

Your Ref.: A/NE-SSH/155

Our Ref.: P23055B/TL24465

3 September 2024

The Secretary
Town Planning Board
15/F., North Point Government Offices
333 Java Road, North Point, Hong Kong

By Post and E-mail
tpbpd@pland.gov.hk

Dear Sir,

Submission of Further Information

**Proposed Temporary Private Vehicle Park (Private Cars) for a Period of 3 Years in
“Village Type Development” Zone, Lot Nos. 537 (Part), 538 (Part), 540 S.A (Part)
and 541 S.A (Part) in D.D. 218, Ma Kwu Lam Village, Sai Kung North, New Territories**

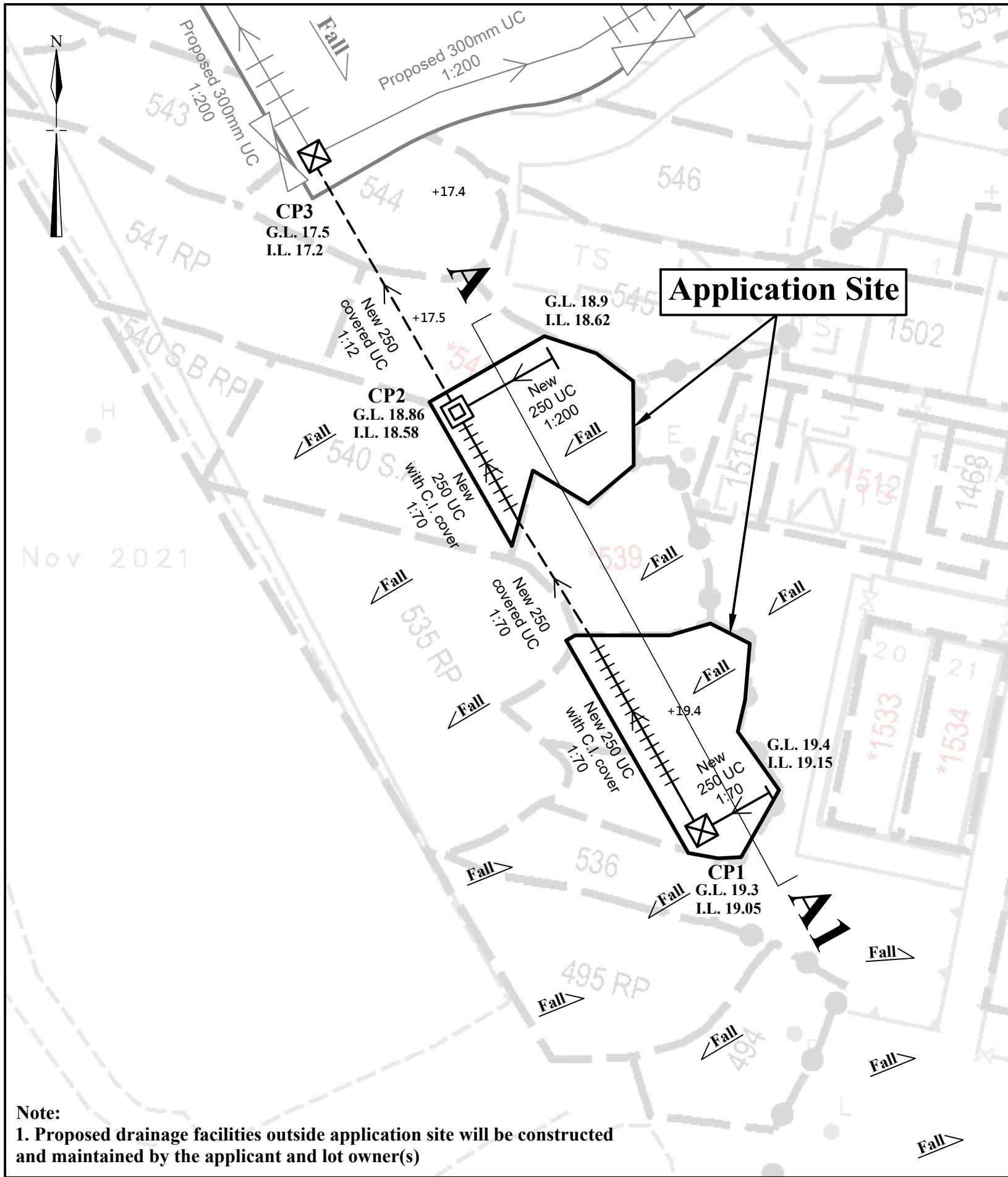
We would like to submit a set of updated drainage proposal for the captioned application.

