Appendix 3

Traffic Impact Assessment

Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application

Revised TIA Report

March 2024

CTA Consultants Limited志達顧問有限公司

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APPENDIX

Appendix A Junction Calculation Sheets

1. INTRODUCTION

1.1 Background

- 1.1.1 CTA Consultants Limited was commissioned as the traffic consultant to prepare a Traffic Impact Assessment (TIA) study for Redevelopment of The Salvation Army Lai King Home at Nos. 200-210 Lai King Hill Road (hereafter called "proposed development").
- 1.1.2 The site currently provides a total of 100 places of Day Activity Centre (DAC), 100 places of Hostel for Severely Mentally Handicapped Persons (HSMHP), 20 places of Extended Care Programme (ECP) and 2 places of Residential Respite Service (RRS). No major renovation or refurbishment works have been carried out since last century, the entire building appears to be dilapidated. Water seepage and spalled concrete are easily found around the buildings.
- 1.1.3 It is planned to demolish the existing three 4-storey main blocks and redevelop into two 7-storey buildings (excluding LG/F) on the site. With the increase in floor areas, more facilities can be provided to serve the community after the redevelopment. The new buildings will provide a total of 178 places of DAC (including 20 places converted ECP) and 20 places of additional ECP, 178 places of Hostel for Severely Mentally Handicapped Persons (HSMH) (including 2 places (casual vacancies) of Residential Respite Service (RRS) and 2 designated places of RRS, 120 places of Integrated Vocational Rehabilitation Services Centre (IVRSC), 80 places of Hostel for Moderately Mentally Handicapped Persons (C&A/SD), and a multi-function hall.
- 1.1.4 The location of the proposed development is shown diagrammatically in **Figure 1.1**.



1.2 Study Objectives

- 1.2.1 The main objectives of this study are as follows:
 - To assess the existing traffic conditions in the vicinity of the proposed development;
 - To forecast traffic demands on the adjacent road network in the design year;
 - To estimate the likely traffic generated by the proposed development;
 - To assess the impacts of traffic generated by the proposed development on the adjacent road network; and
 - To recommend improvement measures, if necessary, to alleviate any traffic problems on the road network

2. THE PROPOSED DEVELOPMENT

2.1 Site Location

2.1.1 The proposed development is located at Nos. 200-210 Lai King Hill Road as shown in **Figure 1.1**. The site is the existing three 4-storey main blocks and it is proposed to redevelop into two 7-storey buildings (excluding LG/F).

2.2 Proposed Development

2.1.2 The development schedule for the proposed development is summarized in Table 2.1.

Type of Facilities	Existing Capacity	Proposed Capacity upon Redevelopment
Day Activity Centre ("DAC") / Hostel for Severely Mentally Handicapped Persons ("HSMH") ⁽¹⁾	100	178
Residential Respite Service ("RSS")	2	2
• Extended Care Programme ("ECP")	20	20
Integrated Vocational Rehabilitation Services Centre ("IVRSC")	0	120
Hostel for Moderately Mentally Handicapped Persons ("HMMH")	0	80
Care & Attention Home for Severely Disabled Persons ("C&A")	0	70
Total	100	448

Table 2.1Development Parameters of the Proposed Development

Note:

(1) Includes 20 places of converted ECP and 2 places (causal vacancies) of RRS

2.2.1 It is anticipated that the proposed development will be completed by 2029 tentatively. Therefore, design year 2032 (i.e. 3 years after the planned commencement year of the proposed development) is adopted for the Traffic Impact Assessment.

2.3 Vehicular Access

- 2.3.1 The existing vehicular access will be adopted for the proposed development. Location of the vehicular access is shown diagrammatically in **Figure 2.1 (Rev A)**. Swept path analysis demonstrates it is feasible to maneuver HGV in/out the proposed vehicular access is shown in **Figure SP-01 (Rev A)**.
- 2.3.2 The proposed routing for vehicles of proposed development is illustrated in Figure 2.2.

2.4 Internal Transport Facilities Provision

2.4.1 There is no relevant requirements stipulated in the latest Hong Kong Planning Standards and Guidelines (HKPSG) published by Planning Department for "DAC", "HSMH", "ECP", "RRS", ""IVRSC", "HMMH" or "C&A/SD". Comments from SWD on the proposed parking provisions have been and are summarized in Table 2.2.

Parking Re	quirement	Loading/	Unloading Requ	irement
Private CarLight BusParking SpaceParking Space5m x 2.5m8m x 3m		LGV 7m x 3.5m	M/HGV 11m x 3.5m	Ambulance 7.5m x 3.5m
5 ⁽¹⁾	6 ⁽²⁾	1 ⁽²⁾	1	1 ⁽²⁾

Table 2.2Proposed Parking Provision

Notes:

(1) Including 1 accessible car parking space for 1-50 car parking spaces.

(2) As per SWD comments dated 11 October 2023,

Comment (A)(I)(6)(i): 4 parking spaces measuring 8mL* 3mW*3.3mH for the 4 light buses for the 178-p HSMH; 1 parking space measuring 7mL*3.5mW*3.6H for a 5.5 ton goods vehicle for the 120-p IVRSC and a parking space measuring 8mL*3mW*3.3mH for a light bus for C&A/SD are required.

Comment (A)(I)(6)(ii): A shared loading / unloading area for the private light buses and ambulance for DAC cum HSMH and C&A/SD, a shared loading /unloading area or lay-by for ambulance for HMMH, a shared loading /unloading area for 5.5 ton goods vehicle of IVRSC are required.

2.4.2 Swept path analysis demonstrates it is feasible to maneuver for the proposed critical parking spaces are shown in **Figure SP-01** (**Rev A**) to **Figure SP-03** (**Rev A**).

2.5 Public Transport Services in the Vicinity

2.5.1 Numerous road-based public transport services are provided in vicinity of the proposed development. Details of the current services of franchised buses and GMB routes within 500 meters catchment area are listed in **Table 2.3**.

Service	Route	Origin-Destination	Headway (min)			
	269M	Tin Yan Estate - Cho Yiu	12-25			
	30	Tsuen Wan (Allway Gardens) - Cheung Sha Wan	25-30			
	32H	Cheung Shan - Lai Chi Kok	30-60			
	42	Tsing Yi (Cheung Hong Estate) - Shun Lee	15-20			
Franchised	45	Kwai Chung (Lai Yiu Estate) - Kowloon City Ferry	25-30			
Buses	46	6 Kwai Chung (Lai Yiu Estate) - Jordan (West Kowloon Station)				
	46X	Hin Keng - Mei Foo	5-15			
	N241	241 Tsing Yi (Cheung Wang Estate) - Hung Hom Station				
	46M	Lai Kong Street - Lai King Station (Circular)	5-15			
	47M	Wonderland Villas - Lai King Station (Circular)	8-15			
	87K	87K Hoi Kwai Road Public Transport Interchange - Kwai Fong Station				
	90M	Mei Foo Station - Lai King Headland (Circular)	4-6			
	91A	Kwai Fong Station - Lai Kong Street	9-15			
	92M	Mei Foo Station - Wah Yuen Chuen (Circular)	5-10			
	93	Wah Yuen Chuen - Tsuen Wan (Ham Tin Street)	6-15			
GMB	93A	Wonderland Villas - Tsuen Wan (Ham Tin Street)	15-25			
	313	Princess Margaret Hospital - Tsuen Wan (Tso Kung Street)	6-11			
	405	Lai King South - Cheung Hang (Circular)	15-20			
	407	Cheung Wang - Princess Margaret Hospital	4-10			
	407A	Princess Margaret Hospital - Kwai Fong Station (Hing Ning Road)	7-13			
	411	Kwai Chung (Lai Kong Street) - Sham Shui Po (Un Chau Street) (Circular)	8-15			
	413	Princess Margaret Hospital - Tsing Yi Public Pier	10-20			

^{2.5.2} It is revealed that the proposed development is well-served by the comprehensive public transport services in the vicinity.

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3. THE EXISTING TRAFFIC CONDITIONS

3.1 Critical Junctions

3.1.1 As shown in Figure 3.1 (Rev B), 5 junctions were identified to be critical for assessment of traffic impact due to the proposed development. They are listed in below Table 3.1 and their existing junction layout arrangements are shown in Figure 3.2 to Figure 3.7 respectively.

Table 3.1Identified Critical Junction

Ref.	Junction	Method of Control	Figure No.
A1	Lai King Hill Road / Joint Street	Priority	3.2
A2	Joint Street / Lai Cho Road	Priority	3.2
В	Lai King Hill Road / King Cho Road (Near Lai King Estate)	Priority	3.3
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	3.4
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	3.5
Е	Lai King Hill Road / Kwai Chung Interchange	Signal	3.6
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	3.7

- 3.1.2 In order to establish the existing traffic condition in the above-mentioned critical junctions, traffic survey in the form of manual classified count was conducted during AM and PM peak periods during 7:30am to 9:30am and 5:30pm to 7:30pm on a typical weekday on 30 March 2023.
- 3.1.3 Analysis of the observed traffic data indicates that the AM and PM peak hour flows occurred from 7:45am to 8:45am and 5:45pm to 6:45pm respectively. The existing traffic flows is presented in **Figure 3.8 (Rev A)**.
- 3.1.4 Existing performance of the identified critical junction and road links are assessed. The results are summarized in **Table 3.2**, **Table 3.3** and the junction calculation sheets are attached in **Appendix A**.

Ref.	Junction	Method of	Year 2023 RC/DFC ⁽¹⁾		
		Control	AM Peak	PM Peak	
A1	Lai King Hill Road / Joint Street	Priority	0.54	0.55	
A2	Joint Street / Lai Cho Road	Priority	0.64	0.60	
В	Lai King Hill Road / King Cho Road (Near Lai King Estate)	Priority	0.67	0.71	
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	0.58	0.36	
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	0.59	0.37	
Е	Lai King Hill Road / Kwai Chung Interchange	Signal	54%	67%	
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	0.57	0.55	

Table 3.2 Operational Performance of Identified Critical Junctions in 2023

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

3.1.5 The assessment results in **Table 3.2** indicate that all critical junctions are at present operating within their capacities during peak hours.

