

# Proposed Development of Hong Kong Sheng Kung Hui St. Christopher's Complex at the Remaining Portion of Taxlord Lot No. T77 In D.D.34. Tai Po

**Sewerage Impact Assessment Report** 

Reference: P060/01 Issue 6 Date: January 2025 Confidential



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## **1** Introduction

#### 1.1 Background

- 1.1.1 The Applicant intends to develop a community service complex (hereafter as "the proposed development") for providing childcare and elderly care services at the remaining portion of Taxlord Lot No. T77 in D.D.34, Tai Po (hereafter as "the Site").
- 1.1.2 According to the Approved Tai Po Outline Zoning Plan (OZP) (OZP No.: S/TP/30) published by Town Planning Board in July 2022, the Site is in the "Government, Institution or Community" Zone.
- 1.1.3 Urban Green Consultants Limited (UGC) was commissioned by the Applicant to conduct a Sewerage Impact Assessment (SIA) to assess the potential sewerage impacts arising from the change of sewerage generation due to the Proposed Development.

#### 1.2 Objective of the SIA

1.2.1 The objectives of this SIA are to assess the potential sewerage impacts arising from the proposed development and recommend the mitigation measures, if necessary, to alleviate the impacts.

#### 1.3 Report Structure

1.3.1 The remaining chapters of this report are shown below:

Chapter 2 – Site Context

Chapter 3 – Evaluation of Sewerage Impact

Chapter 4 – Environmental Considerations

## 2 Site Context

#### 2.1 Site Location and Its Environs

- 2.1.1 The Site is bounded by Tai Po Road Tai Po Kau to its South. The Japanese International School is located at the West of the Site, while Po Leung Kuk Tin Ka Ping Millennium Primary School is at the Northeast of the Site. The site area is approximately 2,210.2m<sup>2</sup>.
- 2.1.2 Figure 2.1 shows the Site location and its environs.

#### 2.2 Proposed Development Scheme

- 2.2.1 The proposed development is a 10-storey building which consists of seven department units, i.e. Special Child Care Centre (SCCC), Care and Attention Home Providing Continuum of Care (CoC Home), Small Group Home (SGH), Foster Care Service and Agency-based Enhancement of Professional Staff Support Services (FCS), Staff Training Unit (STU), and Child Care Centre (CCC). There will also be residential places for the elderly, a basement carpark, and a localized sewerage treatment plant on B2/F. The anticipated commissioning year of the proposed development is 2030.
- 2.2.2 The development plans are shown in Appendix A.

#### 2.3 Existing Sewerage Condition

2.3.1 Based on the review of drainage plans (reference no.: 7-NE-11C-4, 7-NE-11D-3, 7-NE-16A-2 and 7-NE-16B-1) from Drainage Services Department (DSD) in November 2015, it has revealed that the Site is not served by any form of DSD's sewerage facility currently.

## **3 Evaluation of Sewerage Impact**

#### 3.1 Assumptions and Methodology

- 3.1.1 For the sewerage flow estimation from the proposed development, the ceiling unit flow rates as recommended in the "Guidelines for Estimating Sewerage Flows for Sewerage Infrastructure Planning" (hereafter as "GESF") published by the Environmental Protection Department (EPD) in 2005 have been adopted in the assessment.
- 3.1.2 The sewerage generation from the proposed development includes the flow contribution from the staffs, visitors and residents in the proposed development. The population for the proposed development is summarised in Table 3.1.
- 3.1.3 The estimation of pollution loadings from the Proposed Development is based on the basic design parameters of Biochemical Oxygen Demand (BOD) and Suspended Solid (SS) loads for commercial populations as stipulated in "Guidelines for the Design of Small Sewerage Treatment Plants".
- 3.1.4 In the calculation of sewerage generation from both overnight and non-overnight staff, the unit flow factor of 0.28m<sup>3</sup>/person/day was adopted. And the unit flow factor of 0.19m<sup>3</sup>/person/day was adopted for residents. As for visitors, the unit flow factor adopted is 0.015m<sup>3</sup>/person/day. For kitchen staff, the unit flow factor adopted is 1.58m<sup>3</sup>/person/day

