Annex 3 Drainage Impact Assessment



Drainage Impact Assessment

DEC 24

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Drainage Impact Assessment

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1 Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Various Lot in D.D. 76 and Adjoining Government Land (GL), Hok Tau, Fanling, New Territories (the Site) for 'Proposed Temporary Place of Recreation, Sports or Culture (Horse Riding Centre and Barbecue Site) and Holiday Camp with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land'.
- 1.1.2 This report aims to support the development in drainage aspect.

1.2 Application Site

- 1.2.1 The application site is situated beside Hok Tau Road. It has an area of approx. 19,227 m². The site location is shown in **Figure 1**.
- 1.2.2 The existing site is partially hard paved with level various from approx. +24.5mPD to + 28,5mPD.
- 1.2.3 There is an existing stream at the southwest of the application site, which would eventually discharge to Shan Pui River. Figure 2 indicate the existing drainage system of the area.

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2 Development Proposal

2.1 The Proposed Development

- 2.1.1 The total site area is approximately 19,227 m². The Proposed Site would be used for recreation, sports and cultural activities. Part of the site would be paved with concrete not more than 0.2m for site formation of structures and circulation space. The existing pavement area and proposed catchment plan are shown in **Figure 4-1** and **Figure 4-2** respectively.
- 2.1.2 After the proposed development the paved area would be reduced from 5,896 m² to 5095 m².

	Before Development	After Development
Total Site Area (m²)	19,227	19,227
Paved Area (m²)	5,896	5,095

Table 1 - Site Development Area

3 Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2- Design Return Periods under SDM

3.1.2 The proposed drainage system intended to collect runoff from internal site and external catchment.1 in 10 years return period is adopted for the drainage design.

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- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the North District Zone. Therefore, for 10 years return period, the following values are adopted.

a =
$$454.9$$

b = 3.44
c = 0.412

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278 \text{CiA}$

3. The run-off coefficient (C) of surface runoff are taken as follows:

Paved Area: C = 0.95
 Unpaved Area: C = 0.35

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

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Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the pipe flow (m/s)

S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V = velocity of the pipe flow (m/s)

 S_f = hydraulic gradient k_f = roughness value (m)

v = kinematics viscosity of fluid

D = pipe diameter (m) R = hydraulic radius (m)

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4 Proposed Drainage System

4.1. Proposed Channels

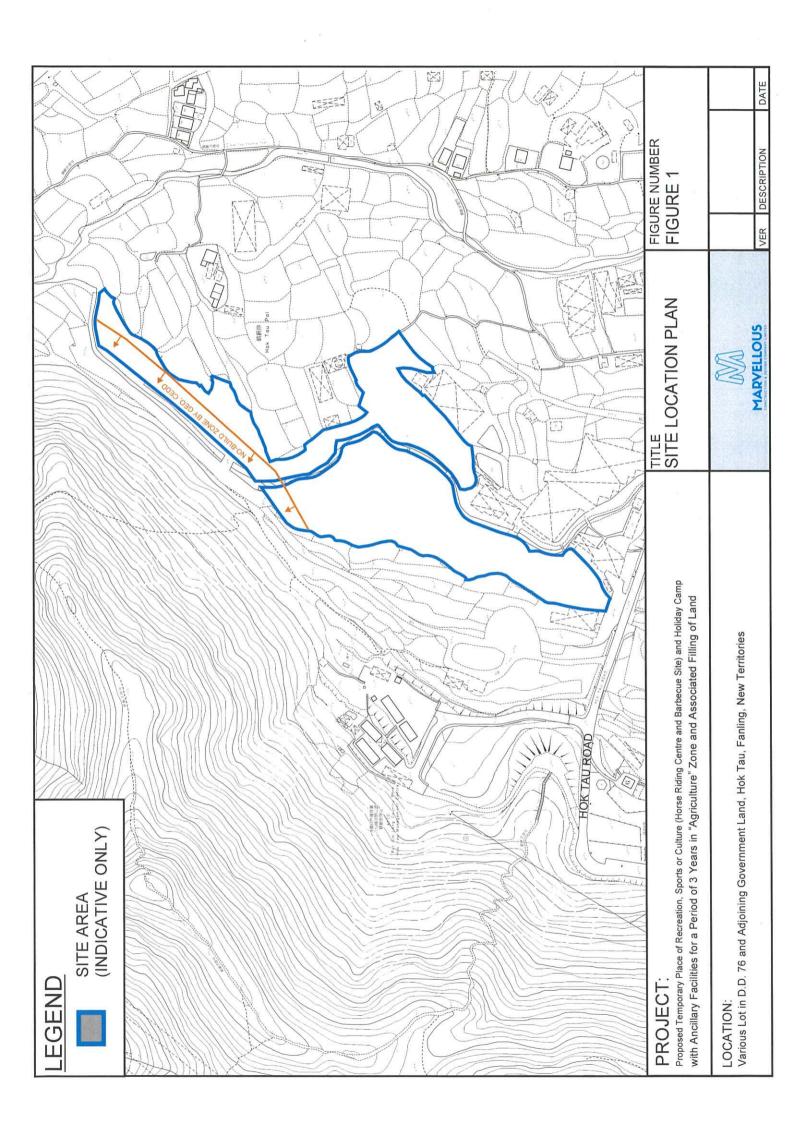
- 4.1.1 As the paved area is reduced from 5,896 m² to 5095 m² after the development, there is no additional runoff due to the proposed development.
- 4.1.2 Proposed channels are designed for collection of runoff for internal and external catchment. They are proposed to connect to existing stream adjacent to the application site.
- 4.1.3 The design calculations of proposed UChannel are shown in Appendix A.
- 4.1.4 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4-1** and **Figure 4-2**.
- 4.1.5 Reference Drawings are shown in **Appendix C** for reference.