Table 3.3Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2023

		Canacity	Year 2023 Observed Traffic Flow			
Road Link	Direction	(pcu/hr)	AM Peal	k Hour	PM Peak Hour	
		(1)(2)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Lai King Hill Road	Northeast bound	1,450	540	0.37	520	0.36
King Cho Road)	Southwest bound	1,450	610	0.42	550	0.38
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	460	0.32	370	0.26
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	610	0.42	560	0.39
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	460	0.32	390	0.27
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	650	0.45	580	0.40
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	570	0.39	560	0.39
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	810	0.56	670	0.46
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	710	0.49	540	0.37

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		Capacity (pcu/hr) (1)(2)	Year 2023 Observed Traffic Flow			
Road Link	Direction		AM Peak Hour		PM Peak Hour	
			Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C
Hospital Road and Kwai Chung Interchange)	Southwest bound	1,450	740	0.51	700	0.48
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	620	0.43	510	0.35
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	720	0.50	690	0.48
Kwai Fuk Road (Between Container Port	Northeast bound	1,450	580	0.40	670	0.46
Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Southwest bound	1,450	520	0.36	430	0.30

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

(3) V/C ratio – volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.

3.1.6 The assessment results in **Tables 3.3** indicate that all critical links are at present operating with ample capacities.

4. TRAFFIC IMPACT ASSESSMENT

4.1 Design Year

4.1.1 The proposed development is anticipated to be completed by year 2029 tentatively. Year 2032 (i.e. 3 years after completion) is therefore adopted as the design year for this TIA.

4.2 Traffic Forecast

- 4.2.1 The traffic growth can be estimated by applying growth factor, based on the following information sources:
 - I. Historical traffic growth in Annual Traffic Census (ATC) published by the Transport Department (TD).
 - II. Territorial planning assumptions prepared by the Planning Department.

Annual Traffic Census

4.2.2 Numerous of traffic count stations are located in the vicinity of the proposed development. The traffic counts reported in the Annual Traffic Census (ATC), which is published by Transport Department, over a period of six years, i.e. 2016 to 2021 are summarized in **Table 4.1**.

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		Annual Average Daily Traffic (AADT)						
ATC Stn	Road Name	2016	2017	2018	2019	2020	2021	Annual Growth Rate
5427	Lai King Hill Rd (From Joint St to Kwai Fuk Rd)	16,830*	17,140*	17,400*	19,320	17,720	18,410*	1.81%
5443	Lai King Hill Rd (From Lai Wan Rd to Kwai Chung INT slip rds)	6,600*	6,720*	6,820*	7,590	7,590	7,880*	3.61%
5476	Lai King Hill Rd slip rds C & D (From Lai King Hill RA to Kwai Chung Rd)	10,840*	10,890*	11,190*	11,430	12,210	11,870*	1.83%
5628	Lai King Hill Rd (From King Cho Rd southern junction to Joint St)	15,770	16,060*	16,300*	16,240*	16,590	18,570	3.32%
6204	Lai King Hill Rd (From Kwai Chung INT slip rds to King Cho Rd)	17,020	17,470	16,860	16,450	16,140	16,050	-1.17%
6642	Lai Cho Road (From Joint Street to Lim Cho Street)	3,010	3,220	3,130	2,400	3,000	2,740	-1.86%
Total		70,070	71,500	71,700	73,430	73,250	75,520	1.51%

Table 4.1Hist	torical Traffic Da	ata from Annual	Traffic Cen	sus (ATC)
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Note: *AADT estimated by Growth factor

Planning Data

4.2.3 Reference has also been made to the latest 2019-based Territorial Population Employment Data Matrices (TPEDM) planning data published by the Planning Department for years 2019 and 2031 in the study district. The average annual growth rates in terms of population and employment from 2019 to 2031 are tabulated in Table 4.2.

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Ta	Table 4.2TPEDM Planning Data from 2019 to 2031							
Zone	Population			Avg.	Ε	Avg.		
	2019	2026	2031	Annual Growth Rate	2019	2026	2031	Annual Growth Rate
Kwai Chung	319,150	315,800	319,700	0.01%	195,950	192,350	183,600	-0.54%

4.2.4 It is indicated that the average annual growth rate of population in the study area from 2019 to 2031 under the 2019-based Territorial Planning Data is +0.01% per year while the growth rate of employment is -0.54% per year.

Adopted Growth Rate

- 4.2.5 A.A.D.T. of ATC indicates that the traffic flow of the local road network has an average annual growth rate of +1.51% from year 2016 to year 2021.
- 4.2.6 Whilst, the planning data indicates that the population and employment in the area are expected to develop with an average annual growth rate of +0.01% and -0.54% respectively from 2019 to 2031.
- 4.2.7 As a conservative approach, annual growth rate $\pm 1.51\%$ p.a. is adopted.

4.3 **Traffic Generations of Adjacent New Developments**

4.3.1 To fully reflect the growth traffic, trip generation of the future vicinity developments have been taken into consideration. The estimated peak hour trips of the planned adjacent development is detailed in Figure 4.1 (Rev A) and Table 4.3.



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Table 4.3 Planned Adjacent Developments in the Vicinity and the Estimated Trip					
Generations and Attractions					

			AM	Peak	PM Peak	
Approved Planning Application	Description	Development Parameter	GEN (pcu/hr)	ATT (pcu/hr)	GEN (pcu/hr)	ATT (pcu/hr)
A/KC/445	Public rental housing development at Lai Cho Road	819 flats	35 ⁽¹⁾	27 ⁽¹⁾	19 ⁽¹⁾	25 ⁽¹⁾
A/KC/447	Proposed Minor Relaxation of Domestic Plot Ratio Restrict ion for Public Housing Development	540 flats	29 ⁽¹⁾	17 ⁽¹⁾	12 ⁽¹⁾	15 ⁽¹⁾
A/KC/451	Redevelopment of Kwai Chung Hospital	1000 beds + 96 consultation rooms	143 ⁽¹⁾	48 ⁽¹⁾	32 ⁽¹⁾	77 ⁽¹⁾
A/KC/470	Expansion of Lai King Building in Princess Margaret Hospital	~850 beds	76 ⁽¹⁾	54 ⁽¹⁾	54 ⁽¹⁾	54 ⁽¹⁾
A/KC/489	Proposed Comprehensive Development including Flat and Community Facility in "Comprehensive Development Area" Zone at Various Lots in S.D.4 and Adjoining Government Land, Kau Wa Keng, Kwai Chung	5,973 flats + Proposed Feeder Service	513 ⁽¹⁾	302 ⁽¹⁾	222 ⁽¹⁾	287 ⁽¹⁾
	Lai Kong Street private housing	410 flats	0.0718 ⁽²⁾	0.0425 ⁽²⁾	0.0286 ⁽²⁾	0.037 ⁽²⁾
-	development	$60m^2$	30	18	12	16

Note:

(1) Extracted from TIA report of the relevant application.

(2) Trip rate as stipulated n TPDM Volume 1 Annex C Table 1.

4.4 Planned Junction Layout under Development Projects

4.4.1 According to the approved TIA report of the adjacent development A/KC/489, Junction Lai King Hill Road / Kwai Chung Interchange (E) will be modified and the detail is presented in **Figure 4.2**. It is intended to be carried out before year 2028.

4.5 Reference Traffic Flows

4.5.1 The reference traffic flow is estimated by applying the adopted growth rate to the observed traffic flow in the current year, and the 2032 reference traffic flows can be computed with the following calculation:

2032Reference Traffic2023Flows= (ObservTraffic FDevelopment)Traffic F	Adopted Growth Factor) + (i.e. +1.51% p.a.) + for 9 year)	Traffic Flows of Planned Adjacent Development
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4.5.2 The 2032 reference traffic flows are shown in Figure 4.3 (Rev B).

4.6 Traffic Generations and Attractions

4.6.1 To estimate the trip generations of the proposed development users, reference has been made to the trip generation rates of the existing Salvation Army Lai King House and sites of similar nature from in-house database. The adopted trip generation rates and the estimated net generation and attraction due to the proposed development users are summarized in **Table 4.4**.

Table 4.4Adopted Generation and Attraction Trip Rates of Proposed
Development Users

		AM Peak	PM Peak			
Reference Sites	Generation (pcu/hr/bed)	Attraction (pcu/hr/bed)	Generation (pcu/hr/bed)	Attraction (pcu/hr/bed)		
Existing Salvation Army Lai King House	0.03	0.06	-	-		
Providence Garden for Rehab	0.10	0.08	-	-		
Hong Chi Fanling Integrative Rehabilitation Complex	0.07	0.08	-	-		
Harmony Manor - Scenic Court (HSMH)	_	0.08	0.08	0.10		
Adopted Rate	0.10	0.08	0.08	0.10		

4.6.2 Based on **Table 4.4**, the estimated traffic generation and attraction due to the proposed development are summarized in **Table 4.5**.

Table 4.5	Estimated Net Traffic Generation and Attraction of Proposed
	Development

Additional	AMI	Peak	PM Peak			
No. of Places	Generation (pcu/hr)	Attraction (pcu/hr)	Generation (pcu/hr)	Attraction (pcu/hr)		
448-100 = 348	35	28	28	35		

4.6.3 It is anticipated that the proposed development would generate and attract +35 pcu/hr and +28 pcu/hr respectively during AM peak hour, and generate and attract +28 pcu/hr and +35 pcu/hr respectively during PM peak hour.

4.7 Design Traffic Forecasts

4.7.1 The future traffic generations of the proposed development were then assigned onto the road network and superimposed onto the 2032 reference traffic flows (without proposed development) to derive the 2032 design traffic forecasts (with proposed development).

2032 Design Traffic Flows	2032 Reference Traffic Flows	+	Proposed
(with proposed development)	= (without proposed development)		Traffic Flows

4.7.2 Year 2032 design traffic flows (with proposed development) and the development traffic flows are shown in **Figure 4.4 (Rev B)** and **Figure 4.5** respectively.

5. TRAFFIC IMPACT ASSESSMENT

5.1 Operational Assessment

5.1.1 To assess the potential traffic impact due to the proposed development, capacity analysis of the identified critical junctions and road links for both reference and design scenarios in year 2032 were carried out. The results are summarized in Table 5.1, Table 5.3, Table 5.4 and the junction calculation sheets are attached in Appendix A.

