Annex 1

Drainage Impact Assessment



Drainage Impact Assessment

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Drainage Impact Assessment

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1 Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Various Lot in D.D. 76 and Adjoining Government Land (GL), Hok Tau, Fanling, New Territories (the Site) for 'Proposed Temporary Place of Recreation, Sports or Culture (Horse Riding Centre and Barbecue Site) and Holiday Camp with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land'.
- 1.1.2 This report aims to support the development in drainage aspect.

1.2 Application Site

- 1.2.1 The application site is situated beside Hok Tau Road. It has an area of approx. 19,227 m². The site location is shown in **Figure 1**.
- 1.2.2 The existing site is partially hard paved with level various from approx. +24.5mPD to + 28.5mPD.
- 1.2.3 There is an existing stream at the south of the application site. The width of the stream various from approx. 5.4m to 9m. The stream pass through the site from south to north and eventually discharge to Tan Shan River at the north. Figure 2 shown the existing stream and existing drainage system of the area.

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2 Development Proposal

2.1 The Proposed Development

- 2.1.1 The total site area is approximately 19,227 m². The Proposed Site would be used for recreation, sports and cultural activities. Part of the site would be paved with concrete not more than 0.2m for site formation of structures and circulation space. The existing pavement area and proposed catchment plan are shown in **Figure 4-1** and **Figure 4-2** respectively.
- 2.1.2 After the proposed development the paved area would be reduced from 5,896 m² to 5095 m².

	Before Development	After Development
Total Site Area (m ²)	19,227	19,227
Paved Area (m ²)	5,896	5,095

Table 1 – Site Development Area

3 Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2– Design Return Periods under SDM

3.1.2 The application site is proposed for a period of 3 years in which it is within village area. The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 10 years return period is adopted for the drainage design.

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- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the North District Zone. Therefore, for 10 years return period, the following values are adopted.

а	=	454.9
b	=	3.44
с	=	0.412

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278$ CiA

where	Q_p	=	peak runoff in m³/s
	С	=	runoff coefficient (dimensionless)
	i	=	rainfall intensity in mm/hr
	А	=	catchment area in km ²

3. The run-off coefficient (C) of surface runoff are taken as follows:

1.	Paved Area:	C = 0.95
2.	Unpaved Area:	C = 0.35

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4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

- $\label{eq:V} \begin{array}{l} V = \mbox{velocity of the pipe flow (m/s)} \\ S_f = \mbox{hydraulic gradient} \\ n = \mbox{manning's coefficient} \end{array}$
- R = hydraulic radius (m)
- 5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:

$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V	=	velocity of the pipe flow (m/s)
S _f	=	hydraulic gradient
k_{f}	=	roughness value (m)
v	=	kinematics viscosity of fluid
D	=	pipe diameter (m)
R	=	hydraulic radius (m)

