

Appendix VII

Drainage Impact Assessment

**DRAINAGE IMPACT ASSESSMENT FOR
PROPOSED RESIDENTIAL DEVELOPMENT AT
LOT 1109 RP (PART) IN D.D. 253/8 KA SHUE
ROAD, SAI KUNG, NEW TERRITORIES
DRAINAGE IMPACT ASSESSMENT**

DECEMBER 16, 2024

CONFIDENTIAL





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Proposed Residential Development at Lot
1109 RP (Part) in D.D. 253/8 Ka Shue Road, Sai
Kung, New Territories**

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DRAINAGE IMPACT ASSESSMENT

DATE: DECEMBER 16, 2024

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1 INTRODUCTION

1.1 BACKGROUND

- 1.1.1 The applicant proposes to redevelop the Application Site at Lot 1109 RP (Part) in D.D. 253,8 Ka Shue Road, Sai Kung, New Territories with minor relaxation of building height and site coverage restrictions. The Application Site falls within “Residential (Group C) 1” zone on the approved Tseng Lan Shue Outline Zoning Plan (OZP) No. S/SK-TLS/10.
 - 1.1.2 The Applicants commissioned WSP (Asia) Limited (WSP) as the Consultant to carry out a Drainage Impact Assessment (DIA) for the proposed development under this planning application.
-

1.2 APPLICATION SITE AND ITS ENVIRONS

- 1.2.1 The Application Site covers an area of approximately 1,572m² and is situated in Sai Kung, New Territories. The proposed development site is bounded by Ka Shue Road.
 - 1.2.2 The existing topography of the Project Site is predominantly flat, with a slight slope. The highest point is located in the middle of the site at +228.8mPD, gently sloping down to approximately +228.7mPD near Clear Water Bay Apartments Blocks F and E, and further descending to +220mPD at the northern edge of the site. The Application Site is surrounded by existing old buildings and areas of greenery. For the location of subject site, please refer to **Figure 1**.
-

1.3 PROJECT DESCRIPTION

- 1.3.1 The proposed development consists of two residential towers with soft landscape.
- 1.3.2 The proposed development's ground level will feature a combination of greenery and concrete paving. The ratio of paved to unpaved areas will remain largely consistent with the existing site layout.
- 1.3.3 The master layout plan is shown in **Appendix A**.

2 ASSESSMENT CRITERIA AND METHODOLOGY

2.1 GENERAL

- 2.1.1 The aim of this study is to assess the changes to runoff from the Application Site as a result of the proposed development and the potential impacts on the existing drainage system.
-

2.2 METHODOLOGY

Catchment Area

- 2.2.1 The catchment areas for the purpose of this DIA have been defined and details could be referred to Sections 3.

Storm Return Period

- 2.2.2 According to DSD's "Stormwater Drainage Manual – Fifth Edition January 2018 (SDM)", the category of stormwater drainage system of the site is classified as "Urban Drainage Branch Systems", so 1 in 50-year storm return period was adopted in this assessment. Effect of rainfall increase and sea level rise due to climate change by end of 21st Century are taken into account by adding 16% and 12.1% to the surface runoff from the proposed development in accordance with the Table 28 and Table 31 SDM Corrigendum No. 1/2022, respectively.

Sea Tidal Effect

- 2.2.3 The proposed project site is not subject to sea tidal effect and therefore sea level is not considered in the DIA.

Basic Formulation of Rational Method

- 2.2.4 Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2(a) of SDM which is to estimate the stormwater runoff as shown below:

$$Q_p = 0.278CiA$$

where Q_p = Peak runoff in m³/s
 C = Runoff coefficient (dimensionless)
 i = Rainfall intensity in mm/hr
 A = Catchment area in km²

Time of Concentration

- 2.2.5 The time of concentration of urban area is assumed as 5 minutes as a conservative approach.

Runoff Intensity

- 2.2.6 The rainfall intensity is referenced from the Section 4.3.2 of SDM (Fifth Edition) and its Corrigendum 1/2024 which is to estimate the Intensity-Duration-Frequency (IDF) Relationship. The design storm return period is 50 years, the values of a, b and c are 505.5, 3.29 and 0.355, respectively.

$$i = \frac{a}{(t_d + b)^c}$$

where i = Extreme mean intensity in mm/hr

t_d = Duration in minutes ($t_d < 240$)
 a, b, c = Storm constants given in SDM Table 3a.