Table 5.1Junction Performance of Identified Critical Junction in Year 2032
(With and Without Proposed Site)

		Method of Control		Year 2032 RC/DFC (1)				
Ref.	Junction			Reference Scenario (Without Proposed Site)		Design Scenario (With Proposed Site)		
				AM Peak	PM Peak	AM Peak	PM Peak	
A1	Lai King Hill Road / Joint Street	Priority	With A/KC/489	0.81	0.55	0.82	0.66	
A2	Joint Street / Lai Cho Road	Priority	With A/KC/489	0.79	0.69	0.79	0.71	
р	Lai King Hill Road / King Cho Road (Near Lai King	Priority	With A/KC/489	0.81	0.79	0.81	0.79	
В	Estate)		Without A/KC/489	0.79	0.78	0.79	0.78	
С	Lai King Hill Road / King Cho Road (Near Lai King Ventilation Building)	Priority	With A/KC/489	0.76	0.43	0.77	0.44	
D	Lai King Hill Road / Kwai Chung Hospital Road	Priority	With A/KC/489	0.76	0.41	0.77	0.45	
Б	Lai King Hill Road / Kwai	Signal	With A/KC/489	-1%	25%	-1%	24%	
E	improvement)	Signal	Without A/KC/489	36%	58%	34%	57%	
F	Kwai Fuk Road / Kwai Yi Road / Container Port Road	Roundabout	With A/KC/489	0.70	0.61	0.71	0.61	

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

5.1.2 The assessment results in **Table 5.1** revealed that all critical junctions would still operate within their capacities in both reference and design year 2032 during the peak hours, except Junction Lai King Hill Road / Kwai Chung Interchange (E).

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5.1.3 Without junction modification, it is anticipated that Junction E will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development. According to approved TIA report of A/KC/489, junction modification work is intended to be carried out before year 2028, i.e. before the completion of the proposed development in year 2029. Capacity analysis of Junction E with junction modification was carried out for both reference and design scenarios. The results are summarized in Table 5.2 and the junction calculation sheets are also attached in Appendix A.

Table 5.2Junction Performance of Modified Junction E in Year 2032
(With and Without Proposed Site)

			Year 2032 RC/DFC ⁽¹⁾					
Ref.	Junction	Method of Control	Reference Scenario (Without Proposed Site)		Design Scenario (With Proposed Site)			
			AM Peak	PM Peak	AM Peak	PM Peak		
Е	Lai King Hill Road / Kwai Chung Interchange (With Junction Modification)	Signal	15%	46%	15%	45%		

Notes: (1) RC = Reserve Capacity

DFC = Design Flow/Capacity ratio for Priority Junction

5.1.4 The assessment results in **Table 5.2** revealed that Junction E would operate within its capacities during the peak hours in both reference and design year with junction modification.

Links in 2002 (Without Hoposed Site)								
	Capacity		Year 2032 Reference Scenario (Without Proposed Site)			nario te)		
Road Link	Direction	(pcu/hr)	AM Pea	k Hour	PM Pea	ak Hour		
		(1)(2)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C		
Lai King Hill Road	Northeast bound	1,450	660	0.46	560	0.39		
King Cho Road)	Southwest bound	1,450	740	0.51	610	0.42		
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	570	0.39	420	0.29		
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	750	0.52	620	0.43		
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	560	0.39	440	0.30		
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	800	0.55	630	0.43		
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	720	0.50	640	0.44		
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	1,020	0.70	760	0.52		
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	900	0.62	610	0.42		
Hospital Road and Kwai Chung Interchange)	Southwest bound	1,450	940	0.65	790	0.54		
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	1,070	0.74	690	0.48		
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	1,050	0.72	910	0.63		
Kwai Fuk Road (Between Container Port	Northeast bound	1,450	740	0.51	750	0.52		
Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Southwest bound	1,450	680	0.47	510	0.35		

Table 5.3Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2032 (Without Proposed Site)

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

(3) V/C ratio – volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.

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Links in 2052 (with Proposed Site)								
			Year 2032 Design Scenario (With Proposed Site)					
Road Link	Direction	$\frac{\text{Capacity}}{(\text{ncu/hr})^{(1)(2)}}$	AM Pea	k Hour	PM Pea	ak Hour		
		(peu/m)	Flow (pcu/hr)	V/C	Flow (pcu/hr)	V/C		
Lai King Hill Road	Northeast bound	1,450	680	0.47	560	0.39		
King Cho Road)	Southwest bound	1,450	750	0.52	610	0.42		
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	590	0.41	420	0.29		
OUHK - Cita Lai King Learning Centre and Proposed Site)	Southwest bound	1,450	760	0.52	620	0.43		
Lai King Hill Road (Between Proposed Site and	Northeast bound	1,450	580	0.40	440	0.30		
King Cho Road near Cho Yiu Chuen)	Southwest bound	1,450	820	0.57	630	0.43		
Lai King Hill Road (Between King Cho Road near	Northeast bound	1,450	740	0.51	640	0.44		
Cho Yiu Chuen and Kwai Chung Hospital Road)	Southwest bound	1,450	1,040	0.72	760	0.52		
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	910	0.63	610	0.42		
Hospital Road and Kwai Chung Interchange)	Southwest bound	1,450	940	0.65	790	0.54		
Lai King Hill Road (Between Kwai Chung	Northeast bound	1,450	1,080	0.74	690	0.48		
Interchange and Access Road to FSD New Territories Workshop)	Southwest bound	1,450	1,050	0.72	910	0.63		
Kwai Fuk Road (Between Container Port	Northeast bound	1,450	740	0.51	750	0.52		
Road/Kwai Yi Road/Kwai Fuk Road Interchange and Joint Street)	Southwest bound	1,450	680	0.47	510	0.35		

Table 5.4Volume to Capacity (V/C) Ratio Assessment of Identified Road
Links in 2032 (With Proposed Site)

Notes:

(1) Capacity based on Table 2.4.1.1 of Section 2.4, Chapter 2, Volume 2, T.P.D.M.

(2) PCU factor of 1.32 has been derived from the result of traffic count survey. Lai King Hill Road and Kwai Fuk Road are district distributor of approximately 10m wide, therefore capacity per direction = 2,200÷2x1.32 = 1,450pcu/hr.

(3) V/C ratio – volume to capacity ratio for road link. A v/c ratio ≤ 1.0 means that a road has sufficient capacity to cope with the anticipated volume of vehicular traffic. A v/c ratio > 1.0 indicates the onset of congestion. A v/c ratio > 1.2 indicates more serious congestion with traffic speeds deteriorating progressively when there is further increase in traffic.

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5.1.5 The assessment results in **Table 5.3** and **Table 5.4** revealed that all critical links would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2032 during the peak hours.

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6. PEDESTRIAN IMPACT ASSESSMENT

6.1 Survey on Pedestrian Flows

- 6.1.1 The pedestrian connectivity and index plan at footpath sections and pedestrian crossing of access road to the proposed development are shown in **Figure 6.1** and **Figure 6.2** respectively.
- 6.1.2 In order to establish the existing pedestrian condition of critical footpath in the abovementioned critical section, pedestrian survey was carried out during AM and PM peak periods during 7:30am to 9:30am and 5:30pm to 7:30pm on a typical weekday, on 9 November 2023.
- 6.1.3 The existing pedestrian flows is presented in **Figure 6.3**, and the level-of-service (LOS) for existing operational performances of the identified critical section and pedestrian crossing are listed in **Table 6.1** and **Table 6.2** respectively.

					Year	2023			
	T ()		1	AM Peak]	PM Peak		
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		
А	2.9	1.9	185	1.6	А	160	1.4	А	
В	1.7	0.7	625	14.9	А	420	10.0	А	
D	1.5	0.5	465	15.5	А	175	5.8	A	

 Table 6.1
 Operational Performance of Critical Footpath in Existing Scenario

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

Table 6.2 Operational Performance of Critical Pedestrian Crossing in **Existing Scenario**

		Year 2023						
	AM Peal	k Hour	PM Pe	ak Hour				
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾				
С	685	3,260	135	3,260				

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.1.4 The results of assessment shown in **Table 6.1** and **Table 6.2** indicate that the critical section of footpath and pedestrian crossing are operating with ample reserved capacity during AM and PM peak hours.

6.2 **Future Pedestrian Condition**

6.2.1 Based on the observed flows and the adopted growth rate of +1.51%, future pedestrian reference flows at the critical section (without proposed development) in Year 2032 are estimated and summarized in Table 6.3, Table 6.4 and Figure 6.4.

Table 6.3 **Operational Performance of Critical Footpath in Year 2032 Reference Scenario (Without Proposed Development)**

		-	Year 2032 Reference Scenario (Without Proposed Development)						
	T ()		1	AM Peak]	PM Peak		
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		
А	2.9	1.9	215	1.9	А	180	1.6	А	
В	1.7	0.7	715	17.0	В	480	11.4	А	
D	1.5	0.5	530	17.7	В	200	6.7	А	

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

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Table 6.4Operational Performance of Critical Pedestrian Crossing in Year2032 Reference Scenario (Without Proposed Development)

	Year 2032 Reference Scenario (Without Proposed Development)						
	AM Peal	k Hour	PM Peak Hour				
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾			
С	780	3,260	155	3,260			

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.2.2 In order to estimate the pedestrian generation and attraction of the proposed development, reference has been made to the trip generation rates from the existing Salvation Army Lai King Home as shown in **Table 6.5**.

Table 6.5Pedestrian Generation and Attraction Rate of Existing SalvationArmy Lai King Home

AM I	Peak	PM Peak		
Generation (ped/hr/place)	Attraction (ped/hr/place)	Generation (ped/hr/place)	Attraction (ped/hr/place)	
0.12	0.42	0.17	0.05	

6.2.3 Based on the trip rate in Table 6.5 and the development parameters listed in Table2.1, the estimated pedestrian generation and attraction due to the proposed development are summarized in Table 6.6.