Type of People	No. of persons	Unit Flow Factor (m³/person/day)	Category
Staff (non-overnight)	235	0.28	Commercial Employee + Commercial activities – J11 Community, Social & Personal Services in Table T-2 of GESF <sup>[1]</sup>
Staff (overnight)	20	0.28	"Services" in Appendix 2 of GDSSTP <sup>[2]</sup>
Residents	454	0.19	Domestic – Institutional and special class in Table T-1 of GESF <sup>[1]</sup>
Visitors	190	0.015	"Visitor" of Table 3-4 of "Wastewater Engineering Treatment and Reuse" published by Metcalf & Eddy
Staff (Kitchen)	5	1.58	Commercial Employee + Commercial Activities J10 - Restaurants & Hotels in Table T-2 of GESF(b) <sup>[1]</sup>

#### Table 3.1 Estimated Population of the Proposed Development

Notes:

<sup>[1]</sup> Environmental Protection Department, HKSARG [EPD] (2005). Guidelines for estimating sewerage flows for sewerage infrastructure planning (EPD/TP 1/05).

<sup>[2]</sup> Environmental Protection Department, HKSARG [EPD] (2006). Guidelines for the Design of Small Sewage Treatment Plants

3.1.5 The estimated daily flow and hourly flow from the proposed development will be 168.41m<sup>3</sup>/day and 42.10m<sup>3</sup>/hr, respectively. The calculations have been provided in Appendix B for reference.

#### 3.2 Proposed Sewerage Treatment Plant

- 3.2.1 As the Site is not served by public sewer, it is required to provide their own sewerage treatment or disposal facilities to ensure sewerage can be discharged in a proper manner and hence to achieve the best protection to the public and the environment. The use of small Sewerage Treatment Plant (STP) with tertiary wastewater treatment technology is recommended to ensure the sewerage generated from the proposed development will be treated to acceptable level before discharge. The location of the proposed underground sewerage treatment plant is shown in Figure 3.1.
- 3.2.2 The "Guidelines for the Design of Small Sewerage Treatment Plants" published by the EPD for the design, construction, operation, and maintenance of STP should be followed. The effluent quality should comply with the "Standards for effluents discharged into Group D inland waters" and the "Standards for effluents discharged into the coastal waters of Tolo and Port Shelter Water Control Zones" as specified in the "Technical Memorandum on Standards for Effluents Discharged into Drainage and Sewerage Systems, Inland and Coastal Waters (TM-DSS)". The discharge standards of the proposed STP shall adopt the higher standards of the above standards. The key parameters are summarised in Table 3.2.

Key Parameters	Standards for effluents discharged into Group D inland waters (for flow ≤200 m³/d)	Standards for effluents discharged into coastal waters of Tolo and Port Shelter Water Control Zones (for flow >10 and ≤200 m3/d)	Standards for effluents discharged into Group D inland waters (for flow >600 and ≤800 m³/d)	Standards for effluents discharged into coastal waters of Tolo and Port Shelter Water Control Zones (for flow >600 and ≤800 m³/d)	Discharge Standard for the Proposed Development
Biochemical Oxygen Demand BOD (mg/L)	20	20	20	20	20
Suspended Solids TSS (mg/L)	30	30	30	30	30
Ammonia Nitrogen NH₃-N (mg/L)	20	-	20	-	20
E.coli (count/100mL)	1000	1000	1000	1000	1000
Total Phosphorus TP (mg/L)	10	8	8	5	5
COD	80	80	80	80	80
Surfactants	15	15	15	15	15
Oil & grease	10	20	10	20	10
Total Nitrogen	-	20	-	15	15
Nitrate + Nitrite Nitrogen	50	-	30	-	30

