

Appendix 5

Sewerage Impact Assessment

Issue No. : 2
Issue Date : Nov 2024
Project No. : 2244EA



SEWERAGE IMPACT ASSESSMENT

FOR

PROPOSED NEW SCIENCE BUILDING AT LINGNAN UNIVERSITY, TUEN MUN, HONG KONG

Prepared by
Allied Environmental Consultants Limited

COMMERCIAL-IN-CONFIDENCE

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1. Introduction

1.1. Project Background

1.1.1. The Applicant intends to develop a New Science Building in Lingnan University (hereafter as “the proposed development”), a new academic and research building centred within the campus’ Wing On Plaza in the North, Leung Kau Kui Building in the East, the Main Building in the West (hereafter as “the Project Site”).

1.1.2. According to the draft Tuen Mun Outline Zoning Plan (OZP No.: S/TM/40), the Project Site is currently zoned as “Government, Institution or Community” (“G/IC”) Zone.

1.1.3. Allied Environmental Consultants Limited (AEC) is commissioned by the Applicant to conduct a Sewerage Impact Assessment (SIA) to meet the land lease’s condition (60) of the captioned project. The proposed development is expected to be completed in 2028.

1.2. Objectives of the SIA

1.2.1. The objectives of this SIA are to assess whether the capacity of the existing sewerage networking to the Project Site is sufficient to cope with the sewage flow from proposed development.

1.3. Report Structure

1.3.1. The remaining chapters of this report are shown below:

Chapter 2 – Site Context

Chapter 3 – Sewerage Impact Assessment

Chapter 4 – Estimation of Sewage Flow from Proposed Development

Chapter 5 – Sewage Capacity

Chapter 6 – Result and Discussion

Chapter 7 – Conclusion

2. Site Context

2.1. Site Location and Its Environs

2.1.1. The proposed development is located at Castle Peak Road – Lingnan, within Lingnan University Campus. It is bounded by Wing On Plaza to the North, Leung Kau Kui Building to the East, the Main Building in the West, and an existing terrain in the South Side.

2.1.2. **Figure 2.1** shows the Site location and its environs.

2.2. Proposed Development Scheme

2.2.1. The proposed site area of the subject site is 2,302.28 m². The proposed development will be of 7-storey in height. The proposed development serves for general education purpose and specialized space such as computer laboratories, acoustic laboratories within Lingnan University Campus. It comprises of Canteen (LG/F), Wet and Dry Laboratories (3/F to 4/F), Teaching Venues, Function Rooms (LG/F and 5/F) and Offices (1/F to 2/F).

2.2.2. The development schedule of the proposed development is tabulated in **Table 2-1**. The GBP of the proposed development are shown in Appendix A.

Table 2-1 Development Schedule

Floor	Use
LG/F	Canteen & Multi-purpose Room
G/F	Lecture Rooms
1/F-2/F	Office
3/F-4/F	Lab
5/F	Exhibition Area & Multi-purpose Room

2.3. Existing Sewerage Condition

2.3.1. Drainage information was obtained from the GeoInfo Map services of the Lands Department in July 2024 to gather background information on sewerage infrastructure in the vicinity of the Project Site. Concerned sewage network was identified for estimation of the potential sewage impact to the downstream sewers associated with the proposed development. A series of public sewers with diameters 450mm were found along Fu Tei Road, towards Castle Peak Road - Lingnan. Sewage generated from the Project Site will be conveyed through existing Lingnan University's internal sewage system (at E-33) indicated in **Figure 3.1** and discharged to public sewer manhole at FMH1015210 and conveyed Northwest toward Castle

Peak Road - Lingnan.

3. Sewerage Impact Assessment

3.1. Legislation, Standards and Guidelines

3.1.1. With reference to ProPECC PN 1/23 Drainage Plans Subject to Comment by the Environmental Protection Department (“EPD”), foul water should be discharged to a foul sewer under the Building (Standards of Sanitary Fitments, Plumbing, Drainage Works and Latrines) Regulations 40(1) and 41(1).

3.1.2. The following standards and guidelines are adopted for estimation, assessment and evaluation of sewerage implication of the proposed development:

- “Hong Kong Planning Standards and Guidelines” issued by the Planning Department;
- “Sewerage Manual Part 1” published by DSD; and
- “Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning Version 1.0 (Report No.: EPD/TP1/05)” (“GESF”) published by Environmental Protection Department (“EPD”).

3.2. Assessment Methodology and Assessment

3.2.1. As shown in the drainage plan of proposed development, the sewage generated from the proposed development will be discharged through the terminal manhole (FTMH) at the Project Site and conveyed towards Lingnan University’s internal manhole, connected to the existing public sewer manhole (S1: FMH1015120) at Castle Peak Road-Lingnan. **Figure 3.1** illustrates an overview of corresponding catchment areas and existing sewerage network for this study. The catchments are divided into 4 upstream catchments and 1 downstream catchment to facilitate estimation of sewage discharge.

- **Downstream Catchment A:** Ho Sin Hang Building, Leung Kau Kui Building, B.Y. Lam Building, Dorothy Y.L. Wong Building, Lau Lee Yuen Hann Amenities Building, Lau Chung Him Building, Patrick Lee Wan Keung Academic Building, Main Building and Swimming Pool;
- **Upstream Catchment B:** Indoor Sports Complex;
- **Downstream Catchment D:** Parkland Villas Block 1-3 (East of Castle Peak Road – Lingnan, 300mm sewers along Castle Peak Road – Lingnan);
- **Downstream Catchment E:** Parkland Villas Block 4-6 and swimming pool (East of Castle

Peak Road – Lingnan, 300mm sewers along Castle Peak Road – Lingnan).

- 3.2.2. The sewage flow from proposed development, Sewage flow from downstream catchment A will be discharge along the internal manhole and sewer from E33 to last FMH-E and discharge to public manhole at FMH1015210. The Sewage flow of Catchment A and the Site then conveyed towards the public sewer series. Upstream catchment D will be discharged to manhole FMH1015204, while catchment B and E will be discharged to FMH1015206 located at Castle Peak Road – Lingnan and collected by the 450mm diameter sewer.
- 3.2.3. A hydraulic assessment, which will take into account of the sewage discharge from proposed development as well as the surrounding developments will be conducted.
- 3.2.4. According to the “Sewerage Manual – Key Planning issues and Gravity Collection System” published by DSD in 2013, the capacities of respective sewers have been calculated based on Colebrook-White equation for circular pipes:

$$V = -2(2gDS)^{0.5} \log \left(\frac{k}{3.7D} + \frac{2.5\nu}{D(2gDS)^{0.5}} \right)$$

Where,

V = mean velocity (m/s)

g = gravitational acceleration (m/s²)

D = pipe diameter (m)

ks = hydraulic pipeline roughness (m) (0.006m is adopted for the “poor” condition of concrete slimed sewers as worst-case scenario)

ν = kinematic viscosity of fluid (m²/s)

s = frictional slope (energy gradient due to frictional loss)

- 3.2.5. The detailed calculations of sewage generation have been provided in **Appendix C** and **Appendix D** for reference.

4. Estimation of Sewage Flow from Proposed Development

4.1. Methodology for Estimation of Average Dry Weather Flow

4.1.1. The global unit flow factors as recommended in the *Guideline for Estimating Sewage Flows for Sewerage Infrastructure Planning* (hereafter as “GESF”) published by EPD in 2005 has been adopted in the assessment to estimate sewage flow. Relative unit flow factors applied for the sewage generation estimation are tabulated in **Table 4-1** below.

Table 4-1 Unit Flow Factors Adopted for the Assessment

Type of People	Unit Flow Factor ^[2]	Category ^[1]
Within Proposed Development		
Staff of Community, Social & Personal Services	0.28 m ³ /person/day	Commercial Employee + Commercial activities (J11 Community, Social & Personal Services)
Student	0.04 m ³ /person/day	School Student
Food Store	1.58 m ³ /person/day	Commercial Employee + Commercial activities (J10 Restaurants & Hotels)
Surrounding Catchments		
Residents of the Residential Development R1	0.19 m ³ /person/day	Domestic (housing type specific) – R1
Staff of Community, Social & Personal Services	0.28 m ³ /person/day	Commercial Employee + Commercial activities (J11 Community, Social & Personal Services)
School Students	0.04 m ³ /person/day	School Students
Food Store	1.58 m ³ /person/day	Commercial Employee + Commercial activities (J10 Restaurants & Hotels)

Notes:

[1] Environmental Protection Department, HKSARG [EPD] (2005). *Guidelines for estimating sewage flows for sewerage infrastructure planning (EPD/TP 1/05)*. Hong Kong

[2] UFF for various occupancy types are adopted according to Table T-1 and Table T-2 of the GESF.

4.2. Estimation of Sewage Flow from Proposed Development

4.2.1. The sewage generated from the proposed development will be collected at FTMH and conveyed to Lingnan University’s internal sewerage system, and discharged into the existing

450mm diameter public sewers via Manhole S1 (No.: FMH1015210) as shown in **Figure 3.1**.

4.2.2. The proposed project comprises of a 7-storey building with an estimated GFA of 11,000m². The estimated sewerage flow for proposed development is given in **Table 4-2** and **Appendix C**.

4.2.3. With reference to **Table 4-2**, the total estimated Average Dry Weather Flow (“ADWF”) from the proposed development is 100.3 m³/day, and peak flow is 0.0102 m³/s. The population estimated ADWF of proposed development is summarized in Table 1 of **Appendix C**.

Table 4-2 Sewage Flow Estimation for the Proposed Development and Existing

Proposed Development		
Generation from Operation		Remark
Generation from Laboratory		
Total Number of Person	210 persons	Based on submitted GBP
Total Number of Staff	30 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28 m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	180 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04 m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	15.6 m ³ /day	
Generation from Multi-Function Room		
Total Number of Person	71 persons	Based on submitted GBP
Total Number of Staff	11 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28 m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	61 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04 m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	5.52 m ³ /day	

Generation from Lecture Room		
Total Number of Person	287 persons	Based on submitted GBP
Total Number of Staff	41 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28 m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	246 persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04 m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	21.32 m ³ /day	
Generation from Food Store & Kitchen		
Total Floor Area	162 m ²	
Worker Density per GFA (in 100 m ²)	5.1 person/100m ²	
Total capacity	9 persons	Referred to submitted GBP.
Unit Flow	1.58 m ³ /person/day	Commercial Employee + Restaurants & Hotels - J10 in Table T-2 of GESF.
Average Sewage Discharge	14.22 m ³ /day	
Generation from Office		
Total capacity	156 persons	Referred to submitted GBP.
Unit Flow	0.28 m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Average Sewage Discharge	43.68 m ³ /day	
Total ADWF of the Proposed Development	100.34 m³/day	
Catchment Inflow Factor (Tuen Mun)	1.1	
Revised ADWF of the Proposed Development	110.37 m³/day	
Peaking Factor	8	
Peak Flow	0.0102 m³/s	

4.2.4. The laboratories are for educational purposes only and the chemical tests will be conducted in small scale, occasionally and in short duration. No extra discharge of wastewater will be

generated during operation.

4.3. Estimation of Sewage Flow from Catchment Areas

4.3.1. Different catchment areas are defined as shown in **Figure 3.1** to consider existing sewage generation. There are 3 upstream catchments and 1 downstream catchment. Sewage generated from upstream Catchments D will be discharged to manhole FMH1015204 located at the Castle Peak Road-Lingnan. Sewage discharge from Catchment B and E will be collected by manhole FMH1015206. Catchment A consisted of Ho Sin Hang Building, Leung Kau Kui Building, B.Y. Lam Building, Dorothy Y.L. Wong Building, Lau Lee Yuen Hann Amenities Building, Lau Chung Him Building, Patrick Lee Wan Keung Academic Building, Main Building and Swimming Pool within Lingnan Campus located at the downstream of Site FMH, manhole FMH1015210 collects the discharge from Catchment A and the proposed development. The sewage is then conveyed into the series of existing 450mm public sewerage pipes along the Castle Peak Road-Lingnan.

4.3.2. The number of employees is estimated with reference to the submitted GBP. Where such information is unavailable from the mentioned reference, “Commercial and Industrial Floor Space Utilization Survey” issued by the Planning Department is used for the estimation of staff. Different worker densities per 100m² of Gross Floor Area (GFA) have been adopted in the assessment to estimate the number of employees within the study area. Relative worker density applied for the sewage generation estimation are tabulated in **Table 4-3** below.