Table 6.6Estimated Pedestrian Generation and Attraction of Proposed
Development

	AN	/I Peak	PM Peak		
Additional No. of Places	Generation (ped/hr)	Attraction (ped/hr)	Generation (ped/hr)	Attraction (ped/hr)	
348 places	42	147	60	18	

6.2.4 The assessment of the design scenario is summarized in Table 6.7, Table 6.8 and Figure 6.5.

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Table 6.7Operational Performance of Critical Footpath in Year 2032 Design
Scenario (With Proposed Development)

			Year 2032 Design Scenario (With Proposed Developm					nt)
	T ()		1	AM Peak]	PM Peak	
Critical Section	Footpath Width (m)	Effective Width (m) ⁽¹⁾	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS	Two-way Pedestrian Flow	Two-way Pedestrian Flow Rate	LOS
			(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾		(ped/hr) ⁽²⁾	(ped/min/ m) ⁽³⁾	
А	2.9	1.9	400	3.5	А	260	2.3	А
В	1.7	0.7	905	21.5	В	560	13.3	А
D	1.5	0.5	625	20.8	В	240	8.0	А

Notes:

(1) Effective Width = Total Footpath Width – Death Width (0.5m from railings).

(2) Two-way Pedestrian Flow Rate (ped/min/m) = Peak Pedestrian Flow / 60 min / Effective Width.

(3) LOS details extracted from the HCM are tabulated in TPDM Volume 6 Chapter 10 Clause 10.4.2.3.

Table 6.8Operational Performance of Critical Pedestrian Crossing in Year2032 Design Scenario (With Proposed Development)

	Year 2032 Design Scenario (With Proposed Development)							
	AM Peal	k Hour	PM Peak Hour					
Crossing	Crossing Demand	Crossing Capacity ⁽¹⁾	Crossing Demand	Crossing Capacity ⁽¹⁾				
С	875	3,260	195	3,260				

Note:

(1) Crossing capacity according to TPDM Volume 2, Table 3.7.2.1.

6.2.5 It is revealed from the assessment results in **Table 6.3** to **Table 6.8** that the critical section of footpath and pedestrian crossing would all operate with ample reserved capacity during AM and PM peak hours in design year 2032.

7. SUMMARY AND CONCLUSION

7.1 Summary

- 7.1.1 CTA Consultants Limited (CTA) is commissioned as the traffic consultant to prepare the Traffic Impact Assessment (TIA) and provide technical justifications in supporting the planning application from traffic engineering point of view.
- 7.1.2 To appraise the existing traffic condition, manual-classified counting surveys were conducted at critical junctions in 2023. Current operational performance of the critical junctions has been assessed. The results reveal that all critical junctions are at present operating within its capacities.
- 7.1.3 Assessment of operational performance of the critical junctions revealed that all critical junctions would still operate within their capacities in both reference scenario (without proposed development) and design scenario (with proposed development) in 2032 during the peak hours, except Junction Lai King Hill Road / Kwai Chung Interchange (E).
- 7.1.4 Without junction modification, it is anticipated that Junction E will have negative reserve capacity during AM peak hour in Year 2032 without and with the proposed development. According to approved TIA report of A/KC/489, junction modification work is intended to be carried out before year 2028, i.e. before the completion of the proposed development in year 2029. The assessment results revealed that Junction E would operate within its capacities during the peak hours in both reference and design year with junction modification.

7.2 Conclusion

- 7.2.1 In conclusion, this TIA has demonstrated that the related traffic trips related to the proposed development can be absorbed by the nearby road network and no insurmountable traffic impact will be induced.
- 7.2.2 Therefore, the proposed development is considered feasible from traffic engineering point of view.



DEVELOPMENT SITE PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Weifare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application PROJECT NO.: DRAWING TITLE: 21149HK DRAWING TITLE: SCALE: DATE: 1: 600 @A4 13 MAP 2024	CUL CUL CUL E 20 LEGEND:	Cul Cul Cul Cul Cul Cul Cul Cul	56.5 + 停車場 Car Park RCP CUL ROAD Nanscence mane I CUL L CUL CU
FIGURE NO.: PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Weifare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application CTA Consultants Limited 志達顧問有限公司 PROJECT NO.: DATE: DATE: LAYOUT PLAN OF PROPOSED DEVELOPMENT CTA Consultants Limited	DEVELOPMENT SITE		27.6 +
	FIGURE NO.: PROJECT TITLE: 2.1(REV A) PROJECT TITLE: PROJECT NO.: DRAWING TITLE: SCALE: DATE: 1.000 @ A.4 42 MAD 2021	Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application	CTA Consultants Limited 志達顧問有限公司





FIGURE NO.: PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application CTA Consultants Limited PROJECT NO.: DRAWING TITLE: EXISTING JUNCTION LAYOUT OF CTA Consultants Limited SCALE: DATE: DATE: EXISTING JUNCTION LAYOUT OF		TS TS TS TS TS TS TS TS TS TS	CHO CHO S S S S S S S S S S S S S S S S S S S
PROJECT NO:: 21149HK 21149HK DRAWING TITLE: SCALE: DATE:	FIGURE NO.: 3.2	PROJECT TITLE: Proposed Minor Relaxation of Building Height Restriction for the Permitted Social Welfare Facility (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application	
LAI KING HILL ROAD / JOINT STREET / LAI CHO ROAD (A1 & A2)	PROJECT NO.: 21149HK SCALE: DATE:	DRAWING TITLE: EXISTING JUNCTION LAYOUT OF LAI KING HILL ROAD / JOINT STREET / LAI CHO ROAD (A1 & A2)	CTA Consultants Limited 志達顧問有限公司
























BODIE NO.: 6.1 PROJECT NO.: 21149HK SCALE: DATE: 1 : 2250 @A4 22 NOV 2023	CRedevelopment of The Salvation Army Lai King Home) (Redevelopment of The Salvation Army Lai King Home) at Nos. 200 - 210 Lai King Hill Road, Kwai Chung, New Territories – S16 Planning Application DRAWING TITLE: PEDESTRIAN CONNECTIVITY	CTA Consultants Limited 志達顧問有限公司

















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APPENDIX A

JUNCTION CALCULATION SHEETS

Li King Hill Road/ Joint Street (Jn A1)

Scenario : 2023 Observed Traffic Flows Li King Hill Road (380)Arm C 370 (140) 170 450 (380) Arm A 220 (210) 130 AM (PM) 120 (PM) (150) (110)Joint Street Arm B The predictive equations of capacity of movement are: Q-BA = D(627 + 14W-CR - Y(0.364q-AC + 0.144q-AB + 0.229q-CA + 0.52q-CB)) Q-BC = E(745 - Y(0.364q-AC + 0.144q-AB))Q-CB = F(745 - 0.364Y(q-AC + q-AB))The geometric parameters represented by D, E, F are: D = (1 + 0.094(w-BA - 3.65))(1 + 0.0009(V-rBA - 120))(1 + 0.0006(V-IBA - 150)) E = (1 + 0.094(w-BC - 3.65))(1 + 0.0009(V-rBC - 120))(1 + 0.0006(V-IBA - 150)) F = (1 + 0.094(w-CB - 3.65))(1 + 0.0009(V-rCB - 120))where Y = 1 - 0.0345W q-AB, etc = the design flow of movement AB, etc W = major road width W-CR = central reserve width w-BA, etc = lane width to vehicle v-rBA, etc = visibility to the right for waiting vehicles in stream BA, etc v-IBA = visibility to the left for waiting vehicles in stream BA, etc Geometry : Calculated Input V-rBA w-BA 3.8 0.893 W 13 50 D V-IBA 50 w-BC 0.877 W-CR 0 3.6 Е V-rBC 50 w-CB 4 0.968 F V-rCB 50 0.552 Traffic Flow Capacity PM Analysis : AM РМ AM pcu/hr pcu/hr 370 380 Q-BA 378 398 q-CA q-CB 170 140 Q-BC 559 572 q-AB 220 210 Q-CB 591 606 380 465 450 Q-BAC 472 q-AC q-BA 130 150 q-BC 120 110 0.480 0.423 Results : Ratio of Flow-to-Capacity ΡM AM 0.377 B-A 0.343 B-C 0.192 0.215 C-B 0.288 0.231 B-AC 0.538 0.551

Li King Hill Road/ Joint Street (Jn A1)

Scenario : 2032 Reference Traffic Flows Li King Hill Road Arm C 470 (410) 190 (150) 560 (430) Arm A 280 (250)140 200 AM (PM) (180) (PM) (110)Joint Street Arm B The predictive equations of capacity of movement are: Q-BA = D(627 + 14W-CR - Y(0.364q-AC + 0.144q-AB + 0.229q-CA + 0.52q-CB)) Q-BC = E(745 - Y(0.364q-AC + 0.144q-AB))Q-CB = F(745 - 0.364Y(q-AC + q-AB))The geometric parameters represented by D, E, F are: D = (1 + 0.094(w-BA - 3.65))(1 + 0.0009(V-rBA - 120))(1 + 0.0006(V-IBA - 150)) E = (1 + 0.094(w-BC - 3.65))(1 + 0.0009(V-rBC - 120))(1 + 0.0006(V-IBA - 150)) F = (1 + 0.094(w-CB - 3.65))(1 + 0.0009(V-rCB - 120))where Y = 1 - 0.0345W q-AB, etc = the design flow of movement AB, etc W = major road width W-CR = central reserve width w-BA, etc = lane width to vehicle v-rBA, etc = visibility to the right for waiting vehicles in stream BA, etc v-IBA = visibility to the left for waiting vehicles in stream BA, etc Geometry : Calculated Input V-rBA w-BA 3.8 0.893 W 13 50 D V-IBA 50 w-BC 0.877 W-CR 0 3.6 Е V-rBC 50 w-CB 4 0.968 F V-rCB 50 0.552 Traffic Flow Capacity PM Analysis : AM ΡM AM pcu/hr pcu/hr 470 380 Q-BA 338 398 q-CA q-CB 190 140 Q-BC 535 572 q-AB 280 210 Q-CB 558 606 380 419 560 Q-BAC 472 q-AC q-BA 200 150 q-BC 140 110 0.412 0.423 Results : Ratio of Flow-to-Capacity ΡM AM 0.377 B-A 0.592 B-C 0.262 0.192 C-B 0.341 0.231 B-AC 0.811 0.551

Priority Junction :

Li King Hill Road/ Joint Street (Jn A1)