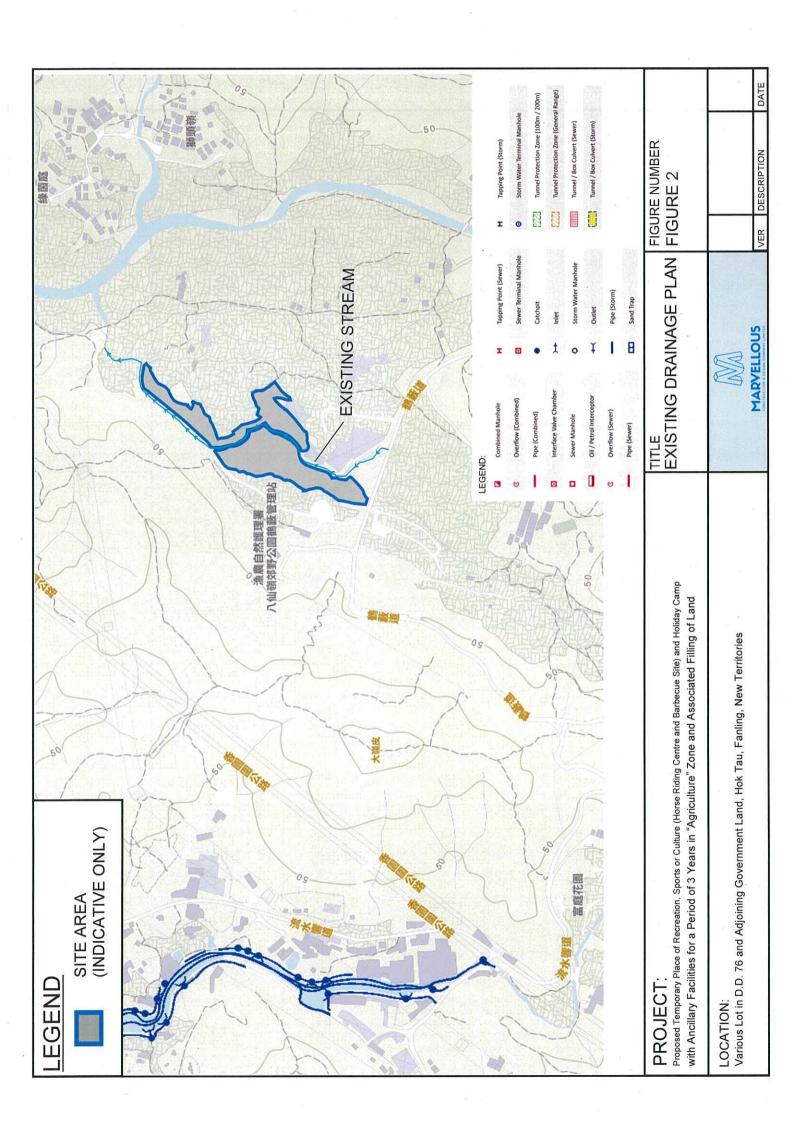
5 Conclusion

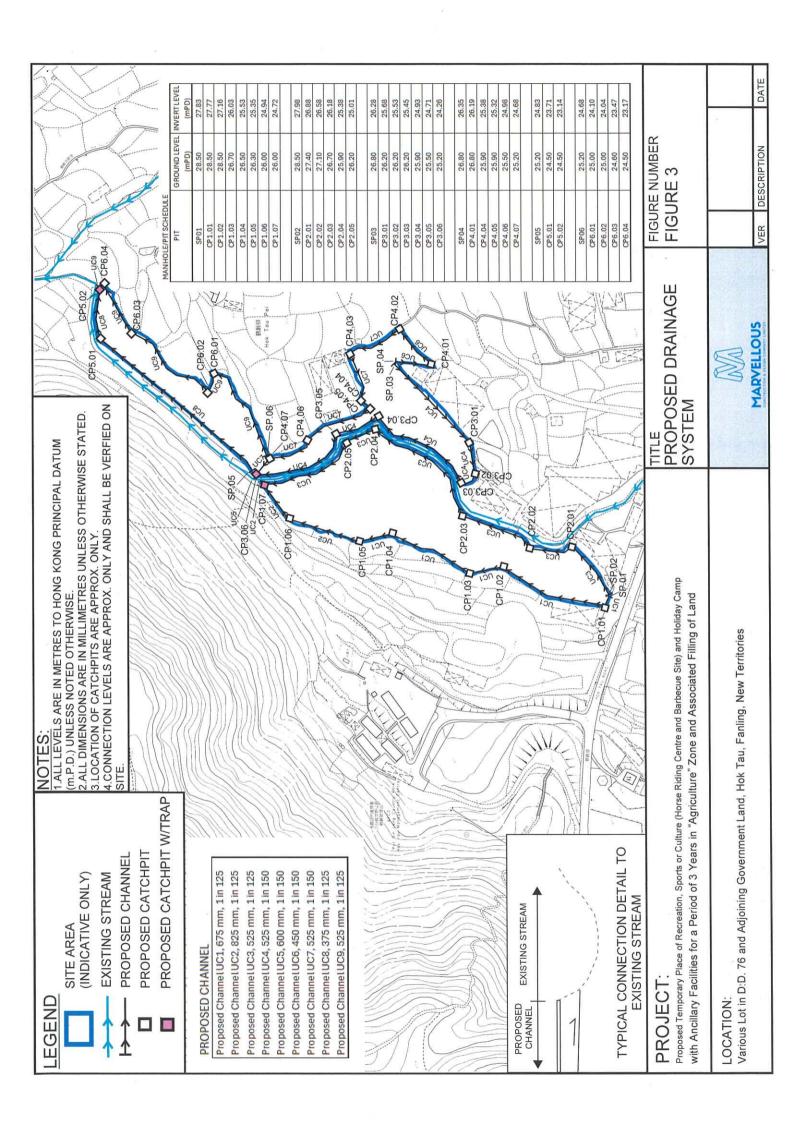
- 5.1.1 Drainage review has been conducted for the Proposed Development. U Channels are proposed to collect the runoff from the catchments. As the paved area is reduced after the development, there is no additional runoff due to the proposed development.
- 5.1.2 With implementation of the above drainage system, the no unacceptable drainage impact is anticipated.