Yours faithfully,
For and on behalf of
Goldrich Planners & Surveyors Ltd.

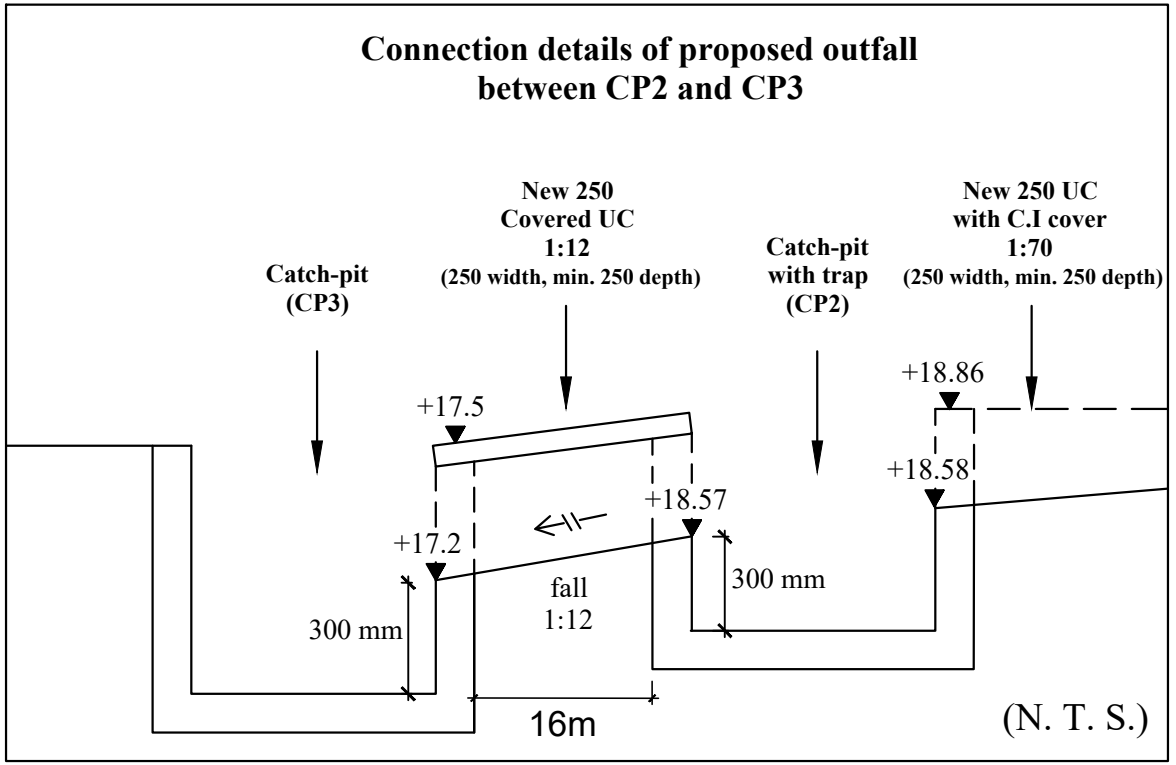


Francis Lau

Encl.



