 3.2 to Figure 3.7 respectively.

Table 3.1	Identified	Critical	Junction
			0

Ref.	Junction	Junction Method of Control		
A1-1	Lai King Hill Road / Joint Street	Priority	3.2	
A1-2	Lai King Hill Road / Bus Terminal Exit	Priority	3.2	
A2	Joint Street / Lai Cho Road	Priority	3.2	
В	Lai King Hill Road / King Cho Road (Near Lai King Estate)	Priority	3.3	
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	3.4	
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	3.5	
Е	Lai King Hill Road / Kwai Chung Interchange	Signal	3.6	
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	3.7	

- 3.1.2 In order to establish the existing traffic condition in the above-mentioned critical junctions, traffic survey in the form of manual classified count was conducted during AM and PM peak periods during 7:30am to 9:30am and 5:30pm to 7:30pm on a typical weekday on 30 March 2023.
- 3.1.3 Analysis of the observed traffic data indicates that the AM and PM peak hour flows occurred from 7:45am to 8:45am and 5:45pm to 6:45pm respectively. The existing traffic flows is presented in **Figure 3.8 (Rev B)**.
- 3.1.4 Existing performance of the identified critical junction and road links are assessed. The results are summarized in Table 3.2, Table 3.3 and the junction calculation sheets are attached in Appendix A.



Ref.	Junction	Method of	Year 2023 RC/DFC ⁽¹⁾	
		Control	AM Peak	PM Peak
A1-1	Lai King Hill Road / Joint Street	Priority	0.34	0.38
A1-2	Lai King Hill Road / Bus Terminal Exit	Priority	0.06	0.10
A2	Joint Street / Lai Cho Road	Priority	0.45	0.35
В	Lai King Hill Road / King Cho Road (Near Lai King Estate)	Priority	0.67	0.72
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	0.58	0.41
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	0.66	0.38
Е	Lai King Hill Road / Kwai Chung Interchange	Signal	43%	63%
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	0.58	0.56

Table 3.2 Operational Performance of Identified Critical Junctions in 2023

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

3.1.5 The assessment results in **Table 3.2** indicate that all critical junctions are at present operating within their capacities during peak hours.

Table 3.3Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2023

		Canacity	Year 2023 Observed Traffic Flow			
Road Link	Direction	(pcu/hr)	AM Pea	k Hour	PM Peak Hour	
		(1)(2)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Lai King Hill Road	Northeast bound	1,450	540	0.37	490	0.34
(Between Joint Street and King Cho Road)	Southwest bound	1,450	610	0.42	550	0.38
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	460	0.32	370	0.26
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	610	0.42	560	0.39
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	460	0.32	390	0.27
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	650	0.45	580	0.40
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	570	0.39	560	0.39
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	810	0.56	670	0.46
Lai King Hill Road	Northeast	1,450	710	0.49	540	0.37

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		Canacity	Year 2023 Observed Traffic Flow				
Road Link	Direction	(pcu/hr)	AM Pea	k Hour	PM Pea	ık Hour	
		(1)(2)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	
(Between Kwai Chung Hospital Road and Kwai Chung Interchange)	bound						
	Southwest bound	1,450	730	0.50	700	0.48	
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	620	0.43	510	0.35	
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	710	0.49	690	0.48	
Kwai Fuk Road (Between Container Port Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Northeast bound	1,450	500	0.34	530	0.37	
	Southwest bound	1,450	670	0.46	590	0.41	

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

- (3) V/C ratio volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.
- 3.1.6 The assessment results in **Tables 3.3** indicate that all critical links are at present operating with ample capacities.

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4. TRAFFIC IMPACT ASSESSMENT

4.1 Design Year

4.1.1 The proposed development is anticipated to be completed by year 2029 tentatively. Year 2032 (i.e. 3 years after completion) is therefore adopted as the design year for this TIA.

4.2 Traffic Forecast

- 4.2.1 The traffic growth can be estimated by applying growth factor, based on the following information sources:
 - I. Historical traffic growth in Annual Traffic Census (ATC) published by the Transport Department (TD).
 - II. Territorial planning assumptions prepared by the Planning Department.

Annual Traffic Census

4.2.2 Numerous of traffic count stations are located in the vicinity of the proposed development. The traffic counts reported in the Annual Traffic Census (ATC), which is published by Transport Department, over a period of six years, i.e. 2016 to 2021 are summarized in **Table 4.1**.

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		Annual Average Daily Traffic (AADT)						Avg.
ATC Stn	Road Name	2016	2017	2018	2019	2020	2021	Annual Growth Rate
5427	Lai King Hill Rd (From Joint St to Kwai Fuk Rd)	16,830*	17,140*	17,400*	19,320	17,720	18,410*	1.81%
5443	Lai King Hill Rd (From Lai Wan Rd to Kwai Chung INT slip rds)	6,600*	6,720*	6,820*	7,590	7,590	7,880*	3.61%
5476	Lai King Hill Rd slip rds C & D (From Lai King Hill RA to Kwai Chung Rd)	10,840*	10,890*	11,190*	11,430	12,210	11,870*	1.83%
5628	Lai King Hill Rd (From King Cho Rd southern junction to Joint St)	15,770	16,060*	16,300*	16,240*	16,590	18,570	3.32%
6204	Lai King Hill Rd (From Kwai Chung INT slip rds to King Cho Rd)	17,020	17,470	16,860	16,450	16,140	16,050	-1.17%
6642	Lai Cho Road (From Joint Street to Lim Cho Street)	3,010	3,220	3,130	2,400	3,000	2,740	-1.86%
Total		70,070	71,500	71,700	73,430	73,250	75,520	1.51%

Table 4.1 Historical Traffic Data from Annual Traffic Census
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Note: *AADT estimated by Growth factor

Planning Data

4.2.3 Reference has also been made to the latest 2019-based Territorial Population Employment Data Matrices (TPEDM) planning data published by the Planning Department for years 2019 and 2031 in the study district. The average annual growth rates in terms of population and employment from 2019 to 2031 are tabulated in Table 4.2.