#### Table 3.2 Key Parameters of Discharge Standards

[i] All units in mg/L [ii] All figures are upper limits

- 3.2.3 In order to fulfil the required discharge standards, the proposed STP will provide tertiary treatment and nitrogen, phosphorus removal treatment. The proposed STP will be equipped with biological treatment by Contact Aeration Process, Ultra Filtration System and Ultra Violet (UV) Sterilization. Furthermore, the treated sewerage from the Site will be discharged in accordance with the Water Pollution Control (General) Regulation. Therefore, no adverse water quality impact due to sewerage generation from the proposed development will be anticipated.
- 3.2.4 The treated sewerage and the drainage discharge from the site boundary will be discharged into the existing drainage system through DSD catchpit SCH1003542. Peak runoff is estimated to be 0.113m<sup>3</sup>/s, which exceeds the capacity of the drainage pipes, as shown in Appendix E. Upgrading works of the drainage system will be required to cater the treated sewerage and drainage discharge.
- 3.2.5 The treated sewage and drainage discharge from the site boundary will be directed into the existing drainage system via DSD catchpit SCH1003542, which connects to box culvert SBP1001645 and subsequently discharges into Tolo Harbour through box culvert SBP100293. These two box culverts are linked by a channelized culvert passing through Villa Castell. No other beneficial uses of inland water were identified

in the downstream receiving waters. The location of the downstream receiving water bodies is shown in Figure 3.3.

#### 3.3 Emergency Measures

- 3.3.1 Submersible pump, air blower, chemical dosing pump and submersible ejector will be provided with standby unit. An emergency generator will be provided in case electric power supply for whole system fails. As such, suspension of whole STP operation is rare under normal operation.
- 3.3.2 Regular preventive maintenance service will be provided to check operation condition of equipment and plan the repairing and replacement scheme for equipment and material of treatment system. An operation and maintenance manual will be prepared by the contractor and then the future operator should appoint competent technician(s) to operate the STP. The operator should be fully conversant with the recommended operating procedures as stipulated in the operation and maintenance manual.
- 3.3.3 In case of STP failure, raw sewerage will be temporarily stored in the equalization tank with volume to 2.19 hours retention time on peak flow, a 30.80m<sup>3</sup> effective tank volume of equalization tank will be provided. The detailed design calculation as shown in Appendix C. Real-time monitor will be provided to monitoring the sewerage flow. In the event of any emergency overflow, on-call crews will follow the overflow emergency response plan and proceed with the best response to correct the problem at once. For example, the alarm system will be activated once overflow occurs. The on-call crews will provide instant response by acknowledging the alarm, to investigate the cause of overflow and correct the problem. The alarm system will be repeated until it is acknowledged. In addition, the on-call crews will ensure the standby pump is switched on and contains the overflow sewage using temporary weirs or vacuum trucks, where applicable. Furthermore, tanker away service will be arranged immediately to draw away the sewerage from equalization tank to avoid flooding from treatment system.

#### 3.4 Recommendation

3.4.1 Upon detailed design stage of the Project, the project proponent will appoint authorized Persons and consultant to design the STP. Detailed information of the proposed STP will be prepared and submitted to EPD and DSD during the detailed design stage.

## **4** Environmental Considerations

- 4.1.1 The proposed on-site STP will be fully enclosed and integrated with ventilation system equipped. Odour treatment facilities shall be installed with the ventilation system to remove any odour from the STP.
- 4.1.2 To be prudent, the STP should also implement good housekeeping practices to ensure that the continuous operation of the on-site STP will not generate any unacceptable odour impact on the surrounding environment. These should include the regular inspection of treatment components where odour could be produced, regular cleaning and flushing of screens and other sewerage handling equipment to remove odorous sources, proper handling, and disposal of collected grit and sludge, regular inspections and maintenance of the deodorisation and ventilation systems.
- 4.1.3 As the proposed on-site STP will be fully enclosed by the building structure of the proposed development, the potential breakout noise from the STP upon the surroundings should be minimal.
- 4.1.4 After the implementation of the above measures, the potential odour and noise impact due to the operation of the on-site STP would be minimal or negligible.

