**Appendix 8 Sewerage Impact Assessment** 

Prepared for

Lo Hing Investment Company Limited

Prepared by

**Ramboll Hong Kong Limited** 

PROPOSED MINOR RELAXATION OF PLOT RATIO (PR) AND SITE COVERAGE (SC) FOR PROPOSED SOCIAL WELFARE FACILITY (RESIDENTIAL CARE HOME FOR THE ELDERLY) (RCHE(S)), TRAINING CENTRE WITH RESIDENTIAL INSTITUTION AND PERMITTED RESIDENTIAL DEVELOPMENT (FLAT) IN LOT 94 IN D.D. 388 AND ADJOINING GOVERNMENT LAND, CASTLE PEAK ROAD – TSING LUNG TAU, TSUEN WANTAU, TSUEN WAN

SEWERAGE IMPACT ASSESSMENT



Date July 2024

Prepared by Miko Wan

**Environmental Consultant** 

Signed

Approved by Calvin Chiu

**Technical Director** 

Signed

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Ramboll Hong Kong Limited 21/F, BEA Harbour View Centre 56 Gloucester Road, Wan Chai, Hong Kong

Tel: (852) 3465 2888 Fax: (852) 3465 2899

Email: hkinfo@ramboll.com

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Tsing Lung Tau, Tsuen Wan

#### 1. INTRODUCTION

#### 1.1 Project Background

- 1.1.1 The Application Site is zoned "Residential (Group B)" ("R(B)") under the Approved Tsuen Wan West Outline Zoning Plan (No. S/TWW/21) with building height restriction of 60 mPD. It is also the subject of a previous planning application (No. A/TWW/122) for proposed minor relaxation of PR from 2.1 to 2.52 for a permitted residential development, which was approved with conditions by the Town Planning Board (TPB) on 12 Aug 2022.
- 1.1.2 The Government has launched the enhanced Incentive Scheme to Encourage Provision of Residential Care Homes for the Elderly (RCHEs) in New Private Developments Time-limited Enhancements (LandsD's Practice Note Issue No. 5/2023). Echoing the incentive scheme, the applicant has now proposed a composite development which contains both private residential use and RCHE.
- 1.1.3 Ramboll Hong Kong Limited (Ramboll) was responsible for the previous planning application (No. A/TWW/122) and prepared the sewerage impact assessment report (SIAR) as one of the technical supporting documents. Ramboll has been appointed to update the SIAR with respect to the current proposal (including private residential use and RCHE) and latest guidelines to address the drainage impact and demonstrate the acceptability of the proposal.
- 1.1.4 Architectural drawings and technical information of the development are provided by the applicant and its project architect.

#### 1.2 Project Location and Environ

- 1.2.1 The Application Site was formerly occupied by an Acid Factory which was already demolished. Currently, the Application Site is a vacant land and most of the area is covered by vegetations.
- 1.2.2 The Application Site is bounded by Castle Peak Road (Tsing Lung Tau) on southern side. It is surrounded by Vale Villa Hong Kong Garden to the north, Hong Kong Garden to the west and Hong Kong Garden Commercial Complex (shopping mall) to the east. Seashore is on the opposite side of Castle Peak Road at over 40m apart.
- 1.2.3 The surrounding is dominated by existing residential development and associated facilities (e.g. shopping mall of the residential development). A vacant site is located to the further north for G/IC uses.
- 1.2.4 The location of the Application Site and surrounding environs are shown in **Figure 1**.

#### 1.3 Proposed Development

- 1.3.1 The Proposed Development consists of a RCHE (G/F to 7/F; 8 floors) and a residential tower (8/F to 15/F). There is also 1 basement floor to cater for car parking area for RCHE, residential and visitor uses.
- 1.3.2 Regarding the RCHE, there will be maximum 320 beds provided in dormitories located at 3/F to 7/F. Other ancillary facilities are mainly located at G/F to 2/F. A rehab facilities (~200m³) is proposed.
- 1.3.3 Regarding the residential portion, clubhouse ( $\sim$ 347m<sup>2</sup>) including an outdoor swimming pool ( $\sim$ 167m<sup>2</sup>) is located at 8/F. A total of 112 flat units are located at 9/F to 15/F.



