

## **APPENDICES**

- Appendix I**      Details of the Affected Business Premises
- Appendix II**     Details of Alternative Sites for Relocation
- Appendix III**    Drainage Impact Assessment

## **Appendix I**

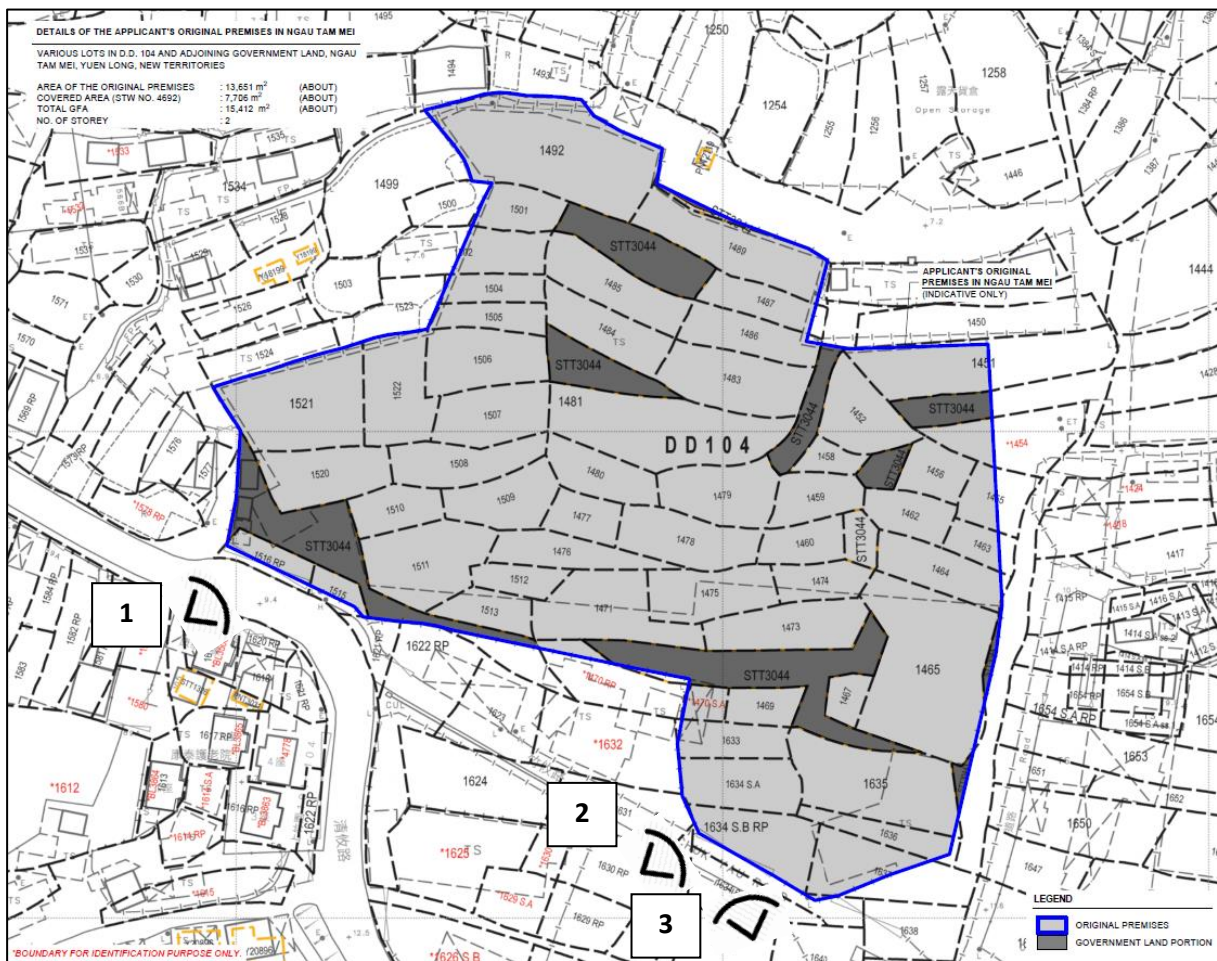
### Details of the Affected Business Premises

### Appendix I – Details of the Affected Business Premises

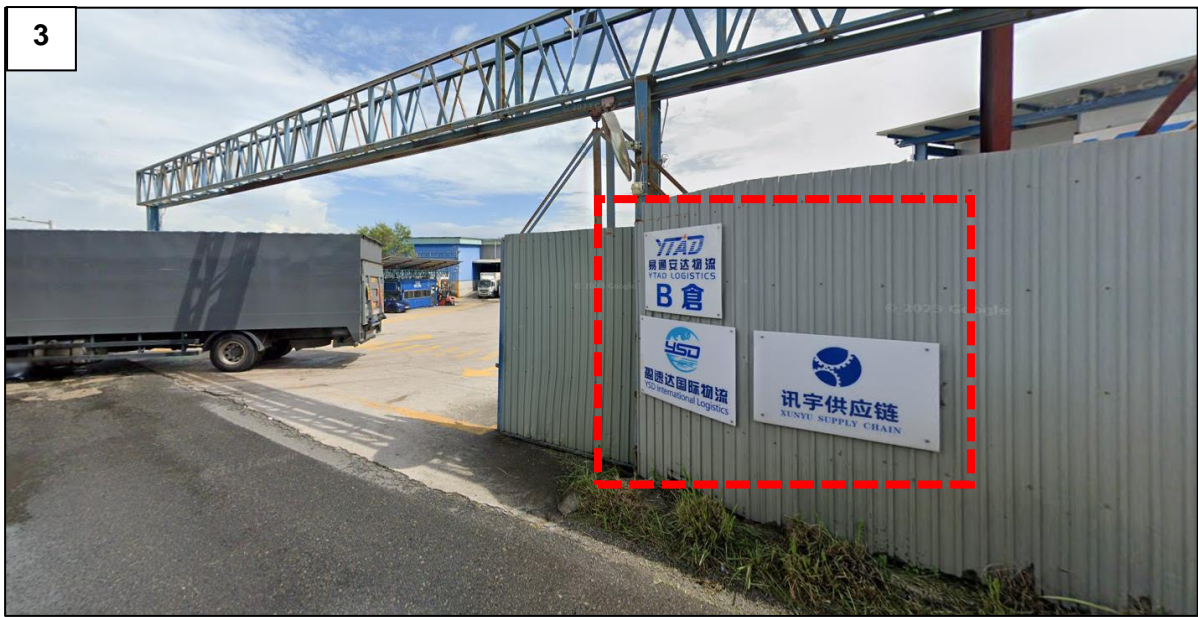
Company Name: **YTAD Warehousing Logistics Co. Limited**  
(authorised Luck Great Global Engineering Limited as applicant of the current application)

#### Details of Business Premises

Location: Various Lots in D.D. 104 and Adjoining Government Land, Ngau Tam Mei, Yuen Long, New Territories  
Use of Premises: Warehouse with Ancillary Facilities



Proposed Temporary Warehouse (Excluding D.G.G.) with Ancillary Facilities for a Period of 3 Years in "Recreation" Zone, Various Lots in D.D.78 and Adjoining GL, Lin Ma Hang, New Territories



**Appendix II**  
Details of Alternative Sites for Relocation

**Appendix II – Alternative Sites for the Relocation of the Applicant’s Original Premises in Ngau Tam Mei, Yuen Long**

Alternative Site / Application Site	Site 1	Site 2	Site 3	Site 4	Site 5	Application Site
<b>Location</b>	Various Lots in D.D. 86, San Uk Ling, Man Kam To, New Territories	Various Lots in D.D. 93, Ma Tso Lung, New Territories	Various Lots in D.D. 122, Long Ping Road, Ping Shan, New Territories	Various Lots in D.D. 129, Lau Fau Shan, Yuen Long, New Territories	Various Lots in D.D. 130, Lam Tei, Tuen Mun, New Territories	Various Lots in D.D.78 and Adjoining GL, Lin Ma Hang, New Territories,
<b>Site Area</b>	3,678 m <sup>2</sup> (about)	30,190 m <sup>2</sup> (about)	2,815 m <sup>2</sup> (about)	10,740 m <sup>2</sup> (about)	7,130 m <sup>2</sup> (about)	24,446m <sup>2</sup> (about)
<b>Accessibility</b>	Accessible from Lin Ma Hang Road via a local access	Accessible from Ma Tso Lung Road via a local access	Accessible from Long Ping Road via a local access	Accessible from Deep Bay Road via a local access	Accessible from Fuk Hang Tsuen Road via a local access	Accessible from Heung Yuen Wai Highway via Lin Ma Hang Road
<b>Distance from Original Premises</b>	12.8 km (about) from the original premises	8.3 km (about) from the original premises	8.4 km (about) from the original premises	15.6 km (about) from the original premises	11.4 km (about) from the original premises	15.4 km (about) from the original premises
<b>Outline Zoning Plan</b>	Approved Man Kam To OZP No. S/NE-MKT/4	Approved Ma Tso Lung and Hoo Hok Wai OZP No. S/NE-MTL/3	Approved Ping Shan OZP No. S/YL-PS/20	Approved Lau Fau Shan & Tsim Bei Tsui OZP No. S/YL-LFS/11	Approved Lam Tei and Yick Yuen OZP No. S/TM-LTTY/12	Approved Ta Kwu Ling North OZP S/NE-TKLN/2
<b>Zoning</b>	"Green Belt"	"Conservation Area (1)"	"Conservation Area"	"Green Belt"	"Comprehensive Development Area"	"Recreation"
<b>Existing Condition</b>	Covered by tree groups and vegetation	Mostly vacant, covered by vegetation and occupied by fishpond.	Woodland and partly vacant	Covered by vegetation and woodland	Hard paved and occupied by temporary structures	Mostly vacant, partially hard-paved and the remaining area consists of soiled ground
<b>Surrounding Area</b>	Surrounded by residential development and woodland	Surrounded by vegetation, pond, some GIC uses and residential use	Surrounded by woodland and graves	Surrounded by tree groups, temporary structures for open storage and residential use	Surrounded by warehouse, workshop, logistic centre and land covered by residential use	Surrounded by vacant land, woodland, public roads, temporary structures and village houses
<b>Suitability for Relocation</b>	<u>Not suitable</u> for relocation - 73% <u>smaller</u> than the original premises - Tree felling is required - Tenancy for portion of the site is not feasible - Not compatible with the surrounding area	<u>Not suitable</u> for relocation - 55% <u>larger</u> than the original premises - Within the closed area - Falls within the "Conservation Area" zone - Tenancy for portion of the site is not feasible - Not compatible with the surrounding area	<u>Not suitable</u> for relocation - 385% <u>smaller</u> than the original premises - Not compatible with the surrounding area - Within "Conservation Area" Zone - Tenancy for portion of the site is not feasible	<u>Not suitable</u> for relocation - 21% <u>smaller</u> than the original premises - Tree felling is required - Tenancy for portion of the site is not feasible - Not compatible with the surrounding area Tenancy for portion of the site is not feasible	<u>Not suitable</u> for relocation - 48% <u>smaller</u> than the original premises - Not compatible with the surrounding area - Tenancy for portion of the site is not feasible	<u>Comparatively Suitable</u> for relocation: - In close vicinity of Heung Yuen Wai Highway - Relatively flat and mostly vacant - No active agricultural activity