Note:
 1. Proposed drainage facilities outside application site will be constructed and maintained by the applicant and lot owner(s)

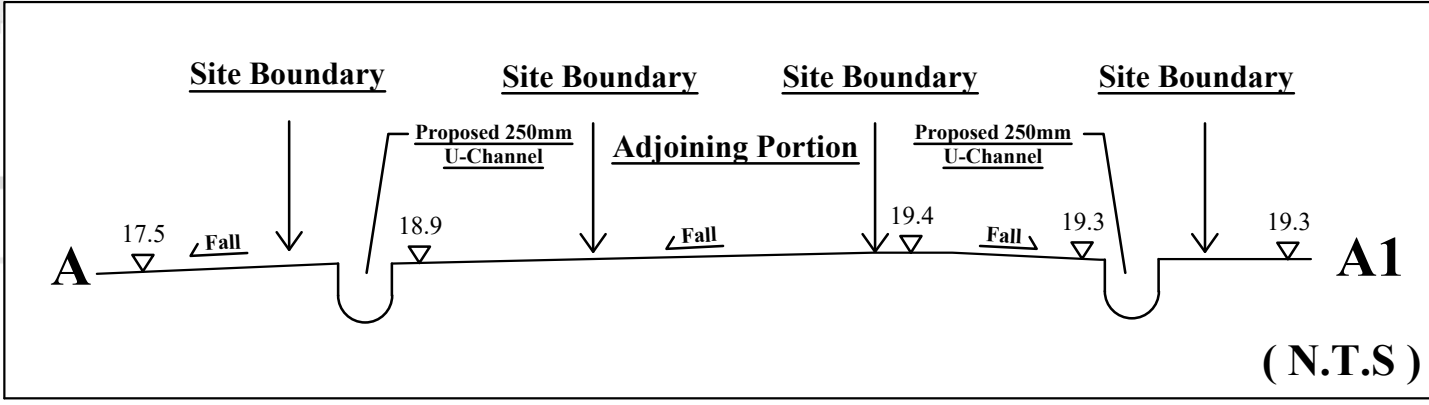


Legend

- ☒ Proposed Catch-pit
- ☐ Proposed Catch-pit with trap
- Proposed U-Channel
- - - Proposed Covered U-Channel
- + + + + U-Channel with C.I. cover
- ⋈ Vehicular Ingress / Egress

Catchpit Schedule

Catchpit No.	G.L.	I.L.
CP1	19.3	19.05
CP2	18.86	18.58
CP3	17.5	17.2



1:300 (A3)

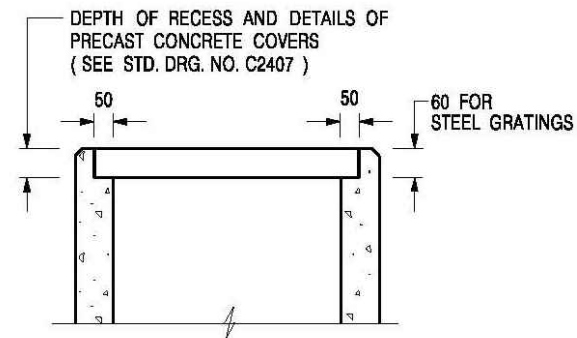
September 2024

Drainage Proposal

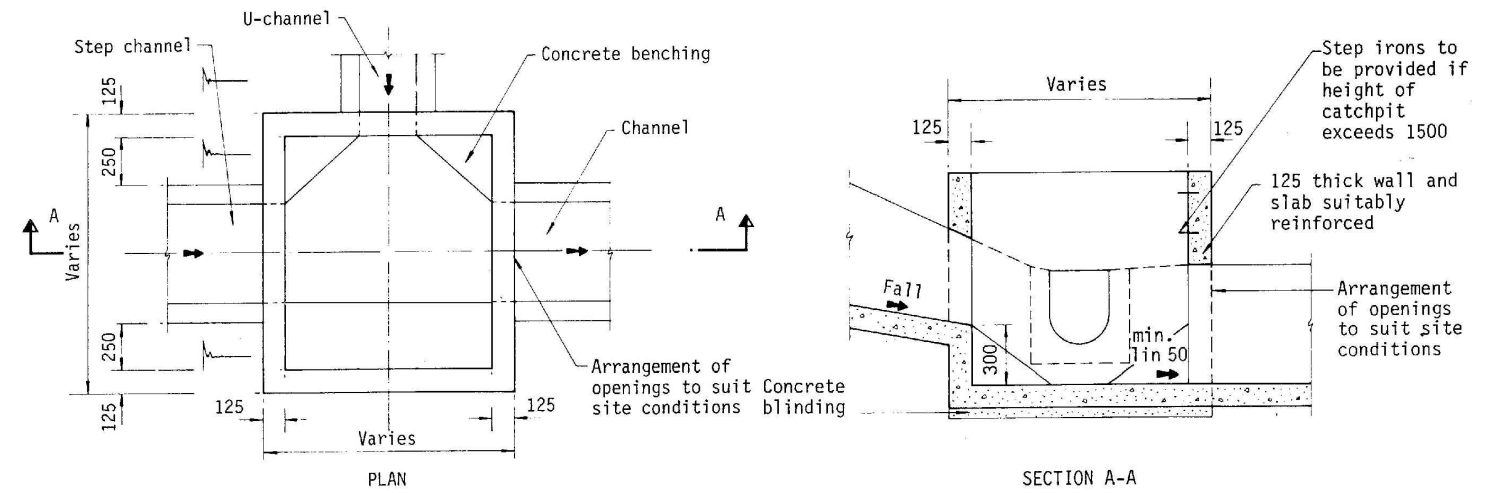
Lots 537(part), 538(part), 540 S.A(part)
 and 541 S.A(part) in D.D.218
 Ma Kwu Lam, Sai Kung North, N.T

