

Annex I - Traffic Impact Assessment

JOINT-USER COMPLEX AND JOINT-USER GENERAL OFFICE BUILDING
AT AREA 29, KWU TUNG NORTH

Quotation Contract No. ASD 101/8705/CX/06/QC6

**Contract Consultancy Services for Joint-user
Complex and Joint-user General Office Building at
Area 29, Kwu Tung North**

**Traffic Impact Assessment Report
April 2025**

Quotation Contract No. ASD 101/8705/CX/06/QC6

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**Traffic Impact Assessment Report
April 2025**

Contents Amendment Record

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1 INTRODUCTION

1.1 Background

- 1.1.1 The Government proposes to develop the lands zoned as Government, Institution of Community (“GIC”) in Area 29, Kwu Tung North, into a Joint-user Complex (“JUC”) and Joint-user General Office Building (“JUB”). To achieving better air ventilation, enhancing the environmental sustainability and quality of the spaces particularly at pedestrian level, mitigating the heat island effect, allowing provision of more greenery on ground levels for public enjoyment and retaining of a Tree of Particular Interest (TPI) of large size near the northwest corner with the Project Site, a planning application for relaxation of the building height restriction of the Project site to 170mPD under Section 16 (S16) of the Town Planning Ordinance, Cap 131 is proposed.
- 1.1.2 Ozzo Technology (HK) Limited has been commissioned to undertake a Traffic Impact Assessment (TIA) Study in support of the above S16 planning application of the Joint-user Complex and Joint-user General Office Building at Area 29, Kwu Tung North (“the Project Site”).

1.2 Study Objectives

- 1.2.1 The main objectives of this Traffic Impact Assessment (“TIA”) Study are as follows:
- To review the existing traffic condition of the surrounding road network;
 - To estimate the potential traffic generations/attractions to be induced by the Proposed Scheme;
 - To assess the future traffic situation of the surrounding road network; and
 - To appraise the potential traffic impact of the Proposed Scheme on the surrounding road network and to recommend improvement proposals, if required.

1.3 Report Structure

- 1.3.1 Following this introductory chapter, this report is arranged as follows:
- Chapter 2 describes the location and the development parameters of the Proposed Scheme;
 - Chapter 3 summarizes the existing traffic conditions in the vicinity of the Project Site;
 - Chapter 4 describes the methodology for estimating the amount of vehicular traffic to be induced by the Proposed Scheme;
 - Chapter 5 details the traffic forecast and the results of traffic impact assessment; and
 - A summary of the findings and conclusion of this TIA study are given in Chapter 6.

2 DESCRIPTIONS OF THE PROPOSED SCHEME

2.1 Site Location and Study Area

- 2.1.1 The Project Site, with a total site area of approx. 20,980m², is situated at Kwu Tong North New Development Area and is bounded by Fanling Highway and Castle Peak Road (Chau Tau) to the south, Heung Tsz Road to the north.
- 2.1.2 **Figure 2-1** shows the location of the Project Site and the proposed Study Area for this TIA Study.

2.2 Development Parameters for the Proposed Scheme

- 2.2.1 The Proposed Scheme consists of the Joint-user General Office Building (JUB) and Joint-user Complex (JUC) with a total GFA of approx. 163,400 m². **Table 2-1** summarizes the development parameters of the Proposed Scheme.

Table 2-1 Summary of Development Parameters

Parameters	JUB & JUC
Site Area	~20,980 m ²
Plot Ratio (PR)	~7.8
Office	~120,900 m ²
Office (supporting community facilities)	~4,440 m ²
Kindergarten (13 classrooms)	~930 m ²
Clinic	~8,070 m ²
Library	~6,060 m ²
Swimming Pool and Sports Centre	~21,020 m ²
Children Care Centre	~1,980 m ²
Total GFA	~163,400 m²

2.3 Parking and Loading/Unloading Facilities

- 2.3.1 The parking and loading / unloading (L/UL) facilities comply with the agreed provisions under the Project Definition Statement (PDS) in Traffic Feasibility Study (TFS) stage in which a total of about 433 parking and L/UL spaces will be provided. **Table 2-2** summarizes the internal transport facilities to be provided by the Proposed Scheme.

Table 2-2 Provision of Parking and L/UL Facilities

Use	Vehicle Type	Proposed Provision	Size (L x W x H)	Total Nos. of Spaces
Parking Spaces	Private Car	~337	5m x 2.5m x 2.4m	Total Nos. of Parking Spaces = ~400
	Coach	4	12m x 3.5m x 3.8m	
	Van	13	7m x 3.5m x 3.6m	
	Ambulance	2	8m x 3.5m x 3.6m	
	Estate cars	7	5m x 3m x 2.4m	
	Medium / Heavy Goods Vehicles	3	11m x 3.5m x 4.7m	
	Bicycle	20	2m x 0.7m x 2.4m	
	Motorcycle	7	2.5m x 1m x 2.4m	
	16-seater Van	3	7m x 3m x 3.5m	
	Large Van	1	5.3m x 2.2m x 2.4m	
	10-seater Van	1	5.7m x 3.13m x 2m	
	GOPC Accessible Parking	2	5m x 3.5m x 2.4m	
Loading / Unloading (L/UL) Spaces	Private Car	4	5m x 2.5m x 2.4m	Total Nos. of L/UL Spaces = 33
	Coach	6	12m x 3.5m x 3.8m	
	Van	6	7m x 3.5m x 3.6m	
	Ambulance	4	8m x 3.5m x 3.6m	
	Ambulance	2	9m x 3m x 3.6m	
	Medium/Heavy Goods Vehicles	6	11m x 3.5m x 4.7m	
	16-seater Van	1	7m x 3m x 3.5m	
	Light Goods Van	1	7m x 3.5m x 3m	
	GOPC Loading Platform	2	11m x 3.5m x 4.7m	
	GOPC Garbage truck	1	9m x 3m x 4.7m	
Total Nos. of Parking and L/UL spaces		~433		

2.4 Vehicular and Pedestrian Access Arrangements

- 2.4.1 The proposed vehicular accesses will be situated at Road L1 at the northern boundary of the Project Site, and at Castle Peak Road at the southern boundary of the Project Site. The pedestrian accesses will be situated at the northern and eastern boundary of the Project Site. A drop-off area will be provided on the G/F of the Proposed Scheme and other parking and loading/unloading facilities will be provided at the basement. The locations of the proposed vehicular and pedestrian accesses are shown in **Figure 2-2**.

3 EXISTING TRAFFIC CONDITIONS

3.1 Existing Road Network

- 3.1.1 The existing road network in the vicinity of the Project Site is shown in **Figure 2-1**.
- 3.1.2 Castle Peak Road - Chau Tau section, a Local Distributor road, is a single-2 lane carriageway connecting Castle Peak Road - San Tin in the west and Castle Peak Road - Kwu Tung in the east.
- 3.1.3 Heung Tsz Road is a local access road with single-2 lane configuration which connects Pak Sau Road to the west, providing access to developments along the road.

3.2 Traffic Surveys at Key Links and Junctions

- 3.2.1 To understand the existing traffic conditions on the road network within the Study Area, vehicular traffic count surveys were conducted at key road junctions and road links on 18 Dec 2024 (Wednesday) between 06:30-09:30 and 16:00-19:00. **Table 3-1** presents a summary of the vehicular traffic count surveys being undertaken and the survey locations are shown in **Figure 3-1**.

Table 3-1 Summary of Vehicular Traffic Count Surveys

Survey Type	Location	Figure	Survey Date	Data Collected
Vehicular Count Surveys	J1, J2, J3, L1, L2	Figure 3-1	2024-12-18 (Wednesday)	Manual Classified count in 15 min intervals

3.3 Existing Vehicle Traffic Conditions

- 3.3.1 All vehicle flows recorded during the traffic surveys are converted to passenger car unit (PCU) based on the PCU factors as indicated in Table 2.3.1.1 of Volume 2 of Transport Planning and Design Manual (TPDM) as summarized in **Table 3-2**.

Table 3-2 Passenger Car Unit Conversion Factors

Vehicle Type	PCU Conversion Factor ⁽¹⁾	
	Traffic Signal	Priority junction/ Roundabout
Car / Taxi	1.00	1.00
Public Light Bus / Minibus	1.50	1.50
Light Goods Vehicle	1.50	1.50
Medium/ Heavy Goods Vehicle	1.75	2.80
Bus / Coach	2.00	2.80

Notes: (1) Table 2.3.1.1, Chapter 2.3, Volume 2, TPDM-2024

3.3.2 By applying the above PCU factors, vehicular traffic flows in PCUs are calculated and the AM and PM peak hour is identified to occur at 07:45-08:45 and 17:15-18:15 respectively. **Figure 3-2** presents the 2024 observed AM and PM peak hour traffic flows.

3.3.3 Based on the existing traffic flows, the peak hour performance of the key junctions in the vicinity of the Project Site is assessed. The assessment results are indicated in **Table 3-3** and detailed junction calculation sheets are given in **Appendix A**.

Table 3-3 2024 Peak Hour Junction Capacity Assessment

Jn. ID.	Location ⁽¹⁾	Type	Capacity Index ⁽²⁾	2024 Weekday	
				AM Peak	PM Peak
J1	Castle Peak Road (Chau Tau) / Pak Sau Road	Priority	DFC	0.33	0.47
J2	Castle Peak Road (Chau Tau) / Unnamed Road	Priority	DFC	0.44	0.37
J3	Castle Peak Road (Kwu Tung) / Ho Sheung Heung Road	Priority	DFC	0.69	0.67

Notes: (1) Refer to Figure 3-1 for junction locations
(2) DFC = Design Flow to Capacity for priority junction

3.3.4 The results reveal that all the key junctions within the Study Area operate satisfactorily during the peak hours currently.

3.3.5 Based on the existing traffic flows, the peak hour performances of the key road links in the vicinity of the Project Site are also assessed and the results are indicated in **Table 3-4**.

Table 3-4 2024 Peak Hour Road Link Capacity Assessment

No.	Location ⁽¹⁾	Direction	Design Capacity (pcu/hr)	Flows (pcu/hr)	2024 Weekday	
					AM Peak	PM Peak
L1	Fanling Highway	EB	6,100	Flows	4,409	4,381
				P/Df ⁽²⁾	0.72	0.72
		WB	6,100	Flows	4,194	4,129
				P/Df ⁽²⁾	0.69	0.68
L2	Castle Peak Road (Chau Tau)	EB	1,250	Flows	575	355
				P/Df ⁽²⁾	0.46	0.28
		WB	1,250	Flows	578	677
				P/Df ⁽²⁾	0.46	0.54

Notes: (1) Refer to Figure 3-1 for road link locations
(2) P/Df = Peak Hourly Flows/Design Flow Ratios (P/Df) for road links

3.3.6 The results reveal that all the key road links in the vicinity of the Project Site operate satisfactorily during the peak hours.

4 TRAFFIC FORECASTS

4.1 Modelling Methodology

- 4.1.1 A Local Area Traffic Model (LATM) is developed for this study based on the 2019-based BDTM (NTE1) obtained from the Transport Planning Division of Transport Department, where the future year assumptions under the 2019-based BDTM (including future development, road network, toll assumptions and public transport services) are adopted.
- 4.1.2 To ensure the robustness of the future year model, validation has been conducted by comparing the 2024 surveyed traffic flows and the LATM 2024 base year modelled flows, at the proposed screenline locations as shown in **Table 4-1**.

Table 4-1 Locations of Screenline for Validation

No.	Location	Direction
L1	Fanling Highway	EB
		WB
L2	Castle Peak Road (Chau Tau)	EB
		WB

- 4.1.3 For the purpose of base year model validation, the validation process shall strictly follow the criteria as stated in **Table 4-2**.

Table 4-2 Validation Criteria

Validation Criteria	Validation Target
Total Link Flows at Screenline	100% within 10% of actual counts
Individual Link Flow at Screenline	100% within 20% of actual counts

4.2 Validation Results

- 4.2.1 The model validation results are shown in **Table 4-3**. The results indicated that all individual link flows are within 20% of actual counts, and the total link flows are within 10% of actual counts. The validation criteria for the base year model are well satisfied.

Table 4-3 Validation Results

No.	Location ⁽¹⁾	Directi on	2024 Observed Flows (pcu/hr)		2024 Modelled Flows (pcu/hr)		Difference (%)	
			AM	PM	AM	PM	AM	PM
L1	Fanling Highway	EB	4,409	4,381	4,422	4,205	-0.3%	4.2%
		WB	4,194	4,129	4,334	3,671	-3.2%	12.5%
L2	Castle Peak Road (Chau Tau)	EB	575	355	525	327	9.5%	8.7%
		WB	578	677	532	616	8.7%	9.9%
Total			9,756	9,542	9,812	8,818	-0.6%	8.2%

4.3 Design Year

- 4.3.1 Based on the latest information, the Proposed Scheme is anticipated to be completed by 2031. Thus, the “Design Year” for this TIA study is 2034, i.e. 3 years after the planned full completion of the Proposed Scheme.

4.4 2034 Peak Hour Reference Flows

- 4.4.1 Model assumptions in 2031 BDTM (NTE1) such as planned road networks, railway and highway networks, new development area and planned road-based public transport services are adopted in the 2034 LATM for forecasting future traffic flows.
- 4.4.2 Also, any adjustments in deriving the 2024 LATM from the 2019 BDTM are applied to the 2031 BDTM as appropriate. In addition, with reference to the 2021-based Territorial Population and Employment Data Matrices (TPEDM) published by Planning Department, a growth factor of 1% p.a. is applied to represent the growth from 2031 to 2034.
- 4.4.3 **Figure 4-1** shows the future road network in the Study Area that are included in the 2034 LATM. Heung Tsz Road will become Road L1 and connect with Road D1 at the western end. **Figure 4-2** also shows the links and junctions to be assessed in this Study.
- 4.4.4 The 2034 Peak Hour Reference Flows (i.e. without the Proposed Scheme) output from the LATM are presented in **Figure 4-3**.

4.5 Peak Hour Project Flows

- 4.5.1 The peak hour vehicular trips to be induced by the Proposed Scheme are estimated with reference to the existing trip rates observed at different uses similar to the corresponding uses by the Proposed Scheme. **Table 4-4** shows the adopted trip rates for different type of uses.

Table 4-4 Adopted Vehicular Trip Rates for the Project Site

Development Type	Trip Rate Unit	Adopted Peak Hour Vehicular Trips			
		Weekday AM		Weekday PM	
		In	Out	In	Out
Office / Office (supporting community facilities) ⁽¹⁾	per 100m ² GFA	0.2452	0.1703	0.1175	0.1573
Kindergarten ⁽²⁾	per classroom	1.9231	1.4615	0.6154	1.1538
Clinic ⁽³⁾	per 100m ² GFA	0.1257	0.1029	0.0800	0.0914
Swimming Pool and Sports Centre ⁽⁴⁾	per 100m ² GFA	0.2856	0.2317	0.3152	0.3125
Children Care Centre ⁽⁵⁾	per 100m ² GFA	0.0663	0.0400	0.0554	0.0518

Notes: (1) Adopt mean trip of Office from TPDM

(2) Peak Hour trip rates observed at Zenith Kindergarten

(3) Peak Hour trip rates observed at Yau Ma Tei Polyclinic

(4) Peak Hour trip rates observed at Fung Kam Street Sports Centre

(5) Peak Hour trip rates observed at Kwu Tung North Multi Welfare Service Complex

4.5.2 Based on the adopted trip rates, the estimated peak hour vehicular trips to be generated by the Proposed Scheme are shown in **Table 4-5**.

Table 4-5 Estimated Peak Hour Vehicle Flows for the Project Site

Development Type	Vehicular Trips for the Project Site			
	AM Peak Hour Trip (pcu/hr)		PM Peak Hour Trip (pcu/hr)	
	In	Out	In	Out
Office / Office (supporting community facilities) (125,340 m ²)	307	213	147	197
Kindergarten (13 classrooms)	18	14	6	11
Clinic (8,070 m ²)	10	8	6	7
Library ⁽¹⁾ (6,060 m ²)	5	5	5	5
Swimming Pool and Sports Centre (21,020 m ²)	38	31	42	42
Children Care Centre (1,980 m ²)	1	1	1	1
Sub-Total	379	272	207	263
2-way Total	651		470	

Notes: (1) No vehicular trips generation or attraction is expected from library. For conservative, peak hour trips of 5 pcu/hr per direction is assumed for library.

4.5.3 It is estimated that the Proposed Scheme would generate two-way flows of 651 pcu/hr (379 in and 272 out) and 470 pcu/hr (207 in and 263 out) in the AM and PM peak hour respectively.

4.6 Future Year Design Peak Hour Traffic Flows

- 4.6.1 The additional development traffic in **Table 4-5** is then assigned onto the nearby road network. The resulting Peak Hour Project flows are shown in **Figure 4-4**.
- 4.6.2 By adding the Project Flows in **Figure 4-3** to the 2034 Peak Hour Reference Flows (i.e. without the Proposed Scheme) in **Figure 4-2**, the 2034 Peak Hour Design Flows (i.e. with the Proposed Scheme) are derived and shown in **Figure 4-5**.

5 TRAFFIC IMPACT ASSESSMENT

5.1 Future Year Junction Capacity Assessments

- 5.1.1 Based on the 2034 Peak Hour Reference Flows shown in **Figure 4-3** and the Design Flows shown in **Figure 4-5**, junction capacity assessment are undertaken and the results are presented in **Table 5-1** with detailed calculation sheets provided in **Appendix B**.

Table 5-1 2034 Peak Hour Junction Capacity Assessment

Jn. ID.	Location ⁽¹⁾	Type	Capacity Index ⁽²⁾	2034 Reference Scenario		2034 Design Scenario	
				AM Peak	PM Peak	AM Peak	PM Peak
J1	Castle Peak Road (Chau Tau) / Pak Sau Road	Priority	DFC	0.32	0.09	0.42	0.11
J3	Castle Peak Road (Kwu Tung) / Ho Sheung Heung Road	Priority	DFC	0.31	0.28	0.31	0.29
J4	Road D1 / Road L1	Signal	RC	100%+	100%+	43.7%	93.1%
J5	Road P1 / Road D1 / Slip Road to Fanling Highway	Roundabout	DFC	0.30	0.39	0.37	0.41
J6	Castle Peak Road (Chau Tau) / Road P1	Roundabout	DFC	0.37	0.27	0.45	0.29

Notes: (1) Refer to Figure 3-2 for junction locations. J2 is removed in future road network.
(2) RC = Reserve Capacity for signal-controlled junction; DFC = Design Flow to Capacity for priority junction

- 5.1.2 It is indicated in **Table 5-1** that all the key junctions in the vicinity of the Project Site would be operating within capacity during the peak hours for both the Reference and Design scenarios.