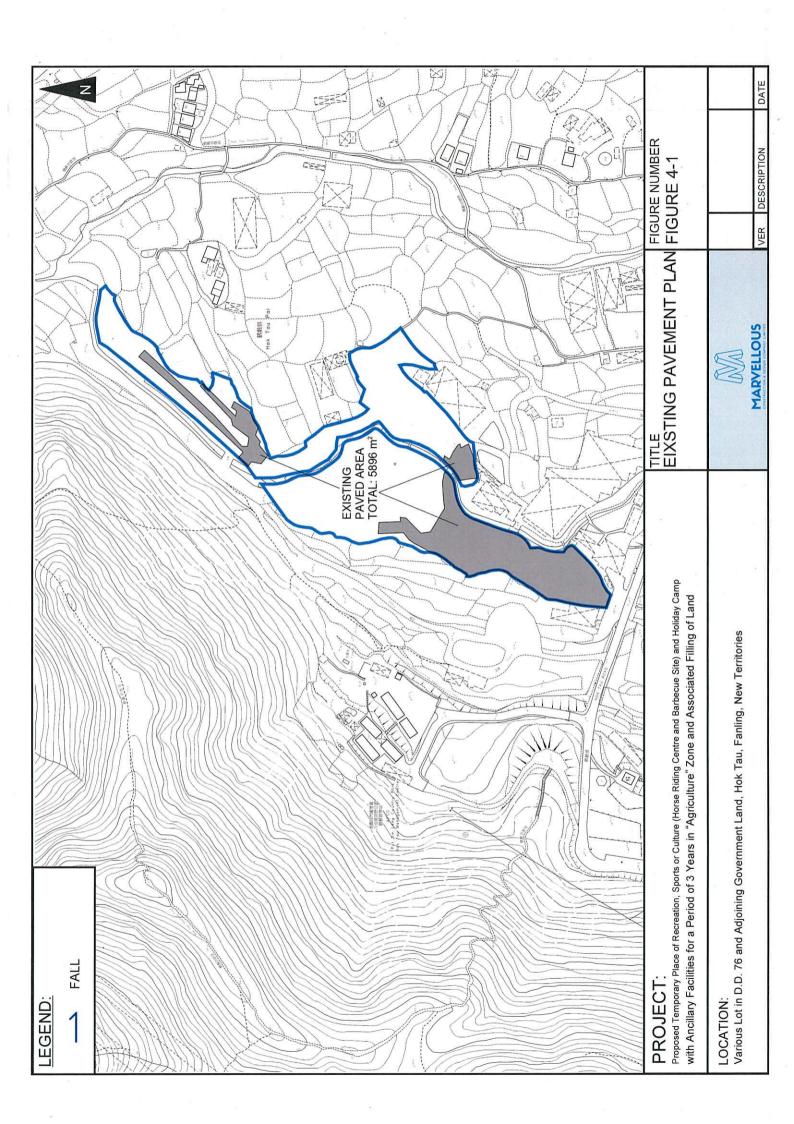
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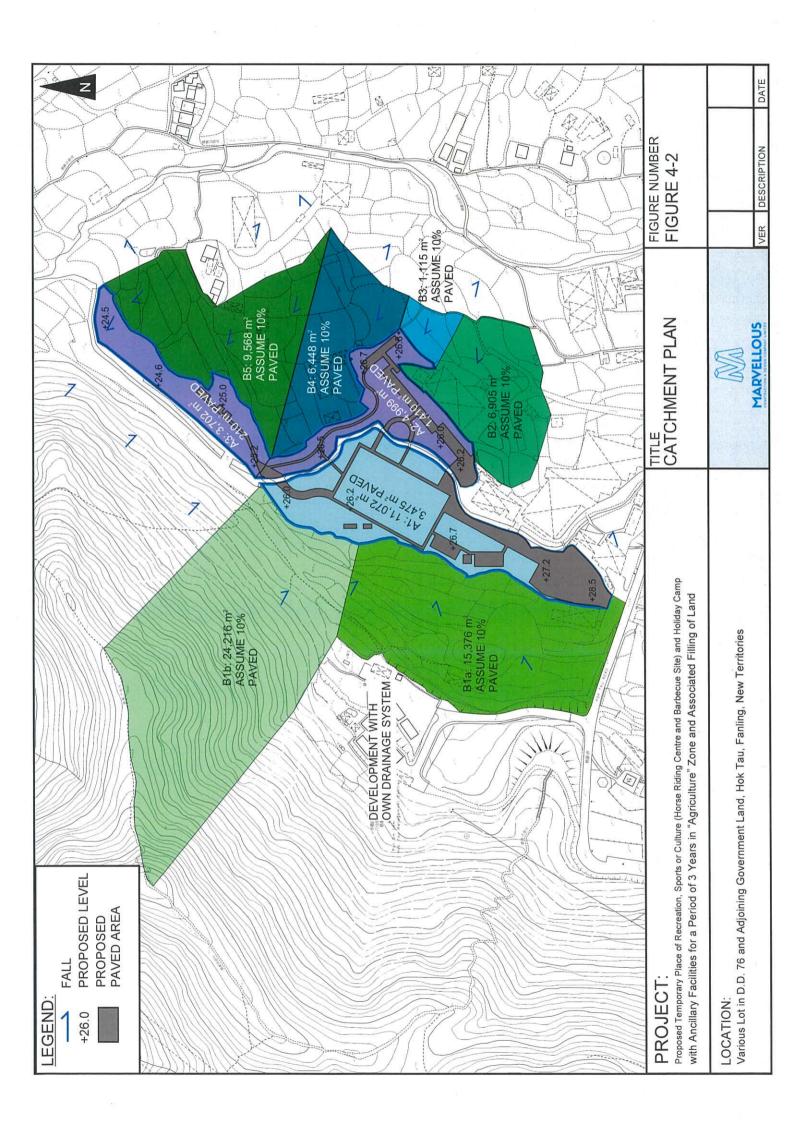
FIGURES