Goldrich Planners &
 Surveyors Ltd.

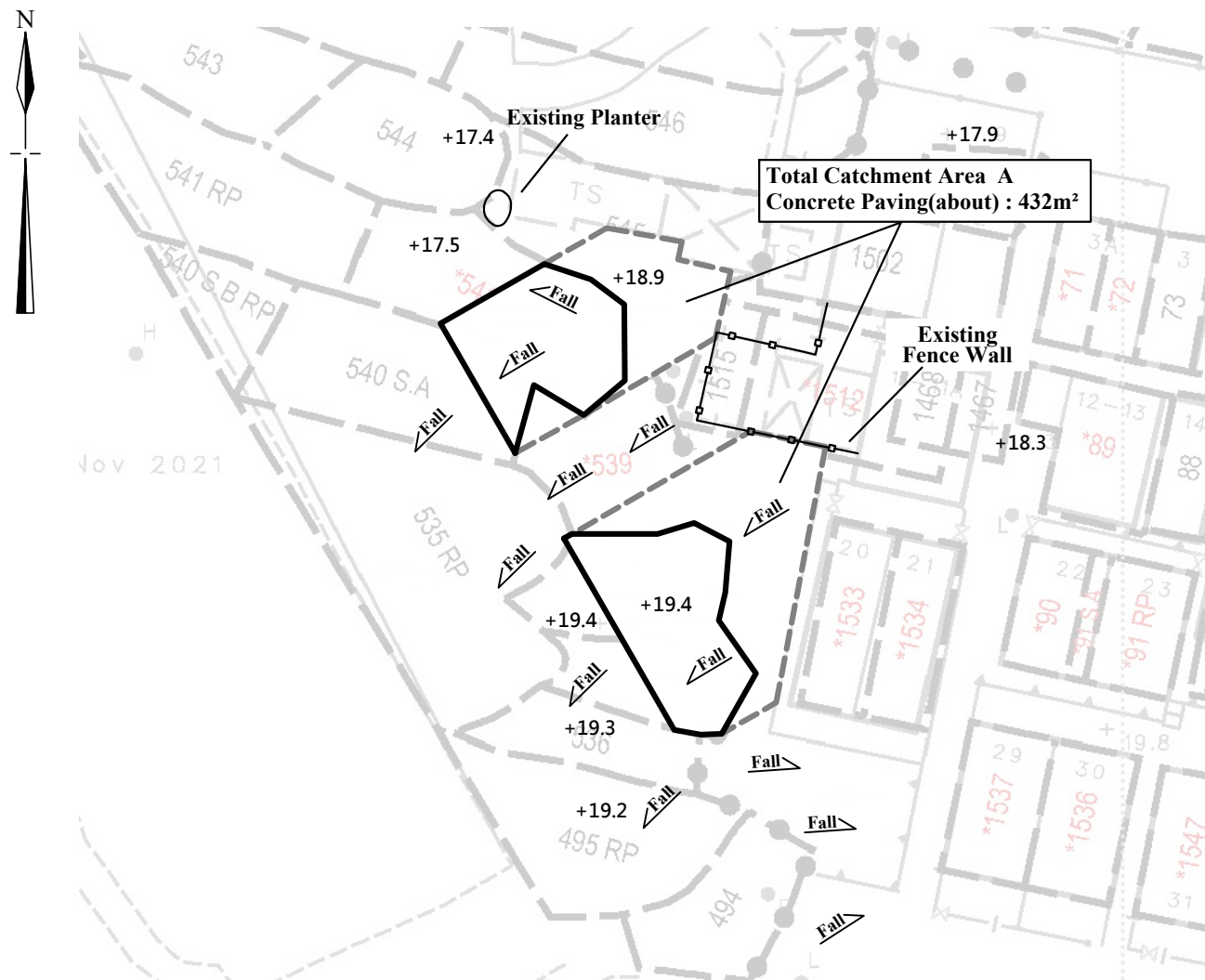
Plan 4.1d
 (P 23055B)