**Appendix III**  
Drainage Impact Assessment

Proposed Temporary Warehouse (Excluding D.G.G.) with  
Ancillary Facilities for a Period of 3 Years in “Recreation” Zone,  
Various Lots in D.D.78 and Adjoining GL,  
Lin Ma Hang, New Territories

Drainage Impact Assessment Report

June 2024



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# 1. Introduction

## 1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) under Section (S.) 16 of the Town Planning Ordinance (Cap. 131)(the Ordinance) to use Various Lots in D.D. 78 and Adjoining Government Land (GL), Lin Ma Hang, New Territories (the Site) for ‘Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years’.
- 1.1.2 This Drainage Impact Assessment aim to support the development in drainage aspect.

## 1.2 The Site

- 1.2.1 The Application Site situate between Lin Ma Hang Road and Shenzhen River. It has an area of about 24,446 m<sup>2</sup>. The site is partially hard-paved at the south east corner and the remaining area is covered by vegetation. The site location plan is shown in **Figure 1**.
- 1.2.2 The existing site ground levels beside Lin Ma Hang Road is about +9.1 mPD. The site generally falling towards Shenzhen river to about +6.6 mPD. There is no major site level changes proposed.
- 1.2.3 There is an existing 800mm channel to the northeast of the site and beside Lin Ma Hang Road. Shenzhen River is situated at the west side of the site. This existing channel pass through the site and discharge toward Shenzhen River at the west. Existing Drainage Plan are shown in **Figure 2** for reference.
- 1.2.4 Proposed Development Layout plan is shown in **Appendix B** for reference.

## 2. Development Proposal

### 2.1 The Proposed Development

2.1.1 The total site area is approximately 24,446 m<sup>2</sup>. The indicative development schedule is summarized in **Table 1** below for technical assessment purpose. The catchment plan is shown in **Figure 4.1** and **Figure 4.2**.

Proposed Development	
Total Site Area (m <sup>2</sup> )	24,446
Paved Area (m <sup>2</sup> )	24,446
Assume all proposed site area as paved area for assessment purpose	

**Table 1 - Key Development Parameters**

## 3. Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this DIA. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

**Table 2– Design Return Periods under SDM**

3.1.2 The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 50 years return period is adopted for the drainage design.

3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.

1. Intensity-Duration-Frequency Relationship – The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the North District Zone. Therefore, for 50 years return period, the following values are adopted.

a	=	474.6
b	=	2.9
c	=	0.371

(Corrigendum\_No.1\_2024)

2. The peak runoff is calculated by the Rational Method  
i.e.  $Q_p = 0.278CiA$

where	$Q_p$	=	peak runoff in $m^3/s$
	C	=	runoff coefficient (dimensionless)
	i	=	rainfall intensity in mm/hr
	A	=	catchment area in $km^2$

3. The run-off coefficient (C) of surface runoff are taken as follows:

1. Paved Area: C = 0.95
2. Unpaved Area: C = 0.35

4. Manning’s Equation is used for calculation of velocity of flow inside the channels:

$$\text{Manning's Equation: } v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the pipe flow (m/s)

S<sub>f</sub> = hydraulic gradient

n = manning’s coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

$$\text{Colebrook-White Equation: } \frac{v}{\nu} = -\sqrt{32gRS} \log \log \left( \frac{k_s}{14.8R} + \frac{1.255\nu}{R\sqrt{32gRS}} \right)$$

where,

V	=	velocity of the pipe flow (m/s)
S <sub>f</sub>	=	hydraulic gradient
k <sub>r</sub>	=	roughness value (m)
ν	=	kinematics viscosity of fluid
D	=	pipe diameter (m)
R	=	hydraulic radius (m)

## 4. Proposed Drainage System and Mitigation Measure

### 4.1. Proposed UChannel

- 4.1.1 The existing U-channel is proposed to be diverted starting at the northeast of the application site and connect to proposed U-channel UC1. It would discharge to the original discharge point and eventually fall to Shenzhen River.
- 4.1.2 Proposed U-channels are designed for collection of runoff within and near the Development Site. Please refer to the **Figure 4.2** for proposed catchment plan. The U-channels are proposed to be connect to original existing channel to the southwest of the site and eventually discharge to Shenzhen River. The design calculations of proposed UChannel are shown in **Appendix A**.

### 4.2. Upgrade of Existing Downstream UChannel

- 4.1.3 The increase in runoff due to the change of application site’s pavement ratio is calculated in **Appendix A**. Please refer to existing catchment plan **Figure 4.1**. The existing channel downstream is proposed to be upgraded such that the increase in capacity is not less than the increase in runoff.
- 4.1.4 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3**.
- 4.1.5 The reference standard drawings of drains are shown in **Appendix C**.

## 5. Conclusion

- 5.1.1 Drainage impact assessment has been conducted for the Proposed Development. With implementation of proposed drainage system and upgrade of existing downstream U-channels, no adverse drainage impact is anticipated.

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# FIGURES

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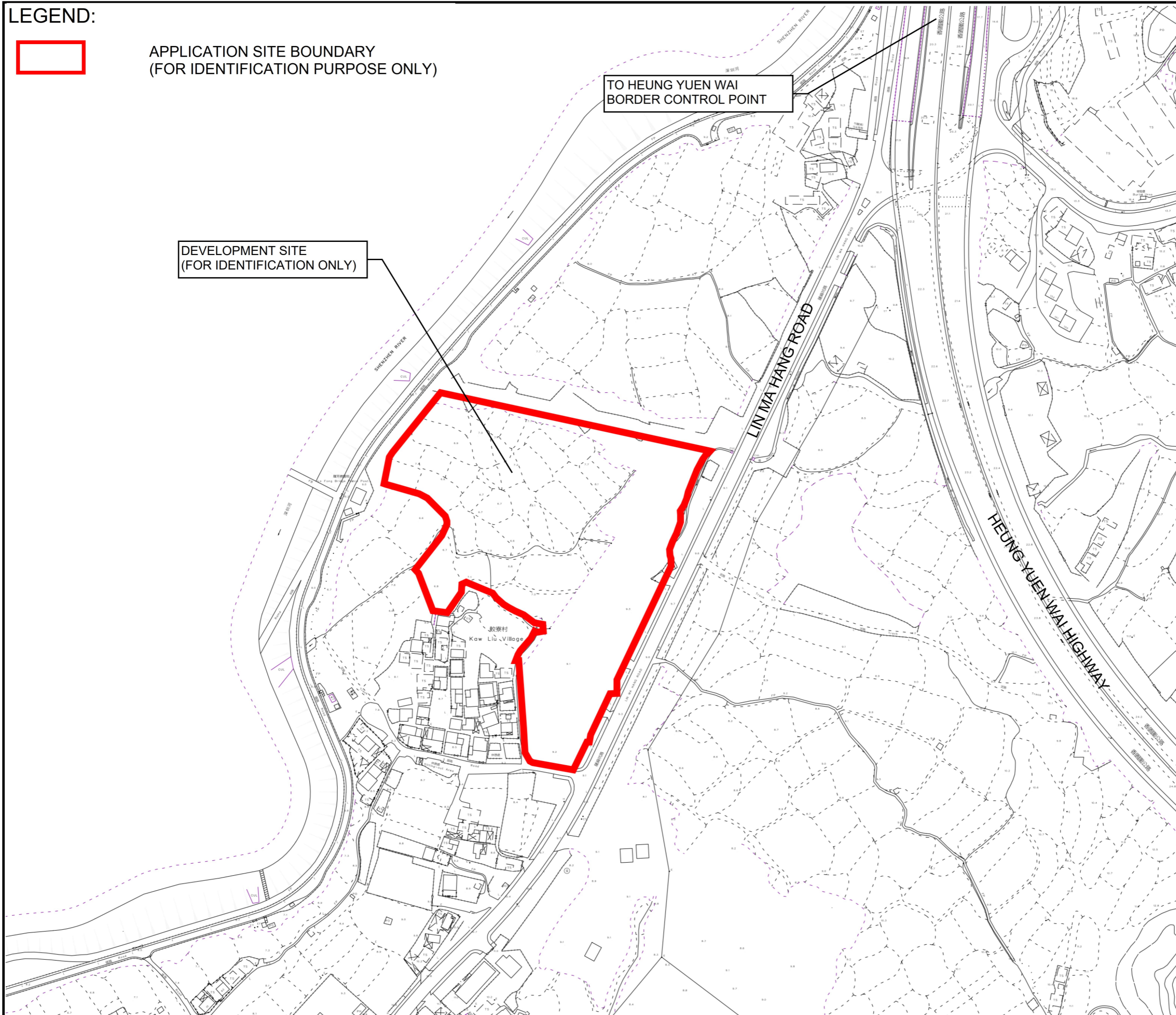
**LEGEND:**