- **Runoff Coefficient**

2.2.7 According to surface characteristics of the catchments, the runoff coefficient for paved area and unpaved area is 0.95 and 0.30 respectively.

•

2.2.8 Colebrook-White Equation is used in hydraulic design for pipe flow.

$$V = -\sqrt{32gRs} \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRs}} \right)$$

where V = Mean velocity (m/s)
 g = Gravitational acceleration (m/s²)
 R = Hydraulic radius (m)
 D = Pipe diameter (m)
 k_s = Equivalent sand roughness (m)
 v = Kinematic viscosity of fluid (m²/s)
 s = Frictional slope (energy gradient due to frictional loss)

2.2.9 Manning's Equation is used in hydraulic design for open channel flow.

$$V = \frac{Ar^{1/6}(rs)^{1/2}}{n}$$

where V = Mean velocity (m/s)
 A = Cross sectional area of flow (m²)
 s = Frictional slope (Energy gradient due to frictional loss)
 r = Hydraulic radius (m)
 n = Manning coefficient of roughness

3 DRAINAGE IMPACT ASSESSMENT

3.1 BEFORE DEVELOPMENT

- 3.1.1 The Application Site is currently occupied by a residential building and characterized by a combination of concrete surfaces and greenery area.
- 3.1.2 The **assessment area** has been divided into Catchment A (including A1 to A3) and Catchment B (B1 to B4) based on their unique boundaries and topographical features. The characteristics of the Catchment A and Catchment B are shown in **Figure 2** and summarised in **Table 3-1**.
- 3.1.3 According to the site observations, the existing drainage system for the Application Site will be collected by the on-site U-channels and eventually discharge to two stepped channels as shown in **Figure 3**. The runoff from Catchment A would be discharged to the Stepped Channel 1, whereas the runoff from Catchment B will be discharged into Stepped Channel 2.

Table 3-1 Summary of Catchment Characteristics Before Development

Catchment	Sub-Catchment	Runoff coefficient (C)	Total Area (m ²)	Estimated Runoff ^[1] (m ³ /s)	Receiving Stepped Channel
A	A1	0.95	317.9	0.049	Existing Stepped Channel 1
	A2	0.30	143.8		
	A3	0.95	302.7		
B	B1	0.95	642.41	0.054	Existing Stepped Channel 2
	B2	0.30	56.78		
	B3	0.30	221.88		
	B4	0.30	15.46		

Note[1] :The estimated runoff would be included 16% climate change factor as conservative.

3.2 AFTER DEVELOPMENT

- 3.2.1 After development, there is no major change on the landscape characteristic (i.e. combination of concrete surface and greenery area).
- 3.2.2 The catchment of the **assessment area** has been re-defined into Catchment P-A (including P-A1 to P-A6) and Catchment P-B (P-B1 to P-B9). The characteristics of the Catchment P-A and Catchment P-B are shown in **Figure 4** and summarised in **Table 3-2**.
- 3.2.3 The Application Site will follow the existing drainage system. The collected surface runoff would be discharged to the existing stepped channels as shown in **Figure 5**. The runoff from Catchment P-A would be discharged to the Stepped Channel 1, whereas the runoff from Catchment P-B will be discharged into Stepped Channel 2.

Table 3-2 Summary of Catchment Characteristics After Development

Catchment	Sub-Catchment	Runoff coefficient (C)	Total Area (m ²)	Estimated Runoff ^[1] (m ³ /s)	Receiving Stepped-Channel
P-A1	P-A1	0.30	144.6	0.048	Existing Stepped Channel 1
	P-A2	0.30	74.1		
	P-A3	0.30	8.4		
	P-A4	0.95	264.9		
	P-A5	0.30	1.9		
	P-A6	0.95	314.8		
P-B1	P-B1	0.30	28.4	0.053	Existing Stepped Channel 2
	P-B2	0.30	42.9		
	P-B3	0.95	55.2		
	P-B4	0.30	24.9		
	P-B5	0.95	249.0		
	P-B6	0.30	131.3		
	P-B7	0.30	3.7		
	P-B8	0.95	343.7		
	P-B9	0.30	1.6		

Note[1] :The estimated runoff would be included 16% climate change factor as conservative.