Table 4-3 Worker Density Adopted for the Assessment

Economic Activities	Worker Density (worker per GFA, in 100m ²)
Community, Social & Personal Services (All Types)	3.3 worker/100m ²
Restaurants (All Types)	5.1 worker/100m ²

4.3.3. Based on information collected from desktop study, sewage flow from different catchments at the study area under proposed developments are calculated as given in **Appendix D**.

4.4. Estimation of Peak Discharge

4.4.1. Catchment inflow factor (“P_{CIF}”) caters for the net overall ingress of wastewater to the sewerage system. They are catchment-dependent and applicable to major sewerage facilities of a catchment.

4.4.2. In accordance with Table T-4 of the GESF, P_{CIF} of 1.10 is adopted for existing sewerage as

concerned sewerage system is identified in “Tuen Mun”.

4.4.3. Revised average dry weather flow (“revised ADWF”) is determined by production of average dry weather flow and catchment inflow factor. Contributing population is then calculated by dividing the revised ADWF by 0.27. The calculated contributing population is finally used for selection of peaking factors.

4.4.4. Based on **Table 4-4** which is also presented in Table T-5 in GESF, the peaking factors for each sewer are chosen in the hydraulic calculation for peak flow estimation.

Table 4-4 Peaking Factors

Population Range for Sewers ^{[1] [2]}	Peaking Factor (including storm water allowance) for facility with existing upstream sewerage	Peaking Factor (excluding storm water allowance) for facility with new upstream sewerage
< 1000	8	6
1000 - 5000	6	5
5000 - 10000	5	4
10000 - 50000	4	3
> 50000	Max (7.3 / N ^{0.15} , 2.4)	Max (6 / N ^{0.175} , 1.6)

4.5. Peak Discharge from Study Areas

4.5.1. Flow rates of peak discharge from the proposed development and other catchment areas are estimated in accordance with the DSD’s “Sewerage Manual Part 1”. Peak flows from different catchments at the study area are summarized in **Table 4-5** and detailed calculation for proposed developments are given in **Appendix C** and **Appendix D**.

Table 4-5 Population and Sewage Flow Estimation

Proposed Development					
Contributing Catchment Area	Connected Manhole	Revised ADWF (m ³ /day)	Contributing Population	Peaking factor	Total Peak Discharge (m ³ /s)
Proposed Development	E-33	110.4	409	8.0	0.010
Proposed Development + Catchment A	E-32	829.0	3070	6.0	0.058
Proposed Development + Catchment A	E-28	829.0	3070	6.0	0.058

Proposed Development + Catchment A	E-16	829.0	3070	6.0	0.058
Proposed Development + Catchment A	E-12	829.0	3070	6.0	0.058
Proposed Development + Catchment A	E-8	829.0	3070	6.0	0.058
Proposed Development + Catchment A1-8	E-1A	829.0	3070	6.0	0.082
Proposed Development + Catchment A1-8	E-1	829.0	3070	6.0	0.082
Proposed Development + Catchment A to E	FMH1015211	1475.8	5466	5.0	0.182
Proposed Development + Catchment A to E	FMH1015265	1475.8	5466	5.0	0.182

Notes:

[1] Revised ADWF (m^3/day) = ADWF (m^3/day) × Catchment Inflow Factor

[2] According to Section 12.1 of GESF,

Contributing Population = Calculated Total Average Flow (m^3/day) ÷ 0.27 ($m^3/person/day$)

[3] According to Table T-5 of GESF

[4] Total Peak Discharge (m^3/s) = (Revised ADWF (m^3/day) × Peaking Factor) ÷ 86400s/day

5. Sewerage Capacity

5.1.1. According to the “Sewerage Manual – Key Planning Issues and Gravity Collection System” (Sewerage Manual) published by DSD in 2013, the capacities of respective sewers have been calculated based on the Colebrook White’s equation. The roughness coefficients (k_s) of 3.0mm for clayware slimed sewer in poor condition are adopted in the assessment in accordance with Table 5 of DSD’s “Sewerage Manual Part 1”.

5.1.2. The sewerage impact on various segments of the sewer were evaluated by comparing the estimated peak flow against the capacity of the respective sewer segments. The detailed calculations are provided in **Appendix E2**.

6. Results and Discussion

6.1. Daily Flow and Peak Flow from Proposed Development

6.1.1. The estimated daily flow and peak flow of the proposed development will be 100.3 m³/day and 0.008 m³/s. **Table 6-1** tabulates the sewage generated from proposed development.

Table 6-1 Sewage Generated from Existing and Proposed Development

Developments	Daily Flow (m ³ /day)	Peak Flow (m ³ /s)
Proposed Development	100.3	0.0102

6.2. Sewage Generation before development

6.2.1. The percentages of used capacity for sewers with existing condition before Proposed Development range from 6.0% to 89.2%. The detailed calculation of flow estimation and capacity are presented in **Appendix D** and **Appendix E1**. The summary of used capacity of existing condition is presented in **Table 6-2** below.

Table 6-2 Used Capacity of Existing Development

Pipe Segments	Maximum Capacity of Sewer (m ³ /s)	Total Discharge through Manhole (m ³ /s)	Used Capacity (%) After Development
E33 – E32	0.098	0.048	48.7%
E32 – E28	0.097	0.048	49.3%
E18 – E16	0.244	0.048	19.6%
E13 – E12	0.079	0.048	60.7%
E9 – E8	0.213	0.048	22.5%
E2 – E1A	0.087	0.073	83.5%
E1A – E1	0.187	0.073	38.8%
FMH1015210 – FMH1015211	0.236	0.189	80.0%
FMH1015211- FMH1015265	0.212	0.189	89.2%

6.3. Sewage Generation after development

6.3.1. After the development, the percentages of used capacity for the existing sewers range from 9.7% to 94.7%. Flow estimation and capacity checking are detailed in **Appendix D** and **Appendix E2**, respectively. Used capacity of proposed development is presented in **Table 6-3**

below.

Table 6-3 Used Capacity from Proposed Development

Pipe Segments	Maximum Capacity of Sewer (m ³ /s)	Total Discharge through Manhole (m ³ /s)	Used Capacity (%) After Development
Site – E33	0.105	0.010	9.7%
E33 – E32	0.098	0.058	58.6%
E32 – E28	0.097	0.058	59.3%
E18 – E16	0.244	0.058	23.6%
E13 – E12	0.079	0.058	73.0%
E9 – E8	0.213	0.058	27.1%
E2 – E1A	0.087	0.082	94.7%
E1A – E1	0.187	0.082	44.0%
FMH1015210 – FMH1015211	0.236	0.182	76.9%
FMH1015211- FMH1015265	0.212	0.182	85.7%

6.3.2. Based on the results shown in the above **Table 6-2** and **Appendix E2**, no sewer pipe exceeded its used capacity.

6.4. Liability

6.4.1. The project proponent will be responsible for the construction works for the pipe convey sewerage from terminal manhole to existing public sewer. All drainage facilities shall be designed and constructed to conform to the requirements laid down in below while future maintenance of the sewers outside the Project Site boundary will be carried out by the DSD.

- a. The Drainage Manual, DSD
- b. The General Specification for Civil Engineering Works, Hong Kong Government
- c. The DSD Standard Drawings

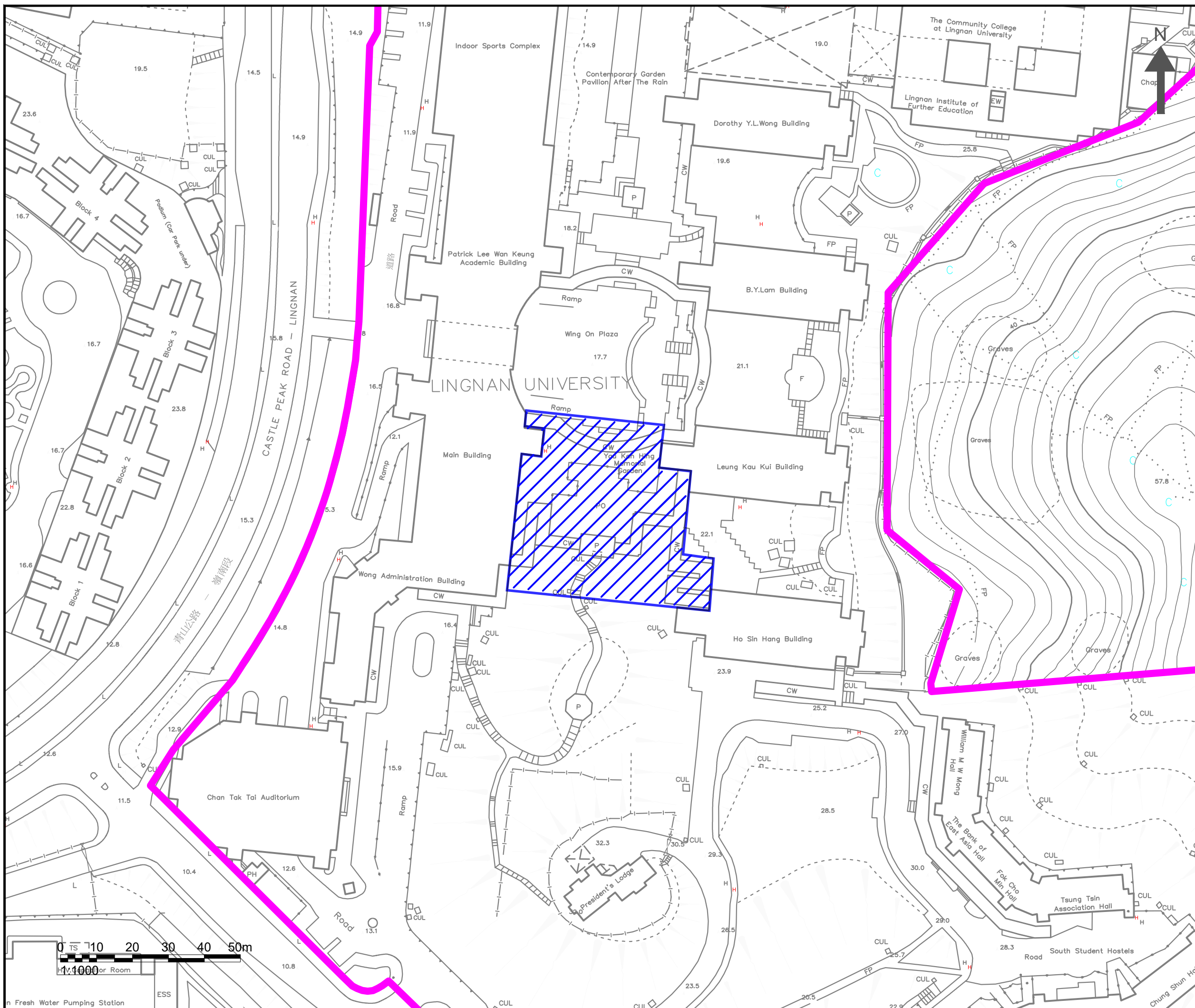
6.4.2. During operational phase, regular inspection of the sewers within the Project Site should be conducted by the property management office to ensure proper performance. Regular maintenance should also be carried out in accordance with standard practices stated in the

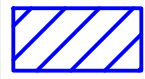

DSD's "Sewerage Manual Part 1".

7. Conclusion

- 7.1.1. A Sewerage Impact Assessment (SIA) has been conducted to evaluate the possible impacts on the public sewerage network due to the proposed development. The sewage generated from the proposed development will be collected by the proposed Manhole (FTMH) and conveyed towards Lingnan University's internal sewerage system, then discharged to public manhole (FMH1015210). The result showed that 100.3 m³/day of average sewage discharge and 0.0102 m³/s of peak sewage discharge are expected to be generated from the proposed development.
- 7.1.2. The assessment results demonstrated that all sewers have sufficient capacity to cater with the sewage flow from catchments and the proposed development. Therefore, significant sewerage impact arising from the proposed development on the existing sewers is not expected, no mitigation measures are considered necessary for the existing sewers.
- 7.1.3. Based on the above, it is concluded that the sewerage impact arising from the proposed development should be acceptable.