Table 4.2 TPEDM Planning Data from 2019 to 2031								
Zone	Population			Avg.	Ε	Avg.		
	2019	2026	2031	Annual Growth Rate	2019	2026	2031	Annual Growth Rate
Kwai Chung	319,150	315,800	319,700	0.01%	195,950	192,350	183,600	-0.54%

4.2.4 It is indicated that the average annual growth rate of population in the study area from 2019 to 2031 under the 2019-based Territorial Planning Data is +0.01% per year while the growth rate of employment is -0.54% per year.

Adopted Growth Rate

- 4.2.5 A.A.D.T. of ATC indicates that the traffic flow of the local road network has an average annual growth rate of +1.51% from year 2016 to year 2021.
- 4.2.6 Whilst, the planning data indicates that the population and employment in the area are expected to develop with an average annual growth rate of +0.01% and -0.54% respectively from 2019 to 2031.
- 4.2.7 As a conservative approach, annual growth rate $\pm 1.51\%$ p.a. is adopted.

4.3 **Traffic Generations of Adjacent New Developments**

4.3.1 To fully reflect the growth traffic, trip generation of the future vicinity developments have been taken into consideration. The estimated peak hour trips of the planned adjacent development is detailed in Figure 4.1 (Rev A) and Table 4.3.



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Table 4.3 Planned Adjacent Developments in the Vicinity and the Estimated TripGenerations and Attractions

			AM	Peak	PM Peak	
Approved Planning Application	Description	Development Parameter	GEN (pcu/hr)	ATT (pcu/hr)	GEN (pcu/hr)	ATT (pcu/hr)
A/KC/445	Public rental housing development at Lai Cho Road	819 flats	35 ⁽¹⁾	27 ⁽¹⁾	19 ⁽¹⁾	25 ⁽¹⁾
A/KC/447	Proposed Minor Relaxation of Domestic Plot Ratio Restrict ion for Public Housing Development	540 flats	29 ⁽¹⁾	17 ⁽¹⁾	12 ⁽¹⁾	15 ⁽¹⁾
A/KC/451	Redevelopment of Kwai Chung Hospital	1000 beds + 96 consultation rooms	143 ⁽¹⁾	48 ⁽¹⁾	32 ⁽¹⁾	77 ⁽¹⁾
A/KC/470	Expansion of Lai King Building in Princess Margaret Hospital	~850 beds	76 ⁽¹⁾	54 ⁽¹⁾	54 ⁽¹⁾	54 ⁽¹⁾
A/KC/489	Proposed Comprehensive Development including Flat and Community Facility in "Comprehensive Development Area" Zone at Various Lots in S.D.4 and Adjoining Government Land, Kau Wa Keng, Kwai Chung	5,973 flats + Proposed Feeder Service	513 ⁽¹⁾	302 ⁽¹⁾	222 ⁽¹⁾	287 ⁽¹⁾
	Lai Kong Street private housing		0.0718 ⁽²⁾	0.0425 ⁽²⁾	0.0286 ⁽²⁾	0.037 ⁽²⁾
-	development	$(av. flat size: 60m^2)$	30	18	12	16

Note:

(1) Extracted from TIA report of the relevant application.

(2) Trip rate as stipulated n TPDM Volume 1 Annex C Table 1.

4.4 Planned Junction Layout under Development Projects

4.4.1 According to the approved TIA report of the adjacent development A/KC/489, Junction Lai King Hill Road / Kwai Chung Interchange (E) will be modified and the detail is presented in **Figure 4.2**. It is intended to be carried out before year 2028.

4.5 Reference Traffic Flows

4.5.1 The reference traffic flow is estimated by applying the adopted growth rate to the observed traffic flow in the current year, and the 2032 reference traffic flows can be computed with the following calculation:

2032Reference Traffic2023Flows= (ObserveObserveWithout ProposedTraffic FloDevelopment)	d x ows	Adopted Growth Factor (i.e. +1.51% p.a. for 9 year)) +	Traffic Flows of Planned Adjacent Development
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4.5.2 The 2032 reference traffic flows are shown in **Figure 4.3 (Rev B)**.

4.6 Traffic Generations and Attractions

4.6.1 To estimate the trip generations of the proposed development users, reference has been made to the trip generation rates of the existing Salvation Army Lai King House and sites of similar nature from in-house database. The adopted trip generation rates and the estimated net generation and attraction due to the proposed development users are summarized in **Table 4.4**.

Table 4.4Adopted Generation and Attraction Trip Rates of Proposed
Development Users

	AM	Peak	PM Peak		
Reference Sites	Generation (pcu/hr/bed)	Attraction (pcu/hr/bed)	Generation (pcu/hr/bed)	Attraction (pcu/hr/bed)	
Existing Salvation Army Lai King House	0.03	0.06	-	-	
Providence Garden for Rehab	0.10	0.08	-	-	
Hong Chi Fanling Integrative Rehabilitation Complex	0.07	0.08	-	-	
Harmony Manor - Scenic Court (HSMH)	-	0.08	0.08	0.10	
Adopted Rate	0.10	0.08	0.08	0.10	

4.6.2 Based on **Table 4.4**, the estimated traffic generation and attraction due to the proposed development are summarized in **Table 4.5**.

Table 4.5	Estimated Net Traffic Generation and Attraction of Proposed
	Development

Additional	AMI	Peak	PM Peak			
No. of Places	Generation (pcu/hr)	Attraction (pcu/hr)	Generation (pcu/hr)	Attraction (pcu/hr)		
448-100 = 348	35	28	28	35		

4.6.3 It is anticipated that the proposed development would generate and attract +35 pcu/hr and +28 pcu/hr respectively during AM peak hour, and generate and attract +28 pcu/hr and +35 pcu/hr respectively during PM peak hour.