5.2 Future Year Link Capacity Assessments

- 5.2.1 Based on the 2034 Peak Hour Reference Flows and Design Flows, link capacity assessments for Design Year 2034 are carried out and the results are presented in **Table 5-2**.

Table 5-2 2034 Peak Hour Road Link Capacity Assessment

No.	Location ⁽¹⁾	Direction	Design Capacity (pcu/hr)	Flows (pcu/hr)	2034 Reference Scenario		2034 Design Scenario	
					AM Peak	PM Peak	AM Peak	PM Peak
L1	Fanling Highway	EB	8,200 ⁽³⁾	Flows	5,112	4,770	5,261	4,925
				P/Df ⁽²⁾	0.62	0.58	0.64	0.60
		WB	8,200 ⁽³⁾	Flows	5,061	4,023	5,179	4,067
				P/Df ⁽²⁾	0.62	0.49	0.63	0.50
L2	Castle Peak Road (Chau Tau)	EB	1,250	Flows	649	489	719	524
				P/Df ⁽²⁾	0.52	0.39	0.58	0.42
		WB	1,250	Flows	634	707	714	742
				P/Df ⁽²⁾	0.51	0.57	0.57	0.59
L3	Road L1	EB	1,250	Flows	74	219	339	364
				P/Df ⁽²⁾	0.06	0.18	0.27	0.29
		WB	1,250	Flows	322	86	512	270
				P/Df ⁽²⁾	0.26	0.07	0.41	0.22

Notes: (1) Refer to Figure 3-2 for road link locations
(2) P/Df = Peak Hourly Flows/Design Flow Ratios (P/Df) for road links
(3) Fanling Highway to be widen to 4-lanes dual carriageway

- 5.2.2 Assessment results indicate that all the key road links in the vicinity of the Project Site would be operating within capacity during the peak hours for both Reference and Design scenario.

5.3 Pedestrian Connections

- 5.3.1 The Project Site is well connected to the future Kwu Tung Station and a Public Transport Interchange (PTI) located at the North of the site, and Castle Peak Road and footbridge at the South of the site. Pedestrian of the Proposed Scheme can travel to the public transport facilities via the Urban Plaza & Greenery at the East of the Project Site.
- 5.3.2 **Figure 5-1** and **Figure 5-2** shows the pedestrian connections to the Kwu Tung Station, PTI, and other local areas, from the G/F and LG/F of the project site respectively.

6 SUMMARY AND CONCLUSION

6.1 Summary

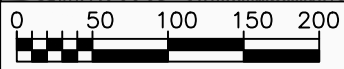
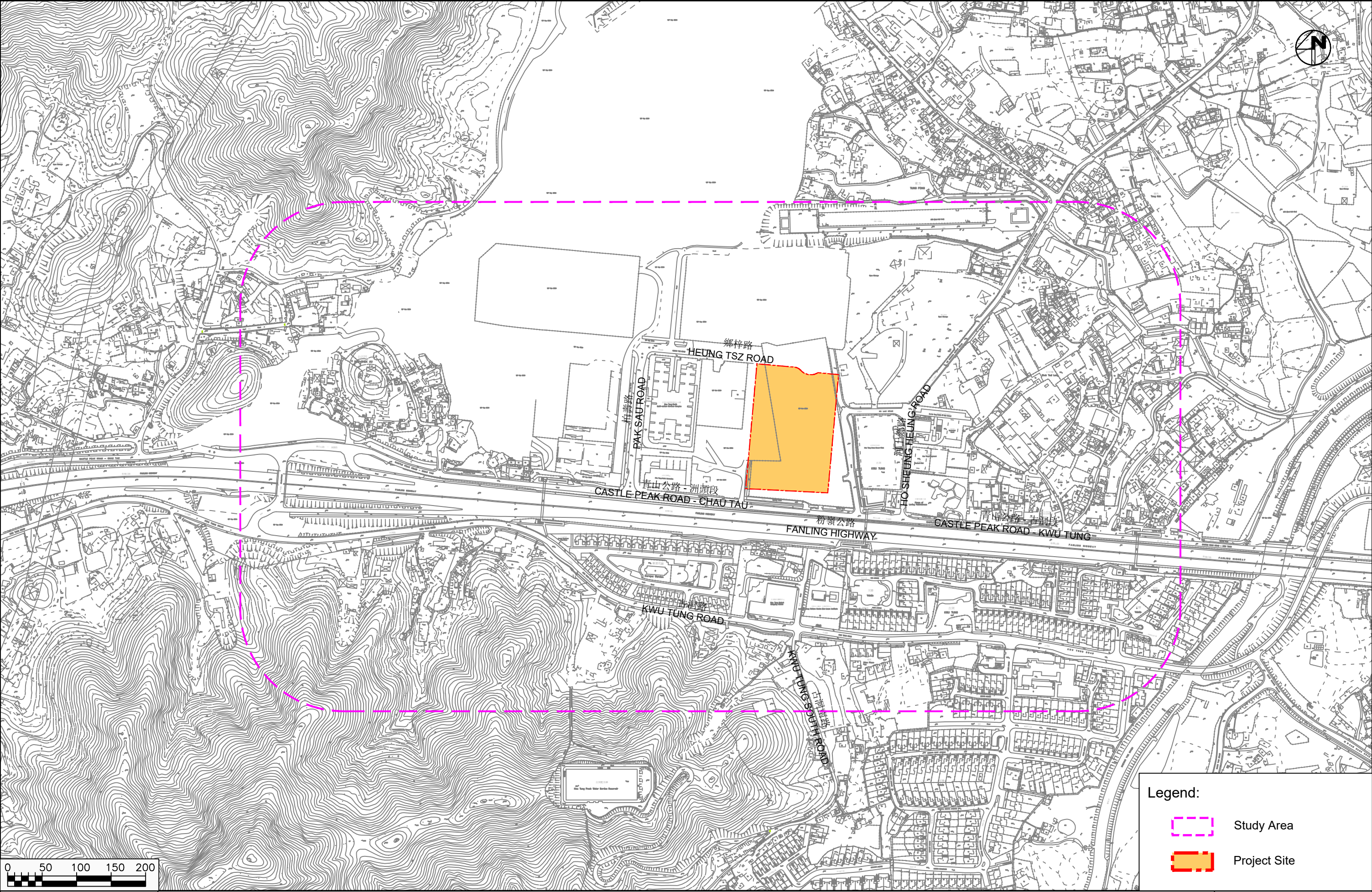
- 6.1.1 The Government proposes to develop the lands zoned as Government, Institution of Community ("GIC") in Area 29, Kwu Tung North, into a Joint-user Complex ("JUC") and Joint-user General Office Building ("JUB"). To achieving better air ventilation, enhancing the environmental sustainability and quality of the spaces particularly at pedestrian level, mitigating the heat island effect, allowing provision of more greenery on ground levels for public enjoyment and retaining of a Tree of Particular Interest (TPI) of large size near the northwest corner with the Project Site, the Applicant proposes a relaxation of the building height restriction of the Project site to 170mPD under Section 16 of the Town Planning Ordinance.
- 6.1.2 Ozzo Technology (HK) Limited is commissioned to undertake a Traffic Impact Assessment (TIA) Study to assess the traffic impact to be induced by the Proposed Scheme on the nearby road networks.
- 6.1.3 Capacity assessments are undertaken to reveal the 2024 AM and PM peak hour traffic conditions in the vicinity of the Project Site. The assessment results indicate that all the key junctions and road links perform satisfactorily during the AM and PM peak hours at present.
- 6.1.4 The anticipated completion year of the Proposed Scheme is 2031. The assessment year for the TIA study is therefore set as 2034, i.e. 3 years after the anticipated completion in 2031.
- 6.1.5 With reference to the 2019-based Base District Traffic Model (NTE-1), a Local Area Traffic Model is developed for forecasting the future traffic flows in the Study Area. Based on the forecast 2034 model flows, junction capacity assessments are undertaken to assess both the 2034 AM and PM peak hour conditions. The assessment results indicate that all the key junctions would perform within capacity during the peak hours for both Reference Scenario (i.e. without Proposed Scheme) and Design Scenario (i.e. with Proposed Scheme).
- 6.1.6 Link capacity assessments are also undertaken and the assessment results indicate that all the key road links in the Study Area would be operating within capacity during the 2034 AM and PM peak hour for both Reference and Design scenario.
- 6.1.7 A total of about 433 parking and loading / unloading (L/UL) spaces will be provided within the Project Site that comply with the agreed provisions under the Project Definition Statement (PDS) in Traffic Feasibility Study (TFS) stage.



6.2 Conclusion


- 6.2.1 The traffic impact assessment results indicate that the Proposed Scheme would not create adverse impact on the future road network in the vicinity of the Project Site.

Figures

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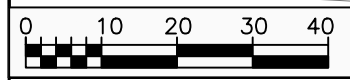
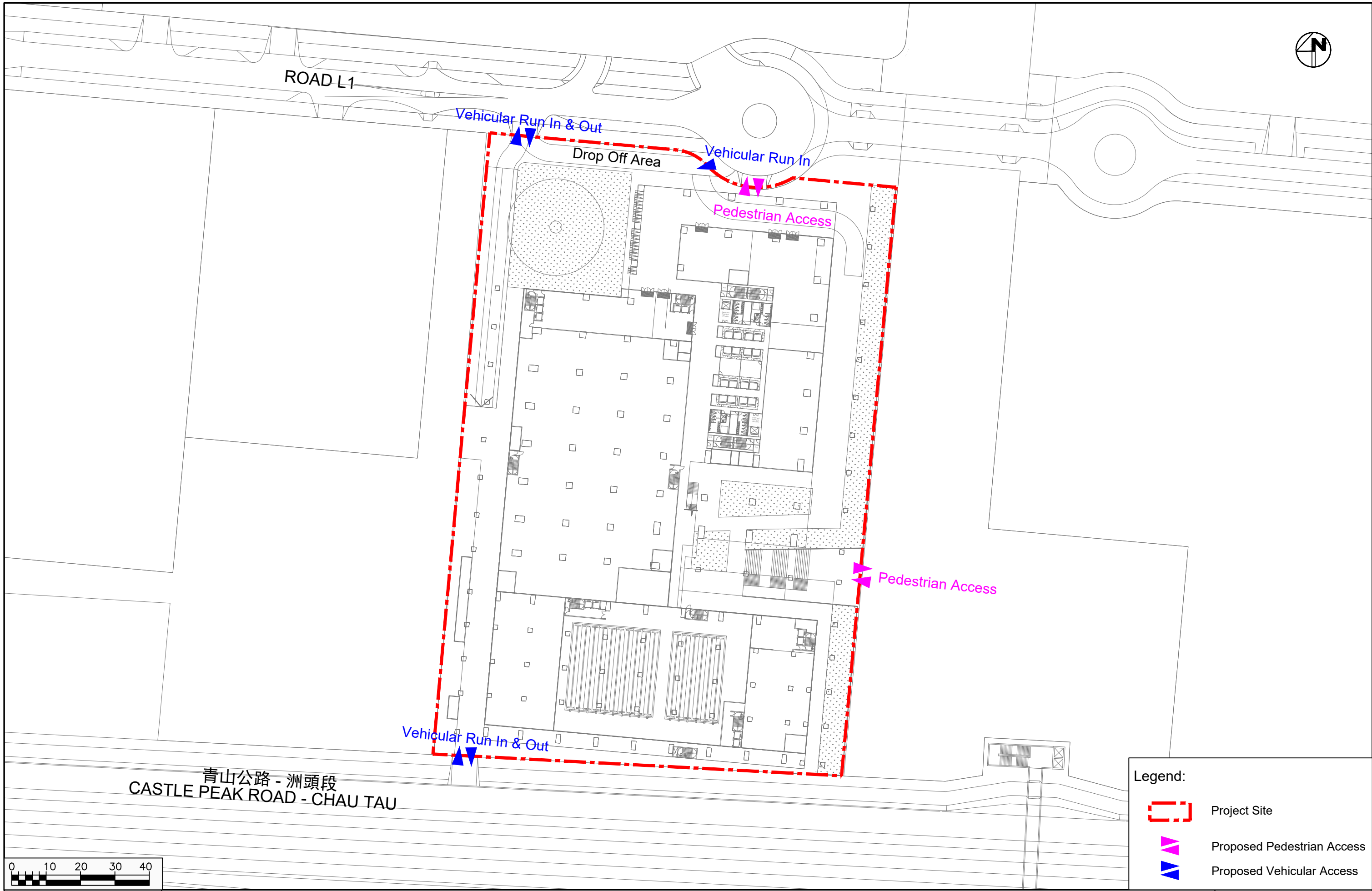


- Legend:
-  Study Area
 -  Project Site

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Site Location and Study Area

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Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

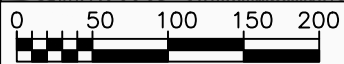
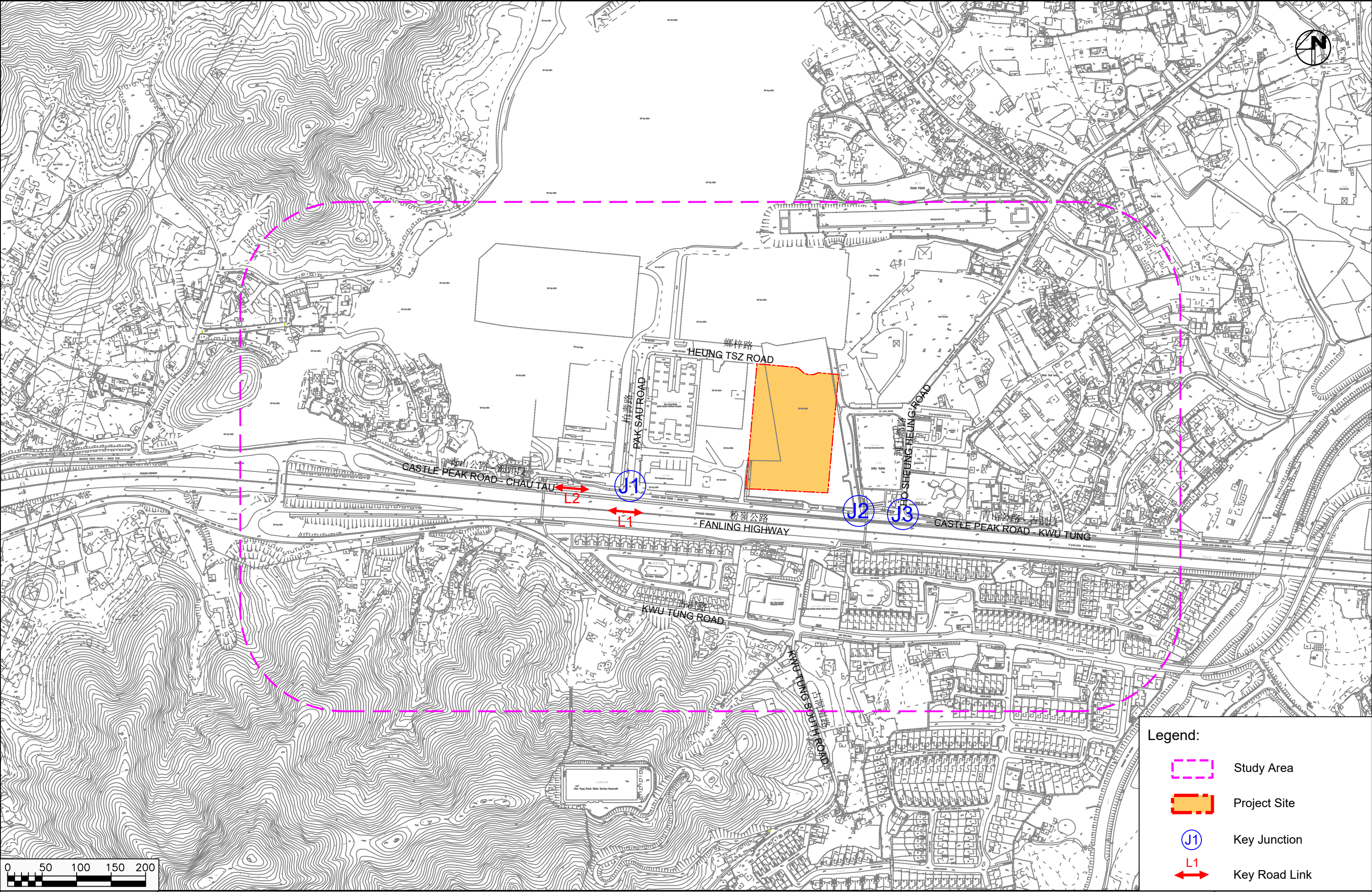
Proposed Vehicular and Pedestrian Accesses

Legend:

- Project Site
- Proposed Pedestrian Access
- Proposed Vehicular Access

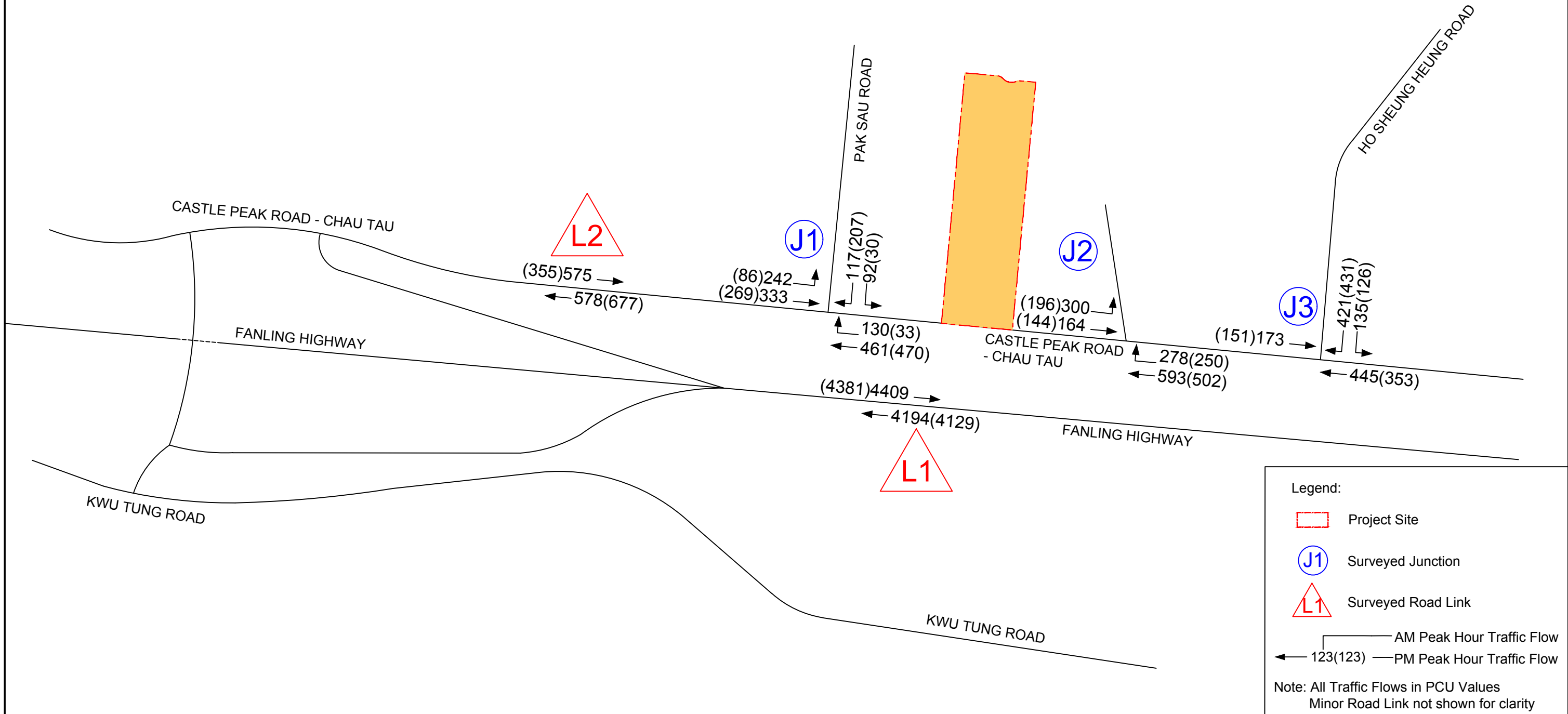
Project No. 83114	Rev.
Dwg No. Figure 2-2	-

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- Legend:
- Study Area
 - Project Site
 - Key Junction
 - Key Road Link

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Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

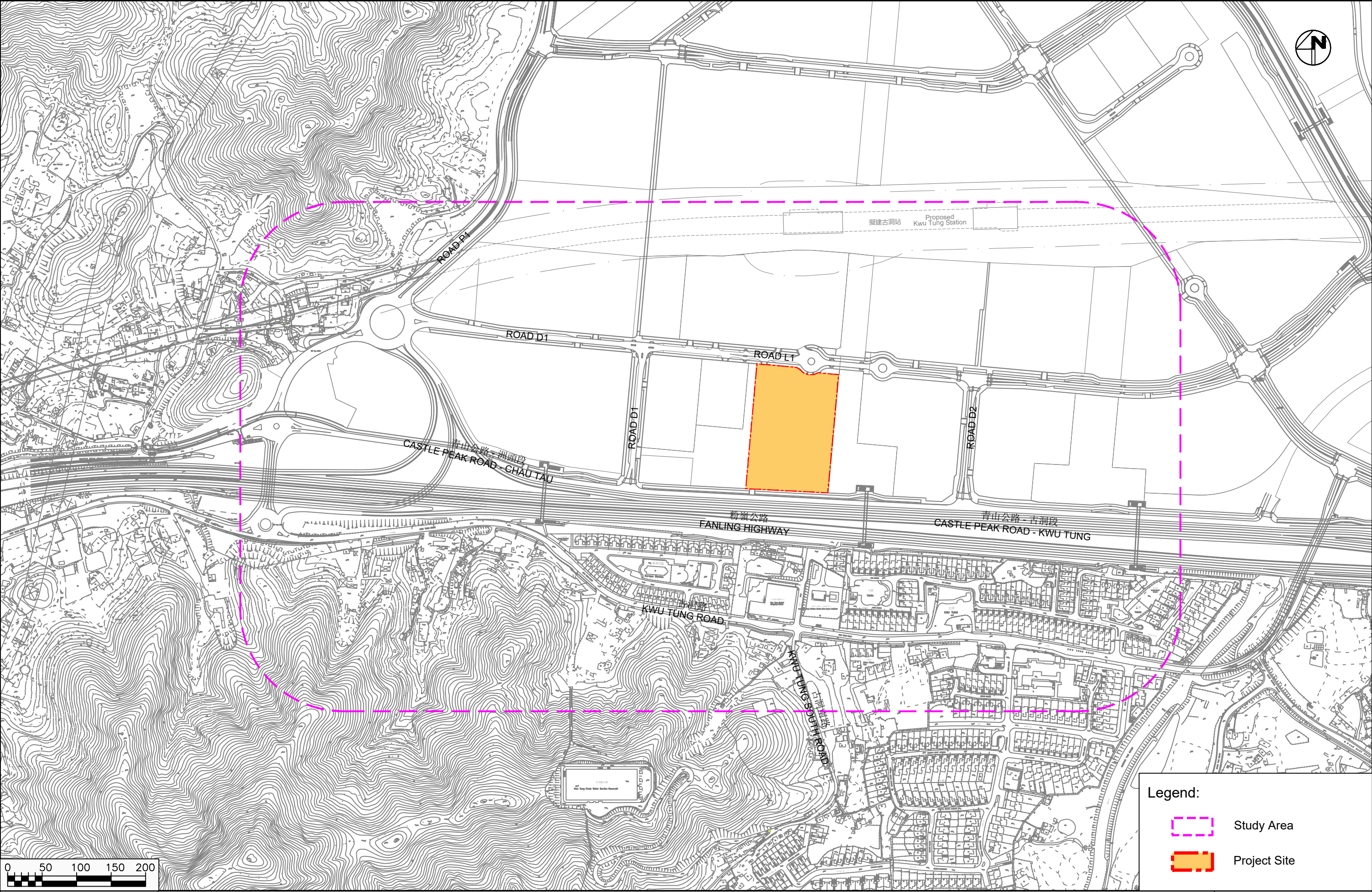
2024 Observed Peak Hours Traffic Flows





Date	Scale
26/12/2024	NTS

Project No. 83114	Rev.
Dwg No. Figure 3-2	-

X:\Ozzol83114_Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North\Drawg\83114_Figure 4-1_20250219.dwg 2025/03/05 10:35:36



- Legend:
-  Study Area
 -  Project Site

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

Date
24/02/2025

Scale
1:5000

Future Road Network in the Study Area



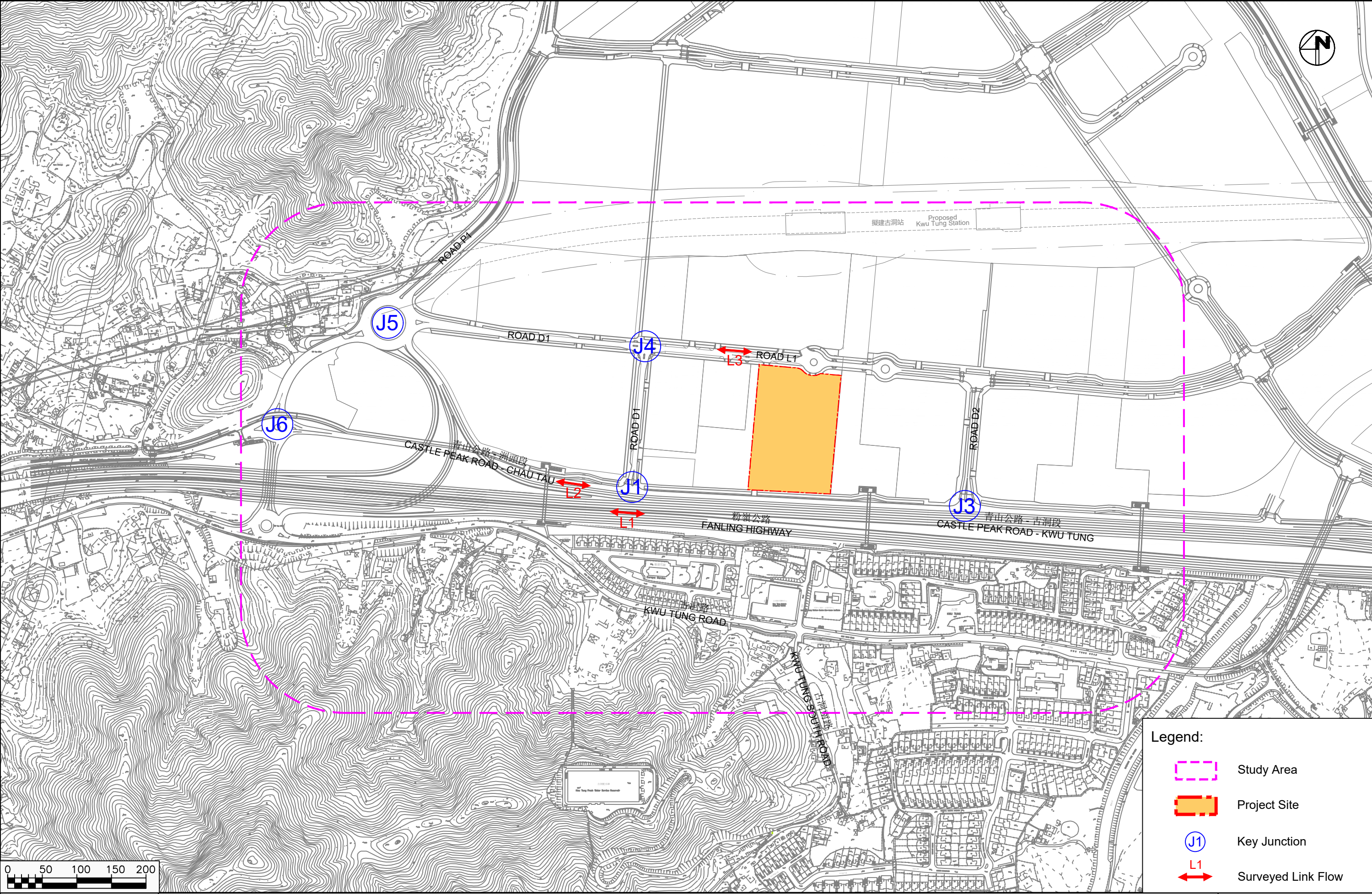
Project No. 83114

Dwg No. Figure 4-1

Rev.

-

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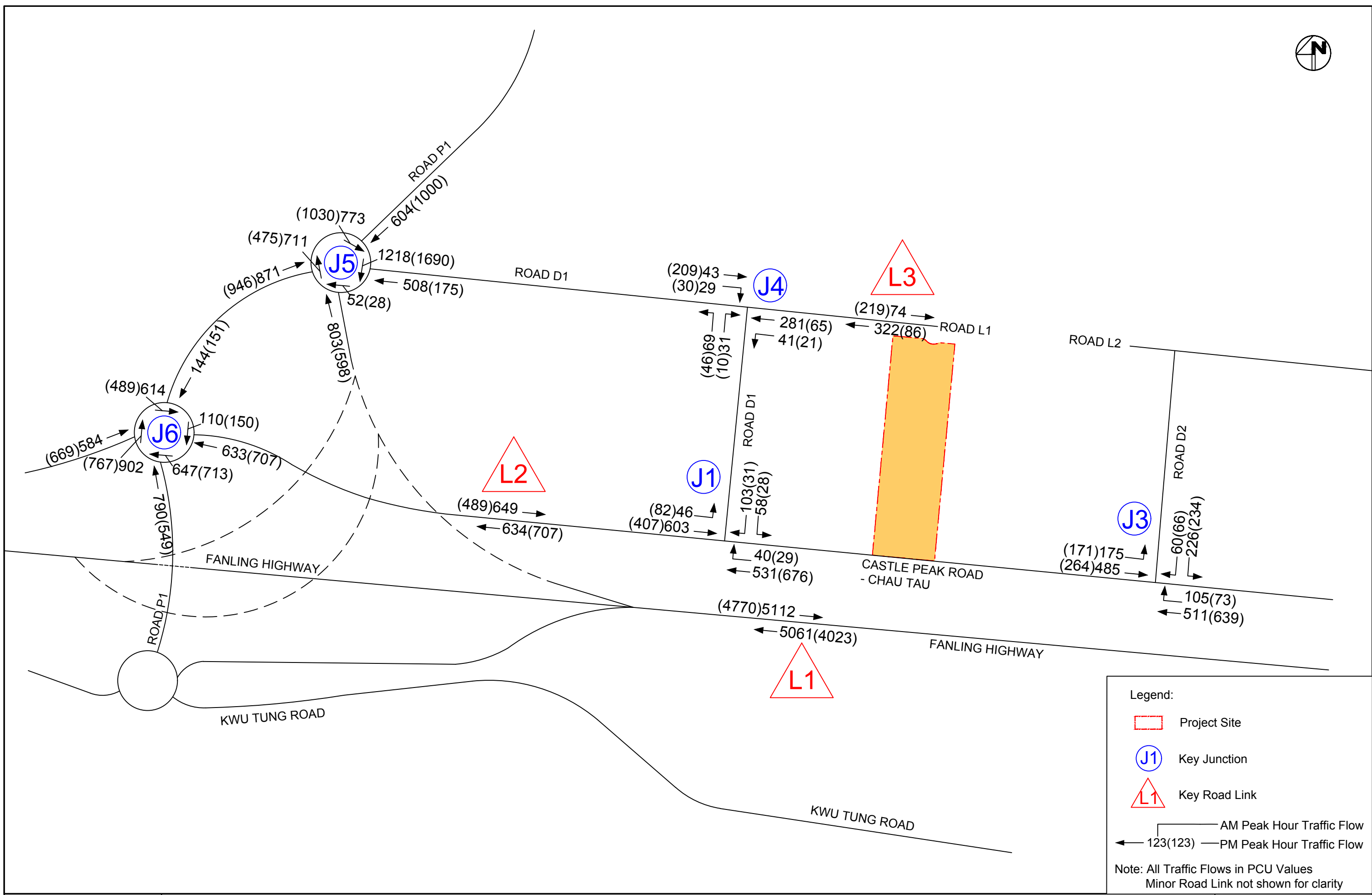
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

Locations of Key Junctions and Road Links for Assessment

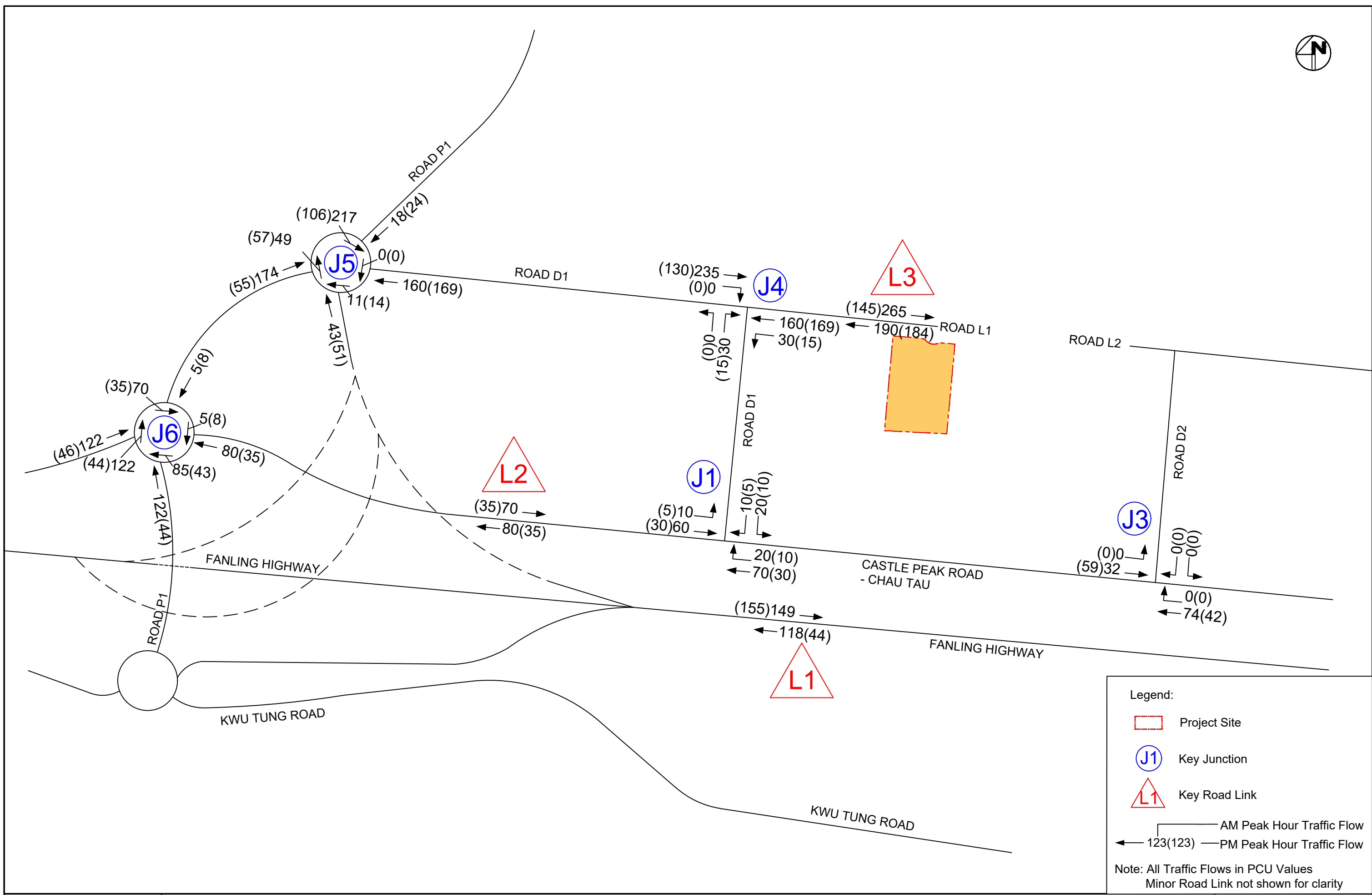
Legend:

- Study Area
- Project Site
- Key Junction
- Surveyed Link Flow

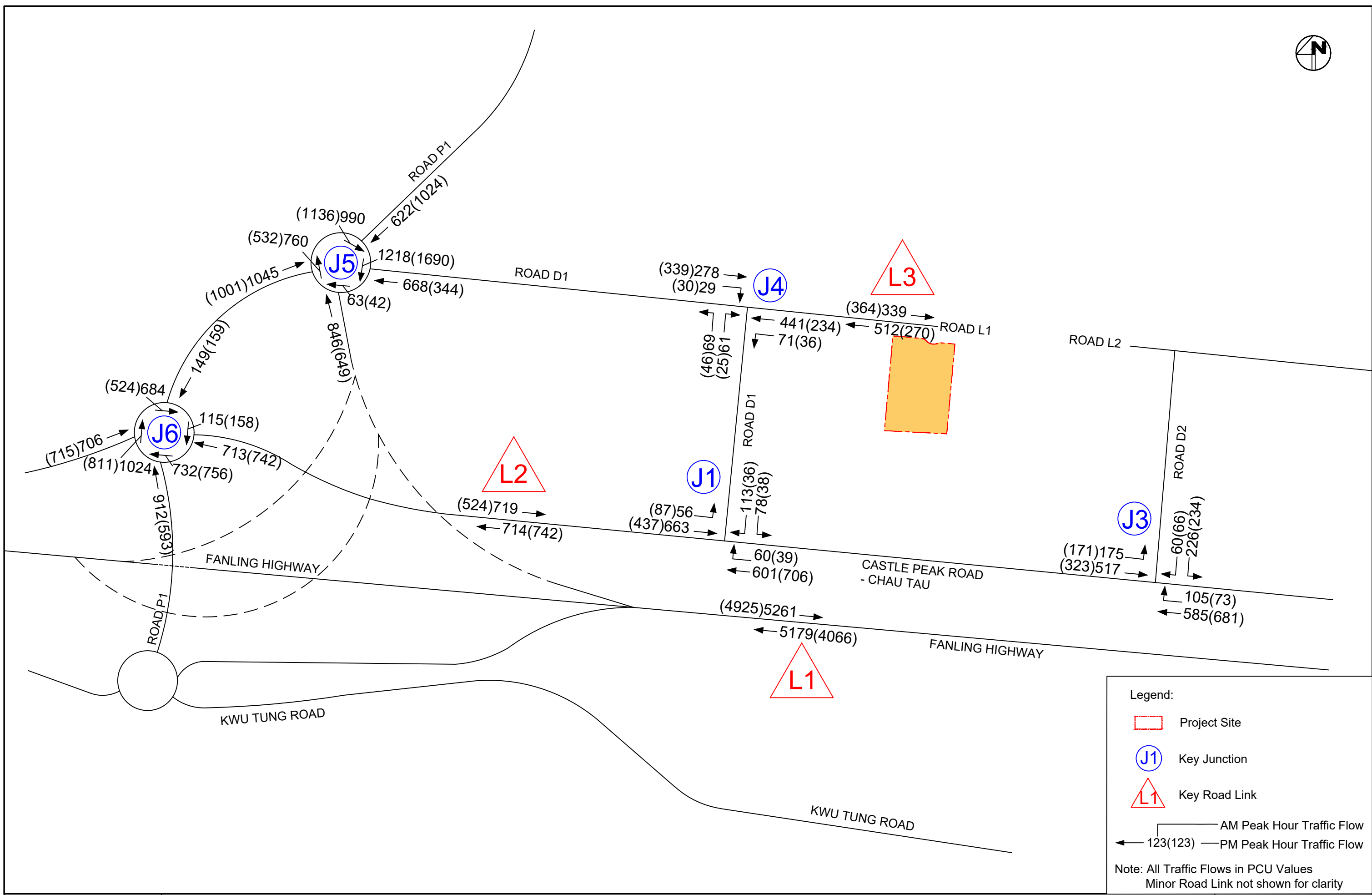
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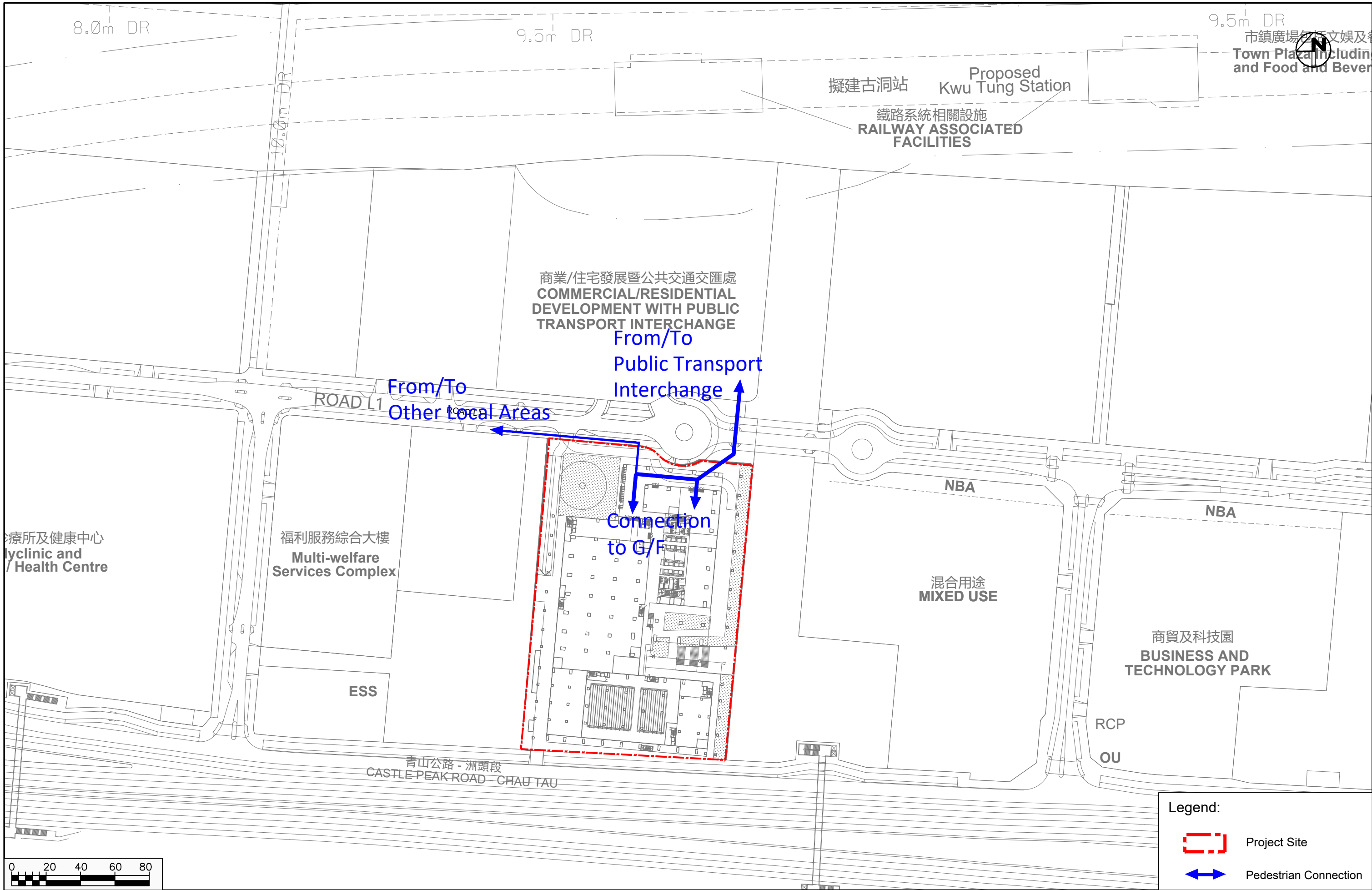
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X:\Ozzol\83114_Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North\Data\Dwg\83114_Figure 5-1A.dwg 2025/03/31 15:01:52

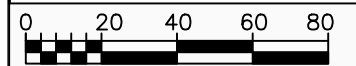
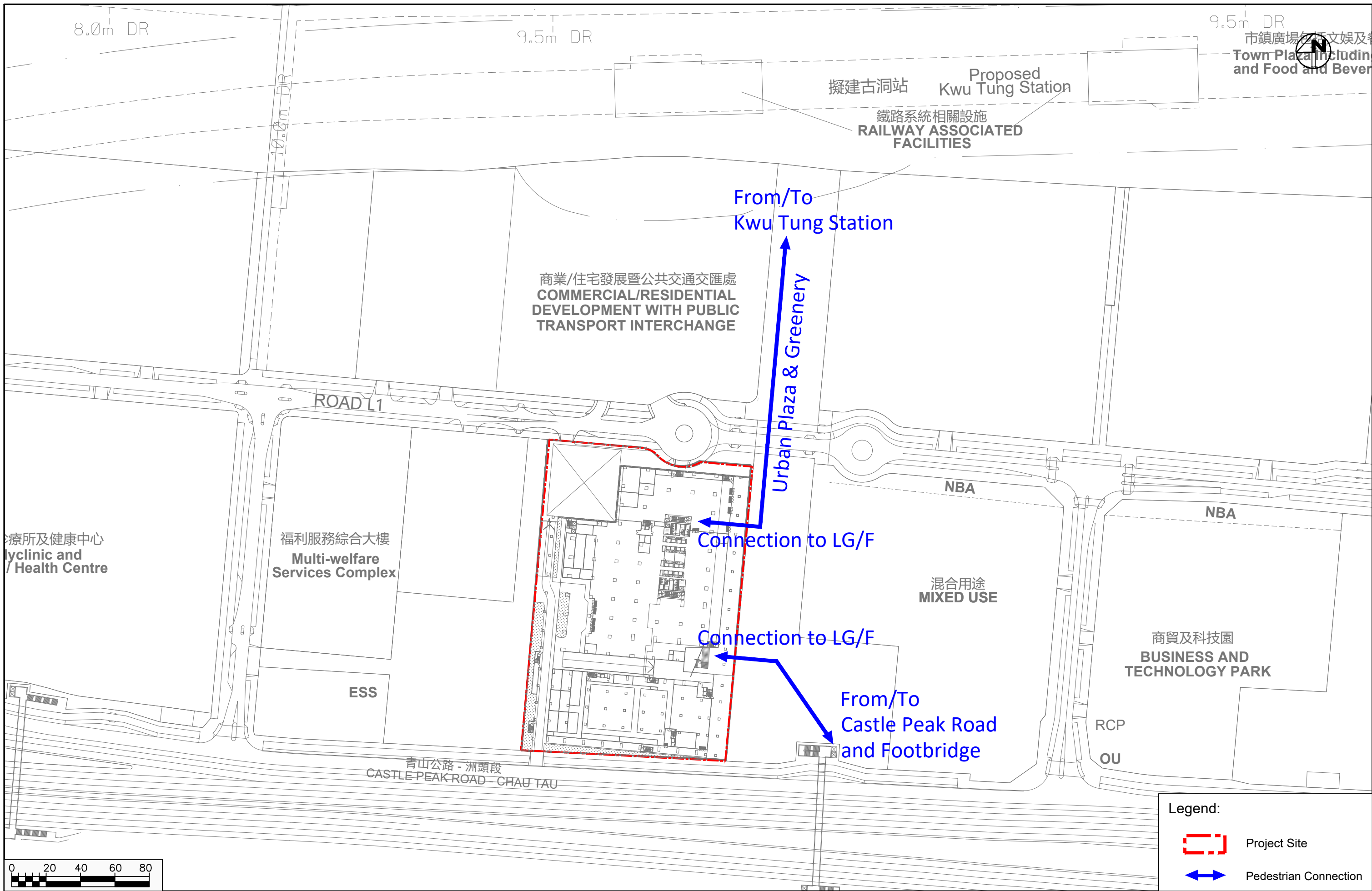


Legend:

 Project Site

 Pedestrian Connection

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Date	Scale
31/03/2025	1:2000

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

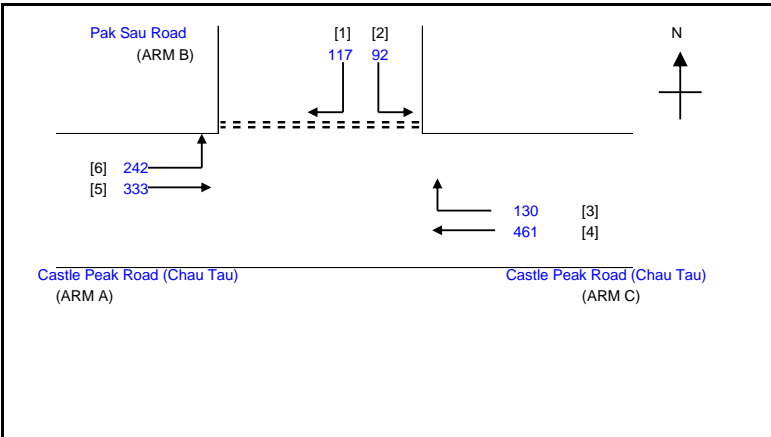
Pedestrian Connections at LG/F

OZZO TECHNOLOGY	
Project No. 83114	Rev.
Dwg No. Figure 5-2	-

Appendix A

2024 Junction Calculation Sheets

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North			PROJECT NO.: 83114	PREPARED BY: CSY	Feb-25
J1: Castle Peak Road (Chau Tau) / Pak Sau Road		2024 AM	FILENAME :	CHECKED BY: LL	Feb-25
2024 Observed AM Peak Hour Traffic Flows			Peak Road (Chau Tau)_Pak Sau Road_P.xls	REVIEWED BY: PCN	Feb-25



NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:	
MAJOR ROAD (ARM A)							
W	= 3.50 (metres)	D	= 1.0484314	Q b-a	= 354	DFC b-a	= 0.3305
W cr	= 0 (metres)	E	= 1.0484314	Q b-c	= 637	DFC b-c	= 0.1444
q a-b	= 242 (pcu/hr)	F	= 1.0568848	Q c-b	= 593	DFC c-b	= 0.2192
q a-c	= 333 (pcu/hr)	Y	= 0.87925	Q b-ac	= 354	DFC b-c (share lane)	= 0.5904
MAJOR ROAD (ARM C)		F for (Qb-ac) = 0		TOTAL FLOW = 1375 (PCU/HR)			
W c-b	= 3.5 (metres)						
Vr c-b	= 200 (metres)						
q c-a	= 461 (pcu/hr)						
q c-b	= 130 (pcu/hr)						
MINOR ROAD (ARM B)							
W b-a	= 3.9 (metres)						
W b-c	= 3.9 (metres)						
Vi b-a	= 150 (metres)						
Vr b-a	= 150 (metres)						
Vr b-c	= 150 (metres)						
q b-a	= 117 (pcu/hr)						
q b-c	= 92 (pcu/hr)						

CRITICAL DFC = 0.33

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY: CSY	Feb-25
J2: Castle Peak Road (Chau Tau)_Unnamed Road	2024 AM	FILENAME :		CHECKED BY: LL	Feb-25
2024 Observed AM Peak Hour Traffic Flows		eak Road (Chau Tau)_Unnamed Road_P .xls		REVIEWED BY: PCN	Feb-25

Diagram illustrating the junction layout and traffic flow data for the 2024 AM peak hour. The diagram shows Castle Peak Road (Chau Tau) (ARM A) and Unnamed Road (ARM B) with associated flow values and lane configurations.

NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH
W cr = CENTRAL RESERVE WIDTH
W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a
W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c
W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b
Vl b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a
Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a
Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c
Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b
D = STREAM-SPECIFIC B-A
E = STREAM-SPECIFIC B-C
F = STREAM-SPECIFIC C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:	
MAJOR ROAD (ARM A)							
W	= 3.50 (metres)	D	= 0.5332189	Q b-a	= 155	DFC b-a	= 0.0000
W cr	= 0 (metres)	E	= 0.5859548	Q b-c	= 384	DFC b-c	= 0.0000
q a-b	= 300 (pcu/hr)	F	= 1.0568848	Q c-b	= 630	DFC c-b	= 0.4413
q a-c	= 164 (pcu/hr)	Y	= 0.87925	Q b-ac	= 155	DFC b-c (share lane)	= 0.0000
MAJOR ROAD (ARM C)		F for (Qb-ac) = 0		TOTAL FLOW = 1335 (PCU/HR)			
W c-b	= 3.50 (metres)						
Vr c-b	= 200 (metres)						
q c-a	= 593 (pcu/hr)						
q c-b	= 278 (pcu/hr)						
						CRITICAL DFC = 0.44	

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY: CSY	Feb-25
J2: Castle Peak Road (Chau Tau)_Unnamed Road	2024 PM	FILENAME : J2_Castle Peak Road (Chau Tau)_Unnamed Road_P.xls	CHECKED BY: LL	Feb-25	
2024 Observed PM Peak Hour Traffic Flows			REVIEWED BY: PCN	Feb-25	

Diagram illustrating the junction layout and traffic flow directions. The diagram shows Castle Peak Road (Chau Tau) with arms A, B, and C, and Unnamed Road (ARM B). Traffic flows are indicated with arrows and values: 196, 144, 250, 502. A north arrow is also present.

NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH
W cr = CENTRAL RESERVE WIDTH
W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a
W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c
W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b
Vl b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a
Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a
Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c
Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b
D = STREAM-SPECIFIC B-A
E = STREAM-SPECIFIC B-C
F = STREAM-SPECIFIC C-B
Y = (1-0.0345W)

GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:	
MAJOR ROAD (ARM A)							
W	= 3.50 (metres)	D	= 0.5332189	Q b-a	= 182	DFC b-a	= 0.0000
W cr	= 0 (metres)	E	= 0.5859548	Q b-c	= 395	DFC b-c	= 0.0000
q a-b	= 196 (pcu/hr)	F	= 1.0568848	Q c-b	= 672	DFC c-b	= 0.3720
q a-c	= 144 (pcu/hr)	Y	= 0.87925	Q b-ac	= 182	DFC b-c (share lane)	= 0.0000
MAJOR ROAD (ARM C)		F for (Qb-ac) = 0		TOTAL FLOW = 1092 (PCU/HR)			
W c-b	= 3.50 (metres)						
Vr c-b	= 200 (metres)						
q c-a	= 502 (pcu/hr)						
q c-b	= 250 (pcu/hr)						
						CRITICAL DFC = 0.37	

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North			PROJECT NO.:	83114	PREPARED BY: CSY Feb-25
J3: Castle Peak Road (Kwu Tung)_Ho Sheung Heung Road		2024 AM	FILENAME :	CHECKED BY: LL Feb-25	REVIEWED BY: PCN Feb-25
2024 Observed AM Peak Hour Traffic Flows			d (Kwu Tung)_Ho Sheung Heung Road_P.xls		
<div><div><div><div><div>Ho Sheung Heung Road (ARM B)</div><div><div>[1] 421</div><div>[2] 135</div></div><div><div>[4] 173</div><div>[3] 445</div></div><div>Castle Peak Road (Kwu Tung) (ARM A)</div><div>Castle Peak Road (Kwu Tung) (ARM C)</div></div><div><div>N</div><div></div></div></div></div><div><div>NOTES : (GEOMETRIC INPUT DATA)</div><div>W = MAJOR ROAD WIDTH</div><div>W cr = CENTRAL RESERVE WIDTH</div><div>W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a</div><div>W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c</div><div>W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b</div><div>Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a</div><div>Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a</div><div>Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c</div><div>Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b</div><div>D = STREAM-SPECIFIC B-A</div><div>E = STREAM-SPECIFIC B-C</div><div>F = STREAM-SPECIFIC C-B</div><div>Y = (1-0.0345W)</div></div></div>					
<div><div><div><div>GEOMETRIC DETAILS:</div><div><div>MAJOR ROAD (ARM A)</div><div>W = 3.50 (metres)</div><div>W cr = 0 (metres)</div><div>q a-b = 0 (pcu/hr)</div><div>q a-c = 173 (pcu/hr)</div></div><div><div>MAJOR ROAD (ARM C)</div><div>W c-b = 3.5 (metres)</div><div>Vr c-b = 200 (metres)</div><div>q c-a = 445 (pcu/hr)</div><div>q c-b = 0 (pcu/hr)</div></div><div><div>MINOR ROAD (ARM B)</div><div>W b-a = 6.9 (metres)</div><div>W b-c = 6.9 (metres)</div><div>Vi b-a = 100 (metres)</div><div>Vr b-a = 120 (metres)</div><div>Vr b-c = 120 (metres)</div><div>q b-a = 421 (pcu/hr)</div><div>q b-c = 135 (pcu/hr)</div></div></div><div><div>GEOMETRIC FACTORS :</div><div>D = 1.2632349</div><div>E = 1.302304</div><div>F = 1.0568848</div><div>Y = 0.87925</div><div>F for (Qb-ac) = 0</div></div><div><div>THE CAPACITY OF MOVEMENT :</div><div>Q b-a = 609</div><div>Q b-c = 898</div><div>Q c-b = 729</div><div>Q b-ac = 609</div><div>TOTAL FLOW = 1174 (PCU/HR)</div></div><div><div>COMPARISION OF DESIGN FLOW TO CAPACITY:</div><div>DFC b-a = 0.6913</div><div>DFC b-c = 0.1503</div><div>DFC c-b = 0.0000</div><div>DFC b-c (share lane) = 0.9130</div><div>CRITICAL DFC = 0.69</div></div></div></div>					

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY: CSY	Feb-25
J3: Castle Peak Road (Kwu Tung)_Ho Sheung Heung Road		FILENAME :		CHECKED BY: LL	Feb-25
2024 Observed PM Peak Hour Traffic Flows		2024 PM J3_Castle Peak Road (Kwu Tung)_Ho Sheung Heung Road_P.xls		REVIEWED BY: PCN	Feb-25

Ho Sheung Heung Road (ARM B)

Castle Peak Road (Kwu Tung) (ARM A)

Castle Peak Road (Kwu Tung) (ARM C)

[1] 431 [2] 126

[4] 151

353 [3]

N

NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

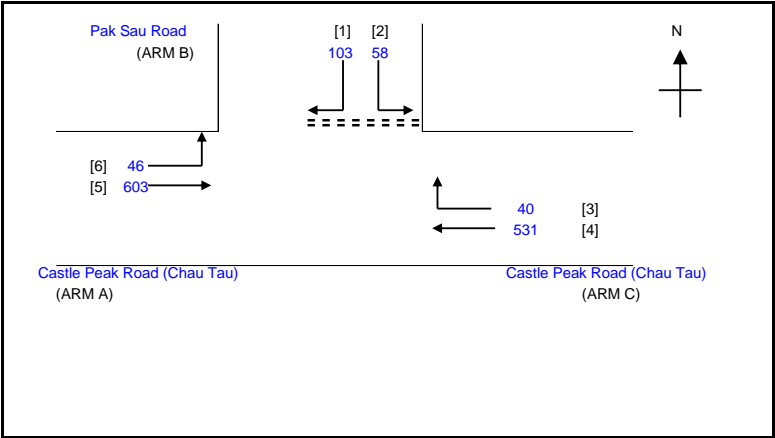
GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:	
MAJOR ROAD (ARM A)							
W	= 3.50 (metres)	D	= 1.2632349	Q b-a	= 641	DFC b-a	= 0.6724
W cr	= 0 (metres)	E	= 1.302304	Q b-c	= 907	DFC b-c	= 0.1389
q a-b	= 0 (pcu/hr)	F	= 1.0568848	Q c-b	= 736	DFC c-b	= 0.0000
q a-c	= 151 (pcu/hr)	Y	= 0.87925	Q b-ac	= 641	DFC b-c (share lane)	= 0.8690
MAJOR ROAD (ARM C)		F for (Qb-ac) = 0		TOTAL FLOW = 1061 (PCU/HR)			
W c-b	= 3.5 (metres)						
Vr c-b	= 200 (metres)						
q c-a	= 353 (pcu/hr)						
q c-b	= 0 (pcu/hr)						
MINOR ROAD (ARM B)							
W b-a	= 6.9 (metres)						
W b-c	= 6.9 (metres)						
Vi b-a	= 100 (metres)						
Vr b-a	= 120 (metres)						
Vr b-c	= 120 (metres)						
q b-a	= 431 (pcu/hr)						
q b-c	= 126 (pcu/hr)						

CRITICAL DFC = 0.67

Appendix B

2034 Junction Calculation Sheets

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION			INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY:	CW	Mar-25
J1: Castle Peak Road (Chau Tau) / Road D1	2034 Ref AM	FILENAME :		CHECKED BY:	DP	Mar-25
2034 Reference AM Peak Hour Traffic Flows		Peak Road (Chau Tau)_Pak Sau Road_P.xls		REVIEWED BY:	OC	Mar-25



NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

VI b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 3.50 (metres)

W cr = 0 (metres)

q a-b = 46 (pcu/hr)

q a-c = 603 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 3.5 (metres)

Vr c-b = 200 (metres)

q c-a = 531 (pcu/hr)

q c-b = 40 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 3.9 (metres)

W b-c = 3.9 (metres)

VI b-a = 150 (metres)

Vr b-a = 150 (metres)

Vr b-c = 150 (metres)

q b-a = 103 (pcu/hr)

q b-c = 58 (pcu/hr)

GEOMETRIC FACTORS :

D = 1.0484314

E = 1.0484314

F = 1.0568848

Y = 0.87925

F for (Qb-ac) = 0

THE CAPACITY OF MOVEMENT :

Q b-a = 318

Q b-c = 573

Q c-b = 568

Q b-ac = 318

TOTAL FLOW = 1381 (PCU/HR)

COMPARISION OF DESIGN FLOW TO CAPACITY:

DFC b-a = 0.3239

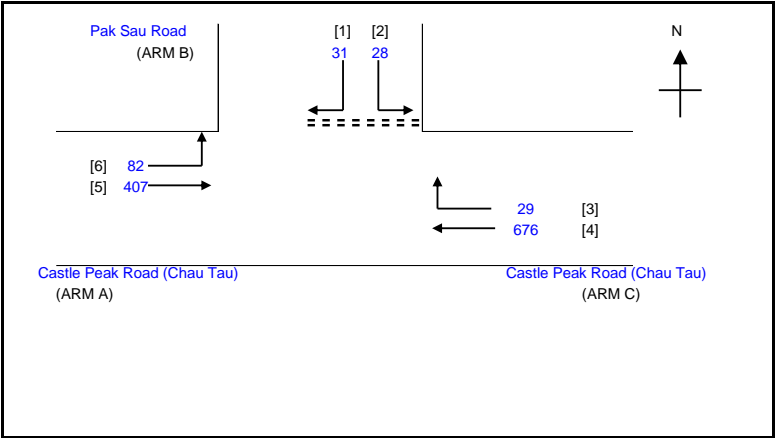
DFC b-c = 0.1012

DFC c-b = 0.0704

DFC b-c (share lane) = 0.5063

CRITICAL DFC = 0.32

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION			INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY:	CW	Mar-25
J1: Castle Peak Road (Chau Tau) / Road D1	2034 Ref PM	FILENAME :		CHECKED BY:	DP	Mar-25
2034 Reference PM Peak Hour Traffic Flows		Peak Road (Chau Tau)_Pak Sau Road_P.xls		REVIEWED BY:	OC	Mar-25



NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 3.50 (metres)

W cr = 0 (metres)

q a-b = 82 (pcu/hr)

q a-c = 407 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 3.5 (metres)

Vr c-b = 200 (metres)

q c-a = 676 (pcu/hr)

q c-b = 29 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 3.9 (metres)

W b-c = 3.9 (metres)

Vi b-a = 150 (metres)

Vr b-a = 150 (metres)

Vr b-c = 150 (metres)

q b-a = 31 (pcu/hr)

q b-c = 28 (pcu/hr)

GEOMETRIC FACTORS :

D = 1.0484314

E = 1.0484314

F = 1.0568848

Y = 0.87925

F for (Qb-ac) = 0

THE CAPACITY OF MOVEMENT :

Q b-a = 353

Q b-c = 634

Q c-b = 622

Q b-ac = 353

TOTAL FLOW = 1253 (PCU/HR)

COMPARISION OF DESIGN FLOW
TO CAPACITY:

DFC b-a = 0.0878

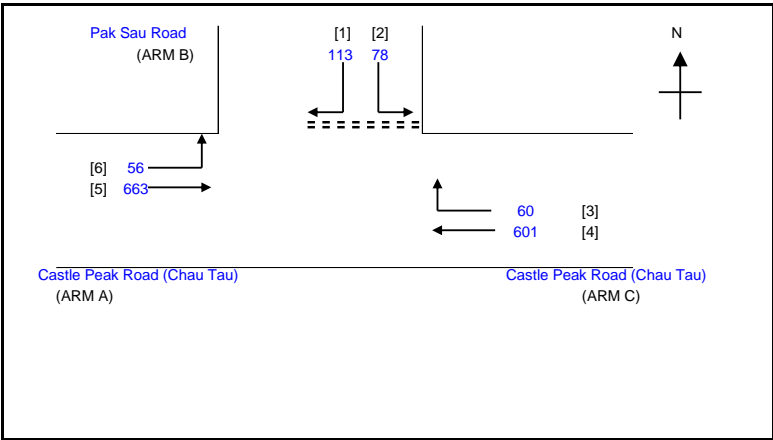
DFC b-c = 0.0442

DFC c-b = 0.0466

DFC b-c (share lane) = 0.1671

CRITICAL DFC = 0.09

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION			INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY:	CW	Mar-25
J1: Castle Peak Road (Chau Tau) / Road D1	2034 Des AM	FILENAME :		CHECKED BY:	DP	Mar-25
2034 Design AM Peak Hour Traffic Flows		Peak Road (Chau Tau)_Pak Sau Road_P.xls		REVIEWED BY:	OC	Mar-25



NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

VI b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 3.50 (metres)

W cr = 0 (metres)

q a-b = 56 (pcu/hr)

q a-c = 663 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 3.5 (metres)

Vr c-b = 200 (metres)

q c-a = 601 (pcu/hr)

q c-b = 60 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 3.9 (metres)

W b-c = 3.9 (metres)

VI b-a = 150 (metres)

Vr b-a = 150 (metres)

Vr b-c = 150 (metres)

q b-a = 113 (pcu/hr)

q b-c = 78 (pcu/hr)

GEOMETRIC FACTORS :

D = 1.0484314

E = 1.0484314

F = 1.0568848

Y = 0.87925

F for (Qb-ac) = 0

THE CAPACITY OF MOVEMENT :

Q b-a = 272

Q b-c = 551

Q c-b = 544

Q b-ac = 272

TOTAL FLOW = 1571 (PCU/HR)

COMPARISION OF DESIGN FLOW TO CAPACITY:

DFC b-a = 0.4154

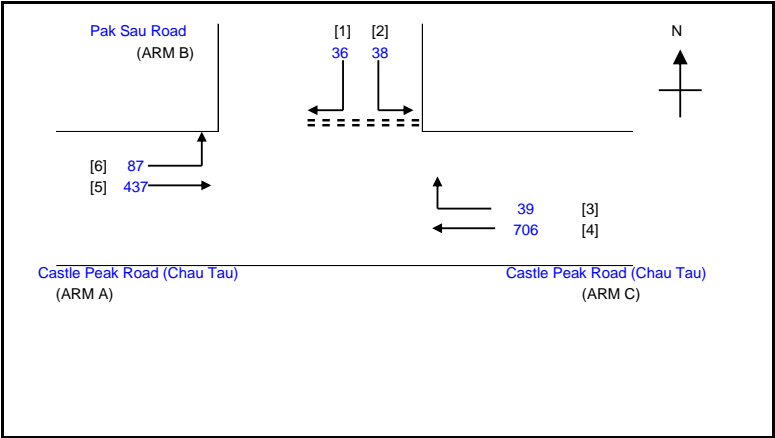
DFC b-c = 0.1416

DFC c-b = 0.1103

DFC b-c (share lane) = 0.7022

CRITICAL DFC = 0.42

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION			INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY:	CW	Mar-25
J1: Castle Peak Road (Chau Tau) / Road D1	2034 Des PM	FILENAME :		CHECKED BY:	DP	Mar-25
2034 Design PM Peak Hour Traffic Flows		Peak Road (Chau Tau)_Pak Sau Road_P.xls		REVIEWED BY:	OC	Mar-25



NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

VI b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 3.50 (metres)

W cr = 0 (metres)

q a-b = 87 (pcu/hr)

q a-c = 437 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 3.5 (metres)

Vr c-b = 200 (metres)

q c-a = 706 (pcu/hr)

q c-b = 39 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 3.9 (metres)

W b-c = 3.9 (metres)

VI b-a = 150 (metres)

Vr b-a = 150 (metres)

Vr b-c = 150 (metres)

q b-a = 36 (pcu/hr)

q b-c = 38 (pcu/hr)

GEOMETRIC FACTORS :

D = 1.0484314

E = 1.0484314

F = 1.0568848

Y = 0.87925

F for (Qb-ac) = 0

THE CAPACITY OF MOVEMENT :

Q b-a = 331

Q b-c = 623

Q c-b = 610

Q b-ac = 331

TOTAL FLOW = 1343 (PCU/HR)

COMPARISION OF DESIGN FLOW TO CAPACITY:

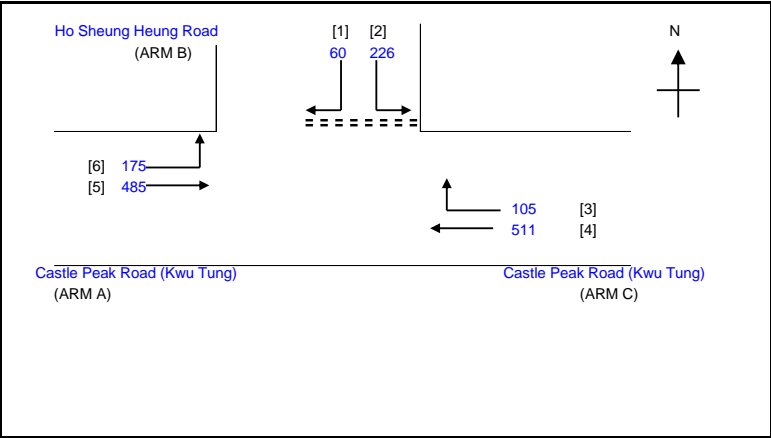
DFC b-a = 0.1088

DFC b-c = 0.0610

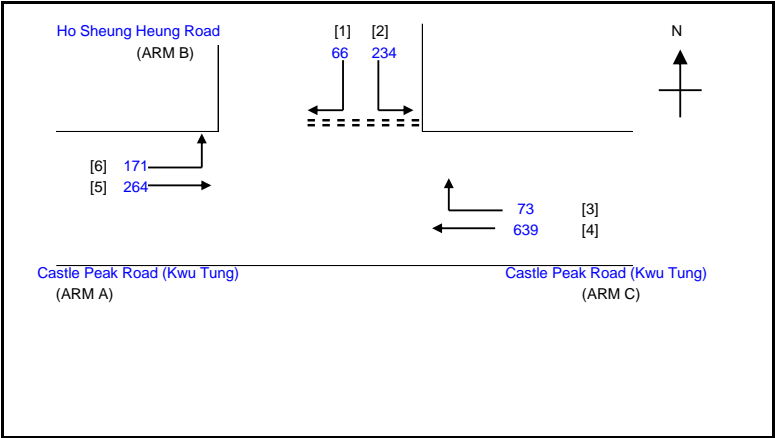
DFC c-b = 0.0639

DFC b-c (share lane) = 0.2236

CRITICAL DFC = 0.11

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY: CW	Mar-25
J3: Castle Peak Road (Kwu Tung)/ Road D2		2034 Ref AM		CHECKED BY: DP	Mar-25
2034 Reference AM Peak Hour Traffic Flows				REVIEWED BY: OC	Mar-25
<div></div>		<div>NOTES : (GEOMETRIC INPUT DATA)</div> <div>W = MAJOR ROAD WIDTH</div> <div>W cr = CENTRAL RESERVE WIDTH</div> <div>W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a</div> <div>W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c</div> <div>W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b</div> <div>Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a</div> <div>Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a</div> <div>Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c</div> <div>Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b</div> <div>D = STREAM-SPECIFIC B-A</div> <div>E = STREAM-SPECIFIC B-C</div> <div>F = STREAM-SPECIFIC C-B</div> <div>Y = (1-0.0345W)</div>			
<div><div><div>GEOMETRIC DETAILS:</div><div>MAJOR ROAD (ARM A)</div><div>W = 3.50 (metres)</div><div>W cr = 0 (metres)</div><div>q a-b = 175 (pcu/hr)</div><div>q a-c = 485 (pcu/hr)</div><div>MAJOR ROAD (ARM C)</div><div>W c-b = 3.5 (metres)</div><div>Vr c-b = 200 (metres)</div><div>q c-a = 511 (pcu/hr)</div><div>q c-b = 105 (pcu/hr)</div><div>MINOR ROAD (ARM B)</div><div>W b-a = 6.9 (metres)</div><div>W b-c = 6.9 (metres)</div><div>Vi b-a = 100 (metres)</div><div>Vr b-a = 120 (metres)</div><div>Vr b-c = 120 (metres)</div><div>q b-a = 60 (pcu/hr)</div><div>q b-c = 226 (pcu/hr)</div></div><div><div>GEOMETRIC FACTORS :</div><div>D = 1.2632349</div><div>E = 1.302304</div><div>F = 1.0568848</div><div>Y = 0.87925</div><div>F for (Qb-ac) = 0</div></div><div><div>THE CAPACITY OF MOVEMENT :</div><div>Q b-a = 377</div><div>Q b-c = 739</div><div>Q c-b = 564</div><div>Q b-ac = 377</div><div>TOTAL FLOW = 1562 (PCU/HR)</div></div><div><div>COMPARISION OF DESIGN FLOW TO CAPACITY:</div><div>DFC b-a = 0.1592</div><div>DFC b-c = 0.3058</div><div>DFC c-b = 0.1862</div><div>DFC b-c (share lane) = 0.7586</div><div>CRITICAL DFC = 0.31</div></div></div>					