Figures



- NOTES:
-  PROJECT SITE
 -  LOT BOUNDARY

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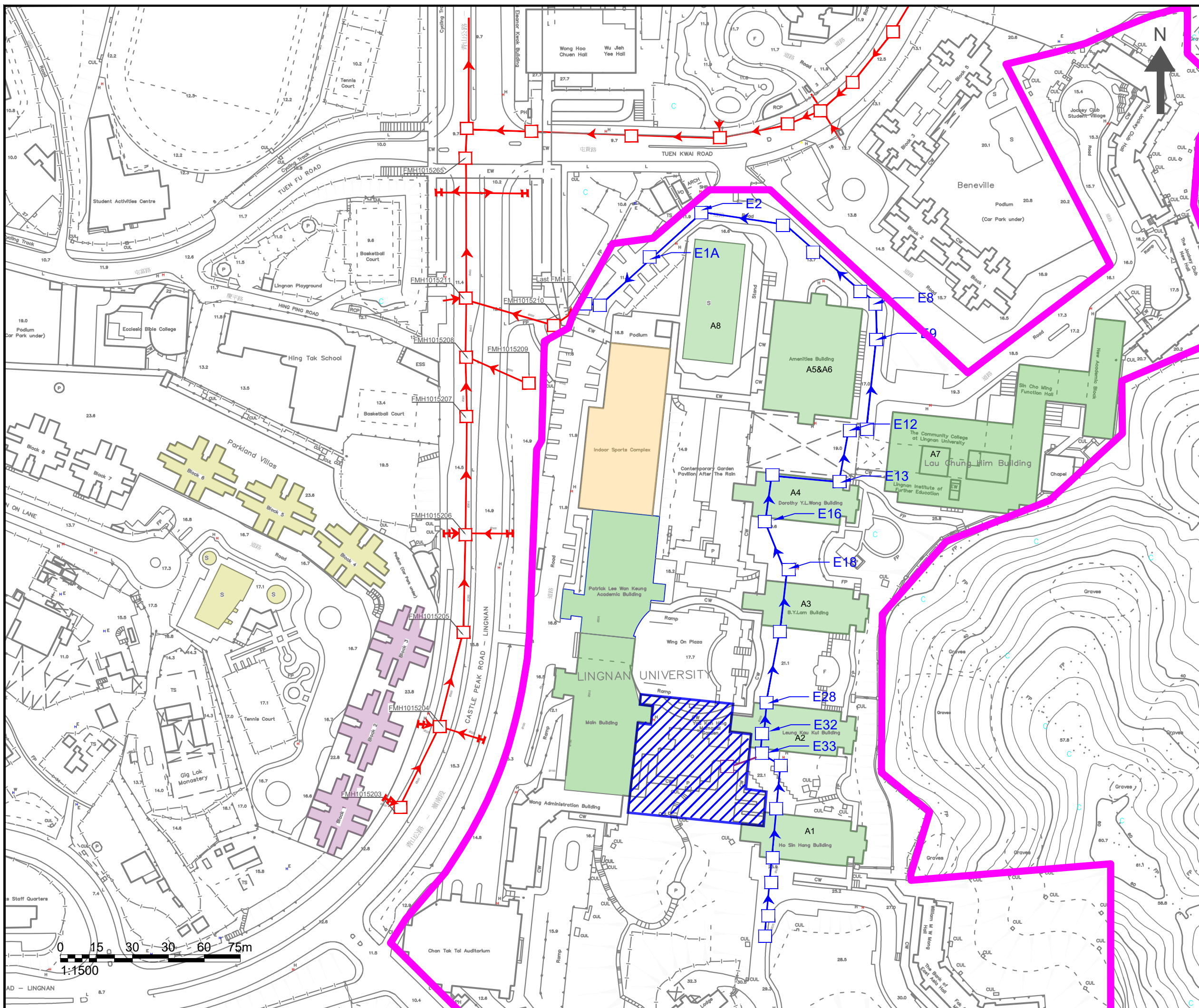
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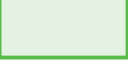


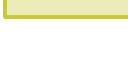

Project :
PROPOSED NEW SCIENCE BUILDING OF
LINGNAN UNIVERSITY

Drawing Title :
PROJECT SITE LOCATION

Drawing No : FIGURE 2.1	Revision : 0
Scale : AS SHOWN	Date : JUN 2024

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- NOTES :
-  PROJECT SITE
 -  EXISTING SEWER AND MANHOLE
 -  INTERNAL MANHOLE AND SEWERS OF LINGNAN UNIVERSITY
 -  PROPOSED MANHOLE AND SEWER
 -  CATCHMENT A
 -  CATCHMENT B
 -  CATCHMENT D
 -  CATCHMENT E
 -  LOT BOUNDARY

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Project No. : 2244EA

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Project :
PROPOSED NEW SCIENCE BUILDING OF LINGNAN UNIVERSITY

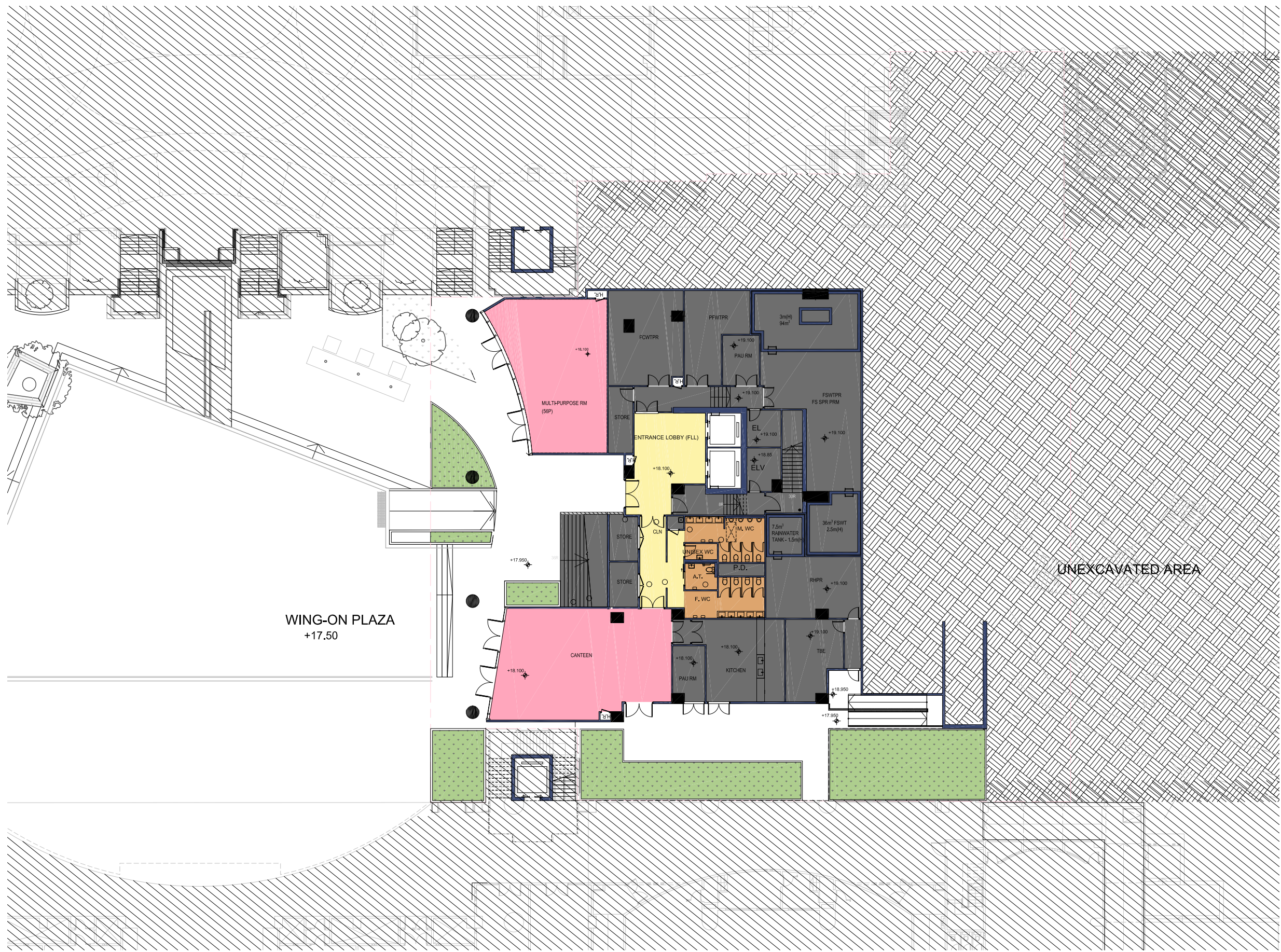
Drawing Title :
OVERVIEW OF EXISTING SEWAGE NETWORK AND CATCHMENT

Drawing No : FIGURE 3.1	Revision : 0
Scale : AS SHOWN	Date : JUN 2024

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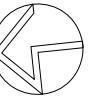
Appendix A