3.3 ASSESSMENT FINDINGS

- 3.3.1 Base on the catchment area as mentioned in **Table 3-1** and **Table 3-2**, the runoff from the proposed development before and after development was estimated based on the return periods of 50 years as detailed in **Appendix B**.
- 3.3.2 **Table 3-3** presents a comparative analysis of the estimated peak runoff under existing and proposed development conditions. For Existing Stepped Channel 1, the peak runoff is projected to decrease marginally from 0.049m³/s to 0.048m³/s, while for Existing Stepped Channel 2, it is expected to reduce slightly from 0.054m³/s to 0.053m³/s. These calculations, based on a 50-year return period, demonstrate that the proposed development will not lead to an increase in peak runoff. Hence, no significant adverse impact is anticipated due to the proposed development on the existing drainage system.

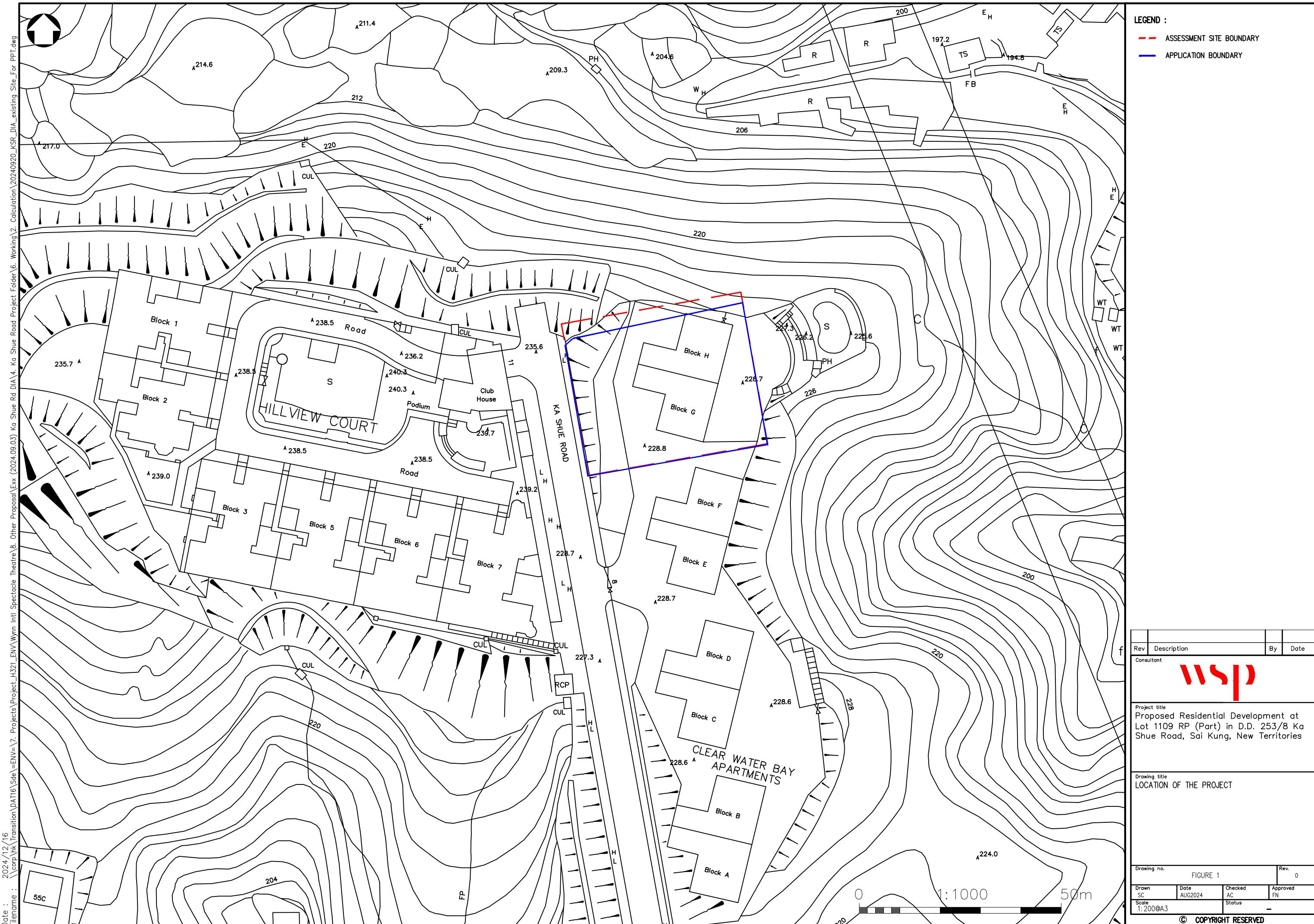
Table 3-3 Estimated Peak Runoff of the Existing Condition and Proposed Development