APPENDIX

Appendix A: Design Calculation

Zone								c	0.014			North District a	454.9	Time of Concentration Checking	ation Check	ing	
North District	Istrict		Batura Bariod	ri p	10	VPars		K	0.15		Storm	North District b	3.44	Catchment	Flow Distance	Flow Distance Highest Level Lowest Lev	Lowest
						2000	T		STATE OF THE PARTY		Constant		The second second	*	7	H	H
									Contractor	_			ALTO DESCRIPTION	(m2)	(144)	(Gdw)	(Gdm)
								Viscosity	0.000001			North District c	0.412	12036	75	36.7	28.5
Catchment Area Table (Area in m ²)	Ne (Area in m ²)									_						-	
			COLUMN TOWNS		8 65 January		S 100 6 4 1 2 3			Total Site Area Total Site Area	Total Site Area				いってい	4	
Catchment	. 41	থ	2	819	418	22	2	z	22	(Arter Development)	(Betone Development)			11			
Total Area	11,071	4,999	3,702	15,376	24,216	6,905	1,115	6,448	8956	19772	19772	THE POST OF THE PARTY OF THE PA			0		
Hard Paved Area	3,475	1,410	210	1,538	2,422	169	112	645	8995	2036.00	2696.00				S	X	1
Unpared Area	7,596	3,569	1,492	13,838	21,794	6,215	1,004	5,803	8611.2	14677.00	13876.00	No. of the last			(1)	1	
Foundament Areas	5.960	2,596	1,422	6,304	9,929	2,831	457	2,644	3622.88	9977.20	10457.80				17	1 11/1	