APPLICATION SITE BOUNDARY  
(FOR IDENTIFICATION PURPOSE ONLY)

DEVELOPMENT SITE  
(FOR IDENTIFICATION ONLY)

TO HEUNG YUEN WAI  
BORDER CONTROL POINT



**PROJECT:**  
Proposed Temporary  
Warehouse (Excluding D.G.G.)  
with Ancillary Facilities for a  
Period of 3 Years in "Recreation"  
Zone, Various Lots in D.D.78 and  
Adjoining GL, Lin Ma Hang, New  
Territories

REV	DESCRIPTION	DATE

DRAWING TITLE  
**SITE LOCATION PLAN**

DRAWING NUMBER  
**FIGURE 1**

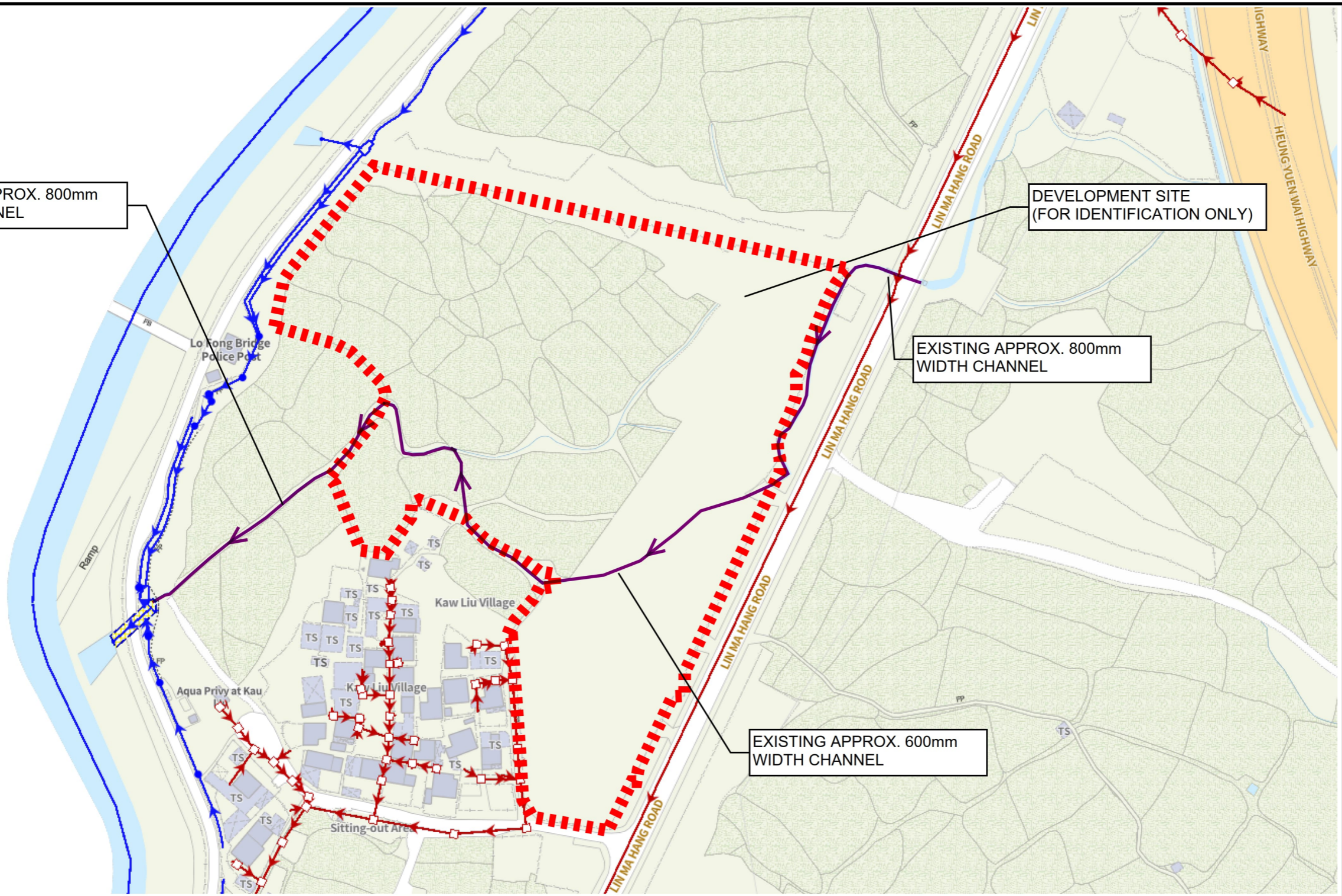


EXISTING APPROX. 800mm WIDTH CHANNEL

DEVELOPMENT SITE (FOR IDENTIFICATION ONLY)

EXISTING APPROX. 800mm WIDTH CHANNEL

EXISTING APPROX. 600mm WIDTH CHANNEL



**PROJECT:**  
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**LEGEND:**

- |  |                          |  |                        |  |  |  |   |
|--|--------------------------|--|------------------------|--|--|--|---|
|  | Combined Manhole         |  | Tapping Point (Sewer)  |  | Tapping Point (Storm)                  |  | APPLICATION SITE BOUNDARY (FOR IDENTIFICATION PURPOSE ONLY) |
|  | Overflow (Combined)      |  | Sewer Terminal Manhole |  | Storm Water Terminal Manhole           |  | EXISTING CHANNELS TOWARDS SHENZHEN RIVER                    |
|  | Pipe (Combined)          |  | Catchpit               |  | Tunnel Protection Zone (100m / 200m)   |  |   |
|  | Interface Valve Chamber  |  | Inlet                  |  | Tunnel Protection Zone (General Range) |  |   |
|  | Sewer Manhole            |  | Storm Water Manhole    |  | Tunnel / Box Culvert (Sewer)           |  |   |
|  | Oil / Petrol Interceptor |  | Outlet                 |  | Tunnel / Box Culvert (Storm)           |  |   |
|  | Overflow (Sewer)         |  | Pipe (Storm)           |  |  |  |   |
|  | Pipe (Sewer)             |  | Sand Trap              |  |  |  |   |

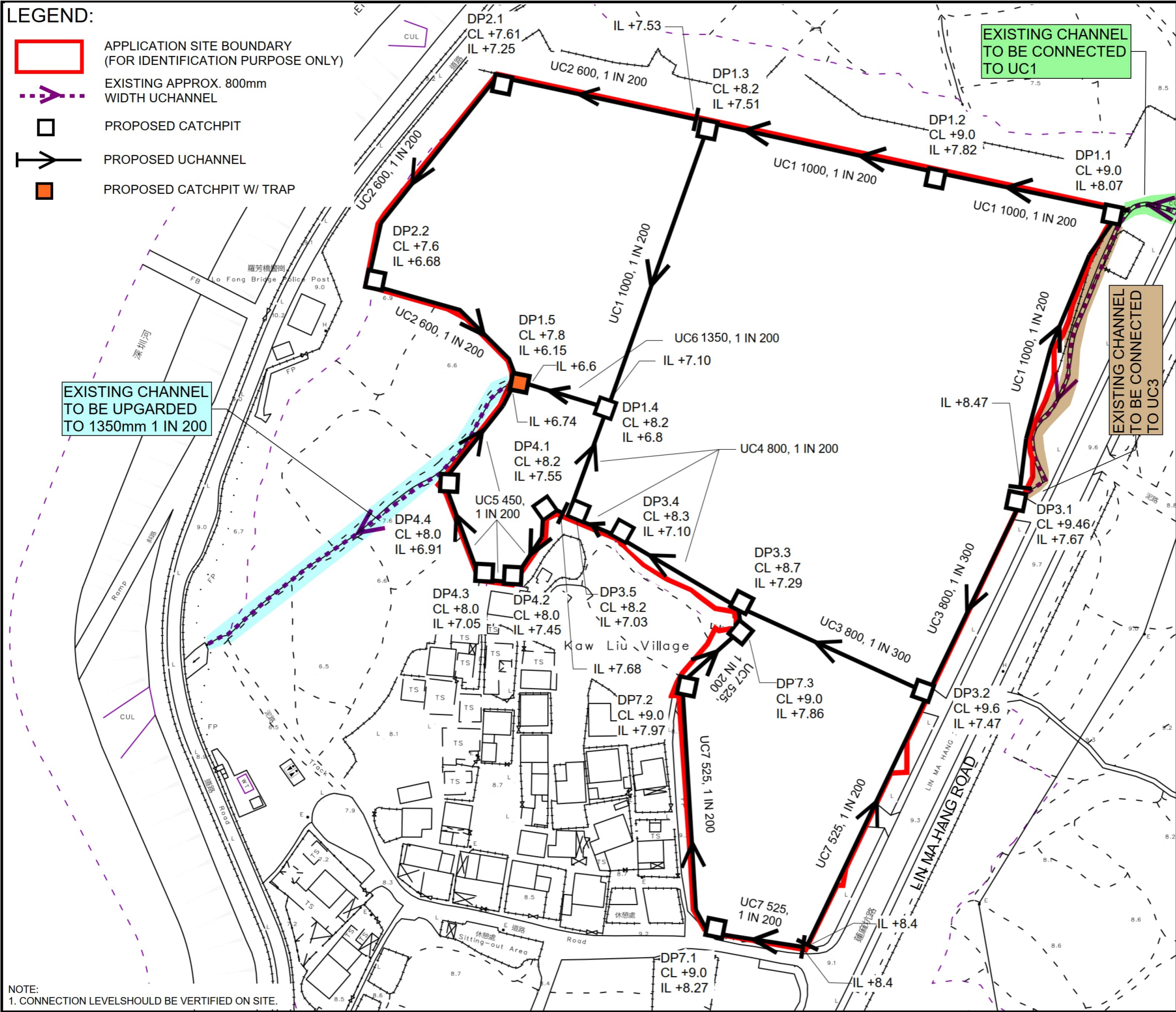
REV	DESCRIPTION	DATE

DRAWING TITLE  
**EXISTING DRAINAGE PLAN**

DRAWING NUMBER  
**FIGURE 2**

**LEGEND:**

- APPLICATION SITE BOUNDARY (FOR IDENTIFICATION PURPOSE ONLY)
- EXISTING APPROX. 800mm WIDTH UCHANNEL
- PROPOSED CATCHPIT
- PROPOSED UCHANNEL
- PROPOSED CATCHPIT W/ TRAP