General Building Plans of the Proposed Development



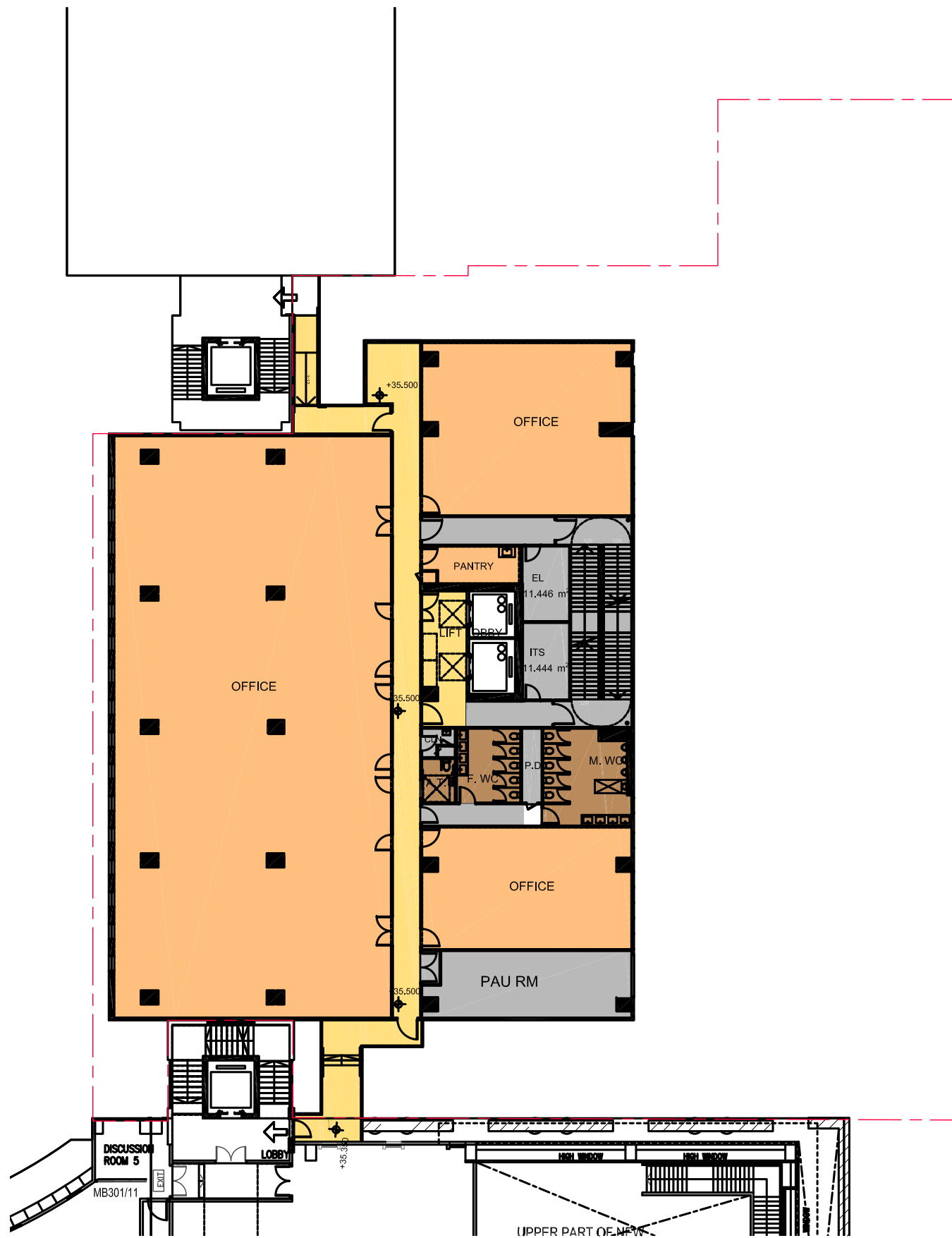
LEGEND

- MULTI-PURPOSE
- CIRCULATION
- PLANT ROOM/ BOH
- WC

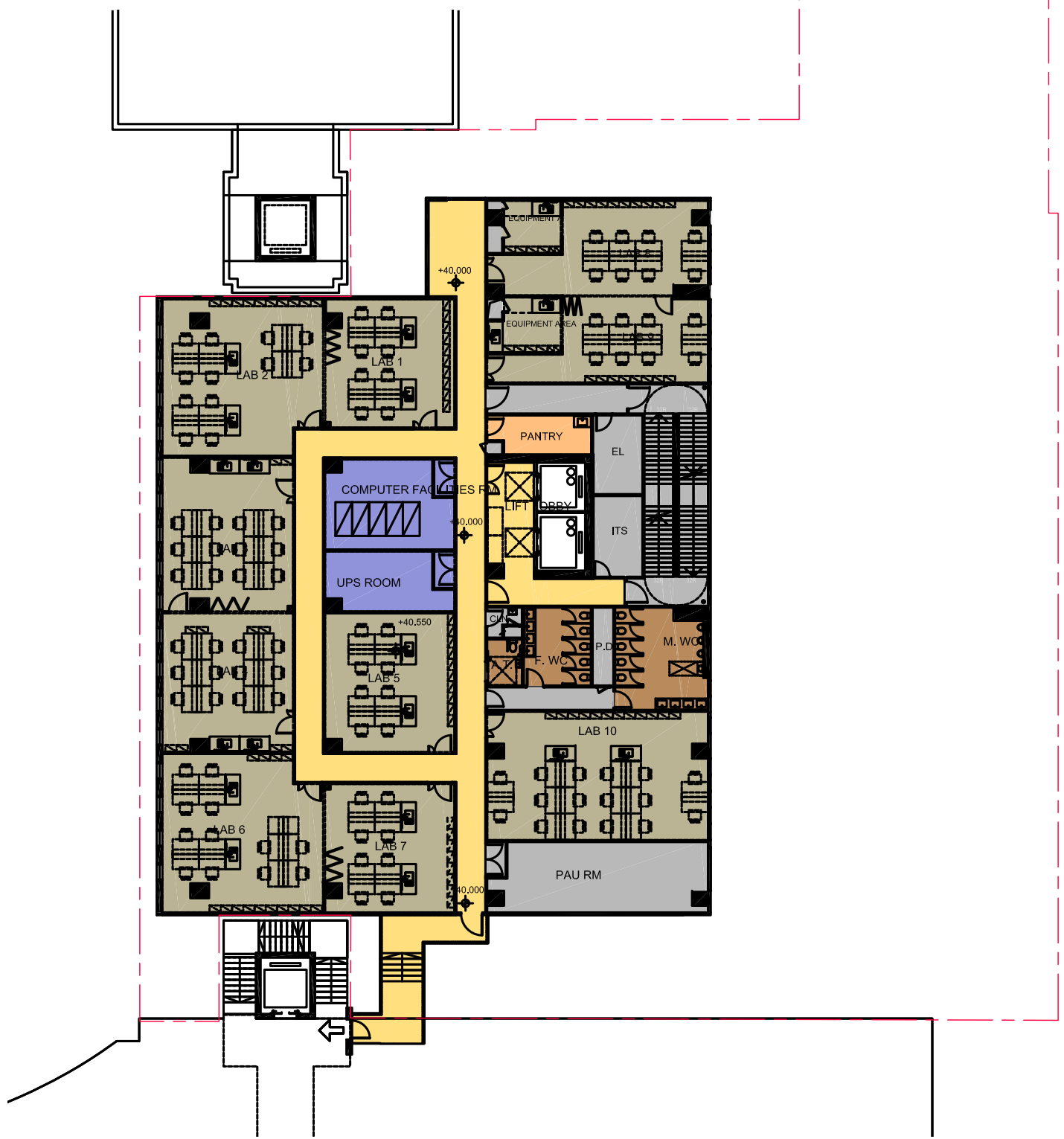


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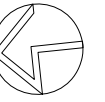
- LECTURE ROOMS
- CIRCULATION
- PLANT ROOM/ BOH
- OFFICES/ PANTRY
- WC
- PLANTERS/ LAWN
- LABORATORIES
- COMPUTER SERVICES



1 2/F LAYOUT PLAN

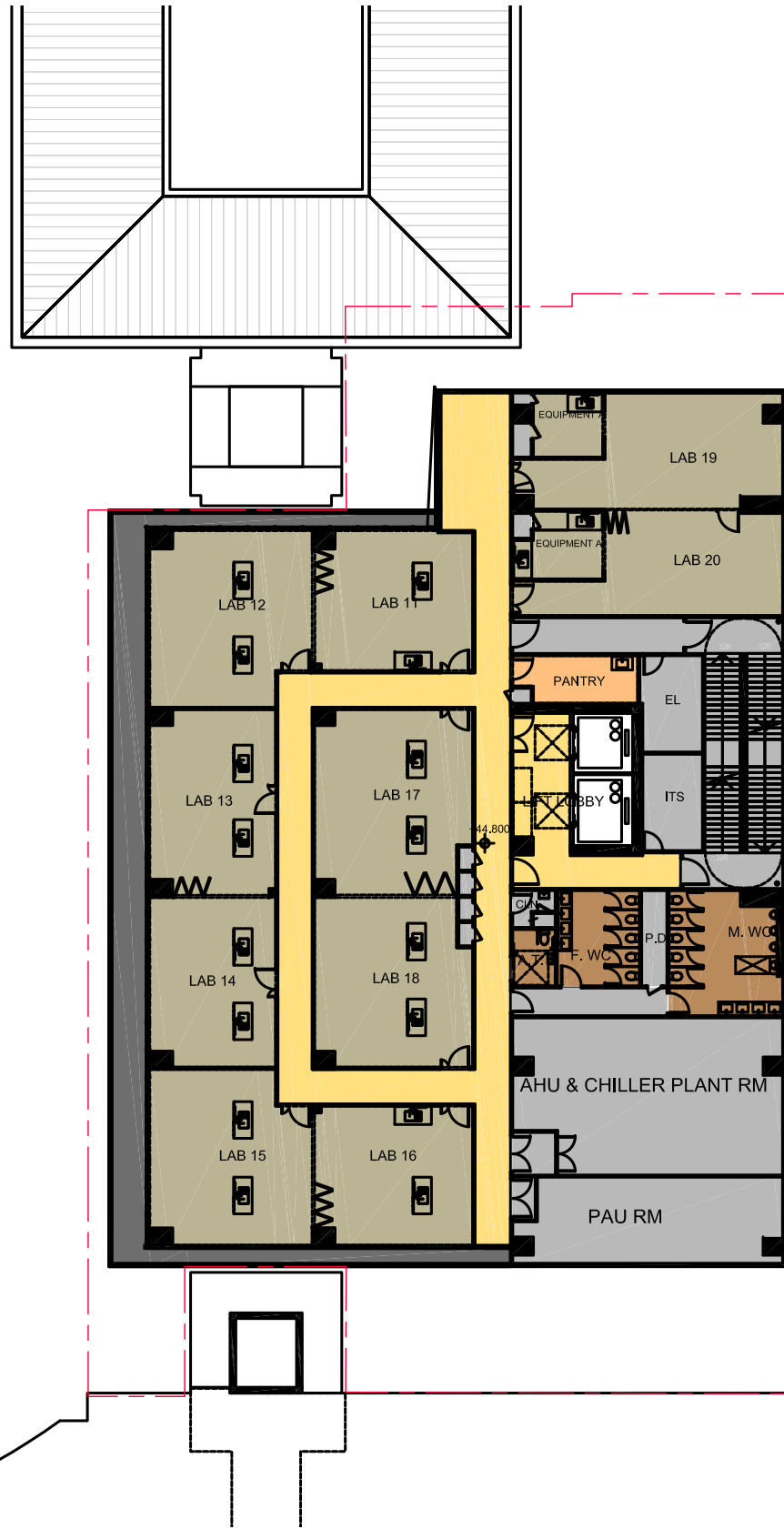


2 3/F LAYOUT PLAN

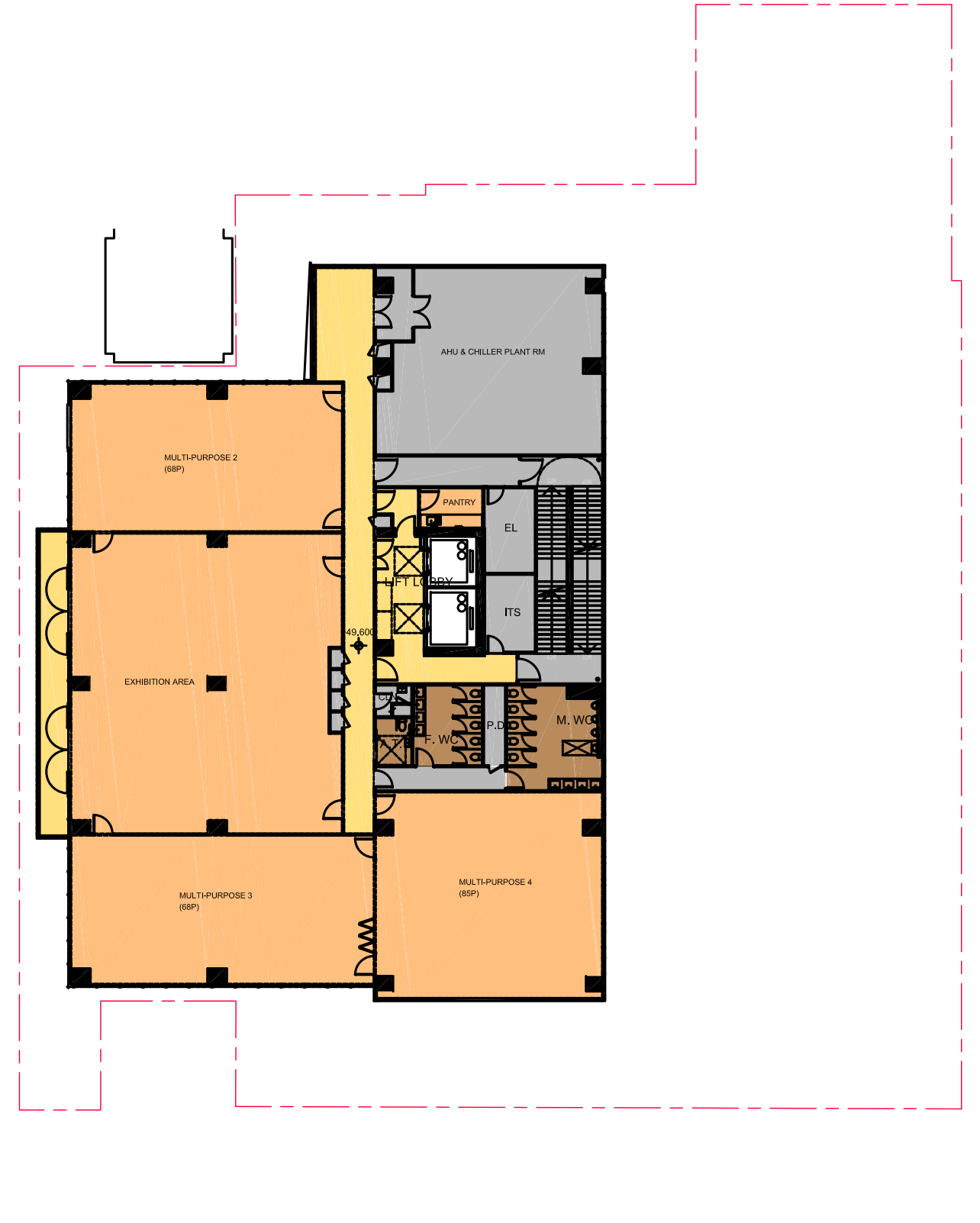


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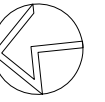
- LECTURE ROOMS
- CIRCULATION
- PLANT ROOM/ BOH
- EXHIBITION/ FUNCTION RMS
- WC
- LABORATORIES