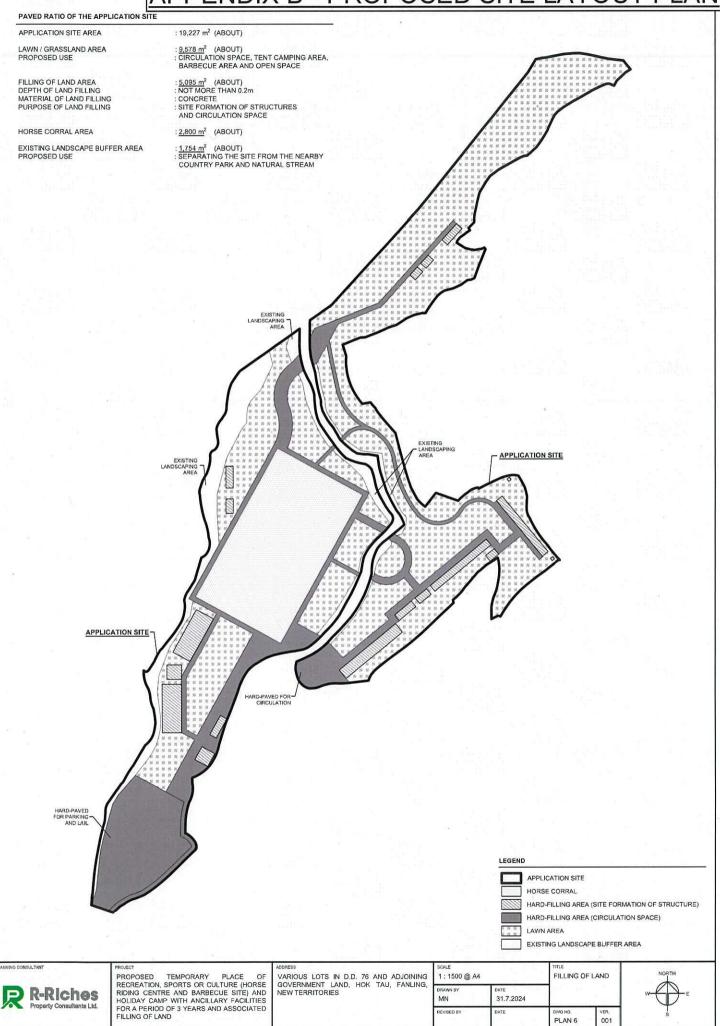
Davedun	0.35
Hard Paved	0.95
Pavement Type	Runoff Coefficient

DRAINAGE DESIGN

ltem	Equivalent Area m2	ToC	Intensity mm/hr	Discharge m3/s	SZS	Gradient 1 in	> 'E	Capacity m3/s	Utilitzation	Remark
	8		(2)	(a)			(4)	(5)	(9)	
Design of Channel UC1 for Catchment, A1,81a	12264	2.60	216.83	0.74	675	125	2.43	0.99	74.8%	
Design of Channel UC2 for Catchment, A1,81a,81b	22193	2.60	216.83	1.34	828	125	2.78	1.69	79.3%	
Design of Channel UC3 for Catchment, A1	2960	2.60	216.83	0.36	525	125	2.05	0.51	71.1%	
Design of Channel UC4 for Catchment, A2,82	5427	2.60	216.83	0.33	525	150	1.88	0.46	70.9%	
Design of Channel UC5 for Catchment, A2,82,83,84	8528	2.60	216.83	0.51	009	150	2.05	99'0	78.0%	
Design of Channel UC6 for Catchment, A2,83	3053	2.60	216.83	0.18	450	150	1.69	0,31	#Z'09	
Design of Channel UC7 for Catchment, A2,83,84	9696	2.60	216.83	0.34	525	150	1.88	0.46	74,4%	
Design of Channel UC8 for Catchment, A3	1422	2.60	216.83	60'0	375	125	164	0.21	41.6%	
Design of Channel UC9 for Catchment, A3,85	5345	2.60	216.83	0.32	525	125	2.05	0.51	82,7%	

1) Sum of Area in Catchment Table $2) \ i = \frac{\alpha}{(4x+b)^4}$ 3) 6.279. In termine, $(4x+b)^4$ 5) 6.275. In the form the Equation $(4x+b)^4$ 5) 6.274. So in the Figure (Equation Fig. Colections, White Equation (5) (4x+b) 4.075.