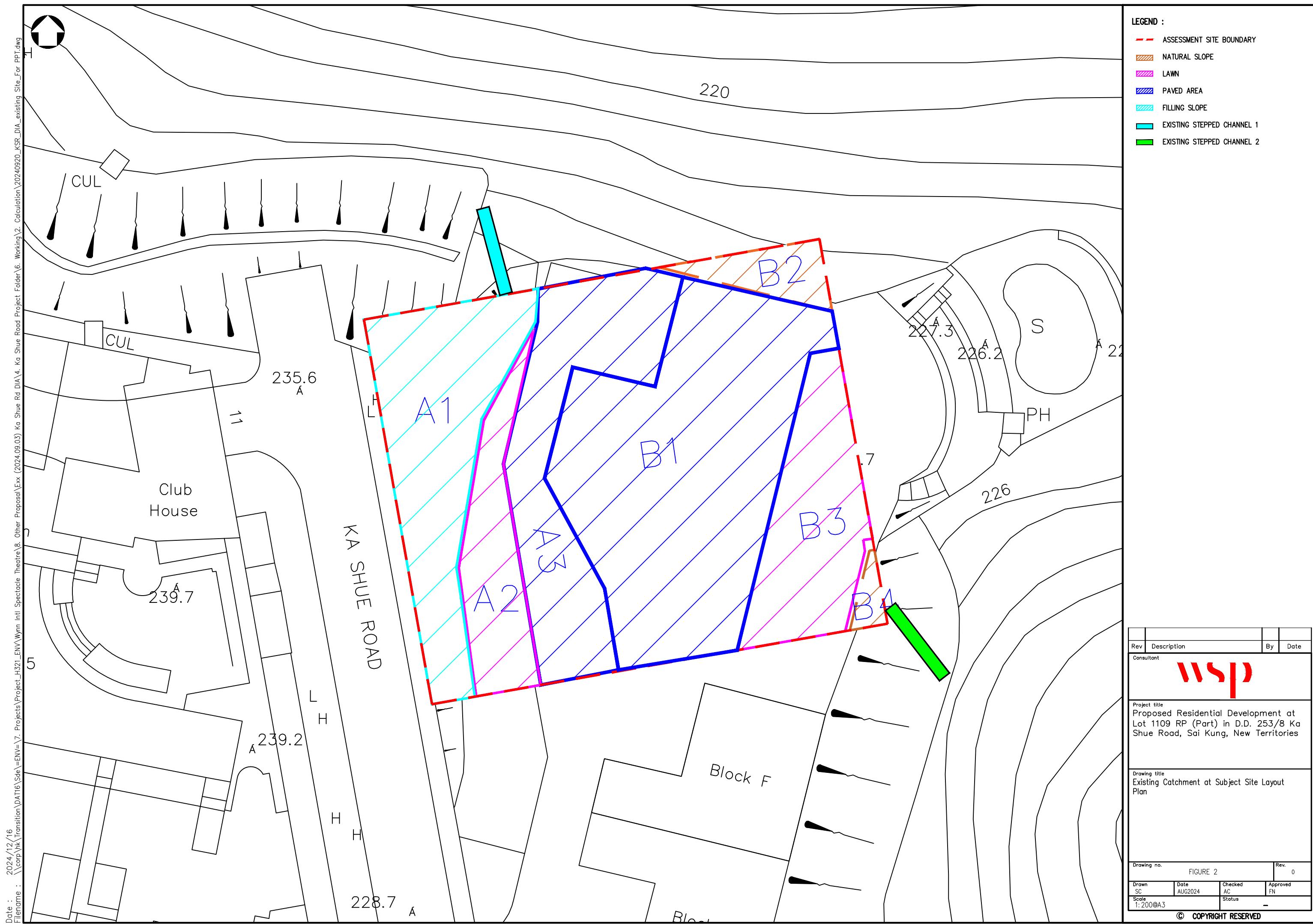
	Estimate Peak Runoff (m ³ /s)		
	Existing Condition	After Development	Increasement ?
Existing Stepped Channel 1	0.049	0.048	No
Existing Stepped Channel 2	0.054	0.053	No