1 4/F LAYOUT PLAN

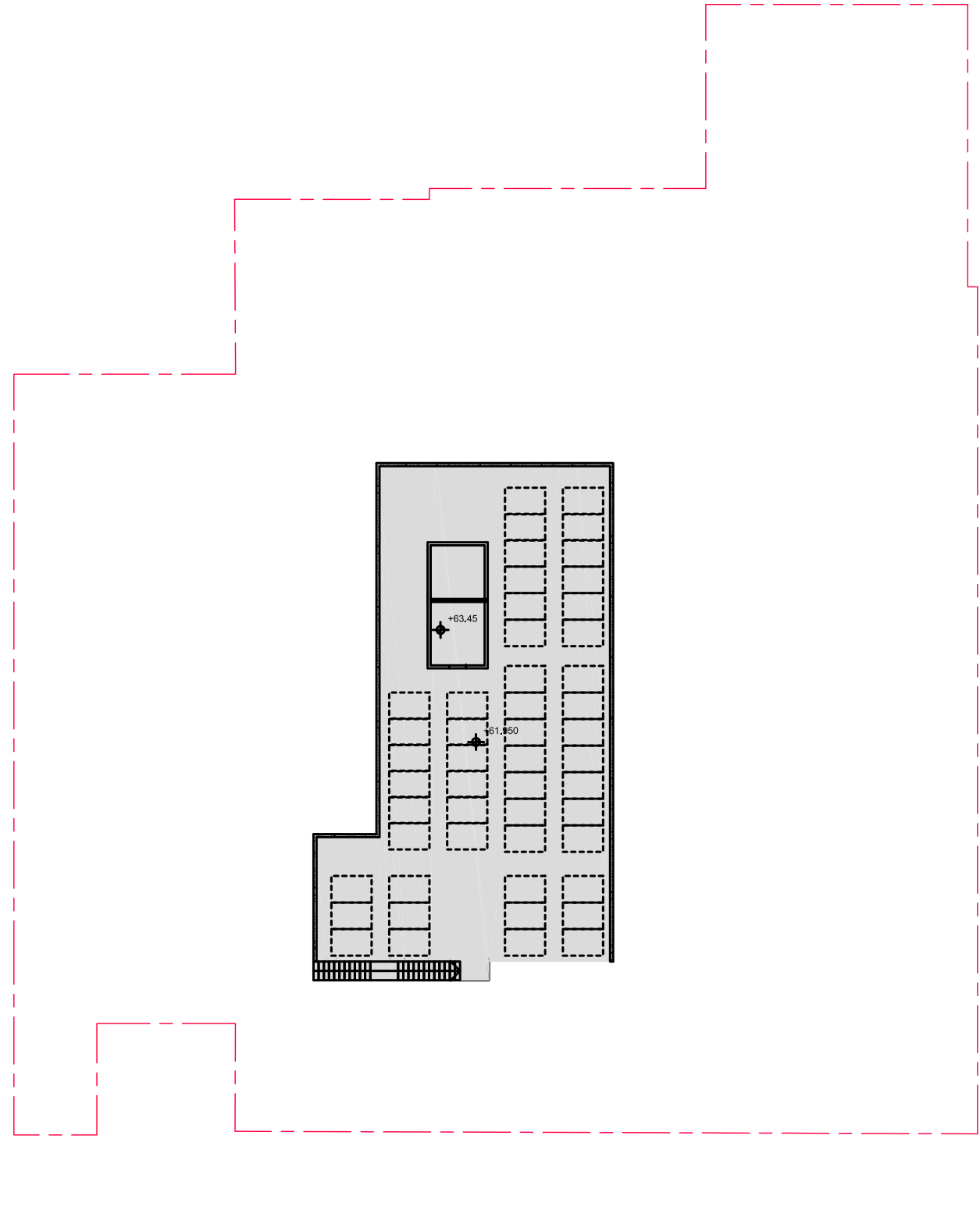
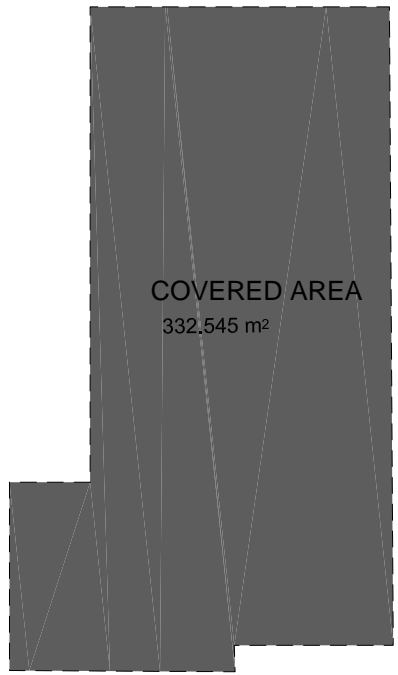
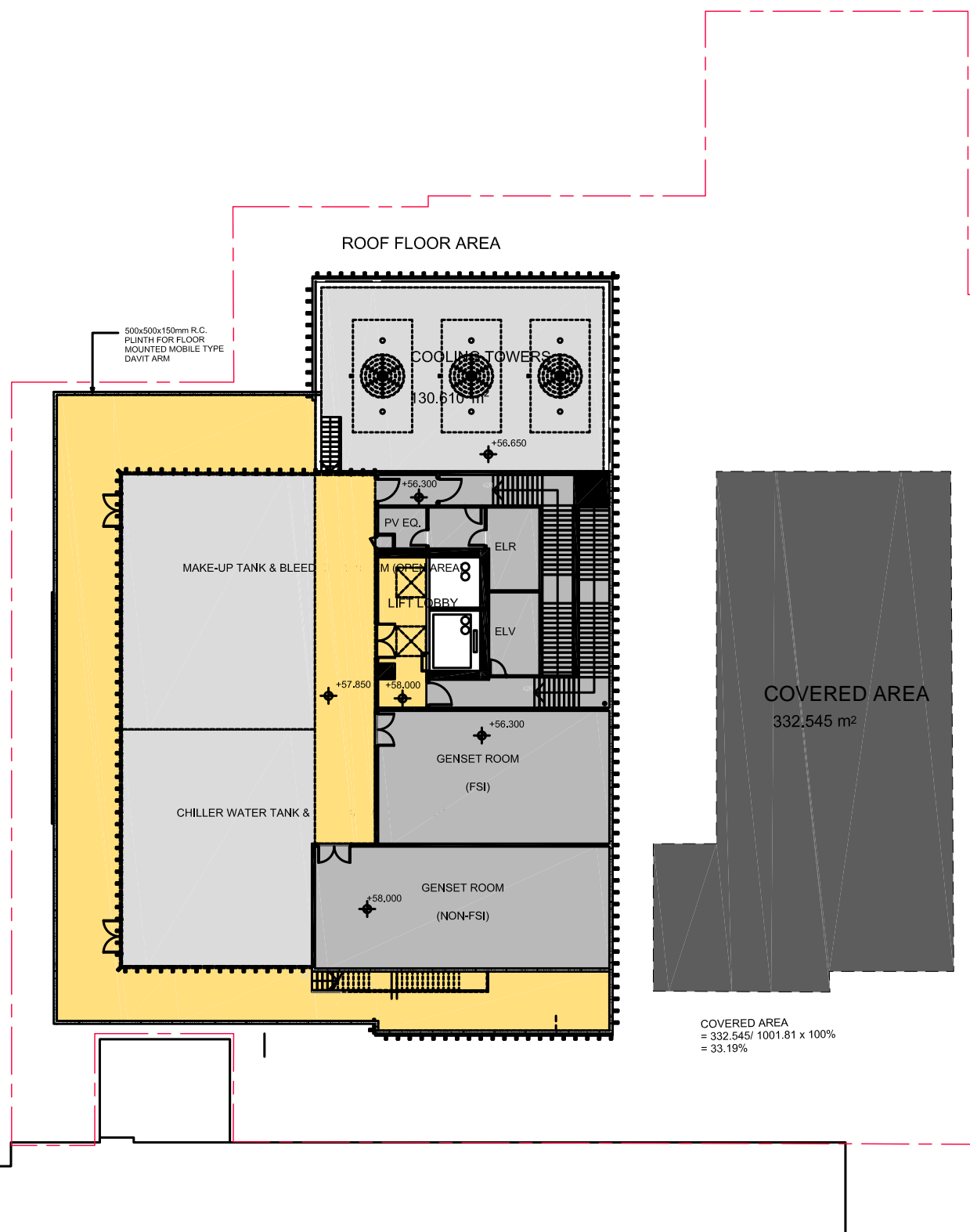