Scenario :		2032 Design	Traffic Flows						
			Li	King Hill Ro	ad				
<u>Arm C</u>	480 200	(420) (150)	140 (120)	<u>200</u> (180)	aint Streat		570 280	(440) (250) AM PM)	<u>Arm A</u> (PM)
The predictive	e equations of ca Q-BA =	pacity of mov D(627 + 14W	<u>Агт В</u> /ement are: /-CR - Y(0.36	J 4q-AC + 0.1	ont Street 144q-AB + 0.22	29q-CA + 0.52	q-CB))		
	Q-DC =	F(745 - 0.364	Y(q-AC + q-A Y(q-AC + q-A	AB))					
The geometri	ic parameters rep D = E = F =	resented by (1 + 0.094(w- (1 + 0.094(w- (1 + 0.094(w-	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	/-rBA - 120))(1 /-rBC - 120))(1 /-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-IE	3A - 150)) 3A - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flo major road wi central reserv lane width to visibility to the visibility to the	w of moveme idth /e width vehicle e right for waiti e left for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	, etc etc			
Geometry :	<u>Input</u> W W-CR	<u>13</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	3.8 3.6 4	<u>Calc</u>	ulated D E F Y	0.893 0.877 0.968 0.552
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC f	AM 480 200 280 570 200 140 0.412	PM 380 140 250 440 180 120 0.400		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 333 533 556 415	PM 385 558 587 454		
Results :	Ratio of Flow-	to-Capacity B-A B-C C-B B-AC		AM 0.601 0.263 0.360 0.819		PM 0.468 0.215 0.239 0.661			

Scenario :	2	023 Observe	ed Traffic Flo	ws					
<u>Arm C</u>	40 160	(30) (200)	La	i Cho Road			30	(30)]	Arm A
			310 (240) <u>Arm B</u>	<u>90</u> (120) J	oint Street		30	(50) (50) AM PM)	(PM)
The predictive	e equations of cap Q-BA = D Q-BC = E Q-CB = F	pacity of mov 0(627 + 14W 2(745 - Y(0.3 2(745 - 0.364	ement are: -CR - Y(0.36 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.′ I44q-AB)) \B))	144q-AB + 0.22	29q-CA + 0.52	q-CB))		
The geometri	ic parameters repr D = (` E = (` F = (`	resented by I 1 + 0.094(w- 1 + 0.094(w- 1 + 0.094(w-	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	/-rBA - 120))(1 /-rBC - 120))(1 /-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-IE	3A - 150)) 3A - 150))		
where	Y = 1 q-AB, etc = th W = n W-CR = c w-BA, etc = la v-rBA, etc = v v-IBA = v	- 0.0345W ne design flo najor road wi entral reserv ane width to isibility to the isibility to the	w of moveme dth re width vehicle e right for wai e left for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	., etc etc			
Geometry :	Input W_ W-CR_	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	3.8 3.8 3.8	<u>Calc</u>	ulated D E F Y	0.893 0.893 0.950 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-AC q-BA q-BC f	AM 40 160 30 30 90 310 0.775	PM 30 200 50 30 120 240 0.667		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 497 657 694 621	PM 485 655 690 598		
Results :	Ratio of Flow-t	o-Capacity B-A B-C C-B B-AC		AM 0.181 0.472 0.230 0.644		PM 0.248 0.367 0.290 0.602			

Scenario :	2	032 Referer	nce Traffic Flo	ows					
	50.1	(00)	La	i Cho Road					
<u>Arm C</u>	50	(30)							
	210	(220)					40	(30)	Arm A
							60	(70)	<u></u>
								(- /	
			370	110				AM	(PM)
			(270)	(140)			((PM)	
			<u>Arm B</u>		loint Street				
The predictiv	e equations of can	acity of mov	vement are:						
rne prodiouv	Q-BA = D)(627 + 14W	-CR - Y(0.36	4q-AC + 0.	144q-AB + 0.22	29q-CA + 0.52	2q-CB))		
	Q-BC = E	(745 - Y(0.3	864q-AC + 0.1	144q-AB))	•		1 - 77		
	Q-CB = F	(745 - 0.364	IY(q-AC + q-A	AB))					
The geometri	ic parameters repr	esented by	D, E, F are:			0.00004715			
	D = (*	1 + 0.094(w-	·BA - 3.65))(1	+ 0.0009(\	/-rBA - 120))(1	+ 0.0006(V-IE	BA - 150))		
	E = (*	1 + 0.094(W- 1 + 0.004(w	·BC - 3.65))(1	+ 0.0009()	/-rBC - 120))(1 / rCB - 120))	+ 0.0006(V-II	3A - 150))		
	г – (1 + 0.094(w-	-CB = 3.05))(1	+ 0.0009(1	/-108 - 120))				
where	Y = 1	- 0.0345W							
	q-AB, etc = th	ne design flo	w of moveme	ent AB, etc					
	W = m	najor road wi	idth	,					
	W-CR = c	entral reserv	ve width						
	w-BA, etc = la	ane width to	vehicle						
	v-rBA, etc = v	isibility to the	e right for wai	ting vehicle	s in stream BA	, etc			
	v-IBA = v	isibility to the	e left for waiti	ng vehicles	in stream BA,	etc			
Geometrv :	Input						Calc	ulated	
	W	10	V-rBA	50	w-BA	3.8		D	0.893
	W-CR	0	V-IBA	50	w-BC	3.8		E	0.893
			V-rBC	50	w-CB	3.8		F_	0.950
			V-rCB	50				Υ	0.655
Analysis :	Traffic Flow	AM	РМ		Capacity	AM	РМ		
-	pcu/hr				pcu/hr				
	q-CA	50	30		Q-BA	476	477		
	q-CB	210	220		Q-BC	652	653		
	q-AB_	60	70		Q-CB	685	685		
	q-AC_	40	30		Q-BAC	612	593		
	q-BA	110	140						
	d-BC	370	270						
	'_	0.771	0.059						
Results :	Ratio of Flow-te	o-Capacity		AM		PM			
		B-A	_	0.231		0.294			
		B-C		0.568		0.413			
		C-B		0.306	_	0.321			
		B-AC		0.785		0.691			

Scenario .		2052 Design	Trailic Tiows						
			La	i Cho Road					
<u>Arm C</u>	50 210	(30) (220)					40 60	(30) (70)	<u>Arm A</u>
			370 (280) <u>Arm B</u>	110 (140) J	pint Street			AM PM)	(PM)
The predictiv	e equations of cap Q-BA = [Q-BC = E Q-CB = F	pacity of mov D(627 + 14W E(745 - Y(0.3 F(745 - 0.364	vement are: '-CR - Y(0.36 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.1 44q-AB)) \B))	44q-AB + 0.22	29q-CA + 0.52	q-CB))		
The geometri	ic parameters rep D = (E = (F = (resented by l 1 + 0.094(w- 1 + 0.094(w- 1 + 0.094(w-	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(V + 0.0009(V + 0.0009(V	-rBA - 120))(1 -rBC - 120))(1 -rCB - 120))	+ 0.0006(V-IE + 0.0006(V-IE	8A - 150)) 3A - 150))		
where	Y = 1 q-AB, etc = t W = r W-CR = c w-BA, etc = k v-rBA, etc = v v-IBA = v	I - 0.0345W he design flo major road wi central reserv ane width to <i>r</i> isibility to the <i>r</i> isibility to the	w of moveme idth /e width vehicle e right for wai e left for waiti	ent AB, etc ting vehicles ng vehicles	s in stream BA in stream BA,	, etc etc			
Geometry :	Input W_ W-CR_	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	3.8 3.8 3.8	<u>Calc</u>	ulated D E F Y	0.893 0.893 0.950 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC	AM 50 210 60 40 110 370 0.771	PM 30 220 70 30 140 280 0.667		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 476 652 685 612	PM 477 653 685 594		
Results :	Ratio of Flow-f	to-Capacity B-A B-C C-B B-AC		AM 0.231 0.568 0.306 0.785		PM 0.294 0.429 0.321 0.707			

Scenario : 2032 Design Traffic Flows



Scenario :		2023 Observe	ed Traffic Flo	ws					
Arm C	430	(330)	La	i King Hill F	Road				
Anno	30	(40)							
							430	(420)	<u>Arm A</u>
							180	(130)	
		-	180	110				AM	(PM)
			(140)	(160)				(PM)	(****)
			Arm B	ł	King Cho Road			<u> </u>	
The predictiv	e equations of ca	apacity of mov	rement are:						
	Q-BA =	E(745 - Y(0.3))	-CR - 1(0.30) $64a_AC + 0^2$	$4q-AC \neq 0.$ $144a-\Delta B()$	144q-AD + 0.22	29 q-CA + 0.52	(q-CB))		
	Q-DC =	F(745 - 0.364	•Y(q-AC + q-/	AB))					
		,		,,					
The geometri	ic parameters rep	presented by I	D, E, F are:			0.0000000			
	D =	(1 + 0.094)	BA - 3.65))(1	+ 0.0009(\	/-rBA - 120))(1	+ 0.0006(V-IE	BA - 150))		
	E - F =	(1 + 0.094) (1 + 0.094)	DC - 3.03))(1 CB - 3.65))(1	+ 0.0009()	/-IBC - 120))(1 /-rCB - 120))	+ 0.0006(v-1	5A - 150))		
	·	(1 * 0.00 1(1	0.00)/(1	0.0000(100 120))				
where	Y =	1 - 0.0345W							
	q-AB, etc =	the design flo	w of moveme	ent AB, etc					
	W =	major road wi	dth						
	w-BA etc =	lane width to	vehicle						
	v-rBA, etc =	visibility to the	e right for wai	ting vehicle	s in stream BA	, etc			
	v-IBA =	visibility to the	e left for waiti	ng vehicles	in stream BA,	etc			
Geometry :	Input						Cal	culated	
	W.	10	V-rBA	50	w-BA	2.5		<u> </u>	0.786
	W-CR	0		50	w-BC	2.5		E	0.786
			V-IBC	50	w-CB	5		Ý-	0.655
			1100_					·	0.000
Analysis :	Traffic Flow	AM	PM		Capacity	AM	PM		
	pcu/nr g-CA	430	330		O-BA	340	355		
	a-CB	30	40		Q-BC	491	497		
	q-AB	180	130		Q-CB	633	648		
	q-AC	430	420		Q-BAC	434	421		
	q-BA	110	160						
	q-BC	180	140						
	Ť.	0.621	0.467						
Results :	Ratio of Flow	-to-Capacity		AM		PM			
		B-A		0.324		0.451			
		B-C		0.366		0.282			
		C-B B-AC		0.047		0.062			
		D-AC		0.000		0.112			