1.3.4 The tentative completion year is 2032. The proposed development scheme is shown in **Appendix 1**.



Tsing Lung Tau, Tsuen Wan

## 2. SEWERAGE IMPACT ASSESSMENT

#### 2.1 Scope of Work

2.1.1 The aim of this SIA is to assess whether the capacity of the existing sewerage network serving the Application Site is sufficient to cope with the sewage flow from the proposed development. Drainage Record Plans from Drainage Services Department (DSD) were obtained for the purposes of this SIA.

#### 2.2 Assessment Criteria and Methodology

- 2.2.1 Environmental Protection Department's (EPD's) Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning, Version 1 (GESF) has been referred to for the purposes of estimating the quantity of the sewage generated from the proposed development and the existing catchment area. Sewage flow parameters and global peaking factors in this document have been adopted for this SIA.
- 2.2.2 Based on the building types in the area, the following unit flow factors are used in the SIA calculation:
  - RCHE residents: 0.19 m<sup>3</sup>/day (Institutional and special class)
  - RCHE Staff: 0.28 m³/day (J11 Community, Social & Personal Services)
  - F&B employee: 1.58 m³/day (J10 Restaurants & Hotels)
  - Domestic residents and G/IC users: 0.27 m³/day (Private R2)
  - Clubhouse Staff: 0.28 m³/day (J11 Community, Social & Personal Services)
- 2.2.3 Catchment Inflow Factor (P<sub>CIF</sub>) of Tsing Yi (1.10) has been applied in the assessment.

### 2.3 Existing and Future Sewerage System

- 2.3.1 According to the Drainage Record Plans obtained from DSD, there is a Ø500 mm sewer along Castle Peak Road Tsing Lung Tau. The sewer eventually discharges the collected sewage to Tsing Lung Tau Sewage Pumping Station. The existing sewers in the vicinity of the Application Site are shown in **Figure 2**.
- 2.3.2 The sewage generated from the Application Site will be discharged to manhole no.: FMH4052284 (S1) as shown in **Figure 2** nearest to the Application Site.

#### 2.4 Wastewater Generated by the Proposed Development

2.4.1 Wastewater arising from the proposed development will be primarily contributed by the residential residents and clubhouse staff. Detailed calculation for the proposed development is given in **Table 1** below and **Appendix 2**.



Table 1 Estimated Peak Flow

|  | Re                   | esidential Portion | on               |         |                   | RCHE               |                         |                     |  |  |  |
|--|----------------------|--------------------|------------------|---------|-------------------|--------------------|-------------------------|---------------------|--|--|--|
| Development Parameters                             | Residential<br>Units | Clubhouse          | Swimming<br>Pool | Beds    | Worker            | Training<br>Centre | Residential Institution | Rehab<br>Facilities |  |  |  |
| Area (m²)  | -                    | 347                | 167              | -       | -                 | 200                | 50                      | 200                 |  |  |  |
| Number of Residential Units                        | 112                  | -                  | -                |         |                   |                    |                         | -                   |  |  |  |
| Average Household Size                             | 2.7(1)               | -                  | -                |         |                   |                    |                         | -                   |  |  |  |
| Assumed Population                                 | 302                  | 11                 | -                | 320     | 33 <sup>(2)</sup> | 7                  | 2                       | -                   |  |  |  |
| Design Flow<br>(m³/person/day)                     | 0.27 <sup>(3)</sup>  | 0.28(4)            | -                | 0.19(5) | 0.28(4)           | 0.28(4)            | 0.28(4)                 | -                   |  |  |  |
| Flow Rate (m³/day)                                 | 81.6                 | 3.2                | -                | 60.8    | 9.2               | 2.0                | 0.6                     | -                   |  |  |  |
| Flow Rate (L/s)                                    | -                    | -                  | 5.6              |         |                   |                    |                         | 10.0                |  |  |  |
| Total Flow Rate (m³/day)                           |                      |                    |                  | 218.    | 2                 |                    |                         |                     |  |  |  |
| Total Flow Rate with P <sub>CIF</sub> (6) (m³/day) | 240.0                |                    |                  |         |                   |                    |                         |                     |  |  |  |
| Peak Flow (L/s) (with swimming pool backwash)      |                      |                    |                  | 37.8    |                   |                    |                         |                     |  |  |  |