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4 Proposed Drainage System

4.1. Proposed Channels

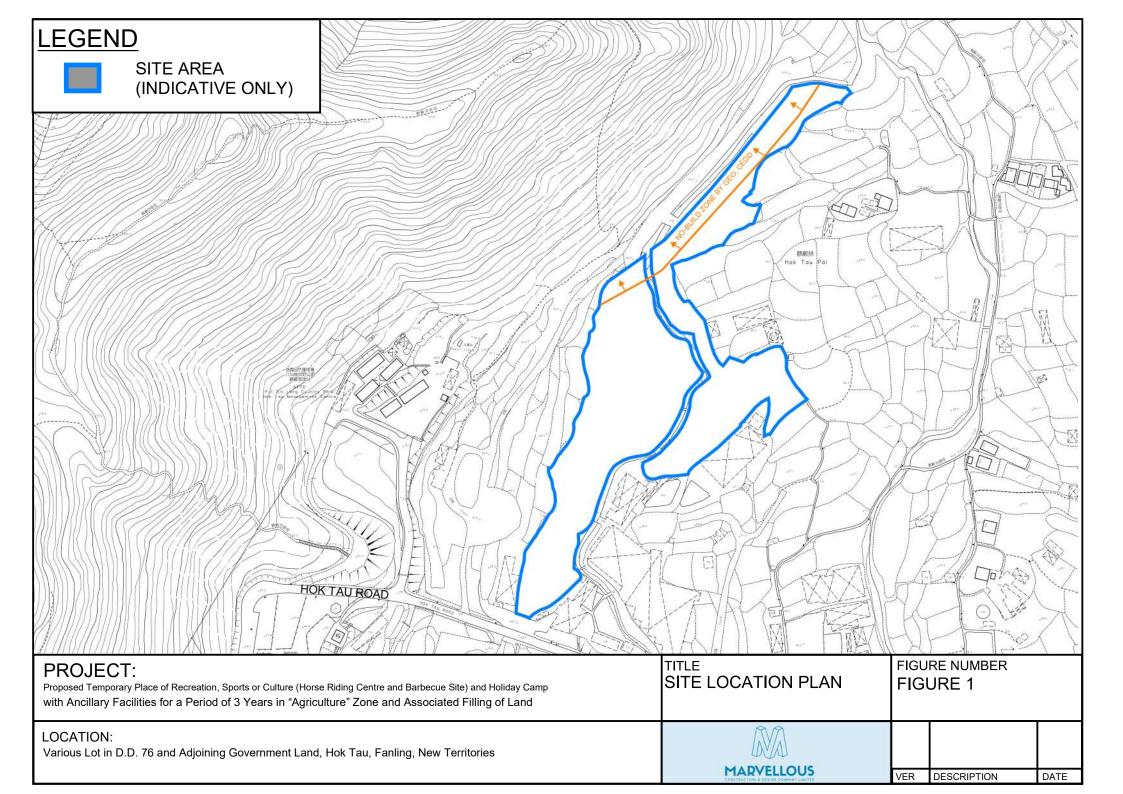
- 4.1.1 As the paved area is reduced from 5,896 m² to 5,095 m² after the development, there is no additional runoff due to the proposed development.
- 4.1.2 Proposed channels are designed for collection of runoff for internal and external catchment. They are proposed to connect to existing stream adjacent to the application site.
- 4.1.3 The design calculations of proposed UChannel are shown in **Appendix A**.
- 4.1.4 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4-1** and **Figure 4-2**.
- 4.1.5 Reference Drawings are shown in **Appendix C** for reference.