## 5 Conclusion

- 5.1.1 A Sewerage Impact Assessment (SIA) has been conducted to evaluate the potential impacts due to the sewerage generation from the proposed development.
- 5.1.2 The proposed development will serve approximately 904 people and the estimated sewerage generation from the proposed development will be approximately 168.41m<sup>3</sup>/day.
- 5.1.3 As there is no existing public sewerage in the vicinity of the Site and, to fulfil the required discharge standards, an underground Sewerage Treatment Plant (STP) will be provided by the Applicant.
- 5.1.4 The proposed STP will be designed, constructed, operated and maintained in accordance with the "Guidelines for the Design of Small Sewerage Treatment Plants" published by the EPD to ensure the sewerage generated from the proposed development will be treated to acceptable standards before discharge to the receiving water. Effluent discharge location from the proposed underground STP will be discharged into the existing drainage system to the west of the Site. No adverse water quality impact due to sewerage generation from the proposed development is anticipated.
- 5.1.5 With the implementation of mitigation measures including the emergency generator, spare pumps, emergency storage and control system etc, no emergency sewage discharge during the operation of STP is anticipated.
- 5.1.6 The flow of the sewerage generated and stormwater runoff from the Site will, however, exceed the capacity of the existing drainage system. Upgrading works of the drainage pipes will be required to cater the treated sewerage and drainage discharge.
- 5.1.7 With the proposed upgrading works, it is concluded that there shall be no sewerage impact arising from the proposed development.

## **Figures**

UGC, ref: P060/01 Issue 6, dated January 2025









# **Appendix A**

**Development Plan** 





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Sheng Kung Hui St. Christopher's Complex
9F Plan
SCALE 1:200 @A3
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Sneng Kung Hui St. Christopher's Complex
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# **Appendix B**

Calculation of Peak Flow

#### Sewage Impact Assessment

Proposed Development of Hong Kong Sheng Kung Hui St. Christopher's Complex at the Remaining Portion of Taxlord Lot No. T77 In D.D.34. Tai Po

A Estimation of Daily Flow from the Propo	osed Development	Remark
Daily Flow 1 <u>Generation from Staff</u> Number of staff (non-overnight) Unit flow	235 persons 0.280 m³/person/day	Provided by client. Referred to the planning unit flow for Commercial Employee + Commercial activities - J11 Community, Social & Personal Services in Table T-2 of GESF <sup>(a)</sup> .
Number of staff (overnight) Unit flow Estimated daily flow	20 persons 0.280 m³/person/dav <u>71.40</u> m³/day	Provided by client. Referred to the planning unit flow for" Services" in Appendix 2 of GDSSTP <sup>(b)</sup> .
2 <u>Generation from Dormitory</u> Number of residents Unit flow Estimated daily flow	454 persons 0.190 m³/person/day <u>86.26</u> m³/dav	Provided by client. Referred to the planning unit flow for Domestic - Institutional and special class in Table T- 1 of GESF <sup>(a)</sup> .
3 <u>Generation from Visitors</u> Number of persons Unit flow Estimated peak daily flow	190 persons 0.015 m³/person/day <u>2.85</u> m³/dav	Provided by client. Referred to the unit flow factor for "Visitor" of Table 3-4 of "Wastewater EngineeringTreatment and Reuse" published by Metcalf & Eddy.
4 <u>Generation from Proposed Kitchen</u> Total Gross Floor Area Worker density Total number of persons Unit flow Estimated daily flow	83.70 m <sup>2</sup> 5.10 persons/ 100m <sup>2</sup> GFA 5 persons 1.58 m <sup>3</sup> /person/day <u>7.90</u> m <sup>3</sup> /day	Provided by client. Referred to worker density for Restaurants + All Types in Table 8 of CIFSUS(a) Referred to the planning unit flow for Commercial Employee + Commercial Activities J10 - Restaurants & Hotels in Table T-2 of GESF(b)
Total estimated daily flow	<u>168.41</u> m³/day	

Notes:

GESF - "Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning" published by Environmental Protection Department (EPD) in 2005. GDSSTP - "Guidelines for the Design of Small Sewage Treatment Plants" published by Environmental Protection Department (EPD) in 2006. (a) (b)

## **Appendix C**

Sewerage Treatment Plant Design Calculation (Tentative)

Proposed Development of Hong Kong Sheng Kung Hui St. Christopher's Complex at the Remaining Portion of Taxlord Lot No. 777 In D.D.34. Tai Po Sewage Treatment Plant Design Calculation