4.7 Design Traffic Forecasts

4.7.1 The future traffic generations of the proposed development were then assigned onto the road network and superimposed onto the 2032 reference traffic flows (without proposed development) to derive the 2032 design traffic forecasts (with proposed development).

2032 Design Traffic Flows	2032 Reference Traffic Flows		Proposed
(with proposed development)	= (without proposed development)	+	Traffic Flows

4.7.2 Year 2032 design traffic flows (with proposed development) and the development traffic flows are shown in **Figure 4.4 (Rev B)** and **Figure 4.5 (Rev B)** respectively.

5. TRAFFIC IMPACT ASSESSMENT

5.1 Operational Assessment

5.1.1 To assess the potential traffic impact due to the proposed development, capacity analysis of the identified critical junctions and road links for both reference and design scenarios in year 2032 were carried out. The results are summarized in Table 5.1, Table 5.3, Table 5.4 and the junction calculation sheets are attached in Appendix A.

Table 5.1Junction Performance of Identified Critical Junction in Year 2032
(With and Without Proposed Site)

		Method of Control		Year 2032 RC/DFC ⁽¹⁾				
Ref.	Junction			Reference Scenario (Without Proposed Site)		Design Scenario (With Proposed Site)		
				AM Peak	PM Peak	AM Peak	PM Peak	
A1-1	Lai King Hill Road / Joint Street	Priority	With A/KC/489	0.59	0.47	0.60	0.48	
A1-2	Lai King Hill Road / Bus Terminal Exit	Priority	With A/KC/489	0.07	0.10	0.07	0.10	
A2	Joint Street / Lai Cho Road	Priority	With A/KC/489	0.54	0.40	0.54	0.40	
Lai King Hill Road / King	Lai King Hill Road / King	Priority	With A/KC/489	0.82	0.79	0.82	0.80	
D	Estate)		Without A/KC/489	0.79	0.78	0.80	0.79	
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	With A/KC/489	0.77	0.48	0.78	0.49	
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	With A/KC/489	0.93	0.42	0.95	0.46	
Б	Lai King Hill Road / Kwai	Signal	With A/KC/489	0%	25%	-1%	23%	
	improvement)	Signal	Without A/KC/489	37%	58%	35%	56%	
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	With A/KC/489	0.73	0.62	0.73	0.63	

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

5.1.2 The assessment results in **Table 5.1** revealed that all critical junctions would still operate within their capacities in both reference and design year 2032 during the peak

hours, except Junction Lai King Hill Road / Kwai Chung Hospital Road (D) and Junction Lai King Hill Road / Kwai Chung Interchange (E).

- 5.1.3 It is anticipated that Junction D will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development.
- 5.1.4 Without junction modification, it is anticipated that Junction E will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development. According to approved TIA report of A/KC/489, junction modification work is intended to be carried out before year 2028, i.e. before the completion of the proposed development in year 2029. Capacity analysis of Junction E with junction modification was carried out for both reference and design scenarios. The results are summarized in **Table 5.2** and the junction calculation sheets are also attached in **Appendix A**.

Table 5.2Junction Performance of Modified Junction E in Year 2032
(With and Without Proposed Site)

			Ē	Year 2032	RC/DFC ⁽¹⁾	
Ref.	Junction	Method of Control	Reference (Without Sit	e Scenario Proposed te)	Design S (With Proj	Scenario posed Site)
			AM Peak	PM Peak	AM Peak	PM Peak
Е	Lai King Hill Road / Kwai Chung Interchange (With Junction Modification)	Signal	16%	46%	15%	45%

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

5.1.5 The assessment results in **Table 5.2** revealed that Junction E would operate within its capacities during the peak hours in both reference and design year with junction modification.

	- (// 10110 440	P				
		Canacity	Year 2032 R (Without			nario te)
Road Link	Direction	(pcu/hr)	AM Pea	k Hour	PM Pea	ak Hour
		(1)(2)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Lai King Hill Road	Northeast bound	1,450	650	0.45	540	0.37
King Cho Road)	Southwest bound	1,450	740	0.51	610	0.42
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	570	0.39	420	0.29
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	750	0.52	620	0.43
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	560	0.39	440	0.30
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	800	0.55	630	0.43
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	720	0.50	640	0.44
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	1,010	0.70	750	0.52
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	900	0.62	620	0.43
Hospital Road and Kwai Chung Interchange)	Southwest bound	1,450	930	0.64	790	0.54
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	1,070	0.74	690	0.48
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	1,040	0.72	910	0.63
Kwai Fuk Road (Between Container Port	Northeast bound	1,450	680	0.47	630	0.43
Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Southwest bound	1,450	840	0.58	680	0.47

Table 5.3Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2032 (Without Proposed Site)

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

(3) V/C ratio – volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.

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		oposed bite)					
			Yea	Year 2032 Design Scenario (With Proposed Site)			
Road Link	Direction	$\frac{\text{Capacity}}{(\text{ncu/hr})^{(1)(2)}}$	AM Pea	k Hour	PM Peak Hour		
		(pcu/m)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C	
Lai King Hill Road	Northeast bound	1,450	670	0.46	550	0.38	
King Cho Road)	Southwest bound	1,450	750	0.52	620	0.43	
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	590	0.41	430	0.30	
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	760	0.52	630	0.43	
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	580	0.40	450	0.31	
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	820	0.57	650	0.45	
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	740	0.51	650	0.45	
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	1,030	0.71	770	0.53	
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	910	0.63	630	0.43	
Hospital Road and Kwai Chung Interchange)	Southwest bound	1,450	940	0.65	800	0.55	
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	1,080	0.74	700	0.48	
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	1,050	0.72	920	0.63	
Kwai Fuk Road (Between Container Port	Northeast bound	1,450	700	0.48	640	0.44	
Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Southwest bound	1,450	850	0.59	690	0.48	

Table 5.4Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2032 (With Proposed Site)

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

(3) V/C ratio – volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.



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5.1.6 The assessment results in **Table 5.3** and **Table 5.4** revealed that all critical links would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2032 during the peak hours.

