Appendix 7

Drainage Impact Assessment





Drainage Impact Assessment Report

for

Redevelopment of The Salvation Army Lai King Home

at

No. 200 – 210 Lai King Hill Road, Kwai Chung,

New Territories (Kwun Chung Town Lot 354)

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CONTENT

1.	Backgrou	nd .		1
2.	Objectives	S		1
3.	Site Inform	nati	on	1
4.	Stormwate	er D	rainage Impact Analysis	3
5.	Conclusio	ons.		7
Арр	oendix			
Арр	endix I	-	Aerial Photo of the Existing Site	
Арр	endix II	-	Approved GBP of the Existing Site	
Арр	endix III	-	Approved Drainage Plan of the Existing Site	
Арр	endix IV	-	DSD Drainage Record Plans	
Арр	endix V	-	Plan Submitted to Section 16	
Арр	endix VI	-	Topographic Survey Record	

1. Background

The Applicant, The Salvation Army intends to develop two(2) 8-storey building block situated at the existing site of No. 200 – 210 Lai King Hill Road, Kwai Chung, New Territories (Kwun Chung Town Lot No. 354). The two(2) building blocks will include Day Activity Centre, Hostel for Severely Mentally Handicapped Persons, Residential Respite Services, Integrated Vocational Rehabilitation Services Centre, Hostel for Moderately Mentally Handicapped Persons, Care & Attention Home for Severely Disable Persons, Extended Care Programme and Ancillary Facilities.

PineBridge Consulting Limited was employed by the Applicant to conduct a Drainage Impact Assessment (DIA) to assess the potential drainage impact arising from the proposed redevelopment.

2. Objectives

This objectives of this DIA are to assess the potential drainage impacts arising from the proposed development and recommend the mitigation measures, if necessary, to alleviate the impacts.

3. Site Information

3.1. Existing Site Information

The site is located at No. 200 – 210 Lai King Hill Road, Kwai Chung, New Territories (Kwun Chung Town Lot No. 354).

Currently, there are three(3) nos. of building erected on site, namely Block A, Block B and Block C. Block A and C are both 5-storey buildings whereas Block B is a 4-storey building. The buildings provides services including Day Activity Centre (DAC), Hostel for Severy Mentally Handicapped Persons (HSMHP), Extended Cre programme (ECP) and Residential Respite Servies (RRS). The buildings will be demolished and two(2) 8-storey building block will be built to provide services in the community and to strengthen comprehensive care and support to the needy persons with disabilities and their carers.





An aerial photo for the existing site is shown on the **Appendix I.**

The existing catchment area is 3,830m² as referred to the approved GBP [Dwg. No. 1182-SD-06] shown on the **Appendix II**.

3.2. Existing Drainage

Approved drainage plans [Dwg. No. 1182-SD-02 & 06] are shown on **Appendix II & III** indicating that there is an existing \emptyset 525 stormwater pipe currently serving for the existing site (catchment area) to collect surface runoff and discharged to the existing Nullah (Done by Government).

Drainage plans (plan no.: 11-NW-1D-4 and 11-NW-6B-2) on **Appendix IV** were obtained from Drainage Services Department (DSD) to gather the background information on drainage infrastructure in the vicinity of the Site.

According to the drainage record plans (plan no.: 11-NW-1D-4 and 11-NW-6B-2), the Site Boundary is currently served by the public drainage facilities near the Kwai Chung Road. The collected surface runoff is collected by an existing U-channel and ultimately discharged to an existing Nullah (Done by Government).

4. Stormwater Drainage Impact Analysis

4.1. Assessment Methodology

As per existing site investigation and DSD's record plan, surface runoff from the proposed development within the Study Boundary is proposed to be connected to DSD's existing stormwater catchpit and finally discharge to the existing Nullah.

This DIA has adopted the Rational Method for runoff estimation:

$$Q_p = 0.273i \sum_{j=1}^m C_j A_j$$

Where,

- Qp is peak runoff (m3/s);
- i is rainfall intensity (mm/hr);
- A_j is the jth catchment (km2);
- C_j is the runoff coefficient of the jth catchment (dimensionless)

The details of the Rational Method can be referred to the Stormwater Drainage Manual (SDM) (DSD, 2018).

Based on a 1:50 year flood protection standard in the SDM and the estimated time of concentration, the appropriate rainfall intensities (i) were calculated based on linear interpolation of the intermediate table values.

4.2. Design Parameter & Drainage Discharge Estimation

The on-site Catchment for the proposed development within the Existing Site Boundary was identified based on the plans as submitted to Section 16 [Appendix V] and topographic survey record of the site boundary and its nearby areas as shown in Appendix VI.