		Remark
Estimation of Peak Flow from Proposed De 1 Generation from Staff	velopment	
Number of staff (non-overnight) Unit flow	235 persons 0.280 m <sup>3</sup> /person/day	Provided by client. Referred to the planning unit flow for Commercial Employee + Commercial activities - J1:
Number of staff (overnight)	20 persons	Community, Social & Personal Services in Table 1-2 of GESF <sup>107</sup> .
Unit flow	0.280 m <sup>3</sup> /person/dav	Referred to the planning unit flow for "Services" in Appendix 2 of GDSSTP <sup>(b)</sup> .
Estimated daily flow Generation from Dormitory	<u>71.40</u> m <sup>3</sup> /day	
Number of bed spaces	454 beds	Provided by client.
Unit flow	0.190 m <sup>3</sup> /person/day	Referred to the planning unit flow for Domestic - Institutional and special class in Table T 1 of GESF <sup>(a)</sup> .
Estimated peak daily flow Generation from Visitors	<u>86.26</u> m <sup>3</sup> /dav	
Number of persons	190 persons	Provided by client.
Unit flow	0.015 m <sup>3</sup> /person/day	Referred to the unit flow factor for "Visitor" of Table 3-4 of "Wastewater EngineeringTreatment and Reuse" published by Metcalf & Eddy.
Estimated daily flow	2.85 m³/dav	
Generation from Kitchen Number of persons	5.0 persons	Provided by client.
Unit flow	1.6 m3/person/day	Referred to the planning unit flow for Commercial Employee + Commercial Activities J10
Estimated daily flow	7.9 m3/day	Restaurants & Hotels in Table 1-2 of GESF(b)
Total estimated daily flow	168.41 m <sup>3</sup> /day	
Average hourly flow (DWF)	7.02 m <sup>3</sup> /hour	Dry Weather Flow
Total estimated peak hourly flow	42.10 m <sup>3</sup> /hour	6 DWF
BOD loading Generation from Proposed Development		
Total Number of population	904 persons	
Unit flow Total BOD loading	0.055 kg/h/day 49.72 kg/day	Referred to Guidelines for the Design of Small Sewage Treatment Plants by EPD.
	Home Ryony	
Design Flow for STP Design Flow	28.07 m <sup>3</sup> /hour	Design flow = average hourly flow * 4 DWF
-		
Equalization tank		
Minimum flow capacity	28.07 m <sup>3</sup>	
Tank length	4 m	
Tank Width	3.5 m	
r anк depth Design water depth	3 m 2.6 m	
Effective water level	2.2 m	
Effective tank volume	30.80 m <sup>3</sup>	
Retention time	2.19 hr	Minimum retention time of 2 hours at peak flow.
		iterennen anno – rain vonainer(Estimatou rean Mouthy Fiow - Design Fiow)
Contact aeration tank Tank length	5 m	
Tank width	4 m	
Tank depth	3.5 m	
Effective tank volume	62 m <sup>3</sup>	
Contact media bed		
Length	5 m	
Vidth Depth	4 m 3 m	
Volume	60 m <sup>3</sup>	
Detention time	2.21 hr	Detention time = Tank Volume/ Design Flow
Final settling tank	2	
Hopper length (L)	2.5	- <del>□</del>
Hopper base width (B)	1.8	
Top water level	+2.5	
Tank bottom level	+0.9	
vvater depth	3 m	
h <sub>2</sub>	0.61 m 2.39 m	$h_2 = ((L-B)/2) \tan 60$ $h_2 = Water depth - h_2$
V <sub>1</sub>	14.96 m <sup>3</sup>	Lx Lx h <sub>1</sub>
V <sub>2</sub>	2.83 m <sup>3</sup>	(1/3) x h₂ x ((L²+ L x B + B²)
Total volume	35.58 m <sup>3</sup>	V <sub>1</sub> +V <sub>2</sub>
Retention time	2.53 hr	Retention time = Tank Volume/(Estimated Peak Hourly Flow - Design Flow)
Filtration pump sump		
Tank length	4 m	
Lank width Design water depth	3 m 3 m	
Effective tank volume	36 m <sup>2</sup>	
Ultra filtration system		
No. of module Effective area	1 module 33 m <sup>2</sup> /module	According to the Manual-Ultrafiltration Technical
Effective area in all module	33 m <sup>2</sup>	
Design filtration rate Flow capacity	0.5 m/h 16.5 m <sup>3</sup> /hour	According to the Manual-Ultrafiltration Technical
Sluge holding tank		
Tank length	5.5 m	
rank worth Design water depth	5 m 5 m	
Effective tank volume	137.5 m <sup>2</sup>	
BOD loading for biological treatment	45.99 kg/day	7.5% BOD removed after fine bar screen
Effluent BOD concentration	273.09 mg/l	=BOD loading/ Total estimated daily flow x 1000
BOD removal rate	42.62 kg/day	=Total estimated daily flow x 1000 x (BOD loading for biological treatment - Effluent BOI
		concentration)x 10
Sludge production rate	42.62 kg/day 2.110 m <sup>3</sup> /day	1.0 kg/kg BOD removal = sludge production rate (kg/day) / 0.02 / 1.01 / 1000
		Assuming that the wet sludge solid content is 2% and specific gravity of 1.01.
Storage time	65.16 days	=Effective tank volume/ Sludge production rate
		at least 60 days