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION			INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY:	CW	Mar-25
J3: Castle Peak Road (Kwu Tung)/ Road D2	2034 Ref PM	FILENAME :		CHECKED BY:	DP	Mar-25
2034 Reference PM Peak Hour Traffic Flows		d (Kwu Tung)_Ho Sheung Heung Road_P.xls		REVIEWED BY:	OC	Mar-25



- NOTES : (GEOMETRIC INPUT DATA)
- W = MAJOR ROAD WIDTH
 - W cr = CENTRAL RESERVE WIDTH
 - W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a
 - W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c
 - W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b
 - Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a
 - Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a
 - Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c
 - Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b
 - D = STREAM-SPECIFIC B-A
 - E = STREAM-SPECIFIC B-C
 - F = STREAM-SPECIFIC C-B
 - Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W = 3.50 (metres)

W cr = 0 (metres)

q a-b = 171 (pcu/hr)

q a-c = 264 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b = 3.5 (metres)

Vr c-b = 200 (metres)

q c-a = 639 (pcu/hr)

q c-b = 73 (pcu/hr)

MINOR ROAD (ARM B)

W b-a = 6.9 (metres)

W b-c = 6.9 (metres)

Vi b-a = 100 (metres)

Vr b-a = 120 (metres)

Vr b-c = 120 (metres)

q b-a = 66 (pcu/hr)

q b-c = 234 (pcu/hr)

GEOMETRIC FACTORS :

D = 1.2632349

E = 1.302304

F = 1.0568848

Y = 0.87925

F for (Qb-ac) = 0

THE CAPACITY OF MOVEMENT :

Q b-a = 453

Q b-c = 832

Q c-b = 640

Q b-ac = 453

TOTAL FLOW = 1447 (PCU/HR)

COMPARISION OF DESIGN FLOW
TO CAPACITY:

DFC b-a = 0.1457

DFC b-c = 0.2813

DFC c-b = 0.1141

DFC b-c (share lane) = 0.6623

CRITICAL DFC = 0.28

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.:	83114	PREPARED BY:	CW Mar-25
J3: Castle Peak Road (Kwu Tung)/ Road D2	2034 Des AM	FILENAME :		CHECKED BY:	DP Mar-25
2034 Design AM Peak Hour Traffic Flows		d (Kwu Tung)_Ho Sheung Heung Road_P.xls	REVIEWED BY:	OC Mar-25	

Ho Sheung Heung Road (ARM B)

Castle Peak Road (Kwu Tung) (ARM A)

Castle Peak Road (Kwu Tung) (ARM C)

Flow values: [1] 60, [2] 226, [3] 105, [4] 585, [5] 517, [6] 175

NOTES : (GEOMETRIC INPUT DATA)

W = MAJOR ROAD WIDTH

W cr = CENTRAL RESERVE WIDTH

W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a

W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c

W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b

Vi b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a

Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a

Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c

Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b

D = STREAM-SPECIFIC B-A

E = STREAM-SPECIFIC B-C

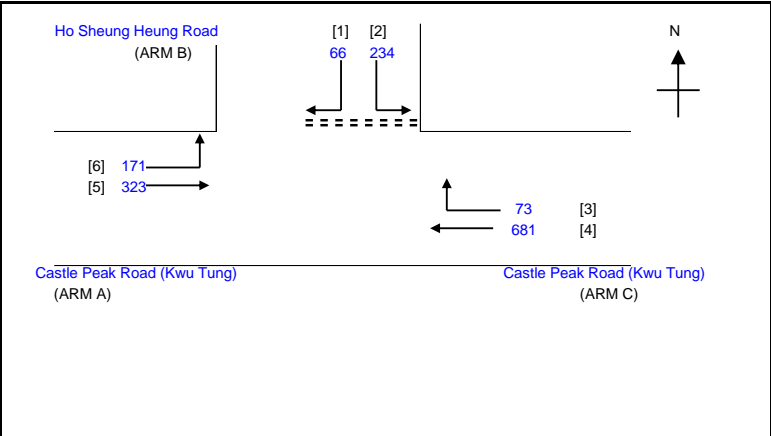
F = STREAM-SPECIFIC C-B

Y = (1-0.0345W)

GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:	
MAJOR ROAD (ARM A)							
W	= 3.50 (metres)	D	= 1.2632349	Q b-a	= 346	DFC b-a	= 0.1734
W cr	= 0 (metres)	E	= 1.302304	Q b-c	= 726	DFC b-c	= 0.3113
q a-b	= 175 (pcu/hr)	F	= 1.0568848	Q c-b	= 553	DFC c-b	= 0.1899
q a-c	= 517 (pcu/hr)	Y	= 0.87925	Q b-ac	= 346	DFC b-c (share lane)	= 0.8266
MAJOR ROAD (ARM C)		F for (Qb-ac) = 0		TOTAL FLOW = 1668 (PCU/HR)			
W c-b	= 3.5 (metres)						
Vr c-b	= 200 (metres)						
q c-a	= 585 (pcu/hr)						
q c-b	= 105 (pcu/hr)						
MINOR ROAD (ARM B)							
W b-a	= 6.9 (metres)						
W b-c	= 6.9 (metres)						
Vi b-a	= 100 (metres)						
Vr b-a	= 120 (metres)						
Vr b-c	= 120 (metres)						
q b-a	= 60 (pcu/hr)						
q b-c	= 226 (pcu/hr)						

CRITICAL DFC

= 0.31

OZZO TECHNOLOGY (HK) LIMITED		PRIORITY JUNCTION CALCULATION		INITIALS	DATE																																																																																																																																																								
Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North		PROJECT NO.: 83114		PREPARED BY: CW	Mar-25																																																																																																																																																								
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<div></div>		<div>NOTES : (GEOMETRIC INPUT DATA) W = MAJOR ROAD WIDTH W cr = CENTRAL RESERVE WIDTH W b-a = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-a W b-c = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM b-c W c-b = LANE WIDTH AVAILABLE TO VEHICLE WAITING IN STREAM c-b Vl b-a = VISIBILITY TO THE LEFT FOR VEHICLES WAITING IN STREAM b-a Vr b-a = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-a Vr b-c = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM b-c Vr c-b = VISIBILITY TO THE RIGHT FOR VEHICLES WAITING IN STREAM c-b D = STREAM-SPECIFIC B-A E = STREAM-SPECIFIC B-C F = STREAM-SPECIFIC C-B Y = (1-0.0345W)</div>																																																																																																																																																											
<table><tr><td colspan="2">GEOMETRIC DETAILS:</td><td colspan="2">GEOMETRIC FACTORS :</td><td colspan="2">THE CAPACITY OF MOVEMENT :</td><td colspan="2">COMPARISION OF DESIGN FLOW TO CAPACITY:</td></tr><tr><td colspan="2">MAJOR ROAD (ARM A)</td><td colspan="2"></td><td colspan="2"></td><td colspan="2"></td></tr><tr><td>W</td><td>= 3.50 (metres)</td><td>D</td><td>= 1.2632349</td><td>Q b-a</td><td>= 419</td><td>DFC b-a</td><td>= 0.1575</td></tr><tr><td>W cr</td><td>= 0 (metres)</td><td>E</td><td>= 1.302304</td><td>Q b-c</td><td>= 807</td><td>DFC b-c</td><td>= 0.2900</td></tr><tr><td>q a-b</td><td>= 171 (pcu/hr)</td><td>F</td><td>= 1.0568848</td><td>Q c-b</td><td>= 620</td><td>DFC c-b</td><td>= 0.1177</td></tr><tr><td>q a-c</td><td>= 323 (pcu/hr)</td><td>Y</td><td>= 0.87925</td><td>Q b-ac</td><td>= 419</td><td>DFC b-c (share lane)</td><td>= 0.7160</td></tr><tr><td colspan="2">MAJOR ROAD (ARM C)</td><td>F for (Qb-ac)</td><td>= 0</td><td>TOTAL FLOW</td><td>= 1548 (PCU/HR)</td><td colspan="2"></td></tr><tr><td>W c-b</td><td>= 3.5 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>Vr c-b</td><td>= 200 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>q c-a</td><td>= 681 (pcu/hr)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>q c-b</td><td>= 73 (pcu/hr)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td colspan="2">MINOR ROAD (ARM B)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>W b-a</td><td>= 6.9 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>W b-c</td><td>= 6.9 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>Vl b-a</td><td>= 100 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>Vr b-a</td><td>= 120 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>Vr b-c</td><td>= 120 (metres)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>q b-a</td><td>= 66 (pcu/hr)</td><td colspan="4"></td><td colspan="2"></td></tr><tr><td>q b-c</td><td>= 234 (pcu/hr)</td><td colspan="4"></td><td colspan="2"></td></tr></table>						GEOMETRIC DETAILS:		GEOMETRIC FACTORS :		THE CAPACITY OF MOVEMENT :		COMPARISION OF DESIGN FLOW TO CAPACITY:		MAJOR ROAD (ARM A)								W	= 3.50 (metres)	D	= 1.2632349	Q b-a	= 419	DFC b-a	= 0.1575	W cr	= 0 (metres)	E	= 1.302304	Q b-c	= 807	DFC b-c	= 0.2900	q a-b	= 171 (pcu/hr)	F	= 1.0568848	Q c-b	= 620	DFC c-b	= 0.1177	q a-c	= 323 (pcu/hr)	Y	= 0.87925	Q b-ac	= 419	DFC b-c (share lane)	= 0.7160	MAJOR ROAD (ARM C)		F for (Qb-ac)	= 0	TOTAL FLOW	= 1548 (PCU/HR)			W c-b	= 3.5 (metres)							Vr c-b	= 200 (metres)							q c-a	= 681 (pcu/hr)							q c-b	= 73 (pcu/hr)							MINOR ROAD (ARM B)								W b-a	= 6.9 (metres)							W b-c	= 6.9 (metres)							Vl b-a	= 100 (metres)							Vr b-a	= 120 (metres)							Vr b-c	= 120 (metres)							q b-a	= 66 (pcu/hr)							q b-c	= 234 (pcu/hr)						
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MAJOR ROAD (ARM C)		F for (Qb-ac)	= 0	TOTAL FLOW	= 1548 (PCU/HR)																																																																																																																																																								
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q c-a	= 681 (pcu/hr)																																																																																																																																																												
q c-b	= 73 (pcu/hr)																																																																																																																																																												
MINOR ROAD (ARM B)																																																																																																																																																													
W b-a	= 6.9 (metres)																																																																																																																																																												
W b-c	= 6.9 (metres)																																																																																																																																																												
Vl b-a	= 100 (metres)																																																																																																																																																												
Vr b-a	= 120 (metres)																																																																																																																																																												
Vr b-c	= 120 (metres)																																																																																																																																																												
q b-a	= 66 (pcu/hr)																																																																																																																																																												
q b-c	= 234 (pcu/hr)																																																																																																																																																												
<div>CRITICAL DFC = 0.29</div>																																																																																																																																																													

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE _____

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

PROJECT NO.	81838
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Prepared By:

	CW
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Mar-25

J4: Road D1 / Road L1

2034 Ref AM

FILENAME :	
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Prepared By:	
Checked By:	

	DP
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Mar-25

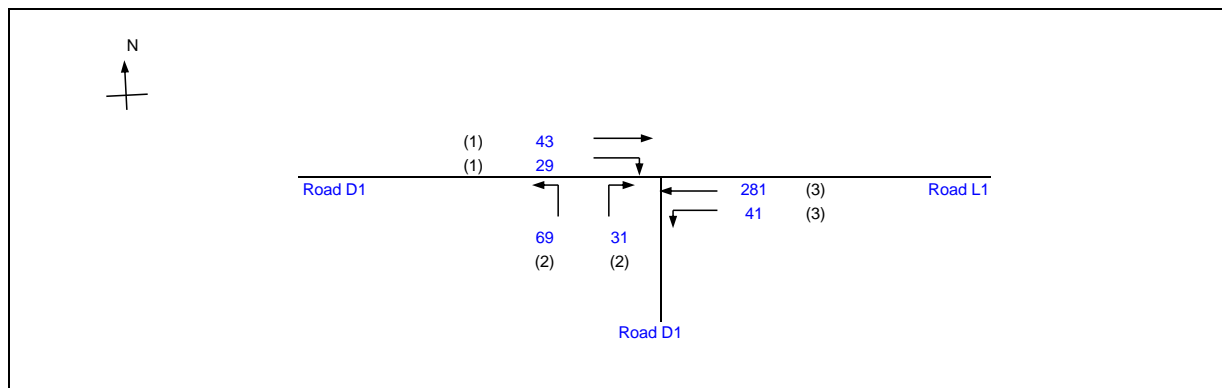
2034 Reference AM Peak Hour Traffic Flows

J4 Road D1 Road L1 S.xls

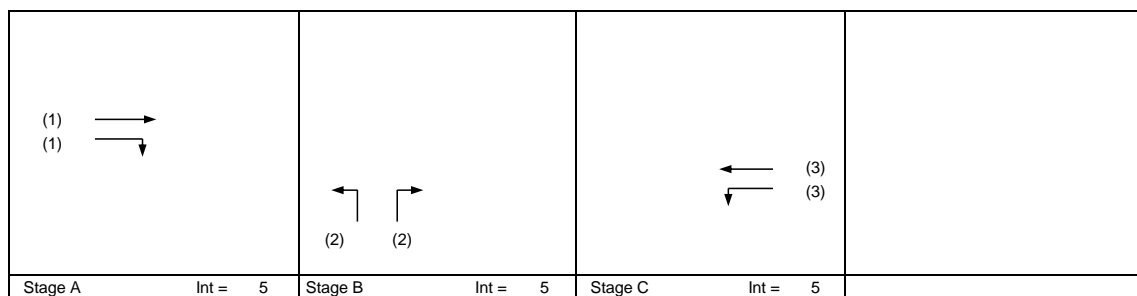
Reviewed By:

	SC
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Mar-25



		Existing Cycle Time	
No. of stages per cycle	N =	3	
Cycle time	C =	90 sec	
Sum(y)	Y =	0.241	
Loss time	L =	24 sec	
Total Flow	=	494 pcu	
Co	= $(1.5 \cdot L + 5) / (1 - Y)$	54.0 sec	
Cm	= $L / (1 - Y)$	31.6 sec	
Yult	=	0.720	
R.C.ult	= $(Yult - Y) / Y \cdot 100\%$	198.4 %	
Cp	= $0.9 \cdot L / (0.9 - Y)$	32.8 sec	
Ymax	= $1 - L / C$	0.733	
R.C.(C)	= $(0.9 \cdot Ymax - Y) / Y \cdot 100\%$	173.6 %	



Pedestrian Phase	Stage	Length (m)	Green Time Required (s)			Green Time Provided (s)	
			SG	FG	Delay	SG	FG

Move- ment	Stage	Lane Width m.	Phase	No. of lane	Radius m.	O	N	Straight- Ahead Sat. Flow	Movement			Total FLOw pcu/h	Proportion of Turning Vehicles	Sat. Flow pcu/h	Flare lane Length m.	Share Effect pcu/hr	Revised Sat. Flow pcu/h	y	Greater y	L sec	g (required) sec	g (input) sec	Degree of Saturation X	Queue Length (m / lane)	Average Delay (seconds)
									Left pcu/h	Straight pcu/h	Right pcu/h														
SA,RT	A	5.00	1	1	20		N	2115		43	29	72	0.40	2053			2053	0.035	0.035	12	10	10	0.329	6	37
LT,RT	B	5.00	2	1	15		N	2115	69		31	100	1.00	1923			1923	0.052	0.052		14	14	0.329	12	33
LT,SA	C	5.00	3	1	15		N	2115	41	281		322	0.13	2088			2088	0.154	0.154		42	42	0.329	24	14
PED	D																		12						