In addition to catchment of the site, there is no change in size before and after development, while the change in paving condition as a result of the proposed development is summarized in **Table A** below.

Catchment	Befor	re Develop	ment	After Development				
	Permeable	Concrete- paved	Catchment Area (m ²)	Permeable	Concrete- paved	Catchment Area (m²)		
Site	13.2%*	86.8%*	3,830	20%	80%	3,830		

Table A: Permeable and Paved Areas of the Site

*denote the percentage to be verified by topographic survey.

As summarized above, there shall be an decrease in stormwater runoff associated with the decrease in the concrete-paved area of the proposed development.

In the hydraulic analysis, runoff coefficient of 0.95 and 0.25 was adopted to represent a concrete-paved area and flatted grassland (heavysoil) respectively. The rainfall intensity of 250mm/hr is assumed for the estimation of the runoff after redevelopment.

	Concrete-paved	Flatted Grassland (heavysoil)					
% of Site Area	80%	20%					
Area (m2)	3,064	766					
Rainfall Coefficient	0.95	0.25					
Peak Runoff (I/s)	202.1	13.3					
Total Peak Runoff = 215.4 I/s							

4.2. Design Parameter & Drainage Discharge Estimation

(CONT'D)

The rain water run-off rate of the entire site is **215.4** I/s and be gone through the existing last manhole S.W.M.H. with connection of the existing \emptyset 525 concrete pipe and discharged to the existing Nullah.

The Manning's roughness coefficients of 0.012 (fair condition for Concrete pipe) as stated in Table 13 of Stormwater Drainage Manual were assumed for the capacity calculation as referred in the Stormwater Drainage Manual.

a) Manning equation,

 $V = 84 \times H^{2/3} \times I^{1/2}$

Where, V = discharge velocity through pipe (m/s)

H = hydraulic meandepth & is $\frac{1}{4}$ when pipe in full bore full

I = Inclination of pipe

b) Discharge pipe formula,

 $Q^1 = A^1 \times V$

Where, Q^1 = quantity of discharge through pipe (m³/s)

 A^1 = Wetted area of flow though pipe (m²)

c) Assumption made,

Assume full depth of full bore flow

Hydraulic meandepth, H = 0.25D

Wetted area of flow through pipe, $A^1 = 0.78D^2$ (D = pipe diameter)

- d) Simplified equation
 - $Q^1 \ = A^1 \ x \ V$

= $0.78D^2 \times 84 \times (0.25D)^{2/3} \times I^{1/2}$

= 25.989 x D^{2.667} x I^{1/2}

e) Available Capacity of the Ø525 concrete pipe

D = 0.525m I = 1:200 (i.e. 5×10^{-3}) Q¹ = 25.989 x D^{2.667} x I^{1/2} Q¹ = 25.989 x (0.525)^{2.667} x (5x10⁻³)^{1/2} = 0.3296 m³/s (**329.6 I/s**) > 215.4 I/s (required)

The capacity of \emptyset 525 concrete pipe with fall 1:200 at full bore is **329.6I/s** which exceeds the required capacity of **215.4 I/s**.

Hence, the existing \emptyset 525 concrete pipe is sufficient to catch up the discharge flow of the Building after redevelopment.

4.3. Drainage Impact Assessment

The catchment of the site is unchanged and the unpaved area is increased after the redevelopment works. The runoff rate is around is **215.4** I/s.

The existing \emptyset 525 concrete pipeworks from the site to the existing Nullah will be retained and reuse after the redevelopment works.

Hence, there will be no adverse impacts on the drainage arising from the proposed redevelopment. However, it is important to provide appropriate internal drainage facilities to collect all runoff from the site itself and connected to the existing \emptyset 525 concrete pipe.

5. Conclusions

The runoff rate of the existing site is around **215.4** I/s and will be collected by the existing Ø525 concrete pipe and discharged to the existing Nullah (Done by Government) after redevelopment. The available capacity of the existing Ø525 concrete pipe is **329.4**I/s that is sufficient to cater for the required runoff. Appropriate internal drainage facilities shall be provided to collect runoff from the site the existing Ø525 concrete pipe.

It is concluded that the redevelopment works would not result in any adverse impacts to the public drainage system.