Notes: (a) (b)

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> . GESF - "Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning" published by Environmental Protection Department (EPD) in 2005. GDSSTP - "Guidelines for the Design of Small Sewage Treatment Plants" published by Environmental Protection Department (EPD) in 2006.

Proposed Hong Kong Sheng Kung Hui St. Christopher's Complex at Tai Po Sewage Treatment Plant Design

#### STP size

	length,m	width,m	depth,m	
Equalization tank	4	3.5	3	
Contact aeration tank	5	4	3.5	
Final settling tank	4.5	3.5	3	
Filtration pump sum	4	3	3	
Sludge holding tank	5.5	5	5	
Side wall	0.5			
Seperation wall	0.3			
	length,m	width,m	depth,m	
STP	15.1	9.8	6	147.98 m2

# **Appendix D**

Relevant Effluent Quality Standards

# Table 6Standards for effluents discharged into Group D inland waters(All units in mg/L unless otherwise stated; all figures are upper limits<br/>unless otherwise indicated)

	Flow rate								
	(m <sup>3</sup> /day)	$\leq 200$	>200	>400	>600	>800	>1000	>1500	>2000
Determinand			and	and	and	and	and	and	and
			≦400	$\leq 600$	$\leq 800$	$\leq 1000$	≦1500	$\leq 2000$	≦3000
pH (pH units)		6-10	6-10	6-10	6-10	6-10	6-10	6-10	6-10
Temperature (°C)		30	30	30	30	30	30	30	30
Colour (lovibond units)		1	1	1	1	1	1	1	1
(25mm cell length)									
Suspended solids		<mark>30</mark>	30	30	<mark>30</mark>	30	30	30	30
BOD		<mark>20</mark>	20	20	<mark>20</mark>	20	20	20	20
COD		<mark>80</mark>	80	80	<mark>80</mark>	80	80	80	80
Oil & Grease		<mark>10</mark>	10	10	<mark>10</mark>	10	10	10	10
Iron		10	8	7	5	4	2.7	2	1.3
Boron		5	4	3.5	2.5	2	1.5	1	0.7
Barium		5	4	3.5	2.5	2	1.5	1	0.7
Mercury		0.1	0.05	0.001	0.001	0.001	0.001	0.001	0.001
Cadmium		0.1	0.05	0.001	0.001	0.001	0.001	0.001	0.001
Other toxic metals individually		1	1	0.8	0.8	0.5	0.5	0.2	0.2
Total Toxic metals		2	2	1.6	1.6	1	1	0.5	0.4
Cyanide		0.4	0.4	0.3	0.3	0.2	0.1	0.1	0.05
Phenols		0.4	0.3	0.2	0.1	0.1	0.1	0.1	0.1
Sulphide		1	1	1	1	1	1	1	1
Sulphate		800	600	600	600	600	400	400	400
Chloride		1000	800	800	800	600	600	400	400
Fluoride		10	8	8	8	5	5	3	3
Total phosphorus		<mark>10</mark>	10	10	<mark>8</mark>	8	8	5	5
Ammonia nitrogen		<mark>20</mark>	20	20	<mark>20</mark>	20	20	20	10
Nitrate + nitrite nitrogen		<mark>50</mark>	50	50	<mark>30</mark>	30	30	30	20
Surfactants (total)		<mark>15</mark>	15	15	<mark>15</mark>	15	15	15	15
<mark>E. coli</mark>		<mark>1000</mark>	1000	1000	<mark>1000</mark>	1000	1000	1000	1000
(count/100ml)									