2 5/F LAYOUT PLAN



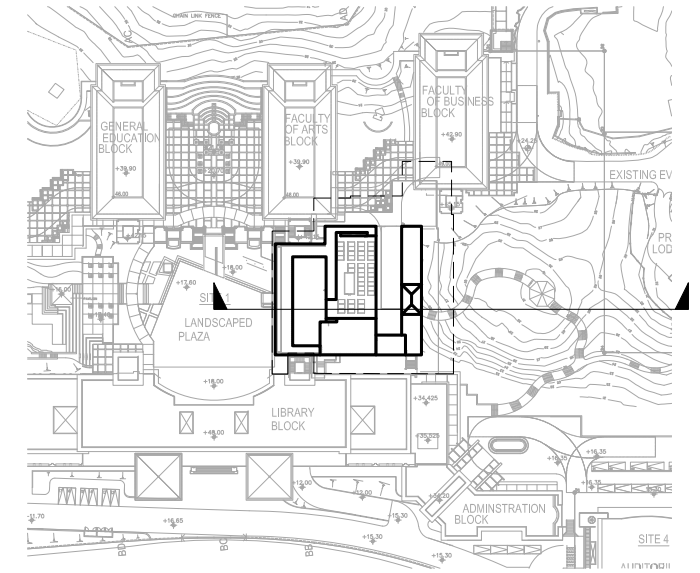
LEGEND

- CIRCULATION
- PLANT ROOM/ BOH





1 R/F LAYOUT PLAN

2 UR/F LAYOUT PLAN

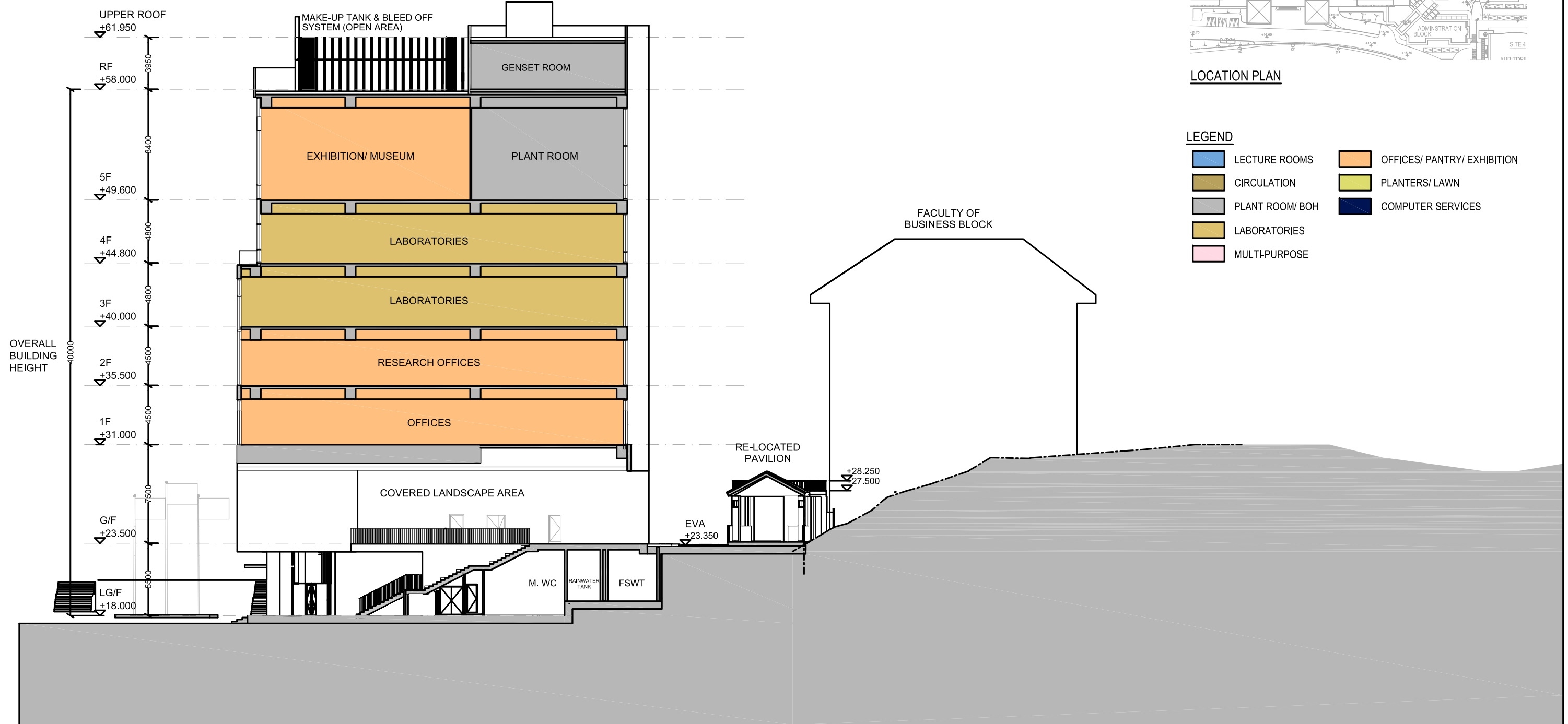


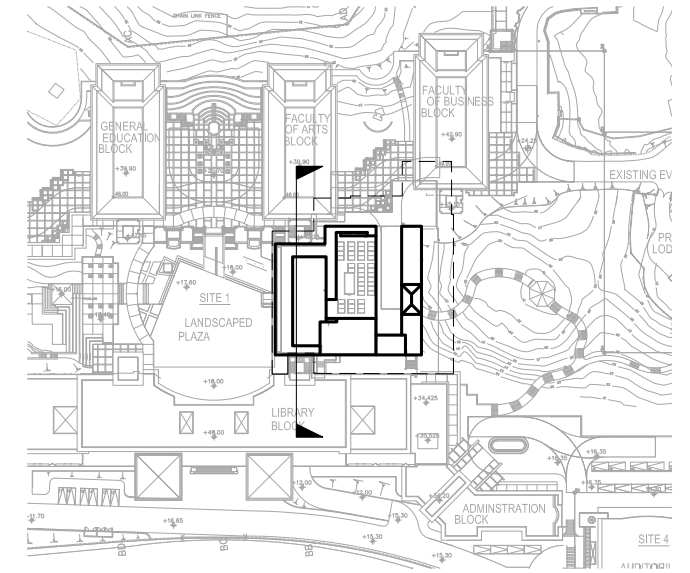
LOCATION PLAN

LEGEND

- | | | | |
|---|-----------------|---|-----------------------------|
|  | LECTURE ROOMS |  | OFFICES/ PANTRY/ EXHIBITION |
|  | CIRCULATION |  | PLANTERS/ LAWN |
|  | PLANT ROOM/ BOH |  | COMPUTER SERVICES |
|  | LABORATORIES | | |
|  | MULTI-PURPOSE | | |




ROOF STRUCTURE 3.95m
< 4m (10% OVERALL BUILDING HEIGHT)

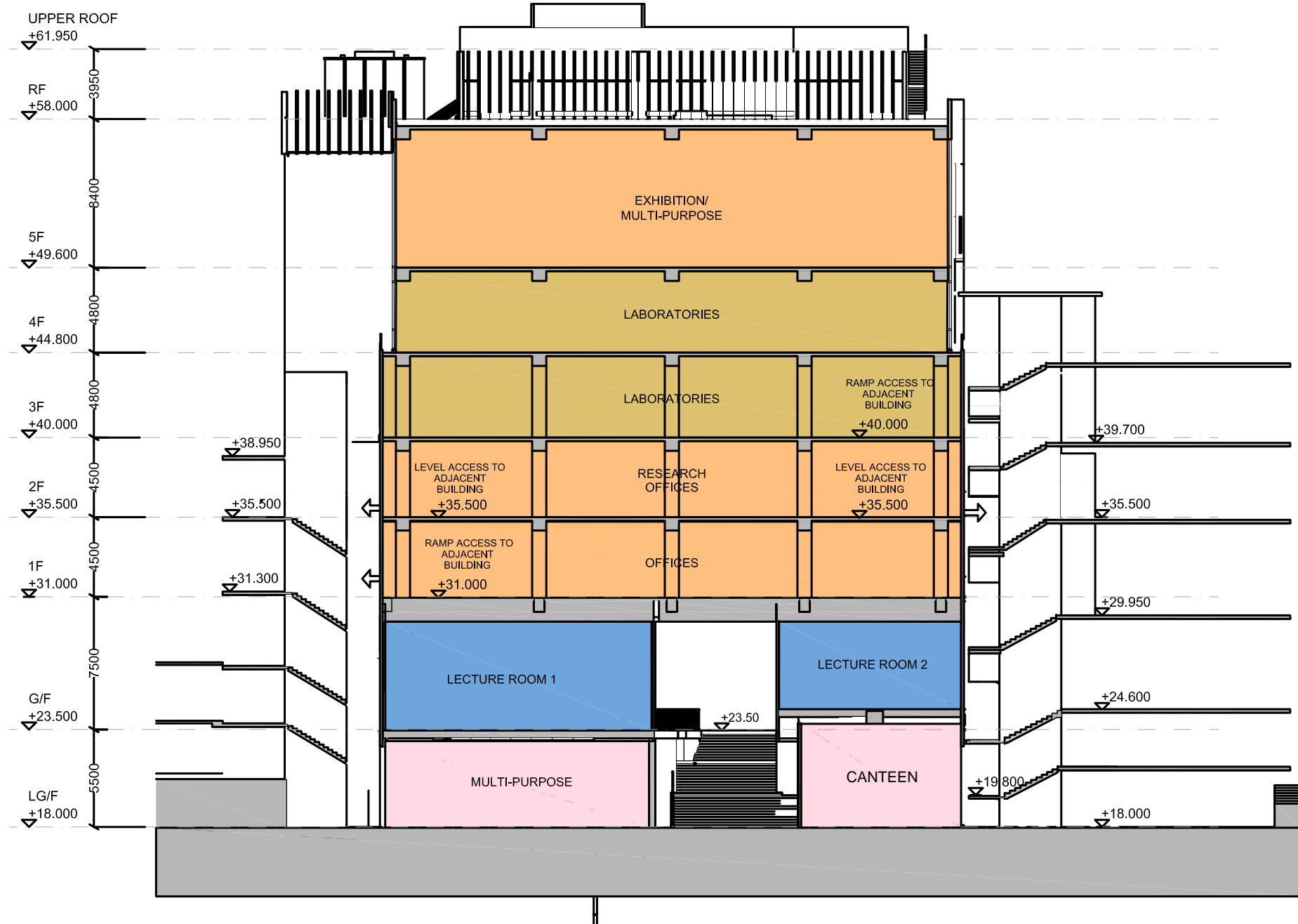




LOCATION PLAN

LEGEND

- | | | | |
|--|-----------------|---|-----------------------------|
|  | LECTURE ROOMS |  | OFFICES/ PANTRY/ EXHIBITION |
|  | CIRCULATION |  | PLANTERS/ LAWN |
|  | PLANT ROOM/ BOH |  | COMPUTER SERVICES |
|  | LABORATORIES | | |
|  | MULTI-PURPOSE | | |



1 SECTION B-B

Appendix B

Lingnan University Drainage Record Plan

NOTE: MANHOLE COVER LEVEL TO BE CHECKED ON SITE.
 * UNLESS OTHERWISE AS SHOWN
 ** CATCH PIT / SAND TRAP INVERT LEVEL DEPENDS ON CHANNEL GRADIENT. EXACT LEVEL TO BE CHECKED ON SITE

COPY
 1/17/95
 11 FEB 1995

NO. DATE AMOUNT
 C OCT 95 WFTY AND PIR EXPANDED MANHOLE TO 10
 B SEPT 94 ROAD WORK CHANGING WALKWAY FROM
 A JAN 92 STATE OF CALIFORNIA, COUNTY OF
 FORMER DWG. NO. 931/PD-3002

Plan Approved
 Chief Building Surveyor
 for BUILDING AUTHORITY
 21 NOV 1995

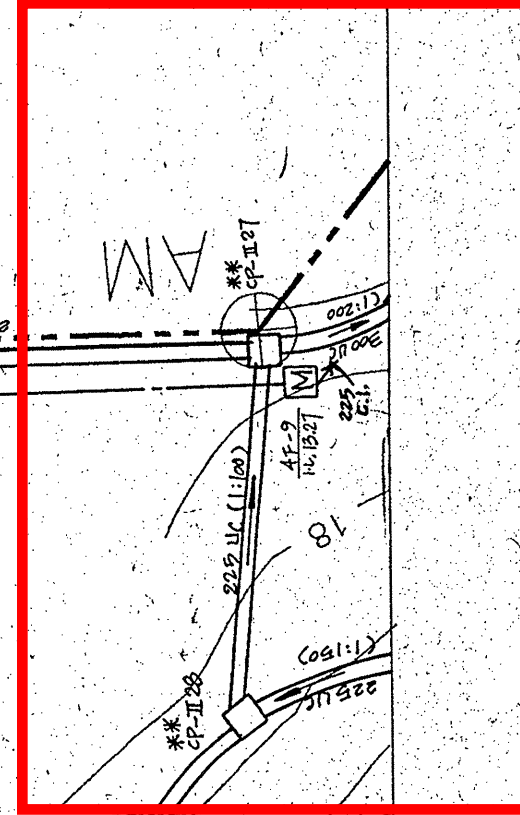
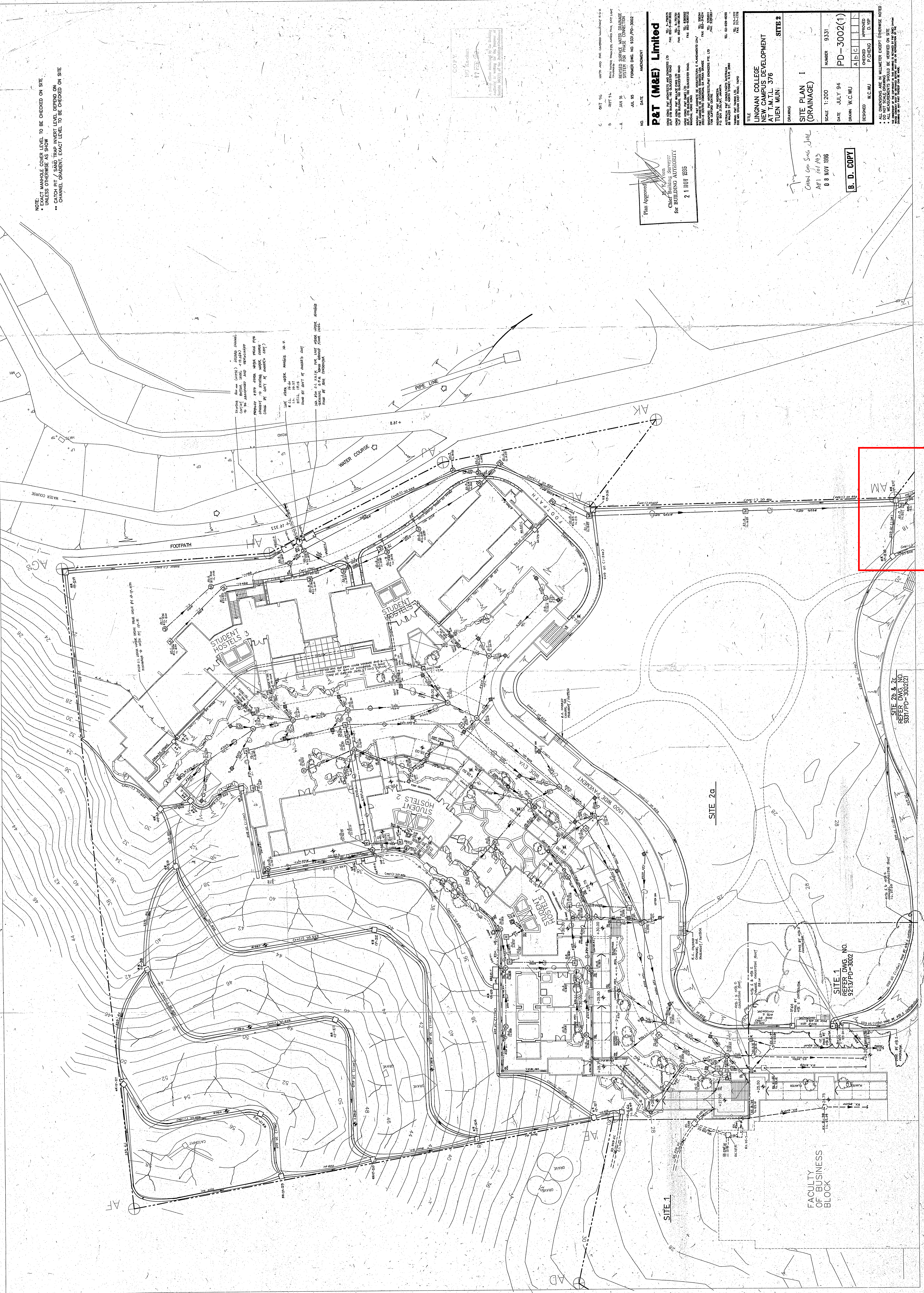
P&T (M&E) Limited
 277 CHURCH STREET, SUITE 200, TORONTO, ONT. M5G 1B5
 TEL: (416) 593-8888 FAX: (416) 593-8889
 1000 BAYVIEW AVENUE, SUITE 100, SCARBOROUGH, ONT. M1B 2Y9
 TEL: (416) 291-1111 FAX: (416) 291-1112
 1000 SHEPPARD AVENUE EAST, SUITE 100, SCARBOROUGH, ONT. M1B 2Y9
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 1000 SHEPPARD AVENUE EAST, SUITE 100, SCARBOROUGH, ONT. M1B 2Y9
 TEL: (416) 291-1111 FAX: (416) 291-1112