EXISTING CHANNEL TO BE UPGRADUED TO 1350mm 1 IN 200

EXISTING CHANNEL TO BE CONNECTED TO UC1

EXISTING CHANNEL TO BE CONNECTED TO UC3

**NOTE:**  
 1. CONNECTION LEVELS SHOULD BE VERIFIED ON SITE.

REV	DESCRIPTION	DATE
DRAWING TITLE <b>PROPOSED DRAINAGE SYSTEM</b>		
DRAWING NUMBER <b>FIGURE 3</b>		

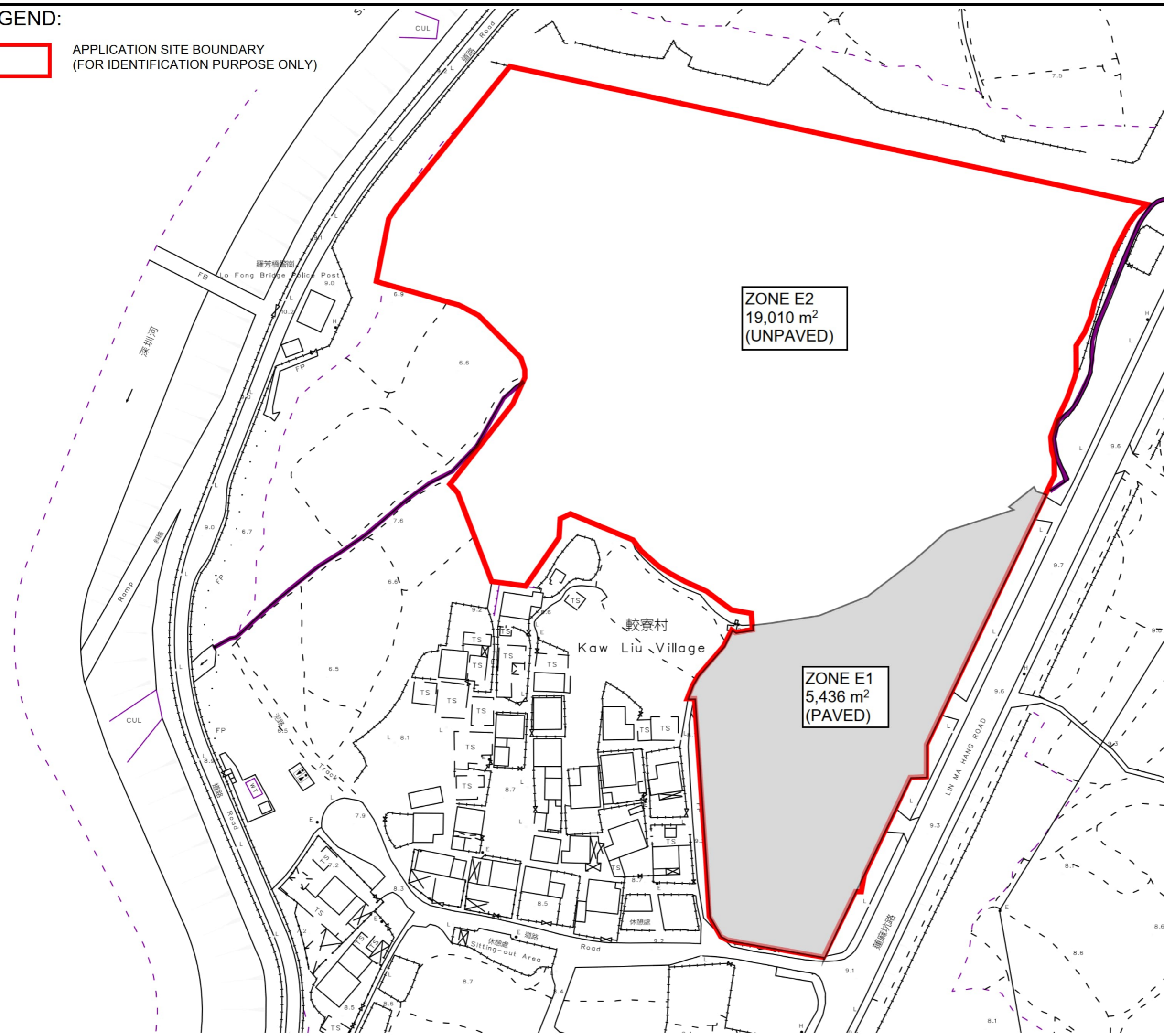
**LEGEND:**



APPLICATION SITE BOUNDARY  
(FOR IDENTIFICATION PURPOSE ONLY)

**PROJECT:**

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Warehouse (Excluding D.G.G.)  
with Ancillary Facilities for a  
Period of 3 Years in "Recreation"  
Zone, Various Lots in D.D.78 and  
Adjoining GL, Lin Ma Hang, New  
Territories




REV	DESCRIPTION	DATE

DRAWING TITLE  
**EXISTING CATCHMENT  
PLAN**

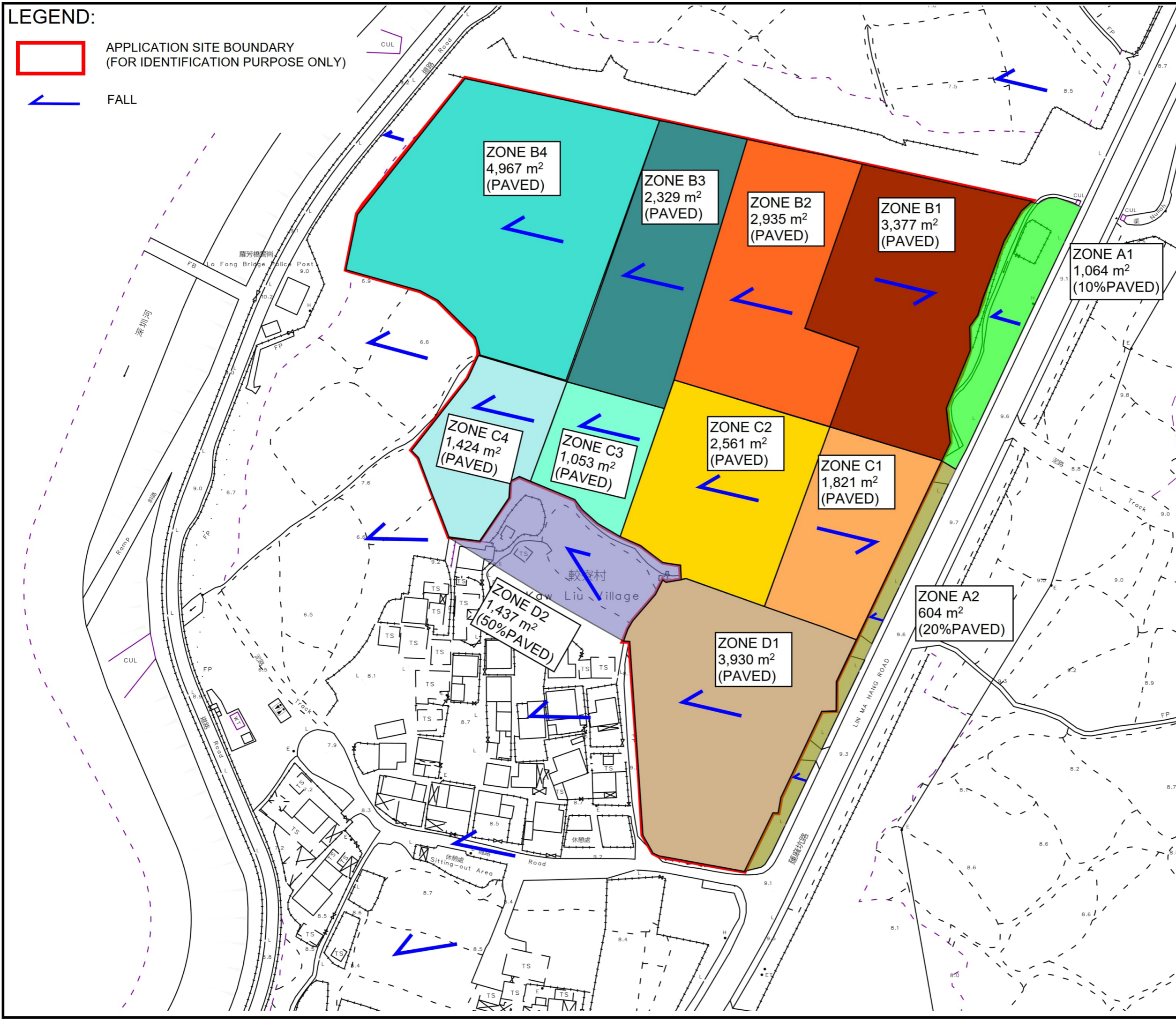
DRAWING NUMBER  
**FIGURE 4.1**

**LEGEND:**

 APPLICATION SITE BOUNDARY  
(FOR IDENTIFICATION PURPOSE ONLY)

 FALL

**PROJECT:**  
Proposed Temporary  
Warehouse (Excluding D.G.G.)  
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Territories



REV	DESCRIPTION	DATE

DRAWING TITLE  
**PROPOSED  
CATCHMENT PLAN**

DRAWING NUMBER  
**FIGURE 4.2**

# Appendix

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# Appendix A - Design Calculation