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6. PEDESTRIAN IMPACT ASSESSMENT

6.1 Survey on Pedestrian Flows

- 6.1.1 The pedestrian connectivity and index plan at footpath sections and pedestrian crossing of access road to the proposed development are shown in **Figure 6.1** and **Figure 6.2** respectively.
- 6.1.2 In order to establish the existing pedestrian condition of critical footpath in the abovementioned critical section, pedestrian survey was carried out during AM and PM peak periods during 7:30am to 9:30am and 5:30pm to 7:30pm on a typical weekday, on 9 November 2023.
- 6.1.3 The existing pedestrian flows is presented in **Figure 6.3**, and the level-of-service (LOS) for existing operational performances of the identified critical section and pedestrian crossing are listed in **Table 6.1** and **Table 6.2** respectively.

					Year	2023		
	T ()		1	AM Peak]	PM Peak	
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾	
Α	2.9	1.9	185	1.6	А	160	1.4	А
В	1.7	0.7	625	14.9	А	420	10.0	А
D	1.5	0.5	465	15.5	А	175	5.8	А

 Table 6.1
 Operational Performance of Critical Footpath in Existing Scenario

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

Table 6.2Operational Performance of Critical Pedestrian Crossing in
Existing Scenario

	Year 2023					
	AM Pea	AM Peak Hour PM Peak Hour				
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾		
С	685	3,260	135	3,260		

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.1.4 The results of assessment shown in **Table 6.1** and **Table 6.2** indicate that the critical section of footpath and pedestrian crossing are operating with ample reserved capacity during AM and PM peak hours.

6.2 Future Pedestrian Condition

6.2.1 Based on the observed flows and the adopted growth rate of +1.51%, future pedestrian reference flows at the critical section (without proposed development) in Year 2032 are estimated and summarized in **Table 6.3**, **Table 6.4** and **Figure 6.4**.

Table 6.3Operational Performance of Critical Footpath in Year 2032Reference Scenario (Without Proposed Development)

			Year 2032	Reference Sc	enario (V	Without Prop	osed Develop	ment)
	T ()		AM Peak			PM Peak		
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾	
А	2.9	1.9	215	1.9	А	180	1.6	А
В	1.7	0.7	715	17.0	В	480	11.4	А
D	1.5	0.5	530	17.7	В	200	6.7	А

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

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Table 6.4Operational Performance of Critical Pedestrian Crossing in Year2032 Reference Scenario (Without Proposed Development)

	Year 2032 Refere	Year 2032 Reference Scenario (Without Proposed Development)						
	AM Peal	AM Peak Hour PM Peak Hour						
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾				
С	780	3,260	155	3,260				

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.2.2 In order to estimate the pedestrian generation and attraction of the proposed development, reference has been made to the trip generation rates from the existing Salvation Army Lai King Home as shown in **Table 6.5**.

Table 6.5Pedestrian Generation and Attraction Rate of Existing SalvationArmy Lai King Home

AM I	Peak	PM Peak		
Generation (ped/hr/place)	Attraction (ped/hr/place)	Generation (ped/hr/place)	Attraction (ped/hr/place)	
0.12	0.42	0.17	0.05	

6.2.3 Based on the trip rate in Table 6.5 and the development parameters listed in Table2.1, the estimated pedestrian generation and attraction due to the proposed development are summarized in Table 6.6.

Table 6.6Estimated Pedestrian Generation and Attraction of Proposed
Development

	AN	/I Peak	PM Peak		
Additional No. of Places	Generation (ped/hr) Attraction (ped/hr)		Generation (ped/hr) Attraction (ped/hr)		
348 places	42	147	60	18	

6.2.4 The assessment of the design scenario is summarized in Table 6.7, Table 6.8 and Figure 6.5.

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Table 6.7Operational Performance of Critical Footpath in Year 2032 Design
Scenario (With Proposed Development)

			Year 20)32 Design Sc	enario (V	With Propose	d Developmer	nt)
	T ()		1	AM Peak]	PM Peak	
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾	
А	2.9	1.9	400	3.5	А	260	2.3	А
В	1.7	0.7	905	21.5	В	560	13.3	А
D	1.5	0.5	625	20.8	В	240	8.0	А

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

Table 6.8Operational Performance of Critical Pedestrian Crossing in Year2032 Design Scenario (With Proposed Development)

	Year 2032 Design Scenario (With Proposed Development						
	AM Peal	k Hour	PM Peak Hour				
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾			
С	875	3,260	195	3,260			

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.2.5 It is revealed from the assessment results in **Table 6.3** to **Table 6.8** that the critical section of footpath and pedestrian crossing would all operate with ample reserved capacity during AM and PM peak hours in design year 2032.

7. SUMMARY AND CONCLUSION

7.1 Summary

- 7.1.1 CTA Consultants Limited (CTA) is commissioned as the traffic consultant to prepare the Traffic Impact Assessment (TIA) and provide technical justifications in supporting the planning application from traffic engineering point of view.
- 7.1.2 To appraise the existing traffic condition, manual-classified counting surveys were conducted at critical junctions in 2023. Current operational performance of the critical junctions has been assessed. The results reveal that all critical junctions are at present operating within its capacities.
- 7.1.3 Assessment of operational performance of the critical junctions revealed that all critical junctions would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2032 during the peak hours, except Junction Lai King Hill Road / Kwai Chung Hospital Road (D) and Junction Lai King Hill Road / Kwai Chung Interchange (E).
- 7.1.4 It is anticipated that Junction D will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development.
- 7.1.5 Without junction modification, it is anticipated that Junction E will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development. According to approved TIA report of A/KC/489, junction modification work is intended to be carried out before year 2028, i.e. before the completion of the proposed development in year 2029. The assessment results revealed that Junction E would operate within its capacities during the peak hours in both reference and design year with junction modification.