- (1) 2021 Population by-Census: Average Household Size of Tsuen Wan DC district
- (2) Refer to Code of Practice for Residential Care Homes (Elderly Person)
- (3) Refer to Table T-1 of GESF Private R2
- (4) Refer to Table T-2 of GESF J11
- (5) Refer to Table T-1 of GESF Institutional and special class
- (6) With P<sub>CIF</sub> of Tsing Yi (1.10) applied

## 2.5 Assessment of Sewerage Impact

2.5.1 **Appendix 2** shows the detailed calculation on the estimated hydraulic capacity of the proposed sewer sections and existing downstream sewers and the calculation of the amount of the sewage entering each segment of the said sewer network. Total flow from the proposed development is assessed in the calculations.

#### 2.6 Discussion

- 2.6.1 It is proposed to connect the Application Site to the nearest manhole (S1) as shown in Figure 2. It is noted that the ramp of the footbridge will be relocated by the Applicant to southeast corner outside the Application Site and will be on top of the proposed connection. As confirmed by the Project Architect, the supporting structure will be designed to take into account the proposed alignment of the sewer and avoid any conflict.
- 2.6.2 The potential sewerage impact due to the proposed development has been quantitatively addressed. Sewage generation rate from the proposed development is estimated to be 240.0 m³/day (i.e. peak flow 37.8 litre/sec including backwash from swimming pool).
- 2.6.3 After calculating the appropriate capacities as mentioned above, the estimated sewage flow from the Application Site and contribution from other existing developments has been compared with the capacity of the existing and proposed sewerage system to determine whether it has adequate spare capacity to accommodate the flow from the proposed development.



- 2.6.4 According to Table 4a of **Appendix 2**, regarding the sewage generation rate from the proposed development and surrounding catchment areas (**Figure 2**), it is found that one of existing Ø500mm pipe segment (S3-S4) is found with inadequate capacity.
- 2.6.5 The proposed upgrading works are summarized in **Table 2** below.

Table 2 Proposed Upgrading Works

| Segment | Manhole<br>Reference | Manhole<br>Reference | Length (m) | Original Size<br>(Ø) (mm) | Upgraded Size<br>(Ø) (mm) |  |  |
|---------|----------------------|----------------------|------------|---------------------------|---------------------------|--|--|
| S3-S4   | FMH4052286           | FMH4052287           | 30.7       | 500                       | 600                       |  |  |
| S4-S5   | FMH4052287           | FSH4001700           | 8.5        | 500                       | 600                       |  |  |

- 2.6.6 The proposed upgrading works will be implemented by the Applicant. With the proposed works in place, the sewerage system will have adequate capacity to cater for the proposed development and the nearby catchments.
- 2.6.7 The key facility in the local sewerage system is the Tsing Lung Tau Sewage Pumping Station (TLTSPS). Detailed calculation for the proposed development is given in **Table 3** below and **Appendix 2**.

