Site Area 3359 m2

Calculation of Runoff from the Proposed Development,

 $= 0.278 \,\mathrm{C}\,\mathrm{i}\,\mathrm{A}$ 

= 0.95

(P.42 of Stormwater Drainage Manual)

Α = 3359  $m^2$ 

= 0.003359

 $km^2$ 

take = 250 mm/hr

Therefore,

= 0.278\*0.95\*250\*0.003359

= 0.222 = 13320

m<sup>3</sup>/sec

lit/min

Calculation Maximum Capacity of Proposed 400 mm dia. Underground pipe.

Manning Equation

 $= R^{2/3}*S_f^{0.5}/n$ 

dia

400 mm

where

R  $= \pi r^2/2 \pi r$ 

 $0.2 \, \text{m}$ 

= r/2

= 0.1

= 0.012

m

s/m<sup>1/3</sup>

(Table 13 of Stormwater Drainage Manual)

1/ 75

 $S_{\rm f}$ = 0.0133

Therefore,

0.1 2/3 \* 0.0133 0.5 / 0.012

2.071

Maximum Capacity (Qmax)

= V\*A

 $= 2.071*\pi r^2$ 

= 0.260

m<sup>3</sup>/sec

1 nos of pipe

= 0.260

= 15600

m<sup>3</sup>/sec

lit/min

> 13320

lit/min

Provide 400 mm dia underground pipe (1:75) is OK