7.2 Conclusion

- 7.2.1 In conclusion, this TIA has demonstrated that the related traffic trips related to the proposed development can be absorbed by the nearby road network and no insurmountable traffic impact will be induced.
- 7.2.2 Therefore, the proposed development is considered feasible from traffic engineering point of view.



DEVELOPMENT SITE PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Weifare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application PROJECT NO.: DRAWING TITLE: 21149HK DRAWING TITLE: SCALE: DATE: 1: 600 @A4 13 MAP 2024	CUL CUL CUL E 20 LEGEND:	Cul Cul Cul Cul Cul Cul Cul Cul	56.5 + 停車場 Car Park RCP CUL ROAD Nanscence mane I CUL L CUL CU
FIGURE NO.: PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Weifare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application CTA Consultants Limited 志達顧問有限公司 PROJECT NO.: DATE: DATE: LAYOUT PLAN OF PROPOSED DEVELOPMENT CTA Consultants Limited	DEVELOPMENT SITE		27.6 +
	FIGURE NO.: PROJECT TITLE: PROJECT NO.: PROJECT NO.: 21149HK DRAWING TITLE: SCALE: DATE: 412 MADE 20231	Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application	CTA Consultants Limited 志達顧問有限公司





FIGURE NO.: PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application CTA Consultants Limited PROJECT NO.: DRAWING TITLE: EXISTING JUNCTION LAYOUT OF CTA Consultants Limited SCALE: DATE: DATE: EXISTING JUNCTION LAYOUT OF		TS TS TS TS TS TS TS TS TS TS	CHORON AS
PROJECT NO:: 21149HK 21149HK DRAWING TITLE: SCALE: DATE:	FIGURE NO.: 3.2	PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application	
LAI KING HILL ROAD / JOINT STREET / LAI CHO ROAD (A1 & A2)	PROJECT NO.: 21149HK SCALE: DATE:	DRAWING TITLE: EXISTING JUNCTION LAYOUT OF LAI KING HILL ROAD / JOINT STREET / LAI CHO ROAD (A1 & A2)	CTA Consultants Limited 志達顧問有限公司










































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APPENDIX A

JUNCTION CALCULATION SHEETS








































TRAFFIC SIGNALS CALCULA	TION								Job No:	21149F	łK							C	ГА С	onsu	ltants	Ltd.
Junction: Description:	Lai K 2023	King Hill Observe	Road / d Traff	Kwai (ïc Flow	Chung /s	Intercl	nange	(E)						-								
	и	otation			(m	Radiu	ıs (m)	0/1	Pro. T	Turning %)	v (pcu/hr)	on Flow r)	Revised Flow (Saturation pcu/hr)	Total Satura (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	:
Approach	Directi	Movement r	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	в	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	170	0.101	0.101	120	0.072	0.072
Kwai Chung Interchange	Е	<	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	100	0.054		110	0.060	
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	325	0.152		268	0.126	
Lai King Hill Road	N	<↓-	3	С	3.8	15	0	1	27%	37%	1990	4120	1940	1920	4070	4050	295	0.152	0.152	242	0.126	0.126
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	610	0.314	0.314	580	0.298	0.298
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	120	0.062		120	0.062	
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)	540(420)	A.I Ey	M. Check P 0.567	hase	P.M Ey	4. Check P 0.496	hase
											610(580) 120(120)	$\overline{}$			$\overline{\nabla}$	80(90)	C (sec) y pract. R.C. (%)	12 120 0.810 43%		C (sec) y pract. R.C. (%)	12 120 0.810 63%	
												170(120)	100(110)									
Stage / Phase Diagrams	,	\ \	1	6			\			2												
	1		T	2		1.1		J.		3												
I/G = 5s	I/G =	: 5s				I/G =	5s															

									300 140.	211471	IIX .							U.	IAU	onsu	ualles	Lta.
Junction: Description:	Lai K	King Hill Reference	Road / ce Trafi	Kwai (fic Flow	Chung I /s	Interch	ange ((E)						-								
	u	otation			(m	Radiu	s (m)	0/1	Pro. T	`urning %)	v (pcu/hr)	on Flow :)	Revised Flow (Saturation pcu/hr)	Total Saturat (po	Revised tion Flow :u/hr)		A.M. Peak			P.M. Peak	
Approach	Directio	Movement n	Phase	Stage	Width (i	Left	Right	Nearside	A.M.	P.M.	Saturation Flow	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	<	2	В	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	250	0.149		160	0.096	
Kwai Chung Interchange	Е	\leq	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	300	0.163	0.163	280	0.152	0.152
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	575	0.270		369	0.173	
Lai King Hill Road	N	€↓-	3	С	3.8	15	0	1	85%	72%	1990	4120	1835	1855	3965	3985	495	0.270	0.270	321	0.173	0.173
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	740	0.380	0.380	630	0.324	0.324
Lai King Hill Road	S	\neg	1	Α	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
Notes:											Traffic Flow 740(630)	(pcu / hr)		AM	(PM)	650(460)	A.M Ey L (sec)	M. Check Pl 0.813 12	hase	P.M Ey L (sec)	 Check P 0.649 12 	hase
											190(160)	<u> </u>			<u>ل</u>	420(230)	C (sec) y pract. R.C. (%)	120 0.810 0%		C (sec) y pract. R.C. (%)	120 0.810 25%	
												250(160)	> 300(280)									
Stage / Phase Diagrams																						
			T	2				J.		3												