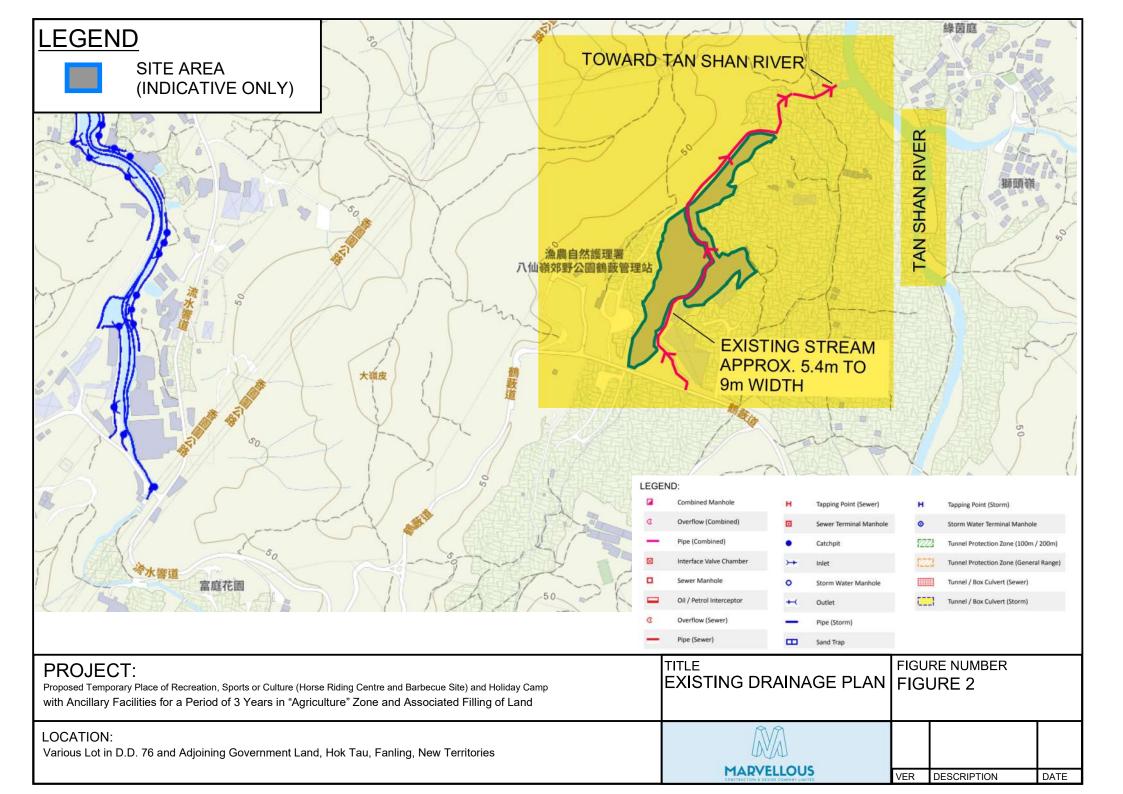
5 Conclusion

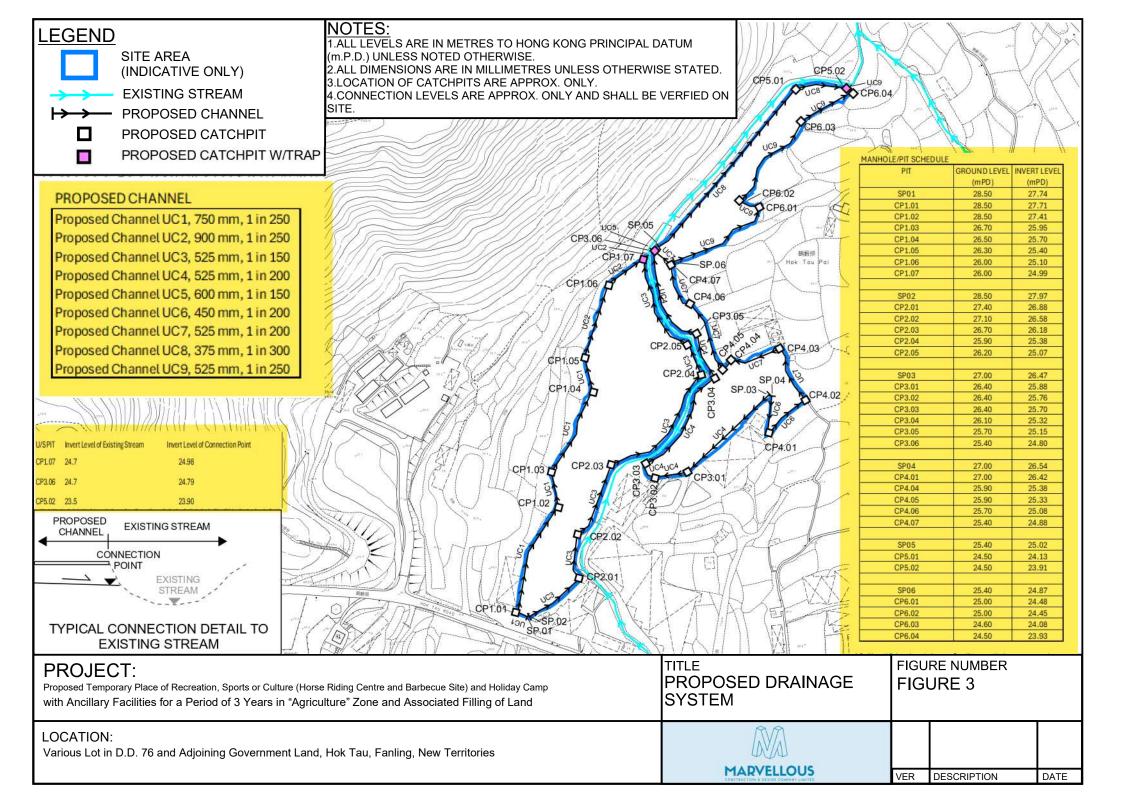
- 5.1.1 Drainage review has been conducted for the Proposed Development. U Channels are proposed to collect the runoff from the catchments. As the paved area is reduced after the development, there is no additional runoff due to the proposed development.
- 5.1.2 With implementation of the above drainage system, the no unacceptable drainage impact is anticipated.