Scenario :		2032 Referen	ce Traffic Flo	ows					
Arm C	530	(370)	La	i King Hill R	oad				
Anne	40	(50)				F	540	(470)	<u>Arm A</u>
			210	120				AM	(PM)
			(150) Arm B	(170) k	íing Cho Road			(PM)	
The predictiv	e equations of ca Q-BA = Q-BC = Q-CB =	apacity of mov D(627 + 14W E(745 - Y(0.3 F(745 - 0.364	ement are: -CR - Y(0.36 64q-AC + 0. ⁻ Y(q-AC + q- <i>,</i>	4q-AC + 0.′ 144q-AB)) AB))	144q-AB + 0.2	29q-CA + 0.52	2q-CB))		
The geometri	ic parameters re D = E = F =	presented by [(1 + 0.094(w-l (1 + 0.094(w-l (1 + 0.094(w-l	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	′-rBA - 120))(1 /-rBC - 120))(1 ⁄-rCB - 120))	+ 0.0006(V-II + 0.0006(V-I	BA - 150)) BA - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flor major road wi central reserv lane width to v visibility to the visibility to the	w of moveme dth e width vehicle e right for waiti e left for waiti	ent AB, etc iting vehicle ng vehicles	s in stream BA in stream BA,	a, etc etc			
Geometry :	<u>Input</u> W W-CR	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	<u>Cal</u>	<u>culated</u> D E F Y	0.786 0.786 1.056 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC f	AM 530 40 200 540 120 210 0.636	PM 370 50 140 470 170 150 0.469		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 303 469 600 409	PM 337 487 633 407		
Results :	Ratio of Flow	-to-Capacity B-A B-C C-B B-AC		AM 0.395 0.447 0.067 0.807		PM 0.504 0.308 0.079 0.786			

Scenario :		2032 Design	Fraffic Flows						
			La	ii King Hill R	load				
<u>Arm C</u>	550 40	(380) (50)							
						E	550 200	(480) (140)	<u>Arm A</u>
						E F			
			210 (150)	120 (170)				AM (PM)	(PM)
			Arm B	ŀ	King Cho Road				
The predictive	e equations of ca Q-BA = Q-BC = Q-CB =	apacity of mov D(627 + 14W E(745 - Y(0.3 F(745 - 0.364	ement are: -CR - Y(0.36 64q-AC + 0.′ Y(q-AC + q-/	4q-AC + 0. 144q-AB)) AB))	144q-AB + 0.22	29q-CA + 0.52	2q-CB))		
The geometri	ic parameters rep D = E = F =	oresented by E (1 + 0.094(w-l (1 + 0.094(w-l (1 + 0.094(w-l), E, F are: 3A - 3.65))(1 3C - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	/-rBA - 120))(1 /-rBC - 120))(1 /-rCB - 120))	+ 0.0006(V-II + 0.0006(V-II	3A - 150)) BA - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flow major road win central reserv lane width to w visibility to the visibility to the	w of moveme dth e width vehicle right for waiti	ent AB, etc iting vehicle ng vehicles	s in stream BA, in stream BA,	ı, etc etc			
Geometry :	Input W	10	V-rBA	50	w-BA	2.5	Cal	culated D	0.786
	W-CR	0	V-IBA V-rBC V-rCB	50 50 50	w-BC w-CB	2.5 5		E F Y	0.786 1.056 0.655
Analysis :	Traffic Flow	AM	РМ		Capacity	AM	РМ		
	a-CA	550	380		Q-BA	299	334		
	q-CB	40	50		Q-BC	467	485		
	q-AB	200	140		Q-CB	598	631		
	q-AC	550	480		Q-BAC	406	405		
	q-BA	120	170						
	q-вс f	0.636	0.469						
Results :	Ratio of Flow	-to-Capacity		AM		PM			
		B-A B-C		0.401		0.309			
		C-B		0.067		0.079			
		B-AC		0.812		0.790			

Scenario :		2023 Observe	ed Traffic Flo	ws					
Arm C	410	(360)	La	i King Hill R	load				
<u></u>	160	(200)				F	610	(540)	<u>Arm A</u>
			200	50		Γ		() AM	(DM)
			(130) Arm B	(30)	King Cho Road			PM)	(FW)
The predictiv	e equations of ca Q-BA = Q-BC = Q-CB =	apacity of mov D(627 + 14W E(745 - Y(0.3 F(745 - 0.364	ement are: -CR - Y(0.36 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.′ 144q-AB)) \B))	144q-AB + 0.22	29q-CA + 0.52	?q-CB))		
The geometr	ic parameters rep D = E = F =	presented by E (1 + 0.094(w-l (1 + 0.094(w-l (1 + 0.094(w-l	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	/-rBA - 120))(1 /-rBC - 120))(1 /-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-II	3A - 150)) 3A - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flor major road wi central reserv lane width to v visibility to the visibility to the	w of moveme dth e width /ehicle e right for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	, etc etc			
Geometry :	Unput W W-CR	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	<u>Calc</u>	Eulated D E F Y	0.786 0.786 1.056 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC q-BC f	AM 410 160 40 610 50 200 0.800	PM 360 200 40 540 30 130 0.813		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 284 468 623 431	PM 293 481 641 446		
Results :	Ratio of Flow	-to-Capacity B-A B-C C-B B-AC		AM 0.176 0.427 0.257 0.580		PM 0.103 0.270 0.312 0.359			

Scenario :		2032 Referen	ce Traffic Flo	ows					
Arm C	510	(410)	La	i King Hill R	load				
	210	(230)				F	750	(590)	<u>Arm A</u>
			260	50		Γ		<u>AM</u>	(DM)
			(160) Arm <u>B</u>	(30)	ing Cho Road		(PM)	(FW)
The predictiv	e equations of ca Q-BA = Q-BC = Q-CB =	apacity of move D(627 + 14W- E(745 - Y(0.36 F(745 - 0.364	ement are: ·CR - Y(0.36 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.′ I44q-AB)) \B))	144q-AB + 0.22	29q-CA + 0.52	?q-CB))		
The geometr	ic parameters rep D = E = F =	oresented by E (1 + 0.094(w-F (1 + 0.094(w-F (1 + 0.094(w-C	0, E, F are: 3A - 3.65))(1 3C - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	/-rBA - 120))(1 /-rBC - 120))(1 /-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-II	3A - 150)) 3A - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flow major road win central reserv lane width to v visibility to the visibility to the	w of moveme dth e width vehicle right for waiti left for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	, etc etc			
Geometry :	Unput W W-CR	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	<u>Calc</u>	Eulated D E F Y	0.786 0.786 1.056 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC q-BC f	AM 510 210 50 750 50 260 0.839	PM 410 230 40 590 30 160 0.842		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 232 441 585 407	PM 269 472 628 440		
Results :	Ratio of Flow	to-Capacity B-A B-C C-B B-AC		AM 0.215 0.589 0.359 0.761		PM 0.111 0.339 0.366 0.432			

Scenario :		2032 Design	Traffic Flows						
<u>Arm C</u>	530	(420)	La	i King Hill R	oad				
	210	(230)					770 50	(610) (40)	<u>Arm A</u>
			260 (160) Arm B	50 (30)	(ing Cho Road			AM (PM)	(PM)
The predictiv	e equations of c Q-BA = Q-BC = Q-CB =	apacity of mov D(627 + 14W E(745 - Y(0.30 F(745 - 0.364	ement are: -CR - Y(0.36 64q-AC + 0.′ Y(q-AC + q-⁄	4q-AC + 0.1 144q-AB)) AB))	144q-AB + 0.22	29q-CA + 0.52	2q-CB))		
The geometri	ic parameters re D = E = F =	presented by E (1 + 0.094(w-l (1 + 0.094(w-l (1 + 0.094(w-l	D, E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	′-rBA - 120))(1 ∕-rBC - 120))(1 ∕-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-II	3A - 150)) 3A - 150))		
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flow major road win central reserv lane width to v visibility to the visibility to the	w of moveme dth e width /ehicle e right for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	, etc etc			
Geometry :	<u>Input</u> W W-CR	<u> </u>	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	<u>Cal</u>	l <u>culated</u> D E F Y	0.786 0.786 1.056 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC f	AM 530 210 50 770 50 260 0.839	PM 420 230 40 610 30 160 0.842		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 226 437 580 403	PM 264 468 623 436		
Results :	Ratio of Flow	- to-Capacity B-A B-C C-B B-AC		AM 0.221 0.595 0.362 0.769		PM 0.114 0.342 0.369 0.436			

Scenario :		2023 Observe	ed Traffic Flov	WS					
Arm C	470	(500)	Lai	i King Hill R	oad				
	240	(40)					610 200	(610) 60	<u>Arm A</u>
			<u>110</u> (90)	<u> </u>			/	AM PM)	(PM)
		<u> </u>	<u>Arm B</u>	K	wai Chung Inte	erchange			
The predictive	e equations of ca Q-BA = I Q-BC = I Q-CB =	pacity of move D(627 + 14W E(745 - Y(0.3 F(745 - 0.364	ement are: -CR - Y(0.364 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.1 44q-AB)) \B))	44q-AB + 0.22	29q-CA + 0.52	ːq-CB))		
The geometri	ic parameters rep D = (E = (F = (resented by E (1 + 0.094(w-l (1 + 0.094(w-l (1 + 0.094(w-l), E, F are: BA - 3.65))(1 BC - 3.65))(1 CB - 3.65))(1	+ 0.0009(V + 0.0009(V + 0.0009(V	′-rBA - 120))(1 ′-rBC - 120))(1 ′-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-IE	3A - 150)) 3A - 150))		
where	Y =	1 - 0.0345W the design flow major road win central reserv lane width to v visibility to the visibility to the	w of moveme dth e width vehicle ∋ right for wai' ∋ left for waitir	nt AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	., etc etc			
Geometry :	Input W W-CR	<u> </u>	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	Calci	<u>ulated</u> D E F Y	0.786 0.786 1.056 0.655
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BA q-BC	AM 470 240 200 610 100 110 0.524	PM 500 40 60 610 60 90 0.600		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 244 456 583 355	PM 304 467 618 402		
Results :	Ratio of Flow-	to-Capacity B-A B-C C-B B-AC	 	AM 0.410 0.241 0.412 0.591		PM 0.197 0.193 0.065 0.373			