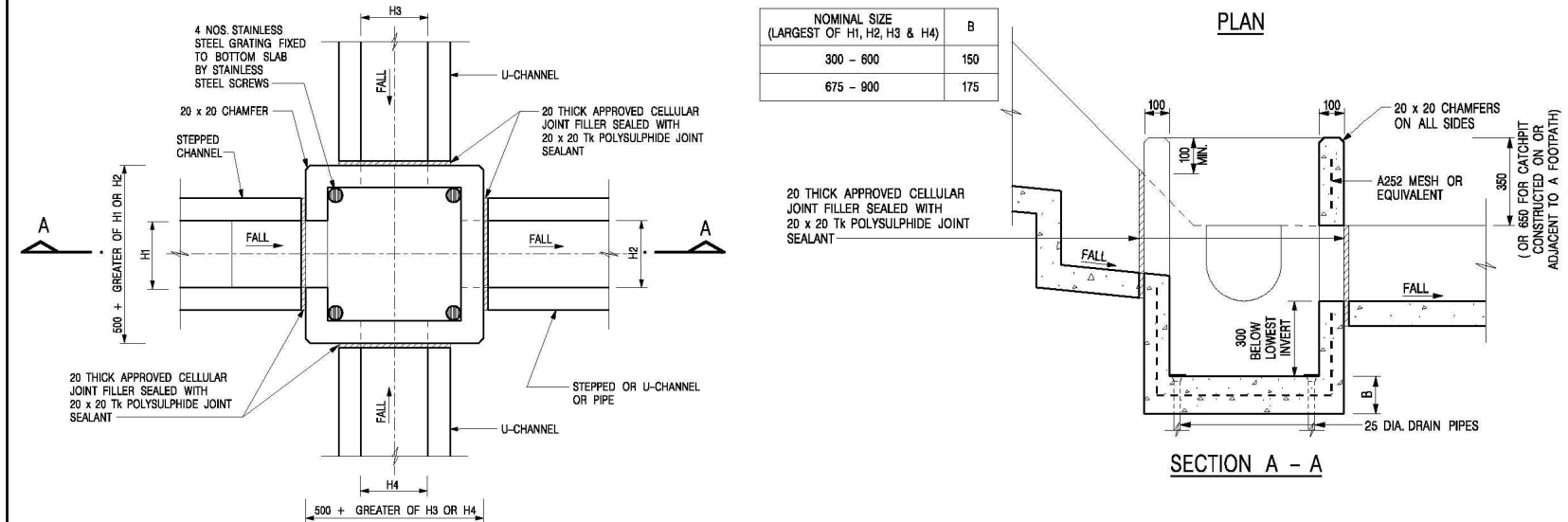
**ALTERNATIVE TOP SECTION
FOR PRECAST CONCRETE COVERS / GRATINGS**
(REFER TO CEDD'S STANDARD DWG. C2405/1 & C2406/2A)