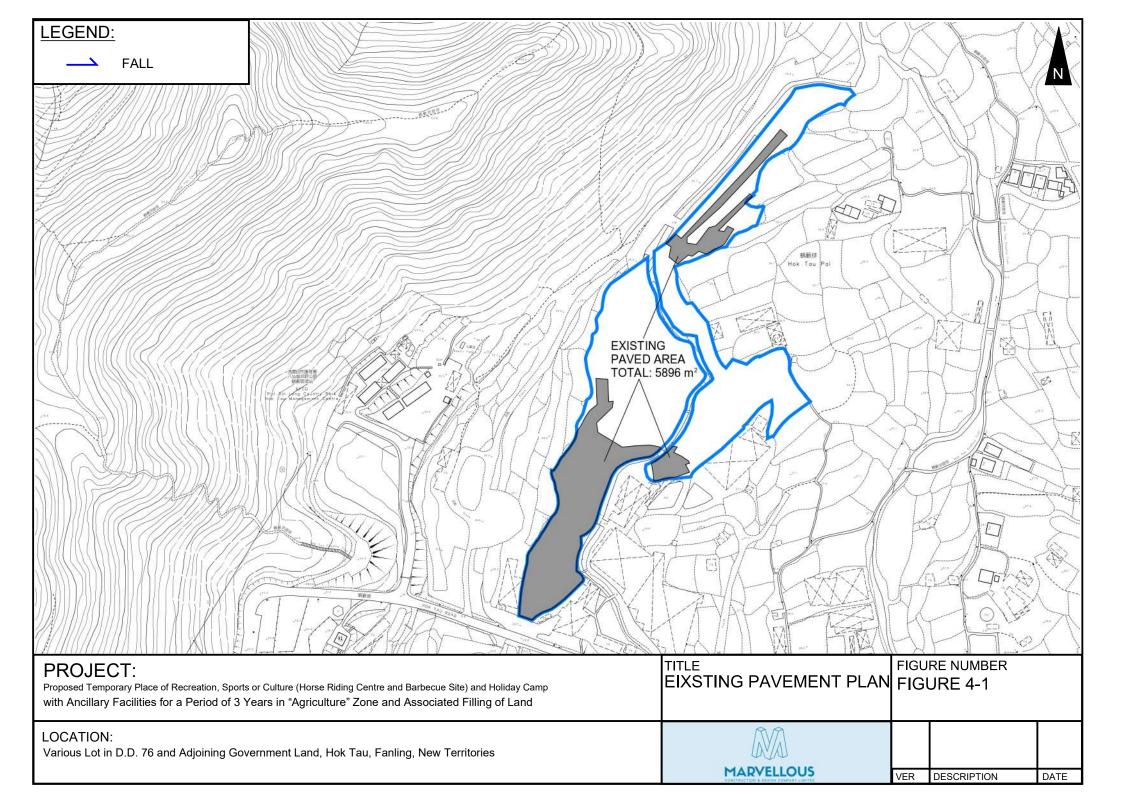
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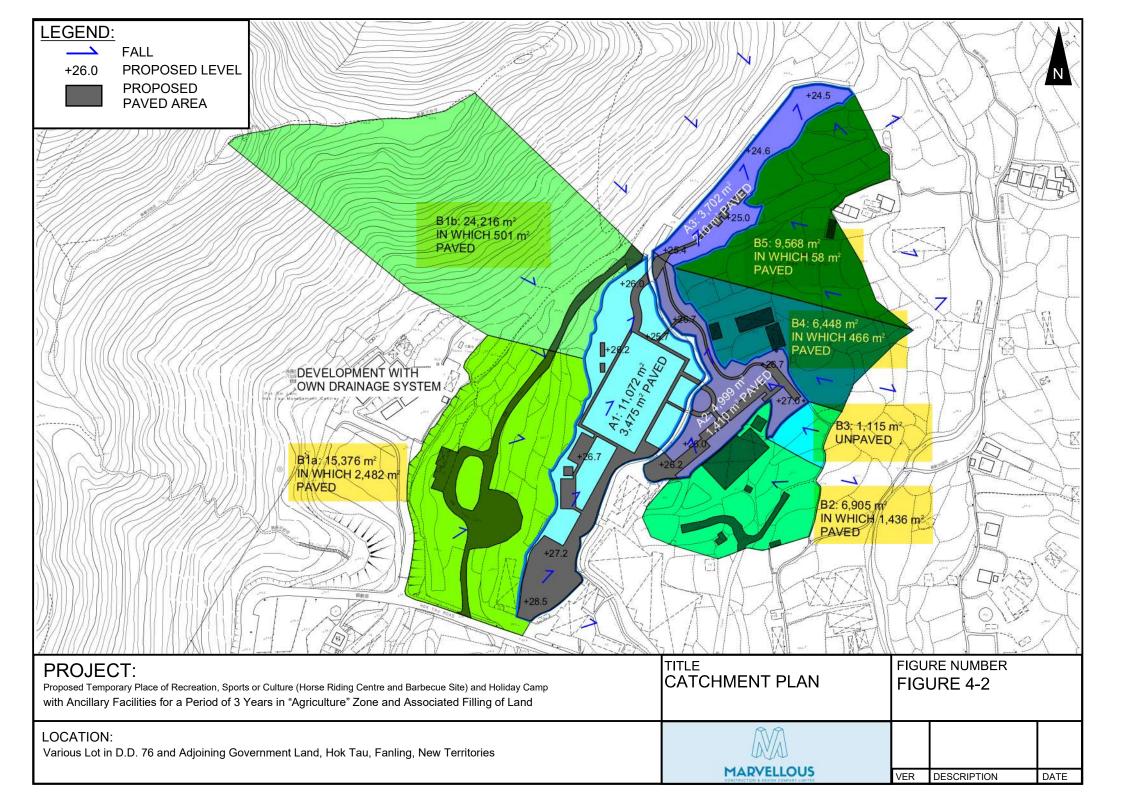
FIGURES