Scenario :	2	2032 Referen	ce Traffic Flo	ows										
			La	ii King Hill R	oad									
Arm C	610	(570)												
	290	(40)												
	۱ <u> </u>						790	(690)	Arm A					
							230	(70)						
								,,						
			120	110				AM	(PM)					
			(100)	(60)			(PM)						
			Arm B	k	wai Chung Int	erchange								
					-	•								
The predictive	e equations of ca	pacity of mov	ement are:											
Q-BA = D(627 + 14W-CR - Y(0.364q-AC + 0.144q-AB + 0.229q-CA + 0.52q-CB))														
	Q-BC = E(745 - Y(0.364q-AC + 0.144q-AB))													
	Q-CB = F(745 - 0.364Y(q-AC + q-AB))													
The geometri	c parameters rep	resented by [D, E, F are:											
	D = ((1 + 0.094(w-	BA - 3.65))(1	+ 0.0009(\	′-rBA - 120))(1	+ 0.0006(V-IE	3A - 150))							
	E = ((1 + 0.094(w-	BC - 3.65))(1	+ 0.0009(\	/-rBC - 120))(1	+ 0.0006(V-I	3A - 150))							
	F = ((1 + 0.094(w-	CB - 3.65))(1	+ 0.0009(\	/-rCB - 120))									
where	Y = 7	1 - 0.0345W												
	q-AB, etc = t	the design flo	w of moveme	ent AB, etc										
	W = r	major road wi	dth											
	W-CR = central reserve width													
	w-BA, etc = lane width to vehicle													
	v-rBA, etc = \	visibility to the	e right for wa	iting vehicle	s in stream BA	, etc								
	v-IBA = v	visibility to the	e left for waiti	ng vehicles	in stream BA,	etc								
Geometry :	Input						Calc	ulatod						
Geometry .	<u>Input</u> W/	10	\∕_rB∆	50	M-BA	2.5	Calc		0 786					
	W_CR -	10		50	W-BC	2.5			0.786					
	W-OR_	0	V-rBC	50	W-DC	2.5			1.056					
			V-rCB	50	W-CD			'v-	0.655					
			V-ICD					'-	0.000					
Analysis :	Traffic Flow	AM	PM		Capacity	AM	РМ							
-	pcu/hr				pcu/hr									
	q-CA	610	570		Q-BA	178	280							
	q-CB	290	40		Q-BC	420	451							
	q-AB	230	70		Q-CB	530	595							
	q-AC	790	690		Q-BAC	304	387							
	q-BA	110	60											
	q-BC	120	100											
	f	0.522	0.625											
Booulto :	Potio of Flow	to Canadity				DM								
Results :	Ratio OF FIOW-1			AIVI 0.618		0.214								
		B-A		0.010		0.214								
		D-0 C-R		0.200		0.222								
		B-AC		0.347		0.007								
		D-AC		0.750		0.414								

Scenario :		2032 Design 1	Traffic Flows											
	Lai King Hill Road													
<u>Arm C</u>	620 290	(590) (40)					040	(700)	A A					
			400	440			230	(700)						
			(100) Arm B	(70)	ƙwai Chung Int	erchange	(1	PM)	<u>(PM)</u>					
The predictive	e equations of ca Q-BA = Q-BC = Q-CB =	apacity of move D(627 + 14W- E(745 - Y(0.36 F(745 - 0.364)	ement are: ·CR - Y(0.36 64q-AC + 0.1 Y(q-AC + q-A	4q-AC + 0.´ I44q-AB)) \B))	144q-AB + 0.22	29q-CA + 0.52	/q-CB))							
The geometri	ic parameters re D = E = F =	presented by E (1 + 0.094(w-E (1 + 0.094(w-C (1 + 0.094(w-C), E, F are: 3A - 3.65))(1 3C - 3.65))(1 CB - 3.65))(1	+ 0.0009(\ + 0.0009(\ + 0.0009(\	′-rBA - 120))(1 ∕-rBC - 120))(1 ∕-rCB - 120))	+ 0.0006(V-IE + 0.0006(V-II	BA - 150)) BA - 150))							
where	Y = q-AB, etc = W = W-CR = w-BA, etc = v-rBA, etc = v-IBA =	1 - 0.0345W the design flow major road win central reserve lane width to w visibility to the visibility to the	w of moveme dth e width vehicle right for waiti	ent AB, etc ting vehicle ng vehicles	s in stream BA in stream BA,	, etc etc								
Geometry :	<u>Input</u> W W-CR	<u>10</u> 0	V-rBA V-IBA V-rBC V-rCB	50 50 50 50	w-BA w-BC w-CB	2.5 2.5 5	<u>Calc</u>	ulated D E F Y	0.786 0.786 1.056 0.655					
Analysis :	Traffic Flow pcu/hr q-CA q-CB q-AB q-AC q-BA q-BC q-BC	AM 620 290 230 810 110 120 0.522	PM 590 40 70 700 700 100 0.588		Capacity pcu/hr Q-BA Q-BC Q-CB Q-BAC	AM 173 416 525 300	PM 276 449 593 378							
Results :	Ratio of Flow	-to-Capacity B-A B-C C-B B-AC		AM 0.635 0.288 0.553 0.766		PM 0.254 0.223 0.067 0.450								

TRAFFIC SIGNALS CALCULATION Job No: 21149HK CTA Consultants Ltd.																						
Junction: Lai King Hill Road / Kwai Chung Interchange (E) Description: 2023 Observed Traffic Flows																						
Approach	Direction	notation	9	Stage	(m)	Radius (m		s 0/1	Pro. Turning (%)		w (pcu/hr)	ion Flow ư)	Revised Saturation Flow (pcu/hr)		Total Revised Saturation Flow (pcu/hr)		A.M. Peak			P.M. Peak		
		Movement r	Phas		Width	Left	Right	Nearsid	A.M.	P.M.	Saturation Flc	Total Satura (pcu/	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	<	2	в	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	150	0.090		120	0.072	
Kwai Chung Interchange	Е	\leq	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	100	0.054	0.054	110	0.060	0.060
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	325	0.152		268	0.126	
Lai King Hill Road	Ν	€↓-	3	С	3.8	15	0	1	27%	37%	1990	4120	1940	1920	4070	4050	295	0.152	0.152	242	0.126	0.126
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	620	0.319	0.319	580	0.298	0.298
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	120	0.062		120	0.062	
Notes:											Traffic Flow	(pcu / hr)		АМ	(PM)		LA	M. Check P	hase	Ρ.Ν	1. Check P	nase
											620(580) 120(120)	→ → 150(120)) 100(110)		← √	540(420) 80(90)	Ey L (sec) C (sec) y pract. R.C. (%)	0.525 12 120 0.810 54%		Ey L (sec) C (sec) y pract. R.C. (%)	0.484 12 120 0.810 67%	
Stage / Phase Diagrams	I/G =	-5s	T	2		1/G =	55	1		3												
TRAFFIC SIGNALS CALCULA	IC SIGNALS CALCULATION Job No: 21149HK Junction: Lai King Hill Road / Kwai Chung Interchange (E) Description: 2032 Reference Traffic Flows															C	ГА С	onsul	tants	Ltd.		
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Junction Description	: Lai k : 2032	King Hill Referen	Road / ce Traf	Kwai fic Flov	Chung vs	Interc	hange	e (E)						-								
	on	otation			m)	Radiu	us (m)	0/1	Pro. T (?	`urning %)	w (pcu/hr)	on Flow r)	Revised S Flow (Saturation pcu/hr)	Total Satura (po	Revised tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directi	Movement r	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	В	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	210	0.125		160	0.096	
Kwai Chung Interchange	E	$\left \right\rangle$	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	300	0.163	0.163	280	0.152	0.152
Lai King Hill Road	Ν	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	575	0.270		369	0.173	
Lai King Hill Road	Ν	<√-	3	С	3.8	15	0	1	85%	72%	1990	4120	1835	1855	3965	3985	495	0.270	0.270	321	0.173	0.173
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	750	0.386	0.386	630	0.324	0.324
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
Notes											Traffic Flow	(pcu / br)		АМ	(PM)			M. Check P	hase	PA	1 Check P	1856
notes.											750(630)	\rightarrow			\leftarrow	650(460)	Ey L (sec)	0.818 12	nuse	Ey L (sec)	0.649 12	mbe
											190(160)	$\overline{}$			V	420(230)	C (sec) y pract. R.C. (%)	120 0.810 -1%		C (sec) y pract. R.C. (%)	120 0.810 25%	
												210(160)	300(280)									
Stage / Phase Diagrams	I/G =	58	T	2		1/G =	5s	L T		3												

TRAFFIC SIGNALS CALCUL	AFFIC SIGNALS CALCULATION Job No: 21149HK CTA Consultants Ltd. Junction: Lai King Hill Road / Kwai Chung Interchange (E) Description: 2032 Reference Traffic Flows (Without A/KC/489)																					
Junction Description	n: Lai k n: 2032	King Hill Referen	Road / ce Traf	Kwai fic Flov	Chung vs (Wi	g Interc thout A	change 4/KC/4	e (E) 489)						-								
	on	otation	0		(II)	Radiu	us (m)	: 0/1	Pro. T	urning %)	w (pcu/hr)	ion Flow r)	Revised S Flow (Saturation pcu/hr)	Total Satura (po	Revised tion Flow cu/hr)		A.M. Peak	_		P.M. Peak	
Approach	Directi	Movement 1	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flor	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	<	2	В	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	210	0.125		160	0.096	
Kwai Chung Interchange	Е	\leq	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	120	0.065	0.065	120	0.065	0.065
Lai King Hill Road	Ν	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	367	0.172		284	0.133	
Lai King Hill Road	Ν	€↓-	3	С	3.8	15	0	1	27%	39%	1990	4120	1940	1915	4070	4045	333	0.172	0.172	256	0.134	0.134
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	700	0.360	0.360	610	0.314	0.314
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		160	0.082	
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)	610(440)	A.l Ey L (sec)	M. Check P 0.597 12	hase	P.M Ey L. (sec)	4. Check P 0.512 12	hase
											190(160)	\neg			√	90(100)	C (sec) y pract. R.C. (%)	120 0.810 36%		C (sec) y pract. R.C. (%)	120 0.810 58%	
												210(160)	120(120)									
Stage / Phase Diagrams						1																
			T	2				L L		3												