*Cap 358AK – TECHNICAL MEMORANDUM STANDARDS FOR EFFLUENTS DISCHARGED INTO DRAINAGE AND SWERAGE SYSTESM, INLAND AND COASTAL WATERS* 

Table 7Standards for effluents discharged into the coastal waters of Tolo and<br/>Port Shelter Water Control Zones

Flow rate												
(m <sup>3</sup> /day)	≦10	>10	>200	>400	>600	>800	>1000	>1500	>2000	>3000	>4000	>5000
Determinand		and	and	and	and	and	and	and	and	and	and	and
		≦200	≦400	≦600	$\leq 800$	$\leq 1000$	≦1500	≦2000	≦3000	≦4000	≦5000	≦6000
pH (pH units)	6-9	6-9	6-9	6-9	6-9	6-9	6-9	6-9	6-9	6-9	6-9	6-9
Temperature (°C)	45	45	45	45	45	45	45	45	45	45	45	45
Colour (lovibond units)	1	1	1	1	1	1	1	1	1	1	1	1
(25mm cell length)												
Suspended solids	30	<mark>30</mark>	30	30	<mark>30</mark>	30	15	15	15	15	15	15
BOD	20	<mark>20</mark>	20	20	<mark>20</mark>	20	10	10	10	10	10	10
COD	80	<mark>80</mark>	80	80	<mark>80</mark>	80	50	50	50	50	50	50
Oil & Grease	20	<mark>20</mark>	20	20	<mark>20</mark>	20	10	10	10	10	10	10
Iron	10	10	10	7	5	4	2.7	2	1.3	1	0.8	0.6
Boron	5	4	3	2.5	2	1.6	1.1	0.8	0.5	0.4	0.3	0.2
Barium	5	4	3	2.5	2	1.6	1.1	0.8	0.5	0.4	0.3	0.2
Mercury	0.1	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001
Cadmium	0.1	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001
Other toxic metals	1	1	0.8	0.5	0.5	0.4	0.1	0.1	0.1	0.1	0.1	0.1
individually												
Total toxic metals	2	2	1.6	1	1	0.8	0.2	0.2	0.2	0.2	0.14	0.1
Cyanide	0.1	0.1	0.1	0.1	0.1	0.1	0.05	0.05	0.03	0.02	0.02	0.01
Phenols	0.5	0.5	0.5	0.25	0.25	0.25	0.1	0.1	0.1	0.1	0.1	0.1
Sulphide	5	5	5	5	5	5	2.5	2.5	1.5	1	1	0.5
Total residual chlorine	1	1	1	1	1	1	1	1	1	1	1	1
Total nitrogen	20	<mark>20</mark>	20	15	<mark>15</mark>	15	15	15	10	10	10	10
Total phosphorus	8	<mark>8</mark>	5	5	<mark>5</mark>	5	5	5	5	5	5	5
Surfactants (total)	15	<mark>15</mark>	15	15	<mark>15</mark>	15	10	10	10	10	10	10
E. coli	1000	<mark>1000</mark>	1000	1000	<mark>1000</mark>	1000	1000	1000	1000	1000	1000	1000
(count/100ml)												

(ALl units in mg/L unless otherwise stated; all figures are upper limits unless otherwise indicated)

*Cap 358AK – TECHNICAL MEMORANDUM STANDARDS FOR EFFLUENTS DISCHARGED INTO DRAINAGE AND SWERAGE SYSTESM, INLAND AND COASTAL WATERS* 