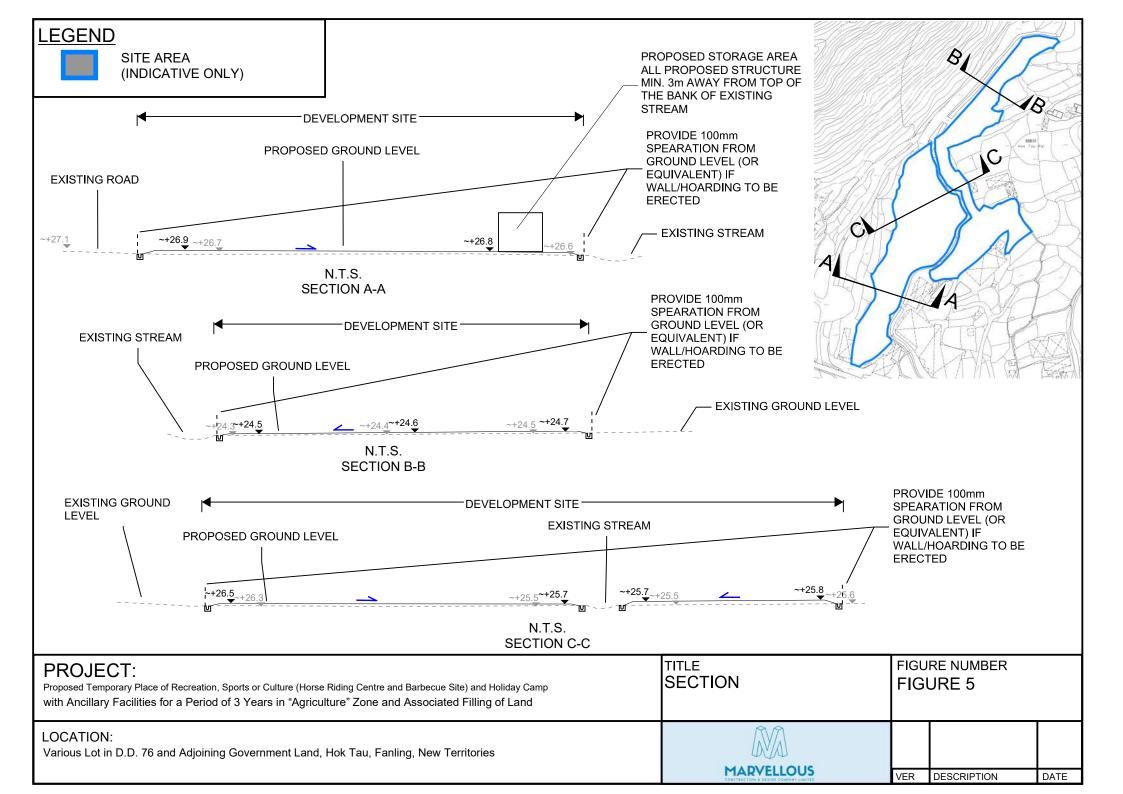








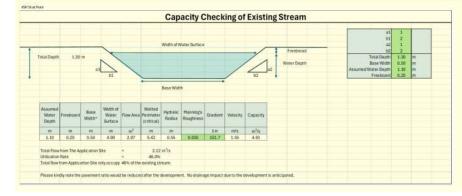


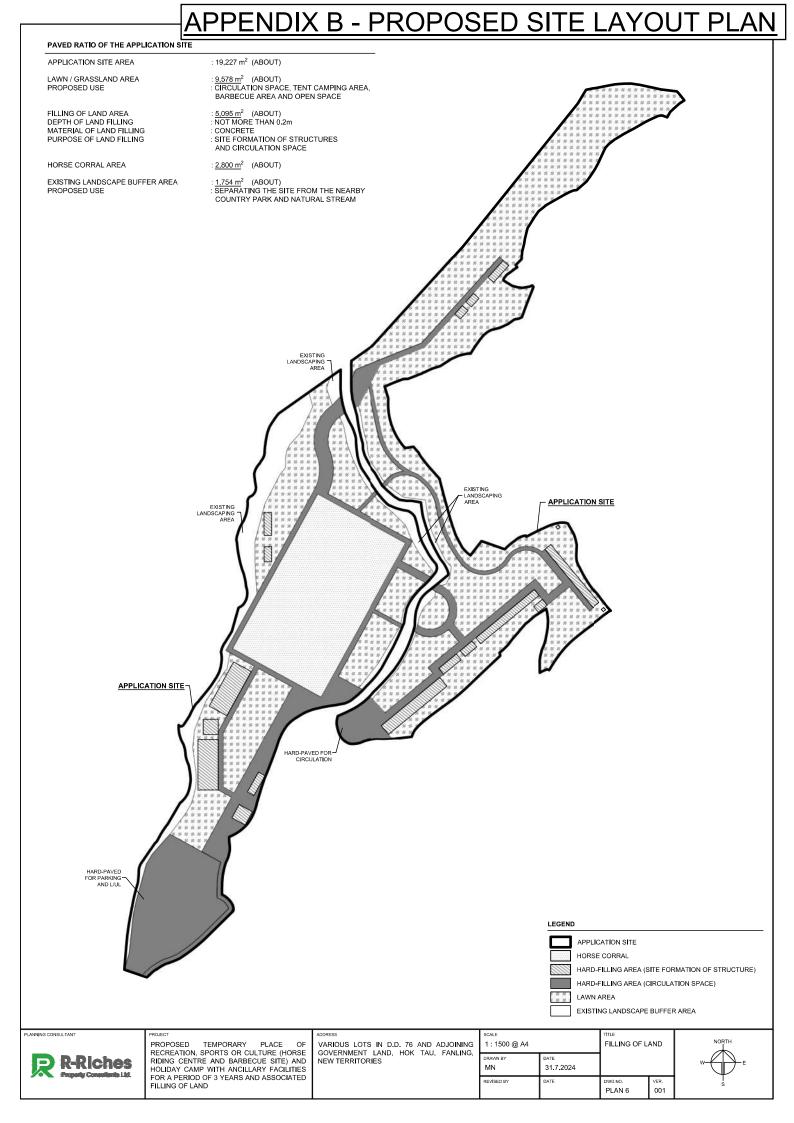


APPENDIX

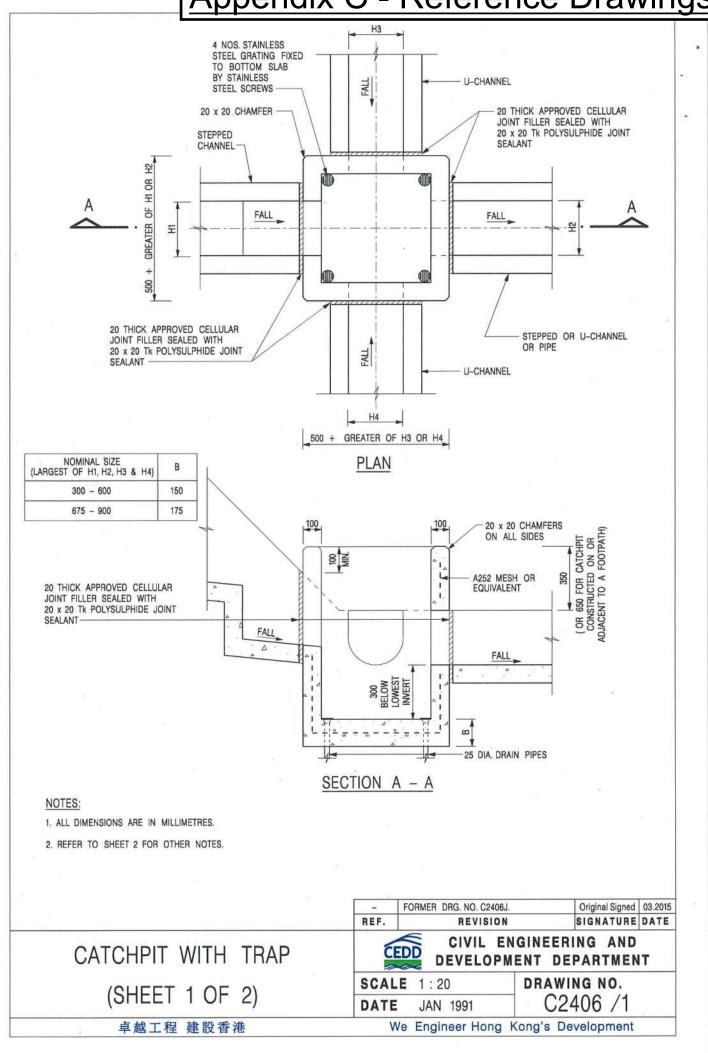
Appendix A: Design Calculation

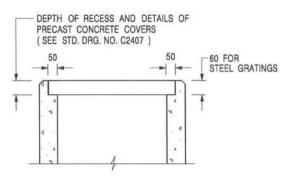
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	CPE.01					200	0.5																
	CP6.01 CP6.02	CP6.03				260	100	24.08															
	CP6.01 CP6.02 OP6.03	CP6.03 CP6.04	24.60	24.50	525																		
	CPE.01 CP6.02 OP6.03	CP6.03 CP6.04	24.60	24.50	525																		





Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.

5. CONCRETE TO BE COLOURED AS SPECIFIED.

- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	A	MINOR AMENDMENT.	Original Signed	04.2016
	-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
	REF.	REVISION	SIGNATURE	DATE
CATCHPIT WITH TRAP			IGINEERING AND	
(SHEET 2 OF 2)	SCA	LE 1:20	DRAWING NO.	
(JILLI 2 01 2)	DAT	E JAN 1991	C2406 /2/	4
卓越工程 建設香港	1	We Engineer Hong	Kong's Development	