Appendix I - Aerial Photo of the Existing Site



Appendix II - Approved GBP of the Existing Site

FIRE RES	ISTANCE	REQUI	REMENT	FOR E	LEMEN	IT OF C	ONSTRUC	TION	
			COMPARTMEN	NT OF BULD'G.	FRE	MIN. DIMENSI	IONS OF ELEM	ENTS OF COM	ISTRUCTIO
BLOCK & STOREY	USAGE	CLASS	AREA sq. metre	VOLUM N cu. metre	REQ'D.	R.C. SLAP thickness	cover to	R.C. BEAM	R. C. CUI
B LEV. I	MULTI- PURPOSE	3	438-30	1402.56	1	100 mm	15 m m	50 mm	225 mn
	TRAINING	3	417.30	1335-36	1	100 mm	15 mm	50 mm	225 m i
A LEV.Z	TRAINING	3	417.30	1335-36	- 1	100 m m	15 m.m.	50 mm	225 mr
B LEV. 2	OFFICE	3	438.30	1402-55	1	100 m m	15 mm	50 m m	225 m
C LEV. 2	DOMESTIC	3	417.30	1335.36	1	100 mm	15 mm	50 mm	225 mn
A LEV 3		3	H	"	1	μ		"	"
B LEV 3	· · · · · · · · · · · · · · · · · · ·	3	438.30	1402.56	1	ti .		"	
C LEV. 3	u jan	3	417.30	1335-36	1	и.	p (11	
A LEV. 4	n	3	//	11	1	11	"	11	11
A LEV. 5		3	379.40	1022-08	1	<i>tl</i>	11	11	"

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STO	REY	USAGE	MAIE	FEMALE	MAL RFQ		REU.	PROV.	REQ.	PROV.	REQ.	PROV.	REQ.	PROV.	REQ.	PROV.
	IEV 1	TRAIN ING	38	38	2	2	2	3	2 ·	2	Z	2		2		
	LEV. 2	TRAINING	38	-38	2	. 2	2	3	2	2	2	2		2		1
	· · · · · · · · · · · · · · · · · · ·				REQ	D.	PR0	VIDED	REQ	'D.	PR0V	IDED				
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Ç	LEV. 2	DOMESTIC	2	3		3		6	3)		8	:		3	4
A	LEV. 3	DOMESTIC	2)	-	3	, v	6		;		8	i Line or the		3 .	4
B	LEV. 3	DOMESTIC	2	1		3		6		3		8			3	4
С	LEV. 3	DOMESTIC	2	3		3		6		3		8			3	
Α	LEV. 4	DOMESTIC	2	1	1	3		6		3		8			3	4
A	LEV. 5	STAFF FLAT		4	1. 1.	1		1		1		1	1 1		1]]
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11 F-50



BLOCK 'A' SAME AS BLOCK 'C'	<u>= 419 33 S.M.</u>
BLOCK 1B1	
(a) 10.35 x 11.00 x 2	22770 S.M.
(b) 6.50 x 12.00	78 COS. M.
(c) 2.50 x 9.25 x 2	=== 46 25 S.M.
(f) 12-10 x 8-15 - 5-90 x 1-50	= 89.77S.M.
	<u>441.725. M.</u>
BLOCK (C)	
(a) 10.35 x 11.00 x 2	= 227 70 S. M.
(h) 6.50 x 12.00	
c) 2.50 x 9.25 x 2	<u> </u>
(d) 12.10 x 6.30 - 5.90 x 1.50	= 67.38 S. M.
	419.33 S.M.
GARBAGE & JANITOR	
8.80 x 1.95	<u> </u>
ACTURAL SITE COVERAGE PROVIDED	

Appendix III - Approved Drainage Plan of the Existing Site



Appendix IV – DSD Drainage Record Plans





Mainland South Division Drainage Services Department \bigcirc The Government of the Hong Kong SAR Map reproduced with permission of the Director of Lands

Map data renewed on August 2020

Appendix V – Plan Submitted to Section 16

APPENDIX V- Plan submitted to Section 16

Appendix VI - Topographic Survey Record

LAND MARKER (1980) H.K. CO., LTD. HEAD OFFICE: UNIT 1303C, 13/F, MIRROR TOWER, NO.61 MODY ROAD TSIM SHA TSUL EAST, KOWLOON, HONG KONG TEL: 2663 9138 FAX: 2666 9921 E-MAIL ADDRESS: general@landmarker.com PLAN NO. 6647/01 REVISION NO. DESCRIPTIONS DATE 0 FIRST ISSUE 05-SEP-2023 WONG YIU CHO JOSEPH AUTHORIZED LAND SURVEYOR FHKIS MRICS RPS(831240 E	AREA=225+141+46+14+56+24 =506m ²				NT VI - TOPOGRAPHIC SURVEY RECORD
JEN LONG	822840 N		822860 N	822880 N	822900 N	822920 N