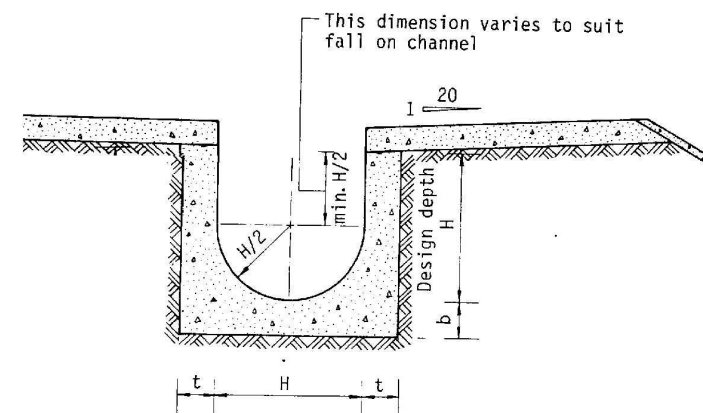
TYPICAL DETAILS OF CATCHPIT



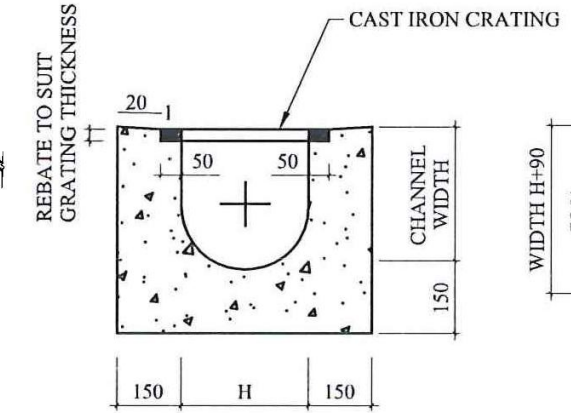
**AREA OF CATCHMENT
(N.T.S)**



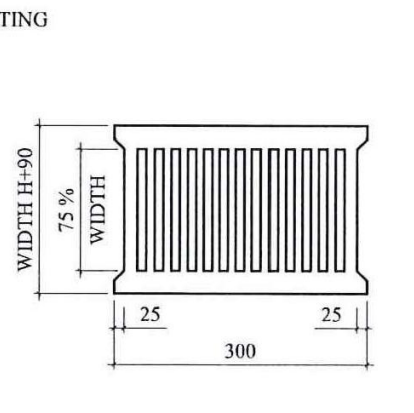
DETAILS OF CATCHPIT WITH TRAP
(REFER TO CEDD'S STANDARD DWG. C2406/1)