## Checking of Peak Flow of Existing Channel at Upstream of the Application Site

### U Channel Capacity Estimation

Channel Size		1 in	800	(mm)
Gradient (approx.)			186	
Area	$\pi \times 0.8^2 / 8 + 0.8 \times 0.8 / 2 =$		0.571	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.8 / 2 + 0.8 / 2 \times 2 =$		2.057	(m)
R	$0.571 / 2.057 =$		0.278	(m)
Velocity			2.23	m/s
Capacity			1.275	m <sup>3</sup> /s

$$v = \frac{R^2}{n} S_y^{1/2}$$

The estimated capacity of existing channel is 1.275 m<sup>3</sup>/s  
 Assume the utilization of the existing channel is 80%  
 The estimated peak flow of existing channel is 1.02 m<sup>3</sup>/s

## Checking of Additional Flow due to the Development

### Runoff Estimation (before development)

Design Return Period		1 in	50	years
Paved Area	5436 =		5436	(m <sup>2</sup> )
Unpaved Area	19010 =		19010	(m <sup>2</sup> )
Total Equivalent Area	$5436 \times 0.95 + 19010 \times 0.35 =$		11818	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	$0.278 \times 19010 \times 220 / 1000000 =$		0.724	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

### Runoff Estimation (after development)

Design Return Period		1 in	50	years
Paved Area	24446 =		24446	(m <sup>2</sup> )
Unpaved Area	0 =		0	(m <sup>2</sup> )
Total Equivalent Area	$24446 \times 0.95 + 0 \times 0.35 =$		23224	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	$0.278 \times 23224 \times 220 / 1000000 =$		1.423	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

Additional flow due to development  $1.423 - 0.724 = 0.699$  m<sup>3</sup>/s

## Proposed Upgrade of Existing Channel at Downstream of Application Site

	Existing U Channel	Proposed Updated Channel
Channel Size		
Gradient	1 in	1 in
Area	$\pi \times 0.8^2 / 8 + 0.8 \times 0.8 / 2 =$	$\pi \times 1.35^2 / 8 + 1.35 \times 1.35 / 2 =$
Wetted Perimeter	$\pi \times 0.8 / 2 + 0.8 / 2 \times 2 =$	$\pi \times 1.35 / 2 + 1.35 / 2 \times 2 =$
R	$0.571 / 2.057 =$	$1.627 / 3.471 =$
Velocity	2.23	3.05
Capacity	1.275	4.96

Capacity of upgraded channel is increased by  $4.959 - 1.275 = 3.684 > 0.699$  m<sup>3</sup>/s  
 Therefore, it is proposed to upgrade of downstream channel from 800mm, 1 in 186 to 1350mm, 1 in 200

## U Channel 1 (Zone B1 + B2 + B3 + Peak Flow from Existing Channel)

### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	3377 + 2935 + 2329 =		8641	(m <sup>2</sup> )
Unpaved Area	0 =		0	(m <sup>2</sup> )
Total Equivalent Area	$8641 \times 0.95 + 0 \times 0.35 =$		8209	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Q	$0.278 \times 8209 \times 220 / 1000000 =$		0.503	m <sup>3</sup> /s
Q + Peak Flow from existing channel	$0.503 + 1.02$		1.523	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

### U Channel

Channel Size		1 in	1000	(mm)
Gradient			200	
Area	$\pi \times 1^2 / 8 + 1 \times 1 / 2 =$		0.893	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 1 / 2 + 1 / 2 \times 2 =$		2.571	(m)
R	$0.893 / 2.571 =$		0.347	(m)
Velocity			2.50	m/s
Capacity			2.228	m <sup>3</sup> /s

$$v = \frac{R^2}{n} S_y^{1/2}$$

Utilization  $1.523 / 2.228 = 68.37$  %

OK (less than 90%, for 10% siltation allowance)

## U Channel 2 (ZONE B4)

### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	4967 =		4967	(m <sup>2</sup> )
Unpaved Area	0 =		0	(m <sup>2</sup> )
Total Equivalent Area	$4967 \times 0.95 + 0 \times 0.35 =$		4719	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	$0.278 \times 4719 \times 220 / 1000000 =$		0.289	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

### U Channel

Channel Size		1 in	600	(mm)
Gradient			200	
Area	$\pi \times 0.6^2 / 8 + 0.6 \times 0.6 / 2 =$		0.321	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.6 / 2 + 0.6 / 2 \times 2 =$		1.542	(m)
R	$0.321 / 1.542 =$		0.208	(m)
Velocity			1.78	m/s
Capacity			0.570	m <sup>3</sup> /s

$$v = \frac{R^2}{n} S_y^{1/2}$$

Utilization  $0.289 / 0.57 = 50.69$  %

OK (less than 90%, for 10% siltation allowance)

### U Channel 3 (Zone A1 + A2 + C1 + D1)

#### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	1821 + 1064 x 0.1 + 604 x 0.2 + 3930 =		5978	(m <sup>2</sup> )
Unpaved Area	0 + 1064 x 0.9 + 604 x 0.8 =		1441	(m <sup>2</sup> )
Total Equivalent Area	5978 x 0.95 + 1441 x 0.35 =		6184	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	0.278 x 6184 x 220 / 1000000 =		0.379	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

#### U Channel

Channel Size		1 in	800	(mm)
Gradient			300	
Area	$\pi \times 0.8^2 / 8 + 0.8 \times 0.8 / 2 =$		0.571	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.8 / 2 + 0.8 / 2 \times 2 =$		2.057	(m)
R	$0.571 / 2.057 =$		0.278	(m)
Velocity			1.76	m/s
Capacity			1.003	m <sup>3</sup> /s

$$v = \frac{R^{2/3}}{n} R^{2/3} S_f^{1/2}$$

Utilization  $\frac{0.379}{1.003} = 37.78\%$

OK (less than 90%, for 10% siltation allowance)

### U Channel 4 (Zone [A1 + A2 + C1 + D1] + C2 + C3 + D2)

#### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	5978 + 2561 + 1053 + 1437 x 0.5 =		10311	(m <sup>2</sup> )
Unpaved Area	1441 + 1053 + 1437 x 0.5 =		2159	(m <sup>2</sup> )
Total Equivalent Area	10311 x 0.95 + 2159 x 0.35 =		10551	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	0.278 x 10551 x 220 / 1000000 =		0.647	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

#### U Channel

Channel Size		1 in	800	(mm)
Gradient			200	
Area	$\pi \times 0.8^2 / 8 + 0.8 \times 0.8 / 2 =$		0.571	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.8 / 2 + 0.8 / 2 \times 2 =$		2.057	(m)
R	$0.571 / 2.057 =$		0.278	(m)
Velocity			2.15	m/s
Capacity			1.229	m <sup>3</sup> /s

$$v = \frac{R^{2/3}}{n} R^{2/3} S_f^{1/2}$$

Utilization  $\frac{0.647}{1.229} = 52.63\%$

OK (less than 90%, for 10% siltation allowance)

### U Channel 5 (Zone C4 + D2)

#### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	1424 + 1437 x 0.5 =		2143	(m <sup>2</sup> )
Unpaved Area	0 + 1437 x 0.5 =		719	(m <sup>2</sup> )
Total Equivalent Area	2143 x 0.95 + 719 x 0.35 =		2287	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	0.278 x 2287 x 220 / 1000000 =		0.140	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

#### U Channel

Channel Size		1 in	450	(mm)
Gradient			200	
Area	$\pi \times 0.45^2 / 8 + 0.45 \times 0.45 / 2 =$		0.181	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.45 / 2 + 0.45 / 2 \times 2 =$		1.157	(m)
R	$0.181 / 1.157 =$		0.156	(m)
Velocity			1.47	m/s
Capacity			0.265	m <sup>3</sup> /s

$$v = \frac{R^{2/3}}{n} R^{2/3} S_f^{1/2}$$

Utilization  $\frac{0.14}{0.265} = 52.91\%$

OK (less than 90%, for 10% siltation allowance)

### U Channel 6 (Whole Site (paved) + A1 + A2 + D2 + Peak Flow from Existing Channel)

#### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	24446 + 1064 x 0.1 + 604 x 0.2 + 1437 x 0.5 =		25392	(m <sup>2</sup> )
Unpaved Area	0 + 1064 x 0.9 + 604 x 0.8 + 1437 x 0.5 =		2159	(m <sup>2</sup> )
Total Equivalent Area	25392 x 0.95 + 2159 x 0.35 =		24878	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Q	0.278 x 24878 x 220 / 1000000 =		1.525	m <sup>3</sup> /s
Q + Peak Flow from existing channel	1.525 + 1.02 =		2.544	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

#### U Channel

Channel Size		1 in	1350	(mm)
Gradient			200	
Area	$\pi \times 1.35^2 / 8 + 1.35 \times 1.35 / 2 =$		1.627	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 1.35 / 2 + 1.35 / 2 \times 2 =$		3.471	(m)
R	$1.627 / 3.471 =$		0.469	(m)
Velocity			3.05	m/s
Capacity			4.959	m <sup>3</sup> /s

$$v = \frac{R^{2/3}}{n} R^{2/3} S_f^{1/2}$$

Utilization  $\frac{1.525}{4.959} = 51.31\%$

OK (less than 90%, for 10% siltation allowance)