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149F	łK							C	ГА С	onsu	ltants	Ltd.
Junction Description	: Lai K : 2032	King Hill Design T	Road / Fraffic	Kwai (Flows	Chung	Interch	nange	(E)						-								
	u	otation			(u	Radiu	ıs (m)	0/1	Pro. T	`urning %)	v (pcu/hr)	on Flow .)	Revised S Flow (Saturation (pcu/hr)	Total Satura (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directio	Movement n	Phase	Stage	Width (1	Left	Right	Nearside	A.M.	P.M.	Saturation Flow	Total Saturati (pcu/hr	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	<	2	в	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	250	0.149		160	0.096	
Kwai Chung Interchange	E	\leq	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	300	0.163	0.163	280	0.152	0.152
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	580	0.272		374	0.175	
Lai King Hill Road	N	<↓	3	С	3.8	15	0	1	84%	71%	1990	4120	1835	1860	3965	3990	500	0.272	0.272	326	0.175	0.175
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	750	0.386	0.386	640	0.329	0.329
Lai King Hill Road	S	\neg	1	Α	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
Notes:											Traffic Flow 750(640)	(pcu / hr)		AM	(PM)	660(470)	A.I Ey L. (sec)	M. Check P 0.821 12	hase	P.M Ey L. (sec)	 Check P 0.656 12 	hase
											190(160)	$\overline{}$			`√	420(230)	C (sec) y pract. R.C. (%)	120 0.810 -1%		C (sec) y pract. R.C. (%)	120 0.810 23%	
												250(160)	300(280)									
Stage / Phase Diagrams	,	\ \	\				,	,														
			T	2		1.1		J.		3												
VG = 5s	I/G =	= 5s	T			I/G =	5s	7		\												

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149H	łK							C	ГА С	onsu	ltants	Ltd.
Junction: Description:	Lai k 2032	King Hill Reference	Road / e Traf	Kwai (fic Flow	Chung /s (Wit	Intercl hout A	hange /KC/4	(E) 89)														
	и	otation			(m	Radiu	ıs (m)	0/1	Pro. T	`urning %)	v (pcu/hr)	on Flow r)	Revised S Flow (Saturation pcu/hr)	Total Saturat (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directi	Movement r	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	в	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	250	0.149		160	0.096	
Kwai Chung Interchange	Е	\leq	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	120	0.065	0.065	120	0.065	0.065
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	367	0.172		284	0.133	
Lai King Hill Road	Ν	<√_	3	С	3.8	15	0	1	27%	39%	1990	4120	1940	1915	4070	4045	333	0.172	0.172	256	0.134	0.134
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	690	0.355	0.355	610	0.314	0.314
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
N. c											T - 65 - F1	(a			(BM)					DI		
ivotes:											690(610)	\rightarrow		AN	(FM)	610(440)	Ey L (sec)	0.592 12	nase	Ey L (sec)	0.512 12	iase
											190(160)	\neg				90(100)	C (sec) y pract. R.C. (%)	120 0.810 37%		C (sec) y pract. R.C. (%)	120 0.810 58%	
												250(160)	> 120(120)									
Stage / Phase Diagrams	I										1						1					
	1		T	2		1,1		J.		3												
I/G = 5s	I/G =	5s				I/G =	5s															

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149F	łK							C'	ГА С	onsu	ltants	Ltd.
Junction	Lai K	King Hill Design T	Road / Fraffic	' Kwai (Flows (Chung Withou	Interch ut A/K	nange C/489)	(E)						-								
	uo	notation			(m	Radiu	ıs (m)	0/1	Pro. T	`urning %)	w (pcu/hr)	ion Flow r)	Revised S Flow (Saturation pcu/hr)	Total Saturat (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directi	Movement r	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	В	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	250	0.149		160	0.096	
Kwai Chung Interchange	Е	Ļ	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	120	0.065	0.065	120	0.065	0.065
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	372	0.175		290	0.136	
Lai King Hill Road	N	<⊥-	3	С	3.8	15	0	1	27%	38%	1990	4120	1940	1915	4070	4045	338	0.174	0.174	260	0.136	0.136
Lai King Hill Road	S	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	700	0.360	0.360	620	0.319	0.319
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)	620(450)	A.I Ey	M. Check Pl 0.599	hase	P.M Ey	4. Check Pl 0.520	nase
											190(160)	$\overline{}$				90(100)	C (sec) y pract. R.C. (%)	120 0.810 35%		C (sec) y pract. R.C. (%)	120 0.810 56%	
												250(160)	120(120)									
Stage / Phase Diagrams	,	\ \	. \	6			\	. 1		0												
	1		T	2		1.1		J.		3												
I/G = 5s	I/G =	= 5s				I/G =	5s															

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149H	IK							C	ГА С	onsu	tants	Ltd.
Junction Description	: Lai K : 2032	ing Hill Referenc	Road / ce Traff	Kwai (ic Flow	Chung //s (Wit	Interch h impr	ange (oveme	E) ent)														
	u	otation			n)	Radiu	s (m)	0/1	Pro. T (9	urning 6)	v (pcu/hr)	on Flow)	Revised S Flow (j	Saturation pcu/hr)	Total Saturat (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directio	Movement n	Phase	Stage	Width (i	Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange Kwai Chung Interchange	E E		1	A A	3.5 3.5	18 0	0 25	1 0	100% 100%	100% 100%	1813.8 1953.8	1813.8 1953.8	1675 1845	1675 1845	1675 1845	1675 1845	250 300	0.149 0.163		160 280	0.096 0.152	0.152
Lai King Hill Road Lai King Hill Road	N N	' ← √	2	B A	3.5 4.3	0 15	0	0 1	0% 100%	0% 100%	2105 2045	2105 2045	2105 1860	2105 1860	2105 1860	2105 1860	650 420	0.309 0.226	0.226	460 230	0.219 0.124	
Lai King Hill Road Lai King Hill Road	S S	\rightarrow	2 3	B C	3.5 3.5	0 0	0 20	1 0	0% 100%	0% 100%	1965 2105	1965 2105	1965 1960	1965 1960	1965 1960	1965 1960	740 190	0.377 0.097	0.377 0.097	630 160	0.321 0.082	0.321 0.082
Pedestrian crossing		↓ - ↓	D E F G	A C C B		Min. g Min. g Min. g Min. g	reen tii reen tii reen tii reen tii	me = 5 me = 5 me = 5	5Gm + 5 5Gm + 9 5Gm + 1 5Gm + 1	FGm = FGm = 1 FGm = 0 FGm =	: 10s : 14s = 16s = 15s											
Notes:											Traffic Flow 740(630) 190(160)	(pcu / hr) → ↓ 250(160)	300(280)	AM((PM)	650(460) 420(230)	A.I Ey L (sec) C (sec) y pract. R.C. (%)	M. Check Pl 0.699 12 120 0.810 16%	hase	P.N Ey L (sec) C (sec) y pract. R.C. (%)	1. Check Pl 0.554 12 120 0.810 46%	nase
Stage / Phase Diagrams		D D D D T S	B1 A2 ED Q C1 D	00	D		D 82	00		D D	 - -											