TYPICAL DETAILS OF U CHANNEL



**TYPICAL SECTION OF
U-CHANNEL WITH COVER
(N.T.S.)**



**CAST IRON CRATING
(HEAVY DUTY)**

N.T.S

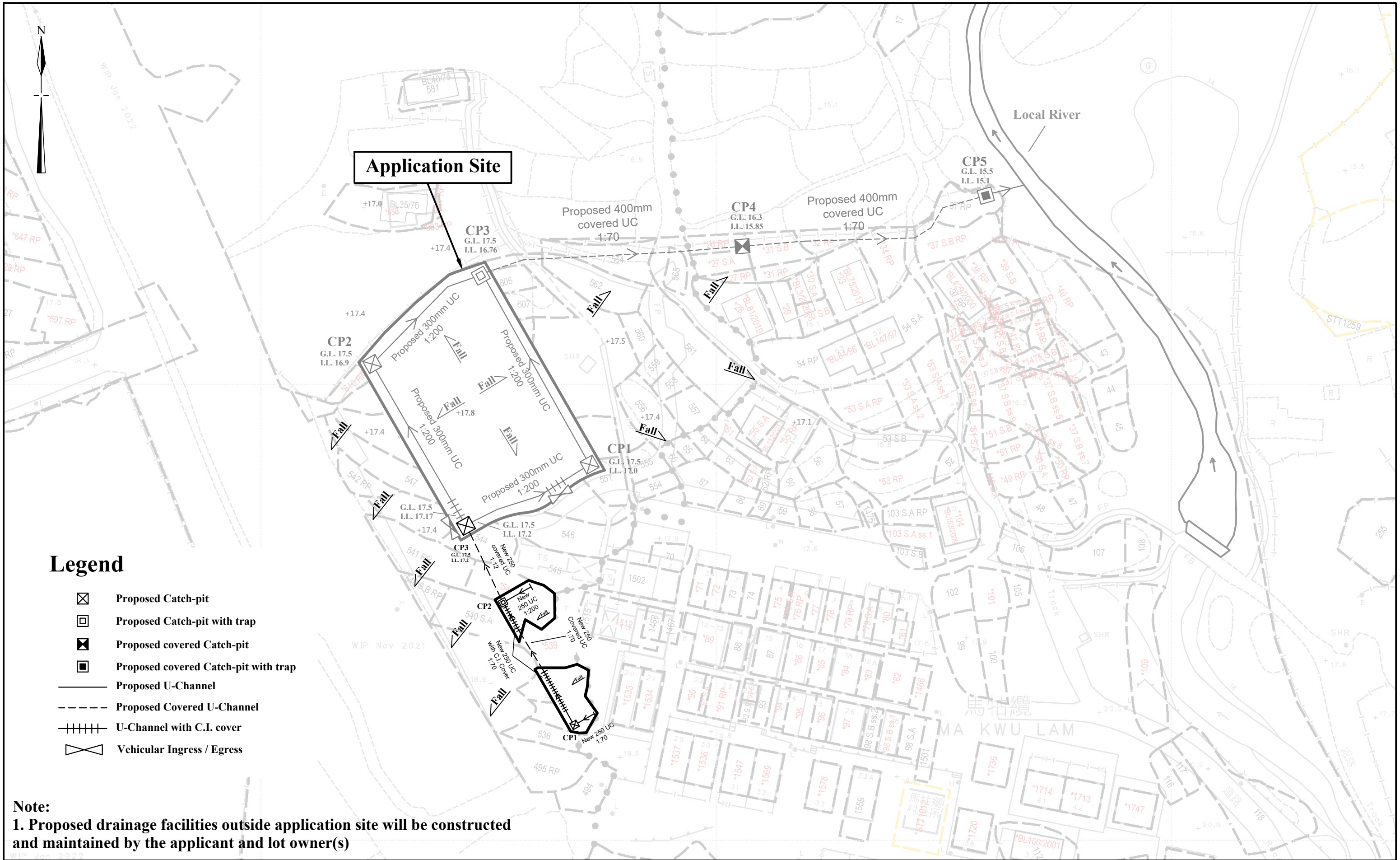
September 2024

Drainage Proposal

Lots 537(part), 538(part), 540 S.A(part)
and 541 S.A(part) in D.D.218
Ma Kwu Lam, Sai Kung North, N.T

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Plan 4.2d
(P 23055B)



Legend

- Proposed Catch-pit
- Proposed Catch-pit with trap
- Proposed covered Catch-pit
- Proposed covered Catch-pit with trap
- Proposed U-Channel
- Proposed Covered U-Channel
- U-Channel with C.I. cover
- Vehicular Ingress / Egress

Note:
1. Proposed drainage facilities outside application site will be constructed and maintained by the applicant and lot owner(s)

1:750 (A3)

August 2024

Drainage Proposal

Lots 537(part), 538(part), 540 S.A(part)
 and 541 S.A(part) in D.D.218
 Ma Kwu Lam, Sai Kung North, N.T

Goldrich Planners & Surveyors Ltd.

**Plan 4.3a
 (P 23055B)**

1 For Catchment Area A

Area, A = 432 m²
 Average slope, H = 0.1 m per 100m
 Distance on the line of natural flow, L = 18.6 m

Time of concentration, $t_c = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (18.6) / (0.1^{0.2} \times 432^{0.1})$
 = 2.3 min

SDM 7.5.2 (d)

2 For Proposed U-Channel in catchment area A

	From	To
Ground level (mPD)	19.30	18.86
Invert level (mPD)	19.05	18.58

Width of u-channel, w = 250 mm
 Length of u-channel, L_c = 33 m
 Depth of vertical part of u-channel, d = 155 mm
 Gradient of u-channel, S_f = (19.05-18.58)/33 = 0.014

Cross-Section Area, a = $0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 125^2 + 250 \times 155$
 = 0.063 m²
 Wetted Perimeter, p = $\pi r + 2 d = 3.14 \times 125 + 2 \times 155$
 = 0.703 m
 Hydraulic radius, R = a / p
 = 0.090 m