NOTE : O - OPPOSING TRAFFIC

N - NEAR SIDE LANE

SG - STEADY GREEN

FG - FLASHING GREEN

PEDESTRIAN WALKING SPEED = 1.2m/s

QUEUING LENGTH = AVERAGE QUEUE * 6m

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE _____

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

PROJECT NO.	81838
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Prepared By:

	CW
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Mar-25

J4: Road D1 / Road L1

2034 Ref PM

FILENAME :	
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Prepared By:	
Checked By:	

	DP
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Mar-25

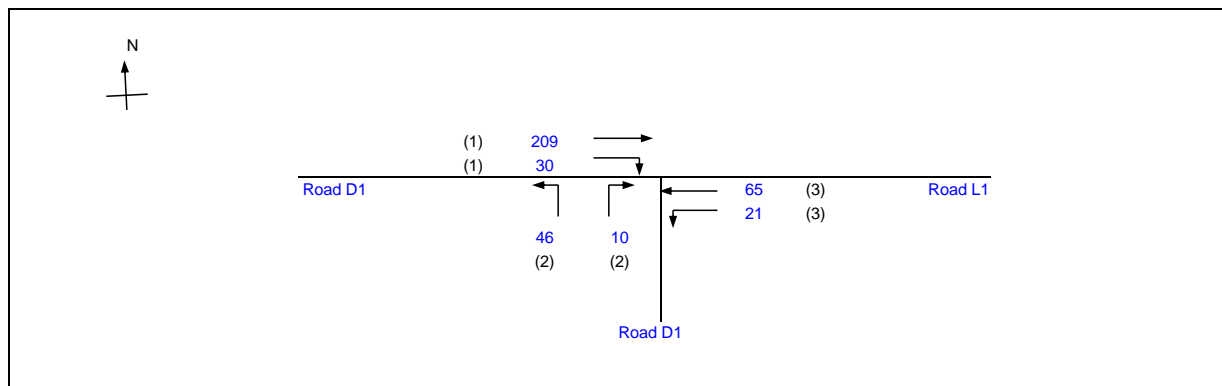
2034 Reference PM Peak Hour Traffic Flows

J4 Road D1 Road L1 S.xls

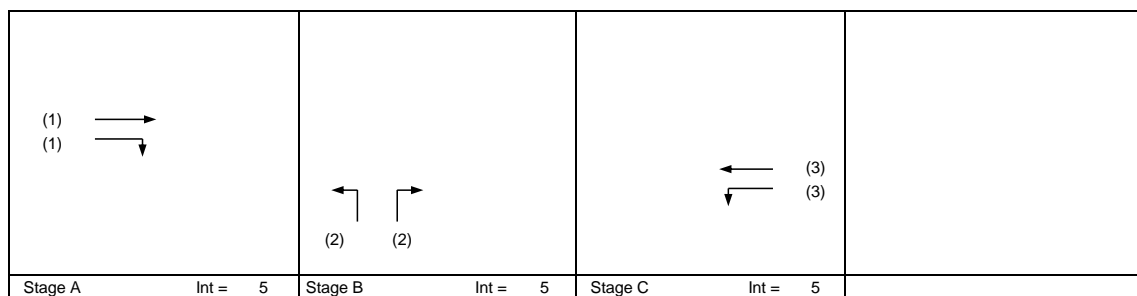
Reviewed By:

	SC
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Mar-25



		Existing Cycle Time	
No. of stages per cycle	N =	3	
Cycle time	C =	90 sec	
Sum(y)	Y =	0.185	
Loss time	L =	24 sec	
Total Flow	=	381 pcu	
Co	= $(1.5*L+5)/(1-Y)$	50.3 sec	
Cm	= $L/(1-Y)$	29.4 sec	
Yult	=	0.720	
R.C.ult	= $(Yult-Y)/Y*100\%$	289.5 %	
Cp	= $0.9*L/(0.9-Y)$	30.2 sec	
Ymax	= $1-L/C$	0.733	
R.C.(C)	= $(0.9*Ymax-Y)/Y*100\%$	257.1 %	



Pedestrian Phase	Stage	Length (m)	Green Time Required (s)			Green Time Provided (s)	
			SG	FG	Delay	SG	FG

Move- ment	Stage	Lane Width m.	Phase	No. of lane	Radius m.	O	N	Straight- Ahead Sat. Flow	Movement			Total FLOw pcu/h	Proportion of Turning Vehicles	Sat. Flow pcu/h	Flare lane Length m.	Share Effect pcu/hr	Revised Sat. Flow pcu/h	y	Greater y	L sec	g (required) sec	g (input) sec	Degree of Saturation X	Queue Length (m / lane)	Average Delay (seconds)
									Left pcu/h	Straight pcu/h	Right pcu/h														
SA,RT	A	5.00	1	1	20		N	2115		209	30	239	0.13	2095			2095	0.114	0.114	12	41	41	0.252	18	14
LT,RT	B	5.00	2	1	15		N	2115	46		10	56	1.00	1923			1923	0.029	0.029		10	10	0.252	6	35
LT,SA	C	5.00	3	1	15		N	2115	21	65		86	0.24	2065			2065	0.042	0.042		15	15	0.252	6	31
PED	D																		12						

NOTE : O - OPPOSING TRAFFIC

N - NEAR SIDE LANE

SG - STEADY GREEN

FG - FLASHING GREEN

PEDESTRIAN WALKING SPEED = 1.2m/s

QUEUING LENGTH = AVERAGE QUEUE * 6m

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE _____

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

PROJECT NO.	81838
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Prepared By:

	CW
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Mar-25

J4: Road D1 / Road L1

2034 Des AM

FILENAME :	
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Prepared By:	
Checked By:	

	DP
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Mar-25

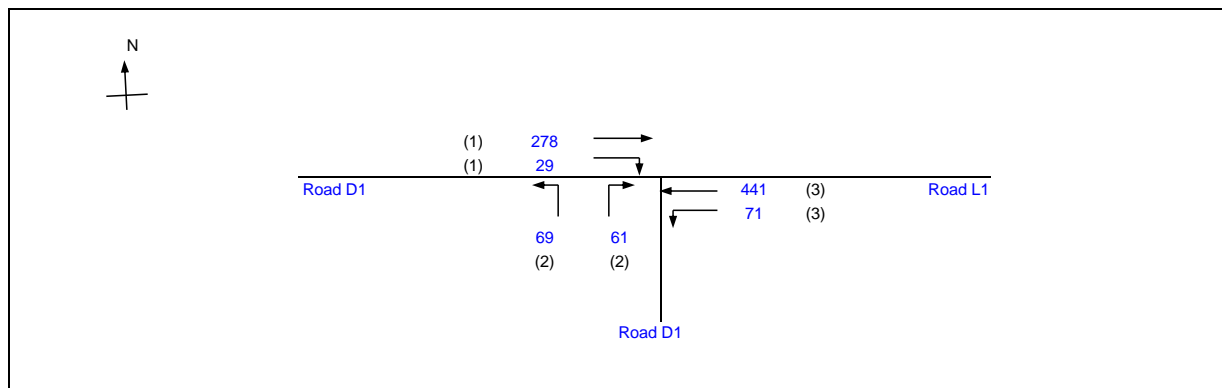
2034 Design AM Peak Hour Traffic Flows

J4 Road D1 Road L1 S.xls

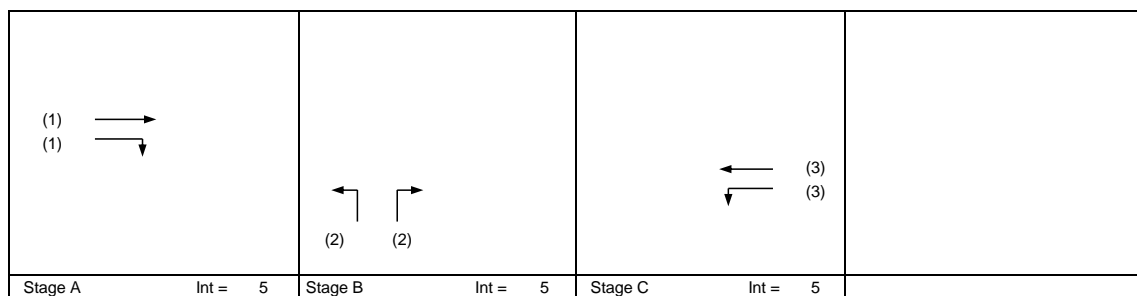
Reviewed By:

	SC
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Mar-25



		Existing Cycle Time	
No. of stages per cycle	N =	3	
Cycle time	C =	90 sec	
Sum(y)	Y =	0.459	
Loss time	L =	24 sec	
Total Flow	=	949 pcu	
Co	= $(1.5 \cdot L + 5) / (1 - Y)$	75.8 sec	
Cm	= $L / (1 - Y)$	44.4 sec	
Yult	=	0.720	
R.C.ult	= $(Yult - Y) / Y \cdot 100\%$	56.8 %	
Cp	= $0.9 \cdot L / (0.9 - Y)$	49.0 sec	
Ymax	= $1 - L / C$	0.733	
R.C.(C)	= $(0.9 \cdot Ymax - Y) / Y \cdot 100\%$	43.7 %	



Pedestrian Phase	Stage	Length (m)	Green Time Required (s)			Green Time Provided (s)	
			SG	FG	Delay	SG	FG

Move- ment	Stage	Lane Width m.	Phase	No. of lane	Radius m.	O	N	Straight- Ahead Sat. Flow	Movement			Total FLOw pcu/h	Proportion of Turning Vehicles	Sat. Flow pcu/h	Flare lane Length m.	Share Effect pcu/hr	Revised Sat. Flow pcu/h	y	Greater y	L sec	g (required) sec	g (input) sec	Degree of Saturation X	Queue Length (m / lane)	Average Delay (seconds)
									Left pcu/h	Straight pcu/h	Right pcu/h														
SA,RT	A	5.00	1	1	20		N	2115		278	29	307	0.09	2100			2100	0.146	0.146	12	21	21	0.626	30	33
LT,RT	B	5.00	2	1	15		N	2115	69		61	130	1.00	1923			1923	0.068	0.068		10	10	0.626	18	48
LT,SA	C	5.00	3	1	15		N	2115	71	441		512	0.14	2086			2086	0.245	0.245		35	35	0.626	42	23
PED	D																		12						

NOTE : O - OPPOSING TRAFFIC

N - NEAR SIDE LANE

SG - STEADY GREEN

FG - FLASHING GREEN

PEDESTRIAN WALKING SPEED = 1.2m/s

QUEUING LENGTH = AVERAGE QUEUE * 6m

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE _____

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung North

PROJECT NO.	81838
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Prepared By:

	CW
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Mar-25

J4: Road D1 / Road L1

2034 Des PM

FILENAME :	
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Prepared By:	
Checked By:	

	DP
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Mar-25

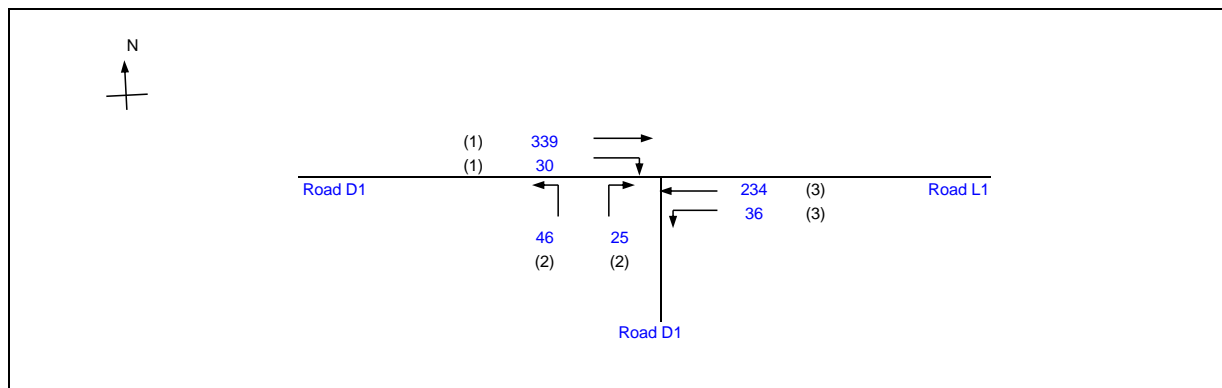
2034 Design PM Peak Hour Traffic Flows

J4 Road D1 Road L1 S.xls




Reviewed By:

	SC
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Mar-25



		Existing Cycle Time	
No. of stages per cycle	N =	3	
Cycle time	C =	90 sec	
Sum(y)	Y =	0.342	
Loss time	L =	24 sec	
Total Flow	=	710 pcu	
Co	= $(1.5 \cdot L + 5) / (1 - Y)$	62.3 sec	
Cm	= $L / (1 - Y)$	36.5 sec	
Yult	=	0.720	
R.C.ult	= $(Yult - Y) / Y \cdot 100\%$	110.6 %	
Cp	= $0.9 \cdot L / (0.9 - Y)$	38.7 sec	
Ymax	= $1 - L/C$	0.733	
R.C.(C)	= $(0.9 \cdot Ymax - Y) / Y \cdot 100\%$	93.1 %	

			
Stage A Int = 5	Stage B Int = 5	Stage C Int = 5	

Pedestrian Phase	Stage	Length (m)	Green Time Required (s)			Green Time Provided (s)	
			SG	FG	Delay	SG	FG

Move- ment	Stage	Lane Width m.	Phase	No. of lane	Radius m.	O	N	Straight- Ahead Sat. Flow	Movement			Total FLOw pcu/h	Proportion of Turning Vehicles	Sat. Flow pcu/h	Flare lane Length m.	Share Effect pcu/hr	Revised Sat. Flow pcu/h	y	Greater y	L sec	g (required) sec	g (input) sec	Degree of Saturation X	Queue Length (m / lane)	Average Delay (seconds)
									Left pcu/h	Straight pcu/h	Right pcu/h														
SA,RT	A	5.00	1	1	20		N	2115		339	30	369	0.08	2102			2102	0.176	0.176	12	34	34	0.466	30	21
LT,RT	B	5.00	2	1	15		N	2115	46		25	71	1.00	1923			1923	0.037	0.037		7	7	0.466	6	45
LT,SA	C	5.00	3	1	15		N	2115	36	234		270	0.13	2087			2087	0.129	0.129		25	25	0.466	24	27
PED	D																		12						

NOTE : O - OPPOSING TRAFFIC

N - NEAR SIDE LANE

SG - STEADY GREEN

FG - FLASHING GREEN

PEDESTRIAN WALKING SPEED = 1.2m/s

QUEUING LENGTH = AVERAGE QUEUE * 6m

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Ref AM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J5: Road P1 / Road D1 / Slip Road to Fanling Hwy

FILENAME :

CHECKED BY: DP

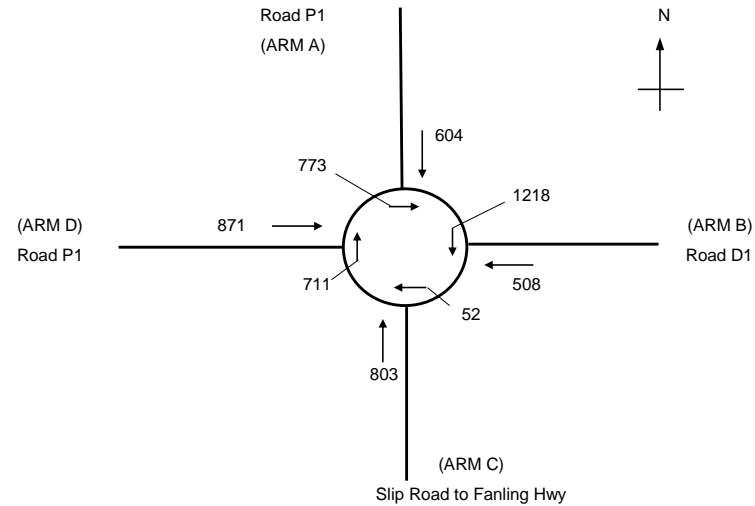
Mar-25

2034 Reference AM Peak Hour Traffic Flows

J5_Road P1_Road D1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	6.8	4.0	7.3
E = Entry width (m)	25.0	19.0	19.0	19.0
L = Effective length of flare (m)	20.0	40.0	20.0	20.0
R = Entry radius (m)	20.0	30.0	100.0	40.0
D = Inscribed circle diameter (m)	80.0	80.0	80.0	80.0
A = Entry angle (degree)	60.0	40.0	20.0	40.0
Q = Entry flow (pcu/h)	604	508	803	871
Qc = Circulating flow across entry (pcu/h)	773	1218	52	711
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	1.42	0.49	1.20	0.94
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.90	0.98	1.07	0.99
X2 = $V + ((E-V)/(1+2S))$	11.92	12.97	8.41	11.37
M = $EXP((D-60)/10)$	7	7	7	7
F = $303 \times X2$	3611	3931	2549	3446
Td = $1+(0.5/(1+M))$	1.06	1.06	1.06	1.06
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.75	0.80	0.60	0.73
Qe = $K(F-Fc \times Qc)$	2714	2902	2704	2898
				Total In Sum =
				2786 PCU
DFC = Design flow/Capacity = Q/Qe	0.22	0.18	0.30	0.30
				DFC of Critical Approach =
				0.30

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Ref PM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J5: Road P1 / Road D1 / Slip Road to Fanling Hwy

FILENAME :