### U Channel 7 (Zone A2, D1)

#### Runoff Estimation

Design Return Period		1 in	50	years
Paved Area	3930 + 604 x 0.2 =		4051	(m <sup>2</sup> )
Unpaved Area	0 + 604 x 0.8 =		483	(m <sup>2</sup> )
Total Equivalent Area	4051 x 0.95 + 483 x 0.35 =		4017	(m <sup>2</sup> )
Rainfall Intensity, I *			220	mm/hr
Design Discharge Rate, Q	0.278 x 4017 x 220 / 1000000 =		0.246	m <sup>3</sup> /s

$$i = \frac{a}{(t_d + b)^c}$$

#### U Channel

Channel Size		1 in	525	(mm)
Gradient			200	
Area	$\pi \times 0.53^2 / 8 + 0.53 \times 0.53 / 2 =$		0.246	(m <sup>2</sup> )
Wetted Perimeter	$\pi \times 0.53 / 2 + 0.53 / 2 \times 2 =$		1.350	(m)
R	$0.246 / 1.35 =$		0.182	(m)
Velocity			1.62	m/s
Capacity			0.400	m <sup>3</sup> /s

$$v = \frac{R^{2/3}}{n} R^{2/3} S_f^{1/2}$$

Utilization  $\frac{0.246}{0.4} = 61.62\%$

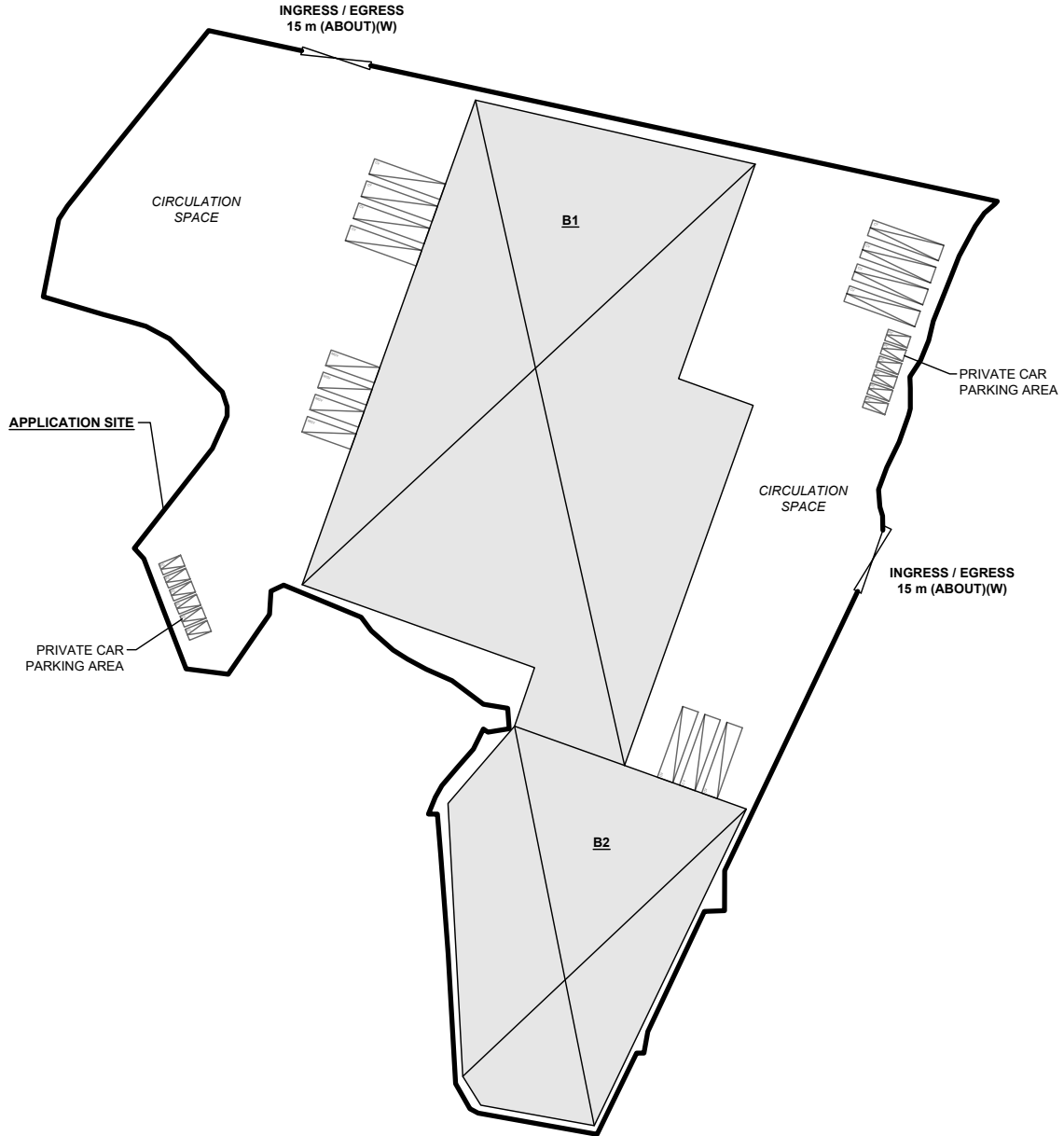
OK (less than 90%, for 10% siltation allowance)

# Appendix B - Proposed Development Layout Plan

## DEVELOPMENT PARAMETERS

APPLICATION SITE AREA	: 24,446 m <sup>2</sup>	(ABOUT)
COVERED AREA	: 11,879 m <sup>2</sup>	(ABOUT)
UNCOVERED AREA	: 12,567 m <sup>2</sup>	(ABOUT)
PLOT RATIO	: 0.97	(ABOUT)
SITE COVERAGE	: 49 %	(ABOUT)
NO. OF STRUCTURE	: 2	
DOMESTIC GFA	: NOT APPLICABLE	
NON-DOMESTIC GFA	: 23,758 m <sup>2</sup>	(ABOUT)
TOTAL GFA	: 23,758 m <sup>2</sup>	(ABOUT)
BUILDING HEIGHT	: 15 m	(ABOUT)
NO. OF STOREY	: 2	

		AREA		HEIGHT
B1	WAREHOUSE (EXCL. D.G.G.) SITE OFFICE AND WASHROOM	8,332 m <sup>2</sup> (ABOUT)	16,664 m <sup>2</sup> (ABOUT)	15 m (ABOUT)(2-STOREY)
B2	WAREHOUSE (EXCL. D.G.G.) SITE OFFICE AND WASHROOM	3,547 m <sup>2</sup> (ABOUT)	7,094 m <sup>2</sup> (ABOUT)	15 m (ABOUT)(2-STOREY)
<b>TOTAL</b>		<b>11,879 m<sup>2</sup> (ABOUT)</b>	<b>23,758 m<sup>2</sup> (ABOUT)</b>	



## PARKING AND LOADING / UNLOADING (L/UL) PROVISIONS

NO. OF PRIVATE CAR PARKING SPACE	: 10
DIMENSION OF PARKING SPACE	: 5 m (L) x 2.5 m (W)
NO. OF CONTAINER VEHICLE PARKING SPACE	: 4
DIMENSION OF LOADING/UNLOADING SPACE	: 16 m (L) x 3.5 m (W)
NO. OF L/UL SPACE FOR MEDIUM GOODS VEHICLE (MGV)	: 4
DIMENSION OF LOADING/UNLOADING SPACE	: 11 m (L) x 3.5 m (W)
NO. OF L/UL SPACE FOR CONTAINER VEHICLE	: 7
DIMENSION OF LOADING/UNLOADING SPACE	: 16 m (L) x 3.5 m (W)

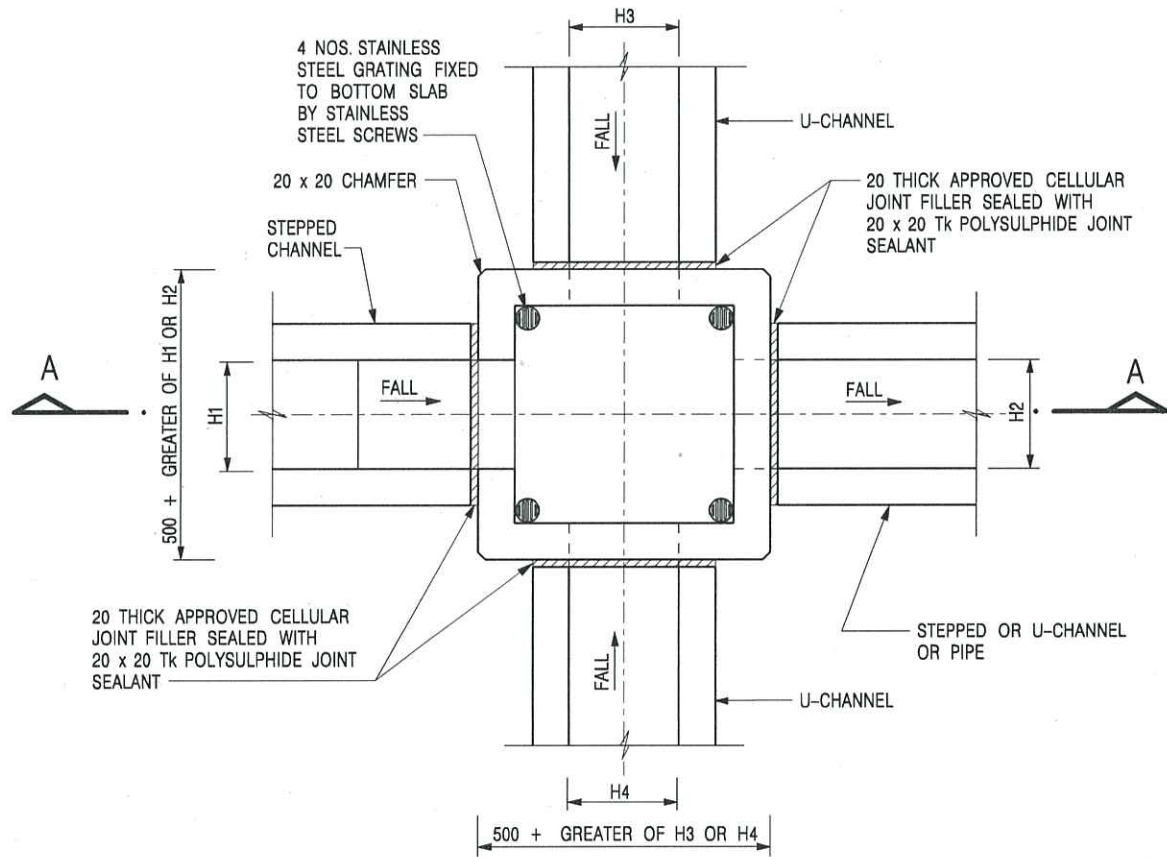
## LEGEND

	APPLICATION SITE
	STRUCTURE
	PARKING SPACE (PC)
	PARKING SPACE (CV)
	LOADING / UNLOADING SPACE (MGV)
	LOADING / UNLOADING SPACE (CV)
	INGRESS / EGRESS