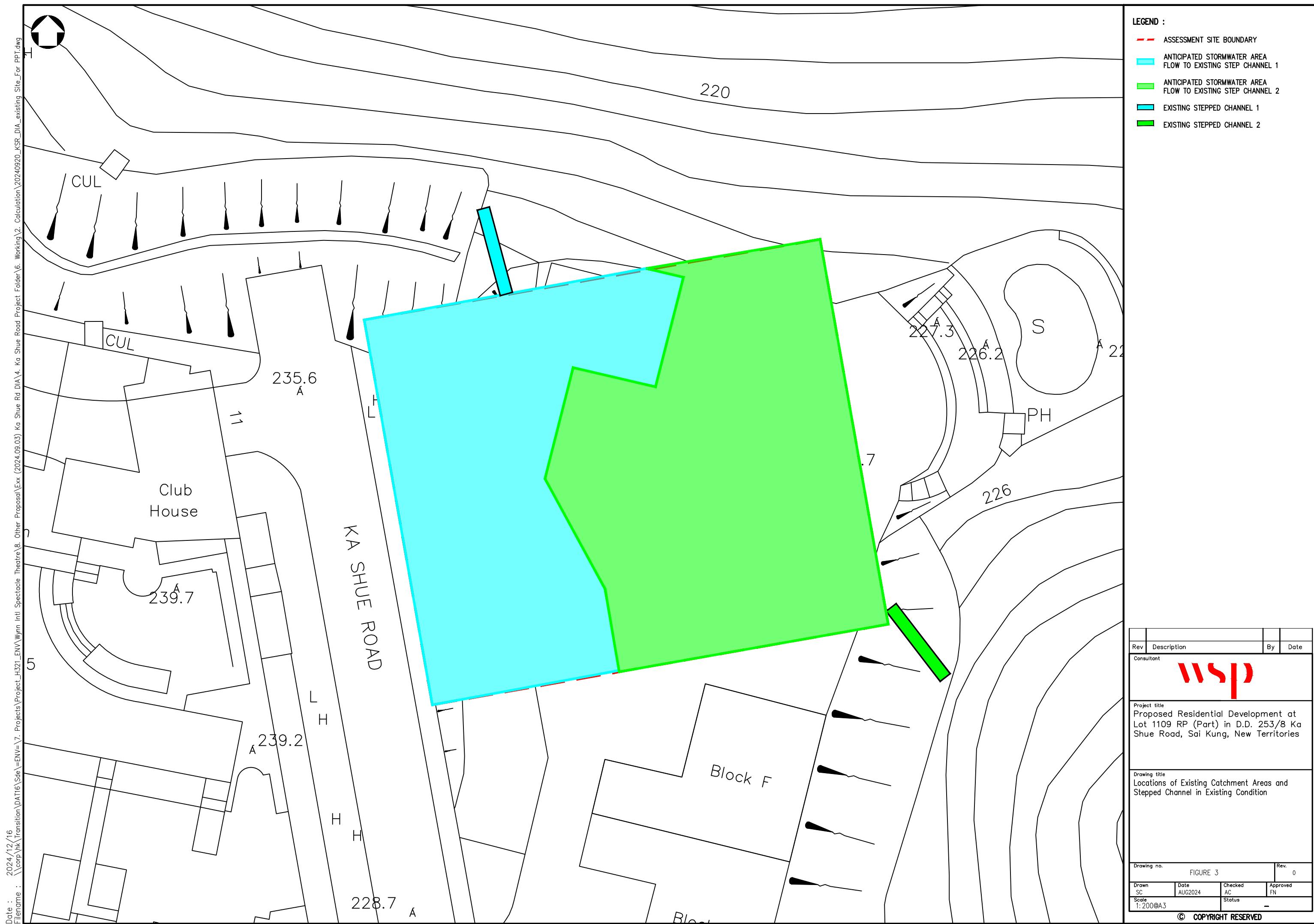
4 CONCLUSIONS

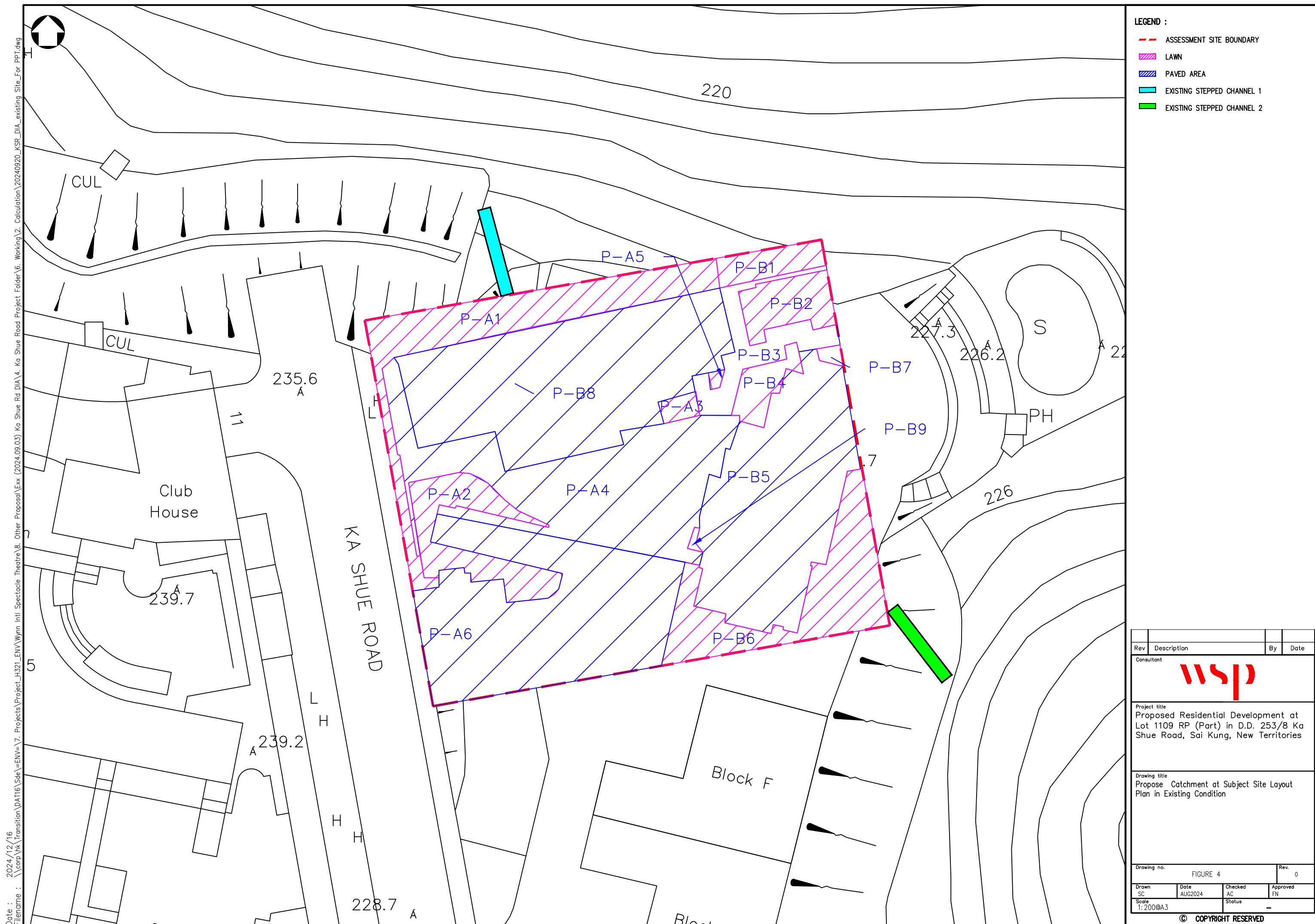
- 4.1.1 Based on the calculation, no additional runoff due to the proposed development is anticipated. The proposed development will follow the existing drainage system, and hence no adverse drainage impact from the Application Site is anticipated.
- 4.1.2 Routine inspection and desilting the drainage facilities within the proposed development and mitigation measures should be carried out during the operation periods to enhance drainage performance without blockage by debris or silting materials being washing down from the proposed development underground drainage system.