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-
 Allowable velocity, v = $R^{1/6} \times (RS_f)^{1/2} / n = (0.09)^{1/6} \times (0.09 \times 0.014)^{1/2} / 0.016$
 = 1.50 m/s
 Time of flow, t_f = 0.4 min

SDM Table 13
 SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, i = $a / (t_c + t_f + b)^c$
 = $505.5 / (2.3 + 0.4 + 3.29)^{0.355}$ for return period T = 50 years
 = 268

SDM 4.3.2
 SDM Table 3(a)

Type of surface	Runoff Coefficient C	Catchment Area A (m ²)	C x A
Flat Glassland (heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	432.0	410.4
			SUM = 410.4

SDM 7.5.2 (b)

Upstream flow, Q_u = 0 m³/s

Design flow, Q_d = $0.278i \sum C_f A_j + Q_u$ where A_j is in km²
 = $0.278 \times 268 \times 410.4 / 1000000 + 0$
 = 0.031 m³/s

SDM 7.5.2 (a)

Allowable flow, Q_a = a x v
 = 0.063 x 1.5
 = 0.095 m³/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD

Scale: NA	Drainage Calculation	Goldrich Planners & Surveyors Ltd.
August 2024		Page 1 (P23055B)

Lots 537(part), 538(part), 540 S.A(part) and 541 S.A(part) in D.D.218
 Ma Kwu Lam, Sai Kung North, N.T

1 For Catchment Area B

Area, A = 0 m²
 Average slope, H = 0.1 m per 100m
 Distance on the line of natural flow, L = 0 m

Time of concentration, $t_o = 0.14465L / (H^{0.2}A^{0.1}) = 0.14465 (0) / (0.1^{0.2} \times 0^{0.1})$
 = 0.0 min

Ref.

SDM 7.5.2 (d)

2 For Proposed U-Channel in catchment area B

	From	To
Ground level (mPD)	18.86	17.50
Invert level (mPD)	18.58	17.20

Width of u-channel, w = 250 mm
 Length of u-channel, $L_c = 16$ m
 Depth of vertical part of u-channel, d = 175 mm
 Gradient of u-channel, $S_f = (18.58-17.2)/16 = 0.086$

Cross-Section Area, $a = 0.5 \pi r^2 + w d = 0.5 \times 3.14 \times 125^2 + 250 \times 175$
 = 0.068 m²
 Wetted Perimeter, $p = \pi r + 2 d = 3.14 \times 125 + 2 \times 175$
 = 0.743 m
 Hydraulic radius, $R = a / p$
 = 0.092 m

SDM 8.2.1

3 Use Manning Equation for estimating velocity of stormwater

Take n = 0.016 for concrete lined channels:-
 Allowable velocity, $v = R^{1/6} \times (RS_f)^{1/2} / n = (0.092)^{1/6} \times (0.092 \times 0.086)^{1/2} / 0.016$
 = 3.74 m/s
 Time of flow, $t_f = 0.1$ min

SDM Table 13
SDM Table 12

4 Use "Rational Method" for calculation of design flow

Design intensity, $i = a / (t_o + t_f + b)^c$
 = $505.5 / (0+0.1+3.29)^{0.355}$ for return period T = 50 years
 = 329

SDM 4.3.2
SDM Table 3(a)

Type of surface	Runoff Coefficient C	Catchment Area A (m ²)	C x A
Flat Glassland(heavy soil)	0.25	0.0	0.0
Concrete Paving	0.95	0.0	0.0
SUM =			0.0

SDM 7.5.2 (b)

Upstream flow, $Q_u = 0.031$ m³/s

Design flow, $Q_d = 0.278i \Sigma C_i A_i + Q_u$ where A_i is in km²
 = $0.278 \times 329 \times 0 / 1000000 + 0.031$
 = 0.031 m³/s

SDM 7.5.2 (a)

Allowable flow, $Q_a = a \times v$
 = 0.068×3.74
 = 0.255 m³/s

> Q_d (O.K.)

Reference was made to Stormwater Drainage Manual (SDM) by DSD