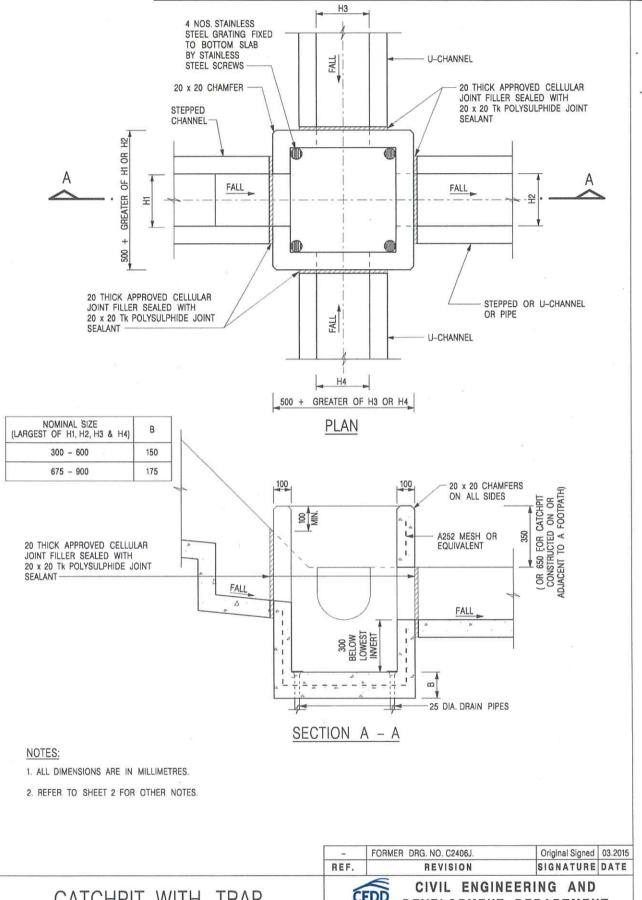
APPENDIX B - PROPOSED SITE LAYOUT PLAN



PLAN 6

001

Appendix C - Reference Drawings



CATCHPIT WITH TRAP (SHEET 1 OF 2)

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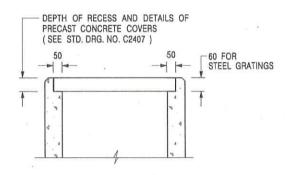
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 DRAWING NO.

DATE JAN 1991

C2406 /1

147 = 1



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

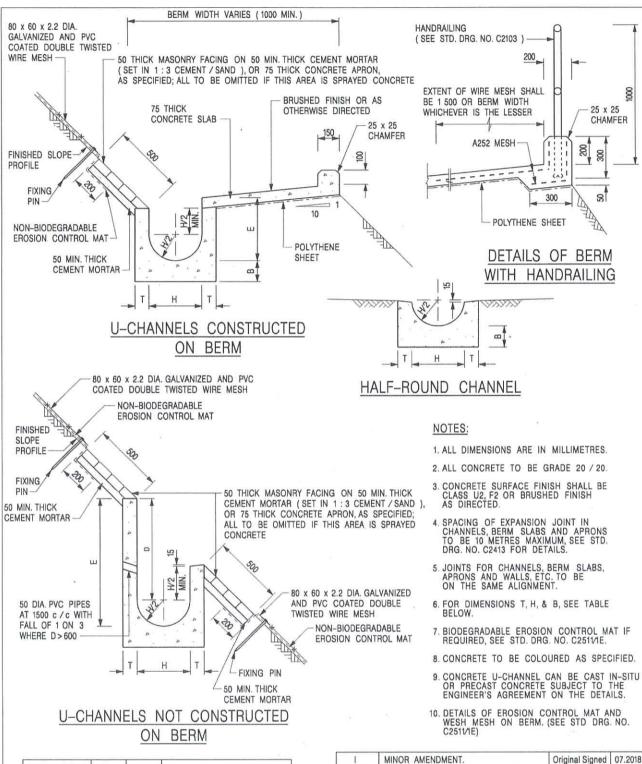
REF.	REVISION	SIGNATURE	DATE
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
Α '	MINOR AMENDMENT.	Original Signed	04.2016

CATCHPIT WITH TRAP (SHEET 2 OF 2) CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 DATE JAN 1991 C2406 /2A