FILE
 LINGNAN COLLEGE
 NEW CAMPUS DEVELOPMENT
 AT T.M.T.L. 376
 TUEN MUN
 SITE 2

DATE JULY 94
 DRAWN W.C.WU
 DESIGNED W.C.WU
 CHECKED P. CHENG
 APPROVED D. YIP

NUMBER 9331
 PD-3002(1)

ALL DIMENSIONS ARE IN MILLIMETERS EXCEPT OTHERWISE NOTED
 * ALL DIMENSIONS SHOULD BE VERIFIED ON SITE
 ** ALL DIMENSIONS SHOULD BE VERIFIED ON SITE
 *** ALL DIMENSIONS SHOULD BE VERIFIED ON SITE



SITE 2b & 2c
 REFER DWG. NO.
 9331/PD-3002(2)

SITE 1
 REFER DWG. NO.
 9218/PD-3002

SITE 2a

SITE 1

RECEIVED 27
 NOV 11 3 22 PM '95

NOTE: MANHOLE COVER LEVEL TO BE CHECKED ON SITE
 * EXACT COVER LEVEL TO BE CHECKED ON SITE
 ** EXACT COVER LEVEL TO BE CHECKED ON SITE
 *** CHANNEL GRADIENT, EXACT LEVEL TO BE CHECKED ON SITE

COPY
 HG Keedra
 17 FEB 97

24 DEC 1996
 Approved Person (List 1)

DEC 16
 JAN 96
 JAN 95
 DATE
 AMENDMENT

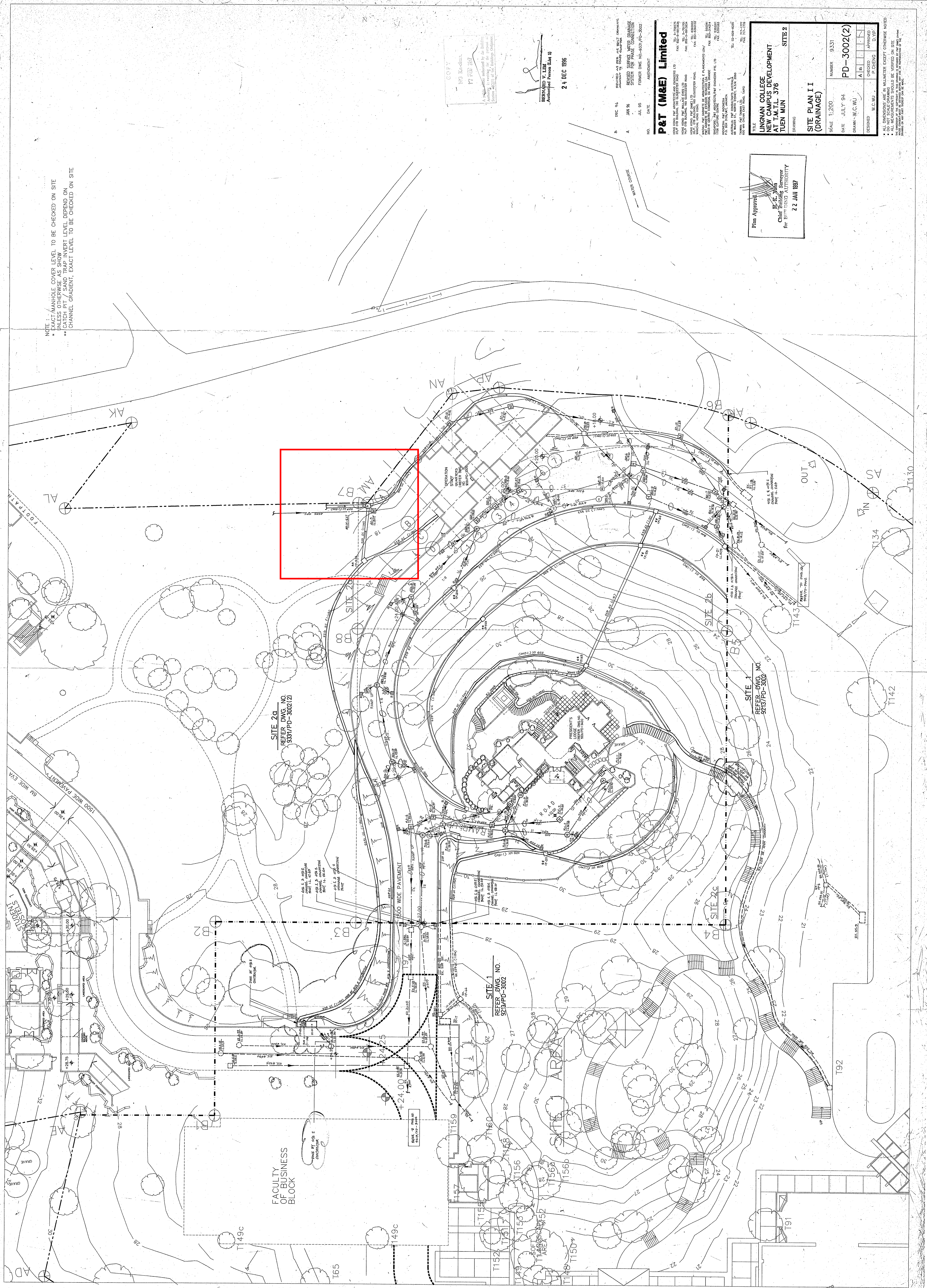
P & T (M&E) Limited
 207 WILSON AVENUE, SUITE 200, WILSON, ONTARIO L0R 2K0
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 FAX: (416) 491-1112
 100 WILSON AVENUE, SUITE 200, WILSON, ONTARIO L0R 2K0
 TEL: (416) 491-1111
 FAX: (416) 491-1112

**LINGNAN COLLEGE
 NEW CAMPUS DEVELOPMENT
 AT L.M.T.L. 376
 TUEN MUN
 DRAWING**

**SITE PLAN II
 (DRAINAGE)**

SCALE 1:200
 DATE JULY 94
 DRAWN: W.C. WU
 CHECKED: P. CHENG
 NUMBER 9331
 REFER-DWG. NO. PD-3002(2)
 APPROVED: D.V.P.

Plan Approval
 Chief Building Surveyor
 for THE HONG KONG AUTHORITY
 22 JAN 1997



Received by
 05 JAN 97

Appendix C

Estimation of Sewage Discharge from the Site

Table 1 Total Average Sewage Discharge from Proposed Development

Proposed Development	Lingnan Hub		
Laboratory			
Total Number of Person	210	persons	Based on submitted GBP
Total Number of Staff	30	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28	m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	180	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04	m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	15.6	m ³ /day	
Multi-Function Room			
Total Number of Person	71	persons	Based on submitted GBP
Total Number of Staff	11	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28	m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	61	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04	m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	5.52	m ³ /day	
Lecture Room			
Total Number of Person	287	persons	Based on submitted GBP
Total Number of Staff	41	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.28	m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Total Number of Student	246	persons	Based on ratio of staff to student: https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
Unit Flow Factor ^[2]	0.04	m ³ /person/day	School Student in Table T-2 of GESF
Average Sewage Discharge	21.32	m ³ /day	
Office			
Total Number of Staff	156	persons	Based on submitted GBP
Unit Flow Factor ^[2]	0.28	m ³ /person/day	Commercial Employee + Community, Social and Personal Services - J11 in Table T-2 of GESF.
Average Sewage Discharge	43.68	m ³ /day	
Food Store & Kitchen			
Total Floor Area	162	m ²	Based on submitted GBP
Worker Density per GFA (in 100m ²)	5.1	person/100 m ²	Referred to the worker density of Community, Social & Personal Services (All Types) in Table 8 of CIFSUS
Total number of person	9	persons	Based on submitted GBP
Unit flow Factor	1.58	m ³ /person/day	Referred to the planning unit flow for Commercial Employee + Restaurants & Hotels J10 in Table T-2 of GESF.
Average Sewage Discharge	14.22	m ³ /day	
Total			
Total Sewage Generation Rate (DWF)	100.34	m ³ /day	
Catchment Inflow Factor (Tuen Mun)	1.1		Reference from Table T-4 of Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning
Revised Dry Weather Flow	110.37	m ³ /day	
Contributing Population	409		
Peaking Factor	8		Refer to Table T-5 of GESF
Peak Flow	0.0102	m ³ /s	

Notes:

[1] The average household size is made reference to "2021 Population Census Summary Results", published by C&SD.

[2] The unit flow factor is made reference to "Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning (Version 1.0)", published by EPD.

[3] The worker density is made reference to Table 8 of CIFSUS, published by PlanD.

Appendix D

Estimation of Sewage Discharge from Catchment Areas

Population Estimation for Catchment Areas

Catchment	Description	Magnitude	Unit	Remark
A1	Ho Sin Hang Building, Leung Kau Kui Building, B.Y. Lam Building, Dorothy Y.L. Wong Building, Lau Lee Yuen Haan Amenities Building, Lau Chung Him Building, Patrick Lee Wan Keung Academic Building, Lingnan University Main Building			
	Total number of Staff (Including Honorary, visiting, temporary & adjunct staff)	972	persons	Referred to University Statistics Booklet 2023 by Lingnan University https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
	Unit flow	0.28	m ³ /person/day	Referred to the planning unit flow for Commercial Employee + Community, Social & Personal Services J11 in Table T-2 of GESF.
	Total Number of Student	5265	persons	Referred to University Statistics Booklet 2023 by Lingnan University https://www.ln.edu.hk/about-lu/facts-and-figures/university-statistics-booklet
	Unit Flow Factor ^[2]	0.04	m ³ /person/day	School Student in Table T-2 of GESF
	Average Sewage Discharge	<u>482.76</u>	m ³ /day	
A2	Canteen (G/F Lau Lee Yuen Haan Amenities Building)			
	Total Floor Area	1770.0	m ²	Refer to Geoinfo map, located at GF
	Worker Density per GFA (in 100m ²)	5.1	person/100 m ²	Referred to the worker density of Community, Social & Personal Services (All Types) in Table 8 of CIFSUS
	Total number of person	91	persons	
	Unit flow Factor	1.58	m ³ /person/day	Referred to the planning unit flow for Commercial Employee + Restaurants & Hotels J10 in Table T-2 of GESF.
	Average Sewage Discharge	<u>143.78</u>	m ³ /day	
A3	Generation from swimming pool			
	Volume of Swimming Pool	213	m ³	Area: 142m ² , assume average height of 1.5m
	Turnover Rate	6	hr	General Specification for Swimming Pool Water Treatment Installation in Government Buildings of The Hong Kong Special Administrative Region (6 hr for outdoor swimming pool)
	Surface Loading Rate of Filter	30	m ³ /m ² /hr	Swimming Pools: Design and Construction, Fourth Edition By Philip H. Perkins (30 m ³ /m ² /hr adopted)
	Required Filter Area	1.183	m ²	
	Backwash Duration	7	mins/day	With reference to Section B8.5.5 of General Specification for Swimming Pool Water Treatment Installation in Government Buildings of the HKSAR published by the ArchSD, "the water velocity chosen shall be effective in cleaning the filter in duration of 7 minutes for sand filter"
	Backwash Flow Rate	30	m ³ /m ² /hr	Technical Paper - Domestic Swimming Pool Filtration by European Union of Swimmingpool and Spa Associations
	Maximum backwash volume	10.35	m ³ /day	
	Peak Flow	0.0247	m ³ /s	
	Total ADWF of Catchment A	<u>626.54</u>	m ³ /day	
B	Indoor Sports Complex			
	Total Floor Area	2005.0	m ²	
	Worker Density per GFA (in 100m ²)	3.3	person/100 m ²	Referred to the worker density of Community, Social & Personal Services (All Types) in Table 8 of CIFSUS
	Total number of person	67	persons	
	Unit flow	0.28	m ³ /person/day	Referred to the planning unit flow for Commercial Employee + Community, Social & Personal Services J11 in Table T-2 of GESF.
	Total ADWF of Catchment B	<u>18.76</u>	m ³ /day	
D	Parkland Villas Block 1-3 Residential Building			
	Total number of units	576	units	Refer to https://hk.centanet.com/estate/en/Parkland-Villas/2-KEPPWPPHPB
	Average Household Size	2.6	persons	Refer to Average Domestic Household Size of Tuen Mun District Council in 2021 Population Census: Summary Result, published by Census and Statistics Department
	Total number of residents	1498	persons	
	Unit flow	0.19	m ³ /person/day	Referred to the planning unit flow for Domestic (housing type specific) - R1 in Table T-1 of GESF.
	Total ADWF of Catchment D	<u>284.6</u>	m ³ /day	