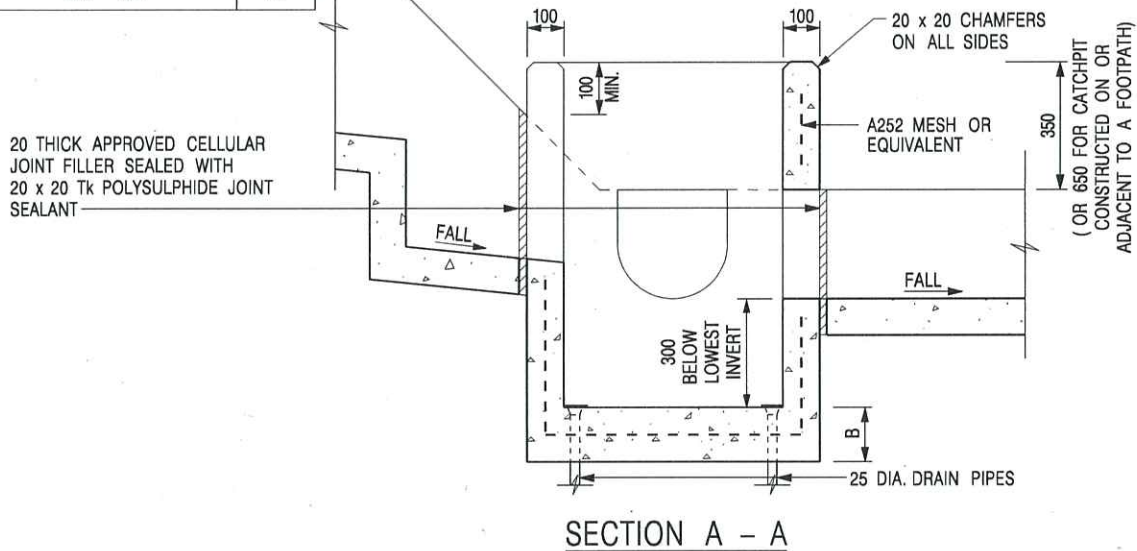
PLANNING CONSULTANT 	PROJECT PROPOSED TEMPORARY WAREHOUSE (EXCLUDING DANGEROUS GOODS GODOWN) WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS	ADDRESS VARIOUS LOTS IN D.D. 78 AND ADJOINING GOVERNMENT LAND, LIN MA HANG, NEW TERRITORIES	SCALE 1 : 1500 @ A4	TITLE LAYOUT PLAN		
			DRAWN BY MN	DATE 14.5.2024	DWG NO. PLAN 9	



# Appendix C - Reference Drawings



NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
300 - 600	150
675 - 900	175



**NOTES:**

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. REFER TO SHEET 2 FOR OTHER NOTES.

CATCHPIT WITH TRAP  
(SHEET 1 OF 2)

REF.	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE



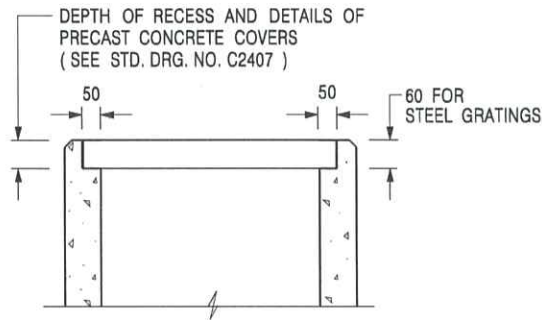
CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT

SCALE 1 : 20

DRAWING NO.

DATE JAN 1991

C2406 /1



ALTERNATIVE TOP SECTION  
FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2 ) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407 ) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE ) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043 ) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

A	MINOR AMENDMENT.	Original Signed	04.2016
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
<b>REF.</b>	<b>REVISION</b>	<b>SIGNATURE</b>	<b>DATE</b>

CATCHPIT WITH TRAP  
(SHEET 2 OF 2)



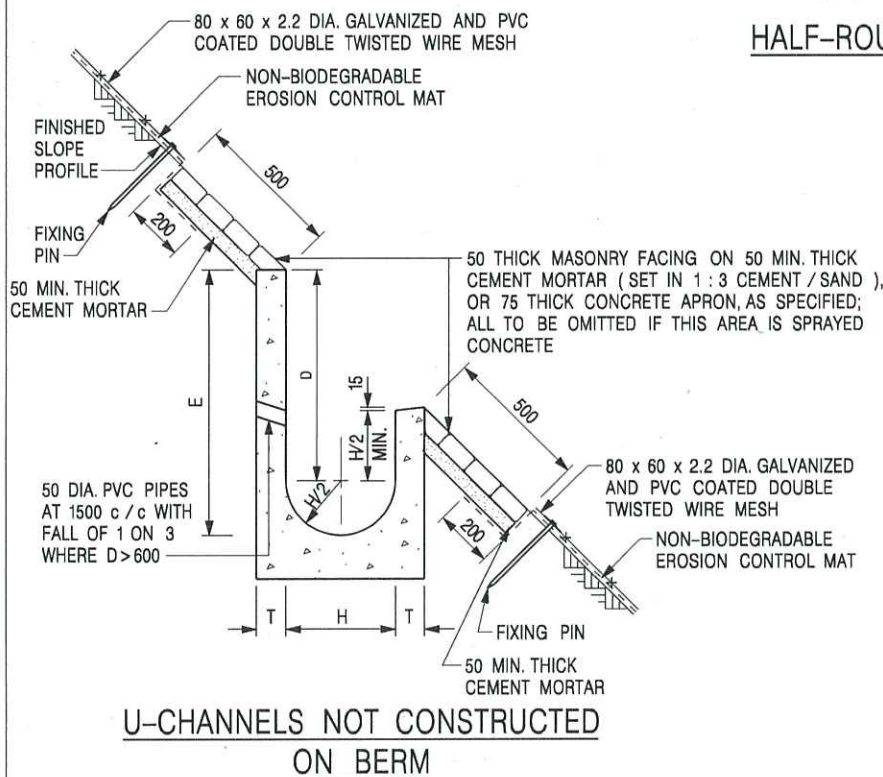
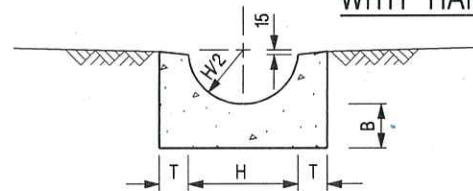
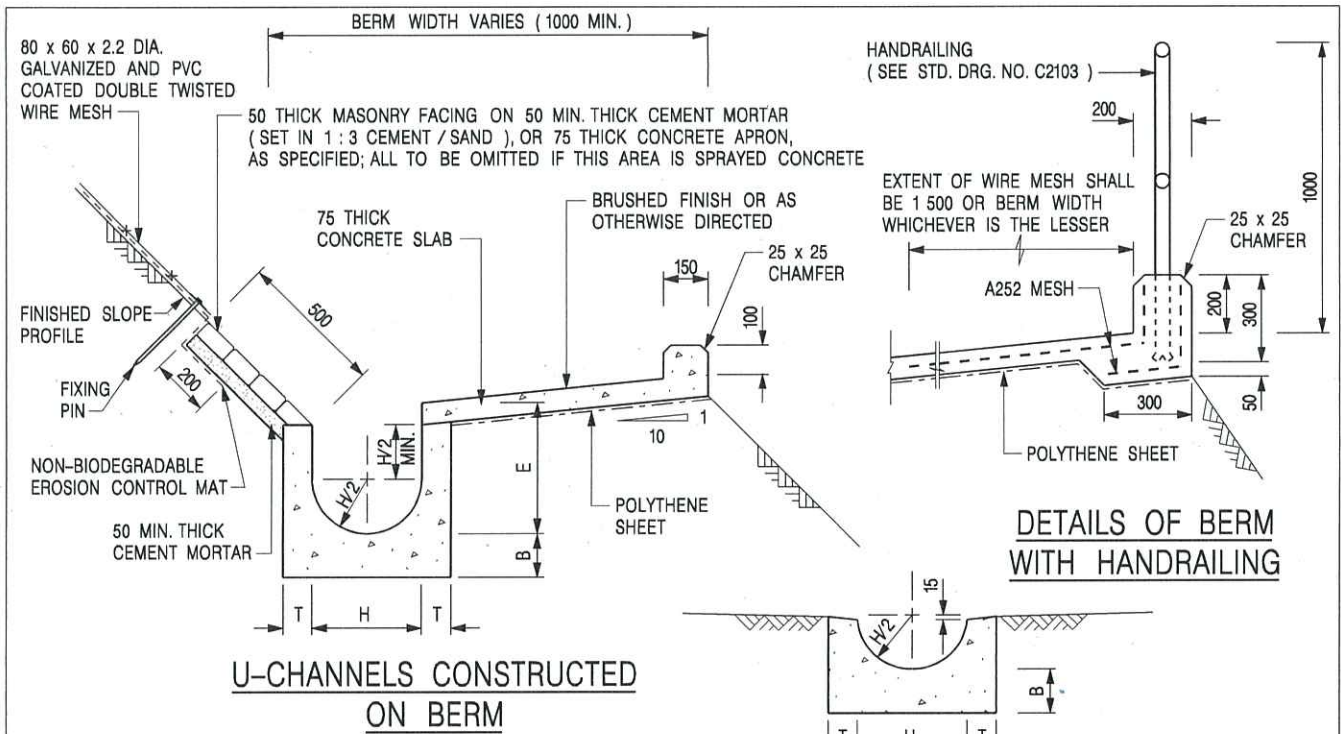
**CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT**