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NOMINAL SIZE H	Т	В	REINFORCEMENT
300	80	100	A252 MESH PLACED
375 - 600	100	150	CENTRALLY AND T=100 WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
В	MINOR AMENDMENTS.	Original Signed	3.94
С	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
D	MINOR AMENDMENT.	Original Signed	08.2001
Е	DRAWING TITLE AMENDED.	Original Signed	11.2001
F	GENERAL REVISION.	Original Signed	12.2002
G	MINOR AMENDMENT.	Original Signed	01.2004
Н	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
- [MINOR AMENDMENT.	Original Signed	07.2018

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

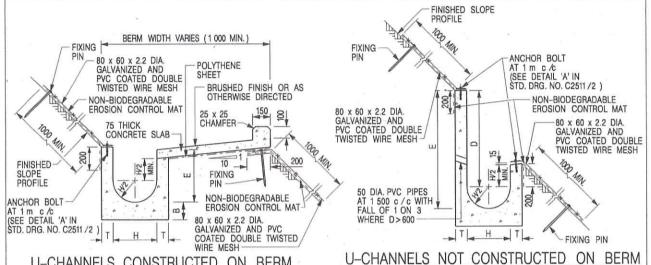
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:25
 DRAWING NO.

 DATE JAN 1991
 C24091

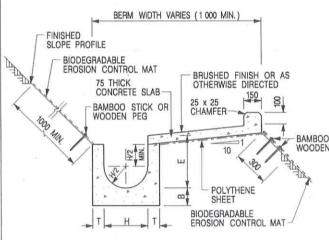
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U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

FINISHED SLOPE



U-CHANNELS CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT

PROFILE CO MA BIODEGRADABLE EROSION CONTROL MAT BAMBOO STICK OR WOODEN PEG ш BAMBOO STICK OR WOODEN PEG 50 DIA. PVC PIPES AT 1 500 c/c WITH FALL OF 1 ON 3 WHERE D>600 H JTL. BAMBOO STICK OR WOODEN PEG-

U-CHANNELS NOT CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE TO BE GRADE 20 /20.
- CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- 4. SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- 6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- 10. THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	Т	В	REINFORCEMENT
300	80	100	A252 MESH PLACED
375 - 600	100	150	CENTRALLY AND T=100 WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE B - WITH EROSION CONTROL MAT APRON)

ć	EDD	CIVIL	EI
REF.	REVISION		
Α	MINOR AMENDMENT.		

MINOR AMENDMENT.

MINOR AMENDMENT

GENERAL REVISION

MINOR AMENDMENT

MINOR AMENDMENT.

G

F

E

D

C

В

FIXING DETAILS OF BIODEGRADABLE

150 x 100 UPSTAND ADDED AT BERM

EROSION CONTROL MAT ADDED.

DIMENSION TABLE AMENDED

ENGINEERING AND EVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC DATE JAN 1991

DRAWING NO. C24101

Original Signed | 07.2018

Original Signed 01.2004

12.2017

01.2005

12.2002

08.2001

6.99

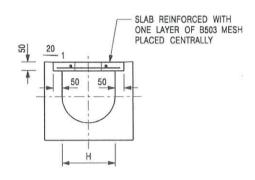
3.94

10.92

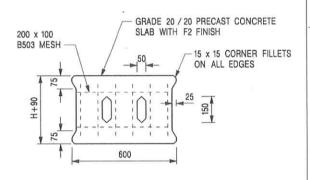
Original Signed

SIGNATURE DATE

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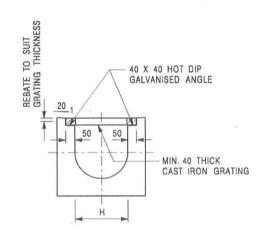
TYPICAL SECTION

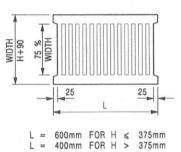


PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H. OF 525)





TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

REF.	REVISION	SIGNATURE	DATE
Α	CAST IRON GRATING AMENDED.	Original Signed	
В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	
D	NOTE 4 ADDED.	Original Signed	
Ε	NOTES 3 & 4 AMENDED.	Original Signed	

COVER SLAB AND CAST IRON GRATING FOR CHANNELS

CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:20
 DRAWING NO.

 DATE JAN 1991
 C2412E

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