Figures







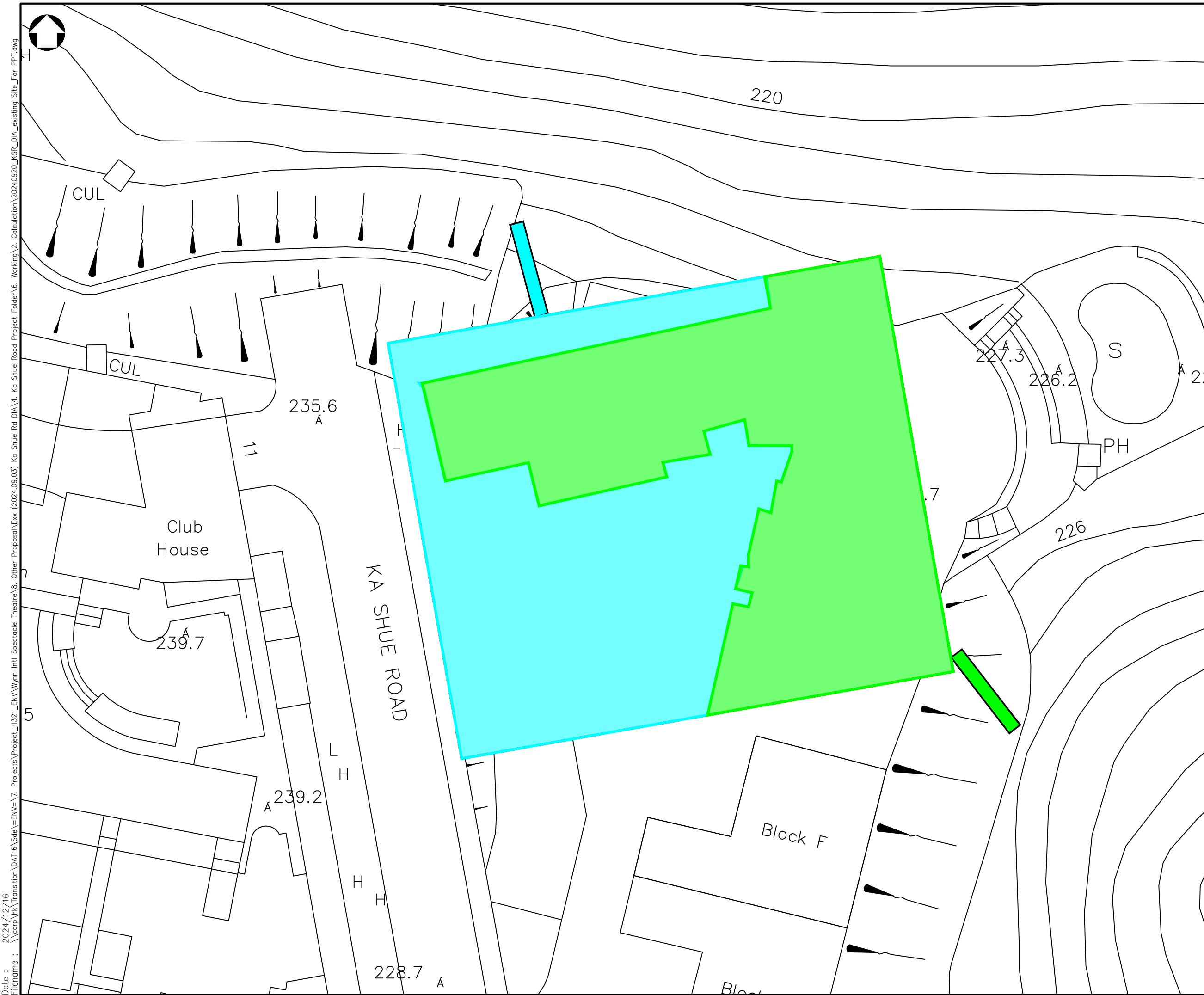


LEGEND :

- ASSESSMENT SITE BOUNDARY
- ANTICIPATED STORMWATER AREA FLOW TO EXISTING STEP CHANNEL 1
- ANTICIPATED STORMWATER AREA FLOW TO EXISTING STEP CHANNEL 2
- EXISTING STEPPED CHANNEL 1
- EXISTING STEPPED CHANNEL 2

Rev	Description	By	Date
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WSP			
Project title			
Proposed Residential Development at Lot 1109 RP (Part) in D.D. 253/8 Ka Shue Road, Sai Kung, New Territories			
Drawing title			
Locations of Catchment Areas and Stepped Channel After Development			
Drawing no.			
FIGURE 5			
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Appendix A Master Layout Plan of Proposed Development