CHECKED BY: DP

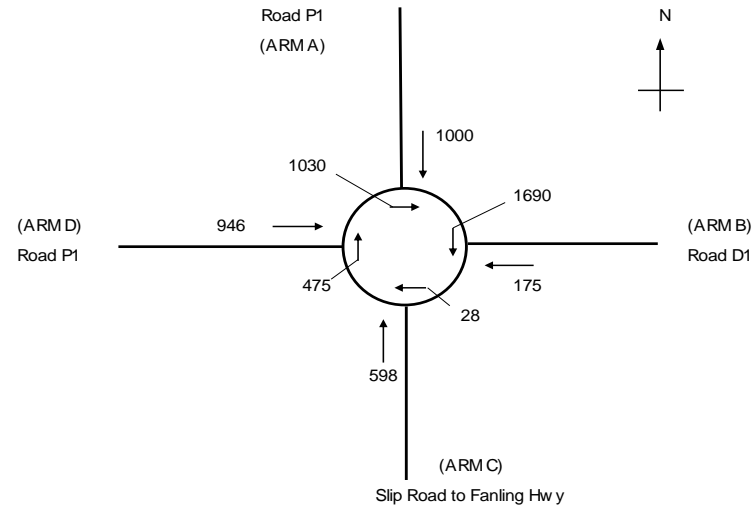
Mar-25

2034 Reference PM Peak Hour Traffic Flows

J5_Road P1_Road D1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	6.8	4.0	7.3
E = Entry width (m)	25.0	19.0	19.0	19.0
L = Effective length of flare (m)	20.0	40.0	20.0	20.0
R = Entry radius (m)	20.0	30.0	100.0	40.0
D = Inscribed circle diameter (m)	80.0	80.0	80.0	80.0
A = Entry angle (degree)	60.0	40.0	20.0	40.0
Q = Entry flow (pcu/h)	1000	175	598	946
Qc = Circulating flow across entry (pcu/h)	1030	1690	28	475
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	1.42	0.49	1.20	0.94
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.90	0.98	1.07	0.99
X2 = $V + ((E-V)/(1+2S))$	11.92	12.97	8.41	11.37
M = $EXP((D-60)/10)$	7	7	7	7
F = $303 \times X2$	3611	3931	2549	3446
Td = $1+(0.5/(1+M))$	1.06	1.06	1.06	1.06
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.75	0.80	0.60	0.73
Qe = $K(F-Fc \times Qc)$	2541	2532	2719	3068
				Total In Sum = 2719 PCU
DFC = Design flow/Capacity = Q/Qe	0.39	0.07	0.22	0.31
				DFC of Critical Approach = 0.39

OZZO TECHNOLOGY (HK) LIMITED

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2034 Des AM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J5: Road P1 / Road D1 / Slip Road to Fanling Hwy

FILENAME :

CHECKED BY: DP

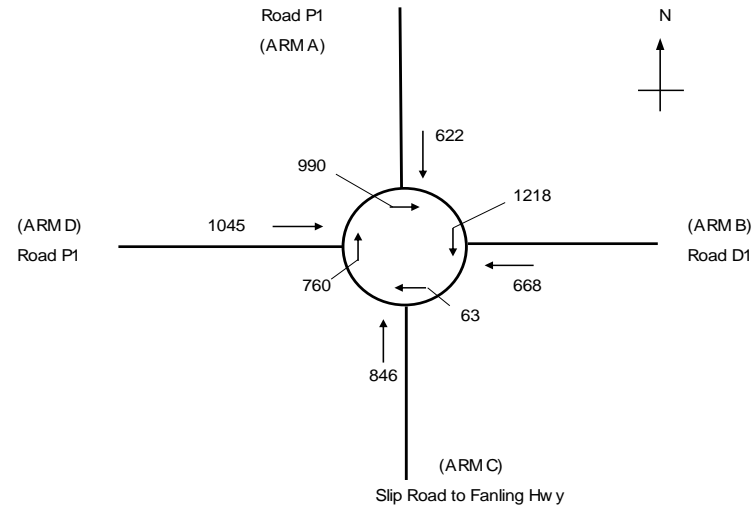
Mar-25

2034 Design AM Peak Hour Traffic Flows

J5_Road P1_Road D1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	6.8	4.0	7.3
E = Entry width (m)	25.0	19.0	19.0	19.0
L = Effective length of flare (m)	20.0	40.0	20.0	20.0
R = Entry radius (m)	20.0	30.0	100.0	40.0
D = Inscribed circle diameter (m)	80.0	80.0	80.0	80.0
A = Entry angle (degree)	60.0	40.0	20.0	40.0
Q = Entry flow (pcu/h)	622	668	846	1045
Qc = Circulating flow across entry (pcu/h)	990	1218	63	760
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	1.42	0.49	1.20	0.94
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.90	0.98	1.07	0.99
X2 = $V + ((E-V)/(1+2S))$	11.92	12.97	8.41	11.37
M = $EXP((D-60)/10)$	7	7	7	7
F = $303 \times X2$	3611	3931	2549	3446
Td = $1+(0.5/(1+M))$	1.06	1.06	1.06	1.06
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.75	0.80	0.60	0.73
Qe = $K(F-Fc \times Qc)$	2568	2902	2697	2863
Total In Sum =				3181 PCU
DFC = Design flow/Capacity = Q/Qe	0.24	0.23	0.31	0.37
DFC of Critical Approach =				0.37

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

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2034 Des PM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J5: Road P1 / Road D1 / Slip Road to Fanling Hwy

FILENAME :

CHECKED BY: DP

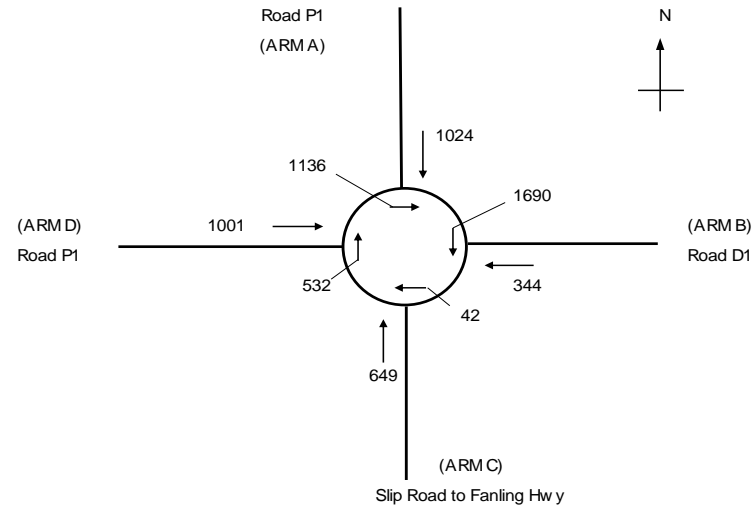
Mar-25

2034 Design PM Peak Hour Traffic Flows

J5_Road P1_Road D1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	6.8	4.0	7.3
E = Entry width (m)	25.0	19.0	19.0	19.0
L = Effective length of flare (m)	20.0	40.0	20.0	20.0
R = Entry radius (m)	20.0	30.0	100.0	40.0
D = Inscribed circle diameter (m)	80.0	80.0	80.0	80.0
A = Entry angle (degree)	60.0	40.0	20.0	40.0
Q = Entry flow (pcu/h)	1024	344	649	1001
Qc = Circulating flow across entry (pcu/h)	1136	1690	42	532
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	1.42	0.49	1.20	0.94
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.90	0.98	1.07	0.99
X2 = $V + ((E-V)/(1+2S))$	11.92	12.97	8.41	11.37
M = $EXP((D-60)/10)$	7	7	7	7
F = $303 \times X2$	3611	3931	2549	3446
Td = $1+(0.5/(1+M))$	1.06	1.06	1.06	1.06
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.75	0.80	0.60	0.73
Qe = $K(F-Fc \times Qc)$	2469	2532	2710	3027
				Total In Sum = 3018 PCU
DFC = Design flow/Capacity = Q/Qe	0.41	0.14	0.24	0.33
				DFC of Critical Approach = 0.41

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Ref AM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J6: Castle Peak Road (Chau Tau) / Road P1

FILENAME :

CHECKED BY: DP

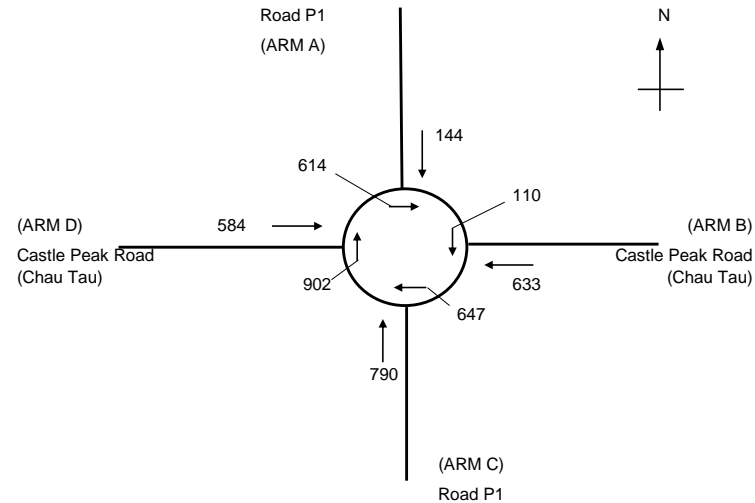
Mar-25

2034 Reference AM Peak Hour Traffic Flows

J6_Castle Peak Road (Chau Tau)_Road P1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	4.5	6.8	6.0
E = Entry width (m)	11.5	12.0	10.0	14.0
L = Effective length of flare (m)	20.0	35.0	15.0	35.0
R = Entry radius (m)	30.0	50.0	20.0	100.0
D = Inscribed circle diameter (m)	45.0	45.0	45.0	45.0
A = Entry angle (degree)	40.0	30.0	30.0	20.0
Q = Entry flow (pcu/h)	144	633	790	584
Qc = Circulating flow across entry (pcu/h)	614	110	647	902
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	0.34	0.34	0.34	0.37
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.98	1.03	1.00	1.07
X2 = $V + ((E-V)/(1+2S))$	9.81	8.95	8.70	10.62
M = $EXP((D-60)/10)$	0	0	0	0
F = $303 \times X2$	2973	2712	2637	3218
Td = $1+(0.5/(1+M))$	1.41	1.41	1.41	1.41
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.88	0.83	0.81	0.92
Qe = $K(F-Fc \times Qc)$	2390	2698	2112	2560
Total In Sum = 2151 PCU				
DFC = Design flow/Capacity = Q/Qe	0.06	0.23	0.37	0.23
DFC of Critical Approach = 0.37				

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Ref PM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J6: Castle Peak Road (Chau Tau) / Road P1

FILENAME :

CHECKED BY: DP

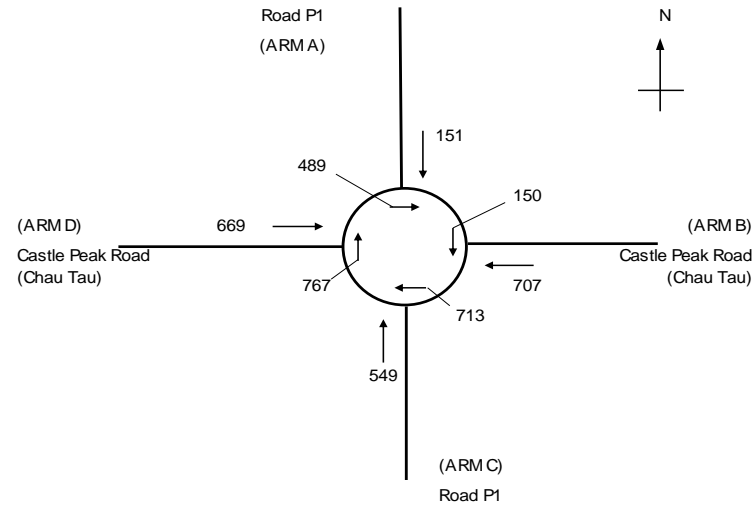
Mar-25

2034 Reference PM Peak Hour Traffic Flows

J6_Castle Peak Road (Chau Tau)_Road P1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	4.5	6.8	6.0
E = Entry width (m)	11.5	12.0	10.0	14.0
L = Effective length of flare (m)	20.0	35.0	15.0	35.0
R = Entry radius (m)	30.0	50.0	20.0	100.0
D = Inscribed circle diameter (m)	45.0	45.0	45.0	45.0
A = Entry angle (degree)	40.0	30.0	30.0	20.0
Q = Entry flow (pcu/h)	151	707	549	669
Qc = Circulating flow across entry (pcu/h)	489	150	713	767
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	0.34	0.34	0.34	0.37
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.98	1.03	1.00	1.07
X2 = $V + ((E-V)/(1+2S))$	9.81	8.95	8.70	10.62
M = $EXP((D-60)/10)$	0	0	0	0
F = $303 \times X2$	2973	2712	2637	3218
Td = $1+(0.5/(1+M))$	1.41	1.41	1.41	1.41
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.88	0.83	0.81	0.92
Qe = $K(F-Fc \times Qc)$	2498	2664	2059	2694
Total In Sum =				2076 PCU
DFC = Design flow/Capacity = Q/Qe	0.06	0.27	0.27	0.25
DFC of Critical Approach =				0.27

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Des AM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J6: Castle Peak Road (Chau Tau) / Road P1

FILENAME :

CHECKED BY: DP

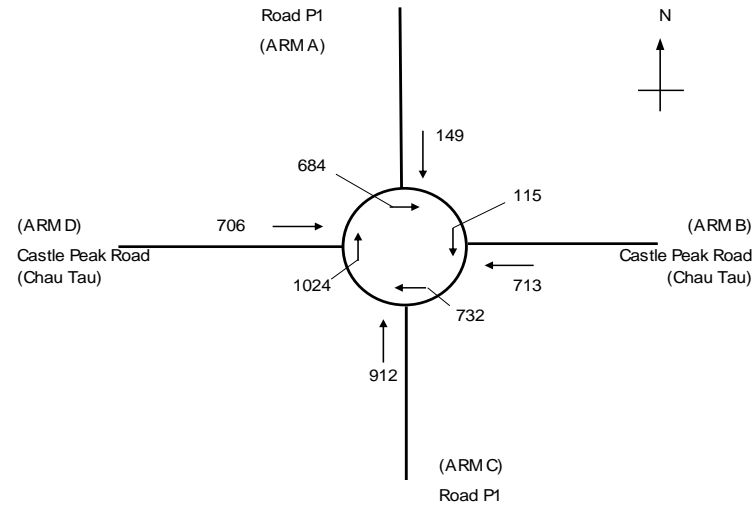
Mar-25

2034 Design AM Peak Hour Traffic Flows

J6_Castle Peak Road (Chau Tau)_Road P1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	4.5	6.8	6.0
E = Entry width (m)	11.5	12.0	10.0	14.0
L = Effective length of flare (m)	20.0	35.0	15.0	35.0
R = Entry radius (m)	30.0	50.0	20.0	100.0
D = Inscribed circle diameter (m)	45.0	45.0	45.0	45.0
A = Entry angle (degree)	40.0	30.0	30.0	20.0
Q = Entry flow (pcu/h)	149	713	912	706
Qc = Circulating flow across entry (pcu/h)	684	115	732	1024
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	0.34	0.34	0.34	0.37
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.98	1.03	1.00	1.07
X2 = $V + ((E-V)/(1+2S))$	9.81	8.95	8.70	10.62
M = $EXP((D-60)/10)$	0	0	0	0
F = $303 \times X2$	2973	2712	2637	3218
Td = $1+(0.5/(1+M))$	1.41	1.41	1.41	1.41
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.88	0.83	0.81	0.92
Qe = $K(F-Fc \times Qc)$	2330	2693	2043	2439
				Total In Sum =
				2480 PCU
DFC = Design flow/Capacity = Q/Qe	0.06	0.26	0.45	0.29
				DFC of Critical Approach =
				0.45

OZZO TECHNOLOGY (HK) LIMITED

TRAFFIC SIGNAL CALCULATION

INITIALS

DATE

Contract No ASD 1018705CX06QC6 S16 for JUC & JUB at Area 29, Kwu Tung

2034 Des PM

PROJECT NO.: 83114

PREPARED BY: CW

Mar-25

J6: Castle Peak Road (Chau Tau) / Road P1

FILENAME :

CHECKED BY: DP

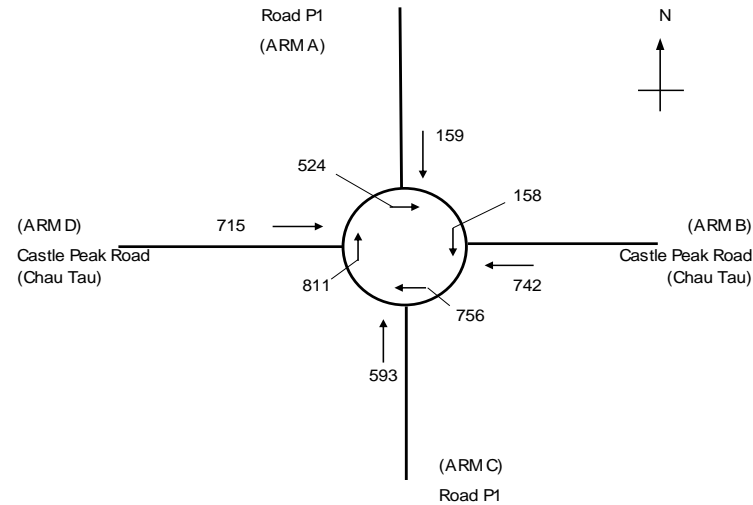
Mar-25

2034 Design PM Peak Hour Traffic Flows

J6_Castle Peak Road (Chau Tau)_Road P1_R.xls

REVIEWED BY: OC

Mar-25



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	7.3	4.5	6.8	6.0
E = Entry width (m)	11.5	12.0	10.0	14.0
L = Effective length of flare (m)	20.0	35.0	15.0	35.0
R = Entry radius (m)	30.0	50.0	20.0	100.0
D = Inscribed circle diameter (m)	45.0	45.0	45.0	45.0
A = Entry angle (degree)	40.0	30.0	30.0	20.0
Q = Entry flow (pcu/h)	159	742	593	715
Qc = Circulating flow across entry (pcu/h)	524	158	756	811
OUTPUT PARAMETERS:				
S = Sharpness of flare = $1.6(E-V)/L$	0.34	0.34	0.34	0.37
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.98	1.03	1.00	1.07
X2 = $V + ((E-V)/(1+2S))$	9.81	8.95	8.70	10.62
M = $EXP((D-60)/10)$	0	0	0	0
F = $303 \times X2$	2973	2712	2637	3218
Td = $1+(0.5/(1+M))$	1.41	1.41	1.41	1.41
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.88	0.83	0.81	0.92
Qe = $K(F-Fc \times Qc)$	2468	2657	2024	2651
				Total In Sum =
				2209 PCU
DFC = Design flow/Capacity = Q/Qe	0.06	0.28	0.29	0.27
				DFC of Critical Approach =
				0.29