TRAFFIC SIGNALS CALCULA	ATION								Job No:	21149H	IK							C	ГА С	onsul	ltants	Ltd.
Junction Description	: Lai K : 2032	King Hill Design 7	Road / Fraffic	' Kwai Flows	Chung	Interc	change	e (E)														
		_							Pro T	urning	hr)	*	Revised 9	Saturation	Total	Revised						
	u	otation			n)	Radiu	ıs (m)	0/1	(%	6)	w (pcu/	on Flo	Flow (pcu/hr)	Saturat (po	tion Flow cu/hr)		A.M. Peak			P.M. Peak	
Approach	Directi	Movement r	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	в	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	210	0.125		160	0.096	
Kwai Chung Interchange	E	$\left \right\rangle$	2	в	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	300	0.163	0.163	280	0.152	0.152
Lai King Hill Road	N	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	580	0.272		374	0.175	
Lai King Hill Road	Ν	←↓	3	С	3.8	15	0	1	84%	71%	1990	4120	1835	1860	3965	3990	500	0.272	0.272	326	0.175	0.175
Lai King Hill Road	S	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	750	0.386	0.386	630	0.324	0.324
Lai King Hill Road	S	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		170	0.088	
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)		A.I	M. Check Pl	hase	P.N	1. Check Pl	hase
											750(630) 190(170)	\rightarrow			← √	660(470) 420(230)	εy L (sec) C (sec) y pract. R.C. (%)	0.821 12 120 0.810 -1%		εy L (sec) C (sec) y pract. R.C. (%)	0.651 12 120 0.810 24%	
												210(160)	300(280)									
Stage / Phase Diagrams	1/G =	-5s	IT IT	2		1/G =	5s	L T		3												

TRAFFIC SIGNALS CALCULA	TION								Job No:	21149H	IK							C	ГА С	onsul	ltants	Ltd.
Junction Description	: Lai k : 2032	King Hill Design 7	Road / Fraffic	Kwai Flows (Chung Witho	intero ut A/K	change C/489	e (E))														
	u	otation			m)	Radiu	us (m)	0/1	Pro. T (?	'urning %)	v (pcu/hr)	on Flow r)	Revised S Flow (j	Saturation pcu/hr)	Total Satura (po	Revised tion Flow cu/hr)		A.M. Peak	:		P.M. Peak	
Approach	Directi	Movement n	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	2	В	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	210	0.125		160	0.096	
Kwai Chung Interchange	Е	$\left \right\rangle$	2	В	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	120	0.065	0.065	120	0.065	0.065
Lai King Hill Road	Ν	\leftarrow	3	С	3.8	0	0	0	0%	0%	2130	0	2130	2130	0	0	372	0.175		290	0.136	
Lai King Hill Road	Ν	\leftarrow	3	С	3.8	15	0	1	27%	38%	1990	4120	1940	1915	4070	4045	338	0.174	0.174	260	0.136	0.136
Lai King Hill Road	s	\rightarrow	1	А	3.3	0	0	1	0%	0%	1945	1945	1945	1945	1945	1945	710	0.365	0.365	610	0.314	0.314
Lai King Hill Road	s	\neg	1	А	3.3	0	20	0	100%	100%	2085	2085	1940	1940	1940	1940	190	0.098		170	0.088	
																	1					
Notes:											Traffic Flow 710(610)	(pcu / hr)		AM	(PM)	620(450)	A.l Ey L (sec)	M. Check P 0.604 12	hase	P.N Ey L (sec)	 Check P 0.515 12 	nase
											190(170)	\neg				90(100)	C (sec) y pract. R.C. (%)	120 0.810 34%		C (sec) y pract. R.C. (%)	120 0.810 57%	
												210(160)	> 120(120)									
Stage / Phase Diagrams	1					1											I					
	1).	T	2		1. 1		T		3												
I/G = 5s	I/G =	= 5s				I/G =	5s															

TRAFFIC SIGNALS CALCULA	ATION								Job No:	21149F	łK							C	ГА С	onsul	tants	Ltd.
Junction Description	: Lai k : 2032	(ing Hill Referen	Road /	Kwai fic Flov	Chung vs (Wit	Interc th imp	hange roveme	(E) ent)														
	u	otation			(III)	Radiu	ıs (m)	0/1	Pro. Ti	urning 6)	v (pcu/hr)	on Flow :)	Revised S Flow (J	aturation pcu/hr)	Total Saturat (po	Revised ion Flow u/hr)		A.M. Peak			P.M. Peak	
Approach	Directi	Movement n	Phase	Stage	Width (Left	Right	Nearside	A.M.	P.M.	Saturation Flov	Total Saturati (pcu/h	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange Kwai Chung Interchange	E E	$\mathbb{A}_{\mathbb{A}}$	1 1	A A	3.5 3.5	18 0	0 25	1 0	100% 100%	100% 100%	1813.8 1953.8	1813.8 1953.8	1675 1845	1675 1845	1675 1845	1675 1845	210 300	0.125 0.163		160 280	0.096 0.152	0.152
Lai King Hill Road Lai King Hill Road	N N	← √	2 1	B A	3.5 4.3	0 15	0 0	0 1	0% 100%	0% 100%	2105 2045	2105 2045	2105 1860	2105 1860	2105 1860	2105 1860	650 420	0.309 0.226	0.226	460 230	0.219 0.124	
Lai King Hill Road Lai King Hill Road	S S	\rightarrow	2 3	B C	3.5 3.5	0 0	0 20	1 0	0% 100%	0% 100%	1965 2105	1965 2105	1965 1960	1965 1960	1965 1960	1965 1960	750 190	0.382 0.097	0.382 0.097	630 160	0.321 0.082	0.321 0.082
Pedestrian crossing		↑ ▼	D E F G	A C B		Min. g Min. g Min. g Min. g	green tij green tij green tij	me = me = me =	5Gm + 5 5Gm + 9 5Gm + 1 5Gm + 1	FGm = FGm = 1 FGm 0 FGm	= 10s = 14s = 16s = 15s											
Notes:											Traffic Flow 750(630) 190(160)	(pcu / hr)		AM	(PM) <	650(460) 420(230)	A.! Ey L (sec) C (sec) y pract. R.C. (%)	 A. Check P 0.704 12 120 0.810 15% 	nase	P.M Ey L (sec) C (sec) y pract. R.C. (%)	I. Check Pf 0.554 12 120 0.810 46%	lase
Storge / Diversione												210(160)	300(280)									
		D P P	B1	00						(F) D D	-											

TRAFFIC SIGNALS CALCULATION Job No: 21149HK Junction: Lai King Hill Road / Kwai Chung Interchange (E) Description: 2032 Design Traffic Flows (With improvement)																		C	ГА С	onsul	ltants	Ltd.
Junction: Description:	Lai K 2032	Ling Hill Design T	Road / Traffic	/ Kwai (Flows (Chung With i	Interc mprov	hange ement)	(E))														
											r)	`			Total	Revised						
	tion	notation	se	še	(m)	Radiu	ıs (m)	le 0/1	Pro. T (%	urning 6)	ow (pcu/h	tion Flow hr)	Revised S Flow (j	Saturation pcu/hr)	Saturat (pc	tion Flow cu/hr)		A.M. Peak	, ,		P.M. Peak	
Approach	Direct	Movement	Pha	Stag	Width	Left	Right	Nearsid	A.M.	P.M.	Saturation Flo	Total Satura (pcu/	A.M.	P.M.	A.M.	P.M.	Flow (pcu/hr)	y Value	Critical y	Flow (pcu/hr)	y Value	Critical y
Kwai Chung Interchange	Е	\leq	1	А	3.5	18	0	1	100%	100%	1813.8	1813.8	1675	1675	1675	1675	210	0.125		160	0.096	
Kwai Chung Interchange	E	$\left \right\rangle$	1	А	3.5	0	25	0	100%	100%	1953.8	1953.8	1845	1845	1845	1845	300	0.163		280	0.152	0.152
Lai King Hill Road	N	\leftarrow	2	В	3.5	0	0	0	0%	0%	2105	2105	2105	2105	2105	2105	660	0.314		470	0.223	
Lai King Hill Road	Ν	V	1	А	4.3	15	0	1	100%	100%	2045	2045	1860	1860	1860	1860	420	0.226	0.226	230	0.124	
Lai King Hill Road	s	\rightarrow	2	В	3.5	0	0	1	0%	0%	1965	1965	1965	1965	1965	1965	750	0.382	0.382	630	0.321	0.321
Lai King Hill Road	S	$\overline{}$	3	С	3.5	0	20	0	100%	100%	2105	2105	1960	1960	1960	1960	190	0.097	0.097	170	0.087	0.087
Pedestrian crossing	Pedestrian crossing \downarrow D A Min. green time = 5Gm + 5 FGm + \downarrow E C Min. green time = 5Gm + 9 FGm + F C Min. green time = 5Gm + 11 FGm + G B Min. green time = 5Gm + 10 FGm + \downarrow																					
Notes:											Traffic Flow	(pcu / hr)		AM	(PM)		A.M	M. Check P	hase	P.N	4. Check Pl	nase
											750(630) 190(170)	\rightarrow			\overleftarrow{V}	660(470) 420(230)	Ey L (sec) C (sec) y pract. R.C. (%)	0.704 12 120 0.810 15%		Ey L (sec) C (sec) y pract. R.C. (%)	0.559 12 120 0.810 45%	
6 (D) D												210(160)	300(280)									
Stage / Phase Diagrams		P P		00	D			00		D D	-											

Roundabout Junction : Kwai Fuk Road / Kwai Yi Road / Container Port Road

Project No. 22149HK

Design Year : 2023 Observed Traffic Flows



СТА

Roundabout Junction : Kwai Fuk Road / Kwai Yi Road / Container Port Road

Project No. 22149HK

Design Year : 2032 Reference Traffic Flows