# **Appendix E**

Existing Drainage System Capacity Calculation

Proposed Development of Hong Kong Sheng Kung Hui St. Christopher's Complex at the Remaining Portion of Taxlord Lot No. T77 In D.D.34. Tai Po Existing Drainage System Capacity Calculation

#### Calculation of Runoff from Site into Storm Drainage Network

Catchment ID	Surface Type	Catchment Area (A), m <sup>2</sup>	Catchment Area (A), km <sup>2</sup>	Average Slope (H), m/100m	Flow path length (L), m	Inlet time (t <sub>0</sub> ), min	Time of Concentration (t <sub>0</sub> ), min	Duration (t <sub>d</sub> ), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	Runoff intensity (i) mm/hr	Runoff coefficient (C)	C × A	Peak runoff (Q <sub>p</sub> ), m <sup>3</sup> /s
Site Boundary	80% Concrete +20% Grassland (heavysoil), flat	2,208	0.002208	-	-	-	-	5.00	451.3	2.5	0.34	227	0.81	0.001788	0.113

#### Capacity Flow Estimation and Adequacy Check for Existing Drainage System

Point (channel no.)	Shape	Diameter, m	Depth, m	Slope	Length, m	Manning's roughness coefficient	Cross Section Area, m <sup>2</sup>	Wetted perimeter, m	Hydraulic radius, m	Mean velocity, m/s	Capacity flow, m³/s	Sewage Treatment Plant and Stormwater Catchment Peak Flow, m <sup>3</sup> /s	% of capacity flow	Sufficient Capacity? (Y/N)
SMD1000600	Circular Pipe × 2 nos.	0.150		0.216	3.0	0.015	0.018	0.471	0.04	3.48	0.123	0.125	102%	N
SWD1011379	Circular Pipe	0.300		0.007	18.5	0.015	0.071	0.942	0.08	0.99	0.070	0.125	178%	N
SWD1011376	Circular Pipe	0.300		0.011	26.6	0.015	0.071	0.942	0.08	1.22	0.086	0.125	145%	N
SWD1011377	Circular Pipe	0.300		0.020	10.7	0.015	0.071	0.942	0.08	1.66	0.117	0.125	106%	N
SWD1011381	Circular Pipe	0.300		0.009	8.8	0.015	0.071	0.942	0.08	1.13	0.080	0.125	156%	N
SWD1011383	Circular Pipe	0.300		0.018	20.0	0.015	0.071	0.942	0.08	1.59	0.113	0.125	111%	N
SWD1014090	Circular Pipe	0.375		0.018	17.4	0.015	0.110	1.178	0.09	1.85	0.204	0.125	61%	Y

# **Appendix F**

Comment from Regional Officer

Dear Emily,

Refer to your SIA report for Taxlord Lot No. T77 in D.D.34, please find our comment below:

"1. In Table 3.2, it is suggested to also include Nitrate + nitrite nitrogen.

2. In part 5 "Ultra filtration system" under Appendix C, the flow capacity (ie. 6m<sup>3</sup>/hour) is not enough for design capacity (ie. 168.41m<sup>3</sup>/day). Please review."

If you have further enquiry please contact me at anytime, thank you.

Regards, Maverick C.K. AU / E(RN)13 Regional Office (North) / EPD 2158 5801 / 9189 0112

 From:
 Emily Tang <emily.tang@urbangreen.hk>

 To:
 "maverickckau@epd.gov.hk" <maverickckau@epd.gov.hk>

 Date:
 23/08/2024 10:09

 Subject:
 Request the Information Advice on District Discharge Standard at Taxlord Lot No. T77 In D.D.34. Tai

 Po

Dear Mr. Au,

We are the environmental consultant undertake the Sewerage Impact Assessment for a service complex development at the Remaining Portion of Taxlord Lot No. T77 In D.D.34. Tai Po. We have received a comment from the EPD-Water Quality Mgt team regarding the discharge standard of the proposed sewerage treatment plant. It would be appreciated if you could advise if the discharge standard listed in Table 3.2 of our SIA report could comply with the district discharge requirements.

Attached the comment below and full set SIA report for your kind reference. We would be grateful if you could provide us the relevant advice at your earliest convenience. Thank you.

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