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149H	IK							C	ГА С	onsu	tants	Ltd.
Junction Description	: Lai K : 2032	Ling Hill Design T	Road / Fraffic I	Kwai (Flows ()	Chung With ir	Interch nprove	ange (ement)	E)														
	u	otation			n)	Radiu	s (m)	0/1	Pro. T (9	urning 6)	v (pcu/hr)	on Flow)	Revised S Flow (j	Saturation pcu/hr)	Total Saturat (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directio	Movement n	Phase	Stage	Width (i	Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	E	< 	1	A	3.5	18 0	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	250 300	0.149		160 280	0.096	0.152
L si Ving Hill Bood	N		2	D	2.5	0	0	0	00%	0%	2105	2105	2105	2105	2105	2105	660	0.214		470	0.152	0.152
Lai King Hill Road	N	← √	1	A	4.3	15	0	1	100%	100%	2045	2045	1860	1860	1860	1860	420	0.226	0.226	230	0.124	
Lai King Hill Road	S	\rightarrow	2	В	3.5	0	0	1	0%	0%	1965	1965	1965	1965	1965	1965	750	0.382	0.382	640	0.326	0.326
Pedestrian crossing	3	\¥	D	A	3.3	Min. g	20 reen tii	0 me = 5	56m + 5	FGm =	2105 10s	2105	1900	1960	1960	1960	190	0.097	0.097	160	0.082	0.082
		•	E F G	C C B		Min. g Min. g Min. g	reen tii reen tii reen tii	me = 5 me = 5 me = 5	5Gm + 9 5Gm + 1 5Gm + 1	FGm = 1 FGm 0 FGm	= 14s = 16s = 15s											
Notes:											Traffic Flow 750(640) 190(160)	(pcu / hr)		AM((PM)	660(470) 420(230)	A.1 Ey L (sec) C (sec) y pract. R.C. (%)	M. Check Pl 0.704 12 120 0.810 15%	hase	P.M Ey L (sec) C (sec) y pract. R.C. (%)	1. Check Pl 0.559 12 120 0.810 45%	nase
												250(160)	300(280)									
Stage / Phase Diagrams		۲ - D - P - P - P - P - P - P - P - P - P - P	B1 A2 ED O C1 O	00	D		D 62	00		D D												

Roundabo	out Junction :	Kwai Fuk Road / Kwai Y	i Road / Conta	iner Port R	oad					P	roject No. 22	2149HK	
Design Ye	ear :	2023 Observed Traffic F	lows										
			AM PM	0 20	Arm D <u>Swai Fuk Road</u> 340 290	430 360	310 350						
AM 320 350 20 0	xrm C PM 370 330 20 0					\sum					ר ב ב	Arm AM 0 430 140 70	A PM 10 360 290 50
	Container Por	t Road		—	1						Kw	ai Yi Road	
			AM	120	270	120	0						
			PM	10	350 Wai Euk Boad	190	0						
					AIIII D								
Input Para	ameters				Arm / Kwai Yi ∣	A - Road	Arm I Kwai Fuk	B - Road	Arm C Container Po	; - ort Road	Arm D Kwai Fuk F	- Road	
					AM	PM	AM	PM	AM	PM	AM	PM	
V F	=	Approach half width (m) Entry width (m)			5.5 9.6	5.5 9.6	7.5 7 9	7.5 7 9	6.7 7	6.7 7	7.1 7.8	7.1 7.8	
L	=	Effective length of flare (m	n)		12	12	1	1	17	17	1	1	
R	=	Entry radius	<i>.</i> .		35	35	29	29	30	30	18.2	18.2	
D △	=	Inscribed circle diameter ((m)		62 18	62 18	65 38	65 38	69	69	60 34	60 34	
Q	=	Entry flow (pcu/hr)			640	710	510	550	690	720	1080	1020	
Qc	=	Circulating flow across en	try (pcu/hr)		790	690	910	970	820	930	490	550	
Output Pa	arameters				Arm	A	Arm	В	Arm	С	Arm D)	
c	_	Sharappage of flare 1.6	*/⊑\/\/I		AM 0.55	PM	AM	PM	AM 0.02	PM 0.02	AM 1.12	PM 1.12	
K	=	1-0.00347*(A-30)-0.978*((L-V)/L (1/R-0.05)		1.06	1.06	0.64	0.64	1.09	1.09	0.98	0.98	
X2	=	V+((E-V)/(1+2*S))			7.46	7.46	7.68	7.68	6.98	6.98	7.32	7.32	
М	=	Exp((D-60)/10)			1.22	1.22	1.65	1.65	2.46	2.46	1.00	1.00	
F	=	303*X2			2260	2260	2326	2326	2116	2116	2217	2217	
id Fo	=	1+(U.5/(1+M))			1.23	1.23	1.19	1.19	1.14	1.14	1.25	1.25	
rc Qe	=	$0.21^{-1} d^{(1+0.2^{*}X2)}$ Capacity = K*(F-Fc*Qc)			0.64 1863	0.64 1931	0.63 1728	0.63 1690	0.58 1796	0.58 1727	0.65 1864	0.65 1826	
DFC	=	Entry Flow/Capacity = Q/C	Qe		0.34	0.37	0.30	0.33	0.38	0.42	0.58	0.56	
DFC of C	ritical Approa	ach =	AM 0.58	PM 0.56									