**SCALE** 1 : 20

**DRAWING NO.**

**DATE** JAN 1991

**C2406 /2A**



**NOTES:**

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE TO BE GRADE 20 / 20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
4. SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
7. BIODEGRADABLE EROSION CONTROL MAT IF REQUIRED, SEE STD. DRG. NO. C2511/E.
8. CONCRETE TO BE COLOURED AS SPECIFIED.
9. CONCRETE U-CHANNEL CAN BE CAST IN-SITU OR PRECAST CONCRETE SUBJECT TO THE ENGINEER'S AGREEMENT ON THE DETAILS.
10. DETAILS OF EROSION CONTROL MAT AND WESH MESH ON BERM. (SEE STD DRG. NO. C2511/E)

NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E > 650
375 - 600	100	150	
675 - 900	125	175	A252 MESH PLACED CENTRALLY

I	MINOR AMENDMENT.	Original Signed	07.2018
H	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
G	MINOR AMENDMENT.	Original Signed	01.2004
F	GENERAL REVISION.	Original Signed	12.2002
E	DRAWING TITLE AMENDED.	Original Signed	11.2001
D	MINOR AMENDMENT.	Original Signed	08.2001
C	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
B	MINOR AMENDMENTS.	Original Signed	3.94
REF.	REVISION	SIGNATURE	DATE

**DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A WITH MASONRY APRON)**



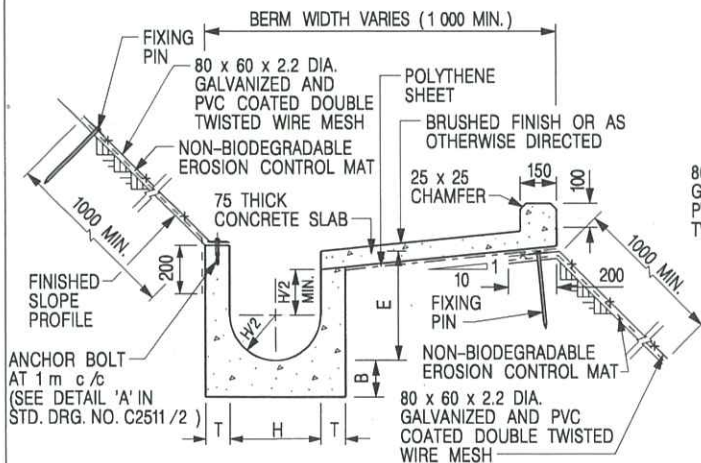
**CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT**

**SCALE** 1 : 25

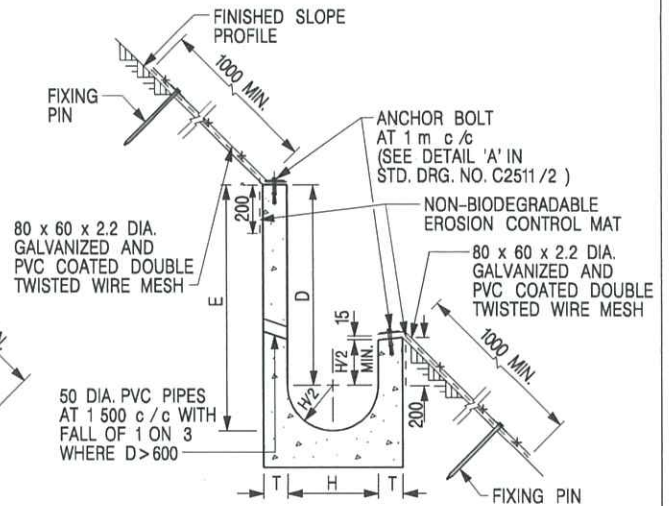
**DRAWING NO.**

**DATE** JAN 1991

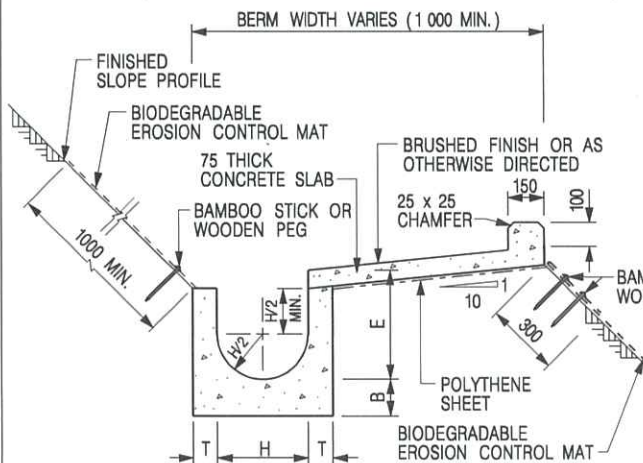
**C24091**



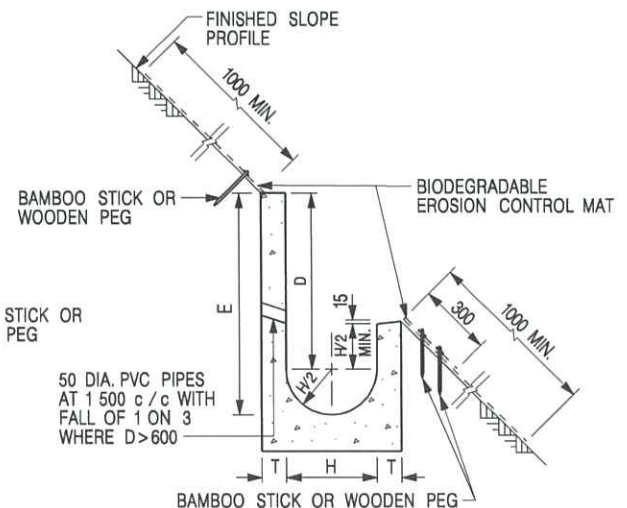
**U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT**



**U-CHANNELS CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT**



**U-CHANNELS NOT CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT**

**NOTES:**

- ALL DIMENSIONS ARE IN MILLIMETRES.
- ALL CONCRETE TO BE GRADE 20 /20.
- CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E > 650
375 - 600	100	150	
675 - 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
I	MINOR AMENDMENT.	Original Signed	07.2018
H	FIXING DETAILS OF BIODEGRADABLE EROSION CONTROL MAT ADDED.	Original Signed	12.2017
G	DIMENSION TABLE AMENDED.	Original Signed	01.2005
F	MINOR AMENDMENT.	Original Signed	01.2004
E	GENERAL REVISION.	Original Signed	12.2002
D	MINOR AMENDMENT.	Original Signed	08.2001
C	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
B	MINOR AMENDMENT.	Original Signed	3.94
A	MINOR AMENDMENT.	Original Signed	10.92

**DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE B - WITH EROSION CONTROL MAT APRON)**



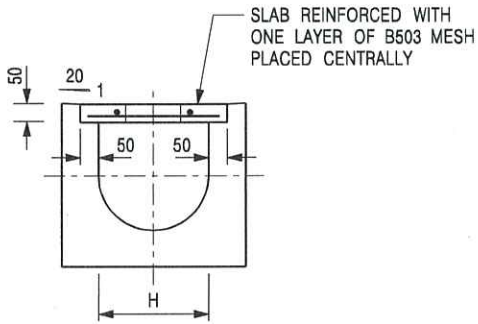
**CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT**

**SCALE** DIAGRAMMATIC

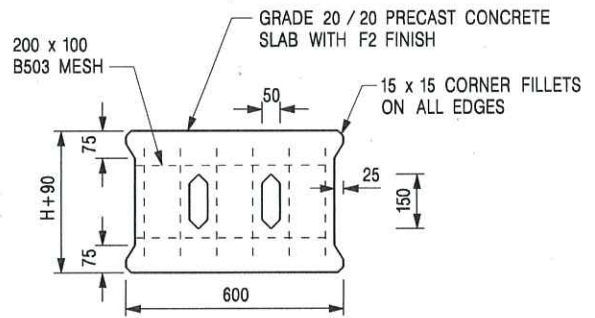
**DRAWING NO.**

**DATE** JAN 1991

**C24101**



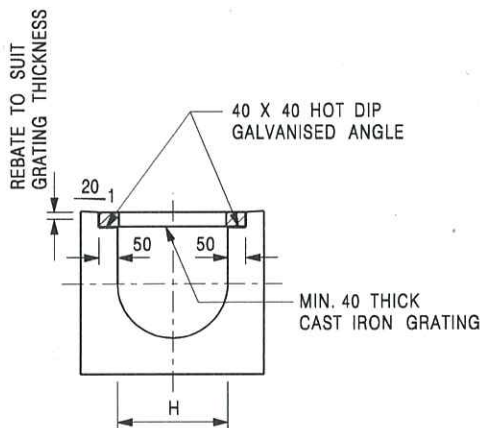
TYPICAL SECTION



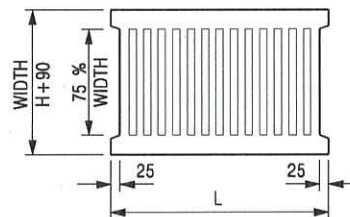
PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)



TYPICAL SECTION



L = 600mm FOR H ≤ 375mm  
L = 400mm FOR H > 375mm

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. H=NOMINAL CHANNEL SIZE.
3. ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

E	NOTES 3 & 4 AMENDED.	Original Signed	12.2014
D	NOTE 4 ADDED.	Original Signed	06.2008
C	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
B	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
A	CAST IRON GRATING AMENDED.	Original Signed	12.2002
REF.	REVISION	SIGNATURE	DATE

COVER SLAB AND CAST IRON  
GRATING FOR CHANNELS



CIVIL ENGINEERING AND  
DEVELOPMENT DEPARTMENT

SCALE 1 : 20

DRAWING NO.

DATE JAN 1991

C2412E