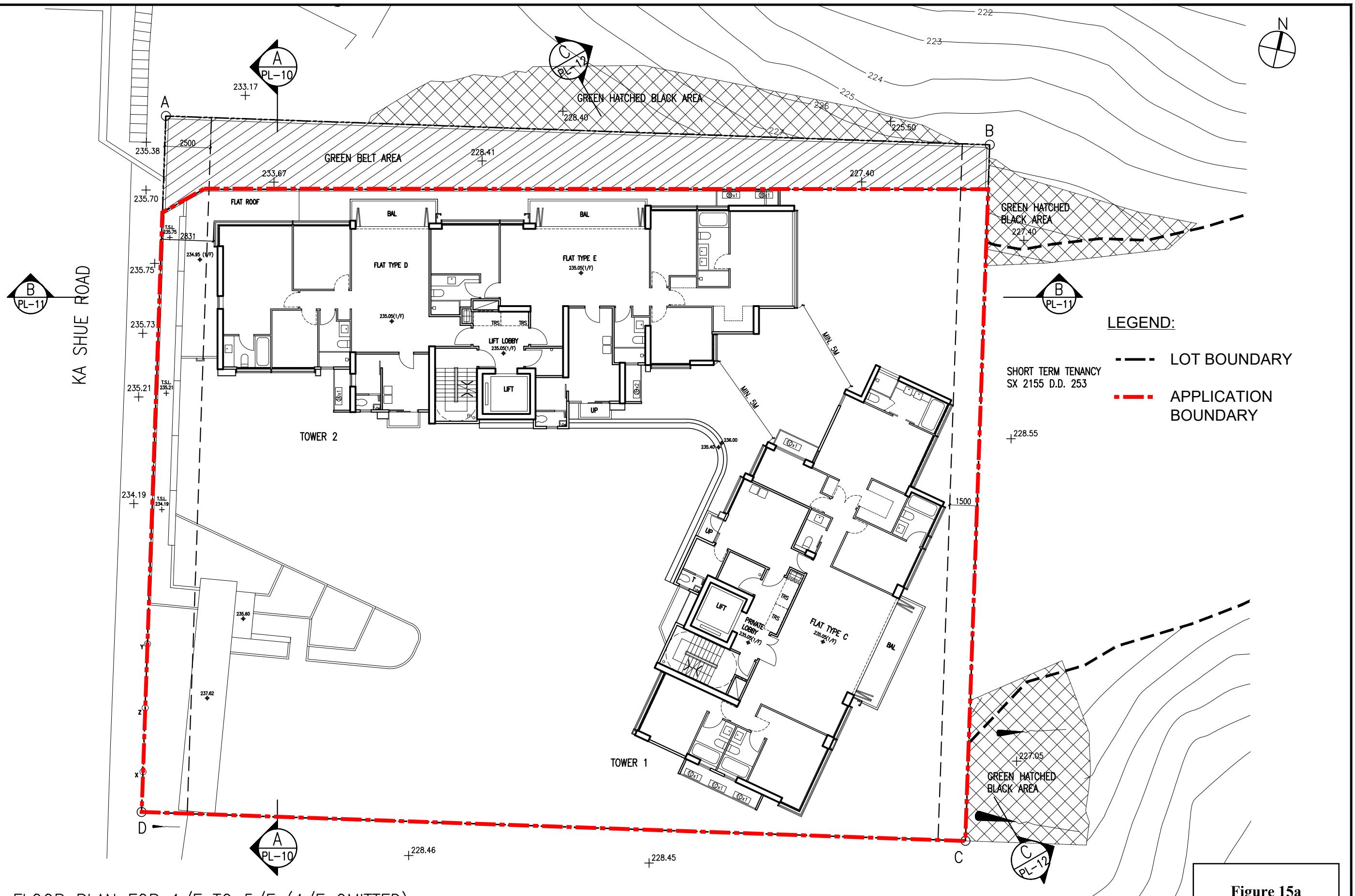


Figure 15a

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PROJECT
Proposed Residential Development at Lot 1109 RP (Part) in D.D. 253, 8
Ka Shue Road, Sai Kung, New Territories

DRAWING TITLE
SECTION 16 APPLICATION
— TYPICAL FLOOR PLAN

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Appendix B Drainage Impact Assessment Calculations