СТА

Roundabo	out Junction :	Kwai Fuk Road / Kwa	ai Yi Road / Conta	iner Port R	oad					P	Project No. 22	2149HK		
Design Ye	ar:	2032 Reference Traff	fic Flows											
			AM PM	0 20	Arm D wai Fuk Road 390 300	580 440	350 370							
A AM 360 400 20 0	rm C PM 390 340 20 0	ר ים ר									<u>ר</u>	Arm AM 0 490 150 80	A PM 10 370 310 50	
(Container Por	t Road		5	Î					L	Kw	ai Yi Road		
l			AM	130	410	140	0							
			PM	10 K	420 wai Fuk Road	200	0							
Input Para	meters				م Arm کر Kwai Yi	A - Road	Arm Kwai Fuk	B - : Road	Arm C Container Po) - ort Road	Arm D Kwai Fuk F	- Road		
					AM	PM	AM	PM	AM	PM	AM	PM		
V	=	Approach half width (n	n)		5.5	5.5	7.5	7.5	6.7	6.7	7.1	7.1		
E L	=	Effective length of flare	e (m)		9.0	9.6	7.9	7.9	17	17	1.0	7.0 1		
R	=	Entry radius			35	35	29	29	30	30	18.2	18.2		
D	=	Inscribed circle diamet	ter (m)		62	62	65	65	69	69	60 24	60 34		
Q	=	Entry flow (pcu/hr)			720	740	680	630	780	750	1320	1130		
Qc	=	Circulating flow across	s entry (pcu/hr)		990	780	1030	1010	1040	1020	560	570		
Output Pa	rameters				Arm	A PM	Arm	B	Arm	C	Arm D) PM		
S	=	Sharepness of flare =	1.6*(E-V)/L		0.55	0.55	0.64	0.64	0.03	0.03	1.12	1.12		
к	=	1-0.00347*(A-30)-0.97	78*(1/R-0.05)		1.06	1.06	0.99	0.99	1.09	1.09	0.98	0.98		
X2	=	V+((E-V)/(1+2*S))			7.46	7.46	7.68	7.68	6.98	6.98	7.32	7.32		
F	=	303*X2			2260	2260	2326	2326	∠.40 2116	∠.40 2116	2217	2217		
Td	=	1+(0.5/(1+M))			1.23	1.23	1.19	1.19	1.14	1.14	1.25	1.25		
Fc	=	0.21*Td*(1+0.2*X2)	c)		0.64	0.64	0.63	0.63	0.58	0.58	0.65	0.65		
DFC	=	Entry Flow/Capacity =	Q/Qe		0.42	0.40	0.41	0.38	0.47	0.45	0.73	0.62		
DFC of Cr	itical Approa	ach =	AM 0.73	РМ 0.62										
													CTA	

СТА

Roundabo	out Junction :	Kwai Fuk Road / Kwai Yi	Road / Conta	iner Port Ro	bad					<u>P</u>	roject No. 22	2149HK	
Design Ye	ear :	2032 Design Traffic Flow	s										
			AM PM	0 20	Arm D wai Fuk Road 390 300	590 450	350 370						
AM 360 400 20	orm C PM 390 340 20 0	1	L			<u> </u>						Arm / AM 0 490 150 80	A PM 10 370 310 50
	Container Por	rt Road		5	1					E	Kw	ai Yi Road	
1			AM	130	410	140	0						
			PM	10 K	420 wai Fuk Road	210	0						
									A (A D		
Input Para	ameters				Arm / Kwai Yi	a - Road	Arm I Kwai Fuk	B - Road	Container Po	, - ort Road	Arm D Kwai Fuk F	- Road	
V E L R D A Q Q C	= = = = = =	Approach half width (m) Entry width (m) Effective length of flare (m) Entry radius Inscribed circle diameter (n Entry angle (degree) Entry flow (pcu/hr) Circulating flow across entr	n) y (pcu/hr)		AM 5.5 9.6 12 35 62 18 720 1000	PM 5.5 9.6 12 35 62 18 740 790	AM 7.5 7.9 1 29 65 38 680 1030	PM 7.5 7.9 1 29 65 38 640 1010	AM 6.7 7 17 30 69 8 780 1040	PM 6.7 7 17 30 69 8 750 1030	AM 7.1 7.8 1 18.2 60 34 1330 560	PM 7.1 7.8 1 18.2 60 34 1140 580	
Output Pa	rameters				Arm	A	Arm	В	Arm	C	Arm D)	
S K X2 M F Td Fc Qe DFC		$\label{eq:states} \begin{aligned} & \text{Sharepness of flare} = 1.6^{*}(1-0.00347^{*}(A-30)-0.978^{*}(1)\\ & \text{V+}((E-V)/(1+2^{*}S))\\ & \text{Exp}((D-60)/10)\\ & \text{303^{*}X2}\\ & 1+(0.5/(1+M))\\ & 0.21^{*}Td^{*}(1+0.2^{*}X2)\\ & \text{Capacity} = K^{*}(F\text{-}Fc^{*}Qc)\\ & \text{Entry Flow/Capacity} = Q/Q. \end{aligned}$	E-V)/L /R-0.05) e		AM 0.55 1.06 7.46 1.22 2260 1.23 0.64 1720 0.42	PM 0.55 1.06 7.46 1.22 2260 1.23 0.64 1863 0.40	AM 0.64 0.99 7.68 1.65 2326 1.19 0.63 1653 0.41	PM 0.64 0.99 7.68 1.65 2326 1.19 0.63 1665 0.38	AM 0.03 1.09 6.98 2.46 2116 1.14 0.58 1658 0.47	PM 0.03 1.09 6.98 2.46 2116 1.14 0.58 1664 0.45	AM 1.12 0.98 7.32 1.00 2217 1.25 0.65 1820 0.73	PM 1.12 0.98 7.32 1.00 2217 1.25 0.65 1807 0.63	
DFC of C	ritical Approa	ach =	AM 0.73	РМ 0.63									

СТА