Population Estimation for Catchment Areas

Catchment	Description	Magnitude	Unit	Remark
E1	Parkland Villas Block 4-6 Residential Building			
	Total number of units	576	units	Refer to https://hk.centanet.com/estate/en/Parkland-Villas/2-KEPPWPPHPB
	Average Household Size	2.6	persons	Refer to Average Domestic Household Size of Tuen Mun District Council in 2021 Population Census: Summary Result, published by Census and Statistics Department
	Total number of residents	1498	persons	
	Unit flow	0.19	m ³ /person/day	Referred to the planning unit flow for Domestic (housing type specific) - R1 in Table T-1 of GESF.
Total ADWF of Catchment E		284.6	m ³ /day	
E2	Generation from swimming pool			
	Volumn of Swimming Pool	621	m ³	Area:414m ² , assume average height of 1.5m
	Turnover Rate	6	hr	General Specification for Swimming Pool Water Treatment Installation in Government Buildings of The Hong Kong Special Administrative Region (6 hr for outdoor swimming pool)
	Surface Loading Rate of Filter	30	m ³ /m ² /hr	Swimming Pools: Design and Construction, Fourth Edition By Philip H. Perkins (30 m ³ /m ² /hr adopted)
	Required Filter Area	3.450	m ²	
	Backwash Duration	7	mins/day	With reference to Section B8.5.5 of General Specification for Swimming Pool Water Treatment Installation in Government Buildings of the HKSAR published by the ArchSD, "the water velocity chosen shall be effective in cleaning the filter in duration of 7 minutes for sand filter"
	Backwash Flow Rate	30	m ³ /m ² /hr	Technical Paper - Domestic Swimming Pool Filtration by European Union of Swimmingpool and Spa Associations
	Maximum backwash volume	30.19	m ³ /day	
Peak Flow	0.0719	m ³ /s		

Appendix E1

Calculation of Flow Capacity of Existing Condition before
Proposed Development

Calculation of Flow Capacity of Proposed Development

Sewer No.				Material	Internal Diameter (m) [a]	Cross-section Area (m ²)	Length (m)	Inlet mPD (m) [a]	Outlet mPD (m) [a]	Hydraulic pipeline roughness (m) [b]	Hydraulic Gradient	Mean Velocity (m/s) [c]	Max Capacity of Sewer (m ³ /s)	Total Average Dry Weather Flow	Catchment Inflow Factor	Revised Total Average Dry Weather Flow [g]	Contributing Population	Peaking Factor	Peak Discharge from Project Site m ³ /day	Peak Discharge through Manhole m ³ /s	Swimming Pool Discharge through Manhole m ³ /s	Total Discharge through Manhol m ³ /s	Utility	Remark
S1	E-33	S2	E-32	Clayware	0.300	0.071	4.0	19.35	19.30	0.003	0.01250	1.39	0.098	626.5	1.1	689.2	2553	6.0	4135.2	0.048	/	0.048	48.7%	Catchment A (A1)
S2	E-32	S3	E-28	Clayware	0.300	0.071	8.2	19.30	19.20	0.003	0.01220	1.37	0.097	626.5	1.1	689.2	2553	6.0	4135.2	0.048	/	0.048	49.3%	Catchment A (A1&A2)
S3	E-18	S4	E-16	Clayware	0.300	0.071	16.2	18.85	17.60	0.003	0.07716	3.46	0.244	626.5	1.1	689.2	2553	6.0	4135.2	0.048	/	0.048	19.6%	Catchment A (A1&A2&A3)
S4	E-13	S5	E-12	Clayware	0.300	0.071	16.1	17.00	16.87	0.003	0.00807	1.12	0.079	626.5	1.1	689.2	2553	6.0	4135.2	0.048	/	0.048	60.7%	Catchment A (A1-A4)
S5	E-9	S6	E-8	Clayware	0.300	0.071	9.4	12.35	11.80	0.003	0.05851	3.01	0.213	626.5	1.1	689.2	2553	6.0	4135.2	0.048	/	0.048	22.5%	Catchment A (A1-A7)
S6	E-2	S7	E-1A	Clayware	0.300	0.071	21.5	10.11	9.90	0.003	0.00977	1.23	0.087	626.5	1.1	689.2	2553	6.0	4135.2	0.048	0.0247	0.073	83.5%	Catchment A (A1-A8)
S7	E-1A	S8	E-1	Clayware	0.300	0.071	22.6	9.90	8.88	0.003	0.04513	2.64	0.187	626.5	1.1	689.2	2553	6.0	4135.2	0.048	0.0247	0.073	38.8%	Catchment A (A1-A8)
S8	FMH1015210	S9	FMH1015211	Clayware	0.450	0.159	19.2	7.64	7.48	0.003	0.00836	1.49	0.236	1214.5	1.1	1336.0	4948	6.0	8016.0	0.093	0.0965	0.189	80.0%	Catchment A-E
S9	FMH1015211	S10	FMH1015265	Clayware	0.450	0.159	52.0	6.04	5.69	0.003	0.00673	1.33	0.212	1214.5	1.1	1336.0	4948	6.0	8016.0	0.093	0.0965	0.189	89.2%	Catchment A-E

[a] Reference from GeoInfo Map
 [b] For all the public sewers, assumed clayware slimed sewers in "poor" condition, ks value of 3.0mm is adopted.
 [c] The velocity is calculated using the Colebrook-White Formula:

$$V = -2 \left(2gDS \right)^{0.5} \log \left(\frac{k}{3.7D} + \frac{2.5v}{D(2gDS)^{0.5}} \right)$$

where
 k = Colebrook-White roughness coefficient, in meter
 V = mean velocity (m/s)
 D = circular cross-section pipe, inside diameter (m)
 S = slope, in meters per meter
 v = kinematic viscosity of water, in meter per second (0.000001306 m²/s)
 g = gravitational acceleration (m/s²) (9.807m/s²)

[d] The Contributing Population is defined as:
 Contributing Population = $\frac{\text{Calculated total average flow (m}^3\text{/day)}}{0.27 \text{ (m}^3\text{/person/day)}}$

[e] Reference from Table T-4 of Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning
 [f] Reference from Table T-4 of Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning
 [g] Revised Total Average Dry Weather Flow = Total Average Dry Weather Flow x Catchment Inflow Factor
 [h] Pipe segment that exceeded 100% used capacity are bolded and underlined

Appendix E2

Calculation of Flow Capacity of Proposed Development

Calculation of Flow Capacity of Proposed Development

Sewer No.				Material	Internal Diameter (m) [a]	Cross-section Area (m ²)	Length (m)	Inlet mPD (m) [a]	Outlet mPD (m) [a]	Hydraulic pipeline roughness (m) [b]	Hydraulic Gradient	Mean Velocity (m/s) [c]	Max Capacity of Sewer (m ³ /s)	Total Average Dry Weather Flow	Catchment Inflow Factor	Revised Total Average Dry Weather Flow [g]	Contributing Population	Peaking Factor	Peak Discharge from Project Site m ³ /day	Peak Discharge through Manhole m ³ /s	Swimming Pool Discharge through Manhole m ³ /s	Total Discharge through Manhol m ³ /s	Utility	Remark
P1	Site TM	S1	E-33	Clayware	0.300	0.071	7.0	19.45	19.35	0.003	0.01429	1.49	0.105	100.3	1.1	110.4	409	8.0	883.0	0.010	/	0.010	9.7%	The Site
S1	E-33	S2	E-32	Clayware	0.300	0.071	4.0	19.35	19.30	0.003	0.01250	1.39	0.098	753.7	1.1	829.0	3070	6.0	4974.2	0.058	/	0.058	58.6%	Catchment A (A1)+ The Site
S2	E-32	S3	E-28	Clayware	0.300	0.071	8.2	19.30	19.20	0.003	0.01220	1.37	0.097	753.7	1.1	829.0	3070	6.0	4974.2	0.058	/	0.058	59.3%	Catchment A (A1&A2)+ The Site
S3	E-18	S4	E-16	Clayware	0.300	0.071	16.2	18.85	17.60	0.003	0.07716	3.46	0.244	753.7	1.1	829.0	3070	6.0	4974.2	0.058	/	0.058	23.6%	Catchment A (A1&A2&A3)+ The Site
S4	E-13	S5	E-12	Clayware	0.300	0.071	16.1	17.00	16.87	0.003	0.00807	1.12	0.079	753.7	1.1	829.0	3070	6.0	4974.2	0.058	/	0.058	73.0%	Catchment A (A1-A4)+ The Site
S5	E-9	S6	E-8	Clayware	0.300	0.071	9.4	12.35	11.80	0.003	0.05851	3.01	0.213	753.7	1.1	829.0	3070	6.0	4974.2	0.058	/	0.058	27.1%	Catchment A (A1-A7)+ The Site
S6	E-2	S7	E-1A	Clayware	0.300	0.071	21.5	10.11	9.90	0.003	0.00977	1.23	0.087	753.7	1.1	829.0	3070	6.0	4974.2	0.058	0.0247	0.082	94.7%	Catchment A (A1-A8)+ The Site
S7	E-1A	S8	E-1	Clayware	0.300	0.071	22.6	9.90	8.88	0.003	0.04513	2.64	0.187	753.7	1.1	829.0	3070	6.0	4974.2	0.058	0.0247	0.082	44.0%	Catchment A (A1-A8)+ The Site
S8	FMH1015210	S9	FMH1015211	Clayware	0.450	0.159	19.2	7.64	7.48	0.003	0.00836	1.49	0.236	1341.7	1.1	1475.8	5466	5.0	7379.1	0.085	0.0965	0.182	76.9%	Catchment A-E+ The Site
S9	FMH1015211	S10	FMH1015265	Clayware	0.450	0.159	52.0	6.04	5.69	0.003	0.00673	1.33	0.212	1341.7	1.1	1475.8	5466	5.0	7379.1	0.085	0.0965	0.182	85.7%	Catchment A-E+ The Site

[a] Reference from GeoInfo Map
 [b] For all the public sewers, assumed clayware slined sewers in "poor" condition, ks value of 3.0mm is adopted.
 [c] The velocity is calculated using the Colebrook-White Formula:

$$V = -2(2gDS)^{0.5} \log \left(\frac{k}{3.7D} + \frac{2.5\nu}{D(2gDS)^{0.5}} \right)$$

where
 k = Colebrook-White roughness coefficient, in meter
 V = mean velocity (m/s)
 D = circular cross-section pipe, inside diameter (m)
 S = slope, in meters per meter
 ν = kinematic viscosity of water, in meter per second (0.000001306 m²/s)
 g = gravitational acceleration (m/s²) (9.807m/s²)

[d] The Contributing Population is defined as:
 Contributing Population = $\frac{\text{Calculated total average flow (m}^3\text{/day)}}{0.27 \text{ (m}^3\text{/person/day)}}$

[e] Reference from Table T-5 of Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning
 [f] Reference from Table T-4 of Guidelines for Estimating Sewage Flows for Sewerage Infrastructure Planning
 [g] Revised Total Average Dry Weather Flow = Total Average Dry Weather Flow x Catchment Inflow Factor
 [h] Pipe segment that exceeded 100% used capacity are bolded and underlined