Table 3 Existing & Future Flows to TLTSPS

| TLTSPS   |           |  |  |  |  |  |  |  |  |  |
|--|-----------|--|--|--|--|--|--|--|--|--|
| Designed Peak Flow (I/s)                         | 240^      |  |  |  |  |  |  |  |  |  |
| Existing Average Daily Flow (m³/day)             | 2,581^    |  |  |  |  |  |  |  |  |  |
| Average Flow with the Proposed Development (I/s) | 241(54%*) |  |  |  |  |  |  |  |  |  |

<sup>^</sup> According to DSD (**Appendix 3**)



<sup>\*</sup> Contribution of average flow to designed Peak Flow of TLTSPS

### 3. OVERALL CONCLUSION

#### 3.1 Conclusion

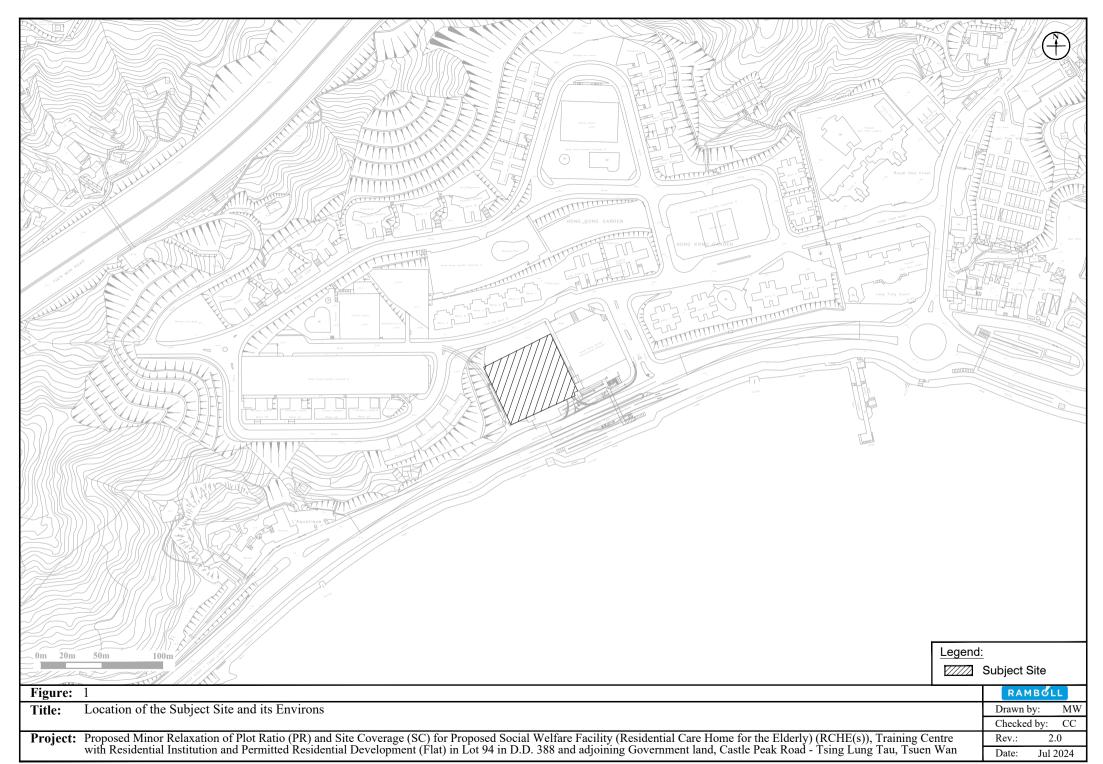
- 3.1.1 A development is proposed at Lot 94, D.D. 388 And Adjoining Government Land, Castle Peak Road Tsing Lung Tau, New Territories. The potential sewerage impact has been quantitatively addressed.
- 3.1.2 It is proposed to connect the Application Site to the nearest manhole for discharge of sewage to public sewer. Based on the sewerage impact assessment results, it is found that the capacity of some of the existing sewerage system serving the area would not be sufficient to cater for the sewage generation from the proposed development and nearby catchment areas. Upgrading works of sewers will be required.
- 3.1.3 With the proposed upgrading works in place, this SIA confirms the feasibility of the proposed development in terms of impacts to the public sewerage system. This project will also provide spare capacity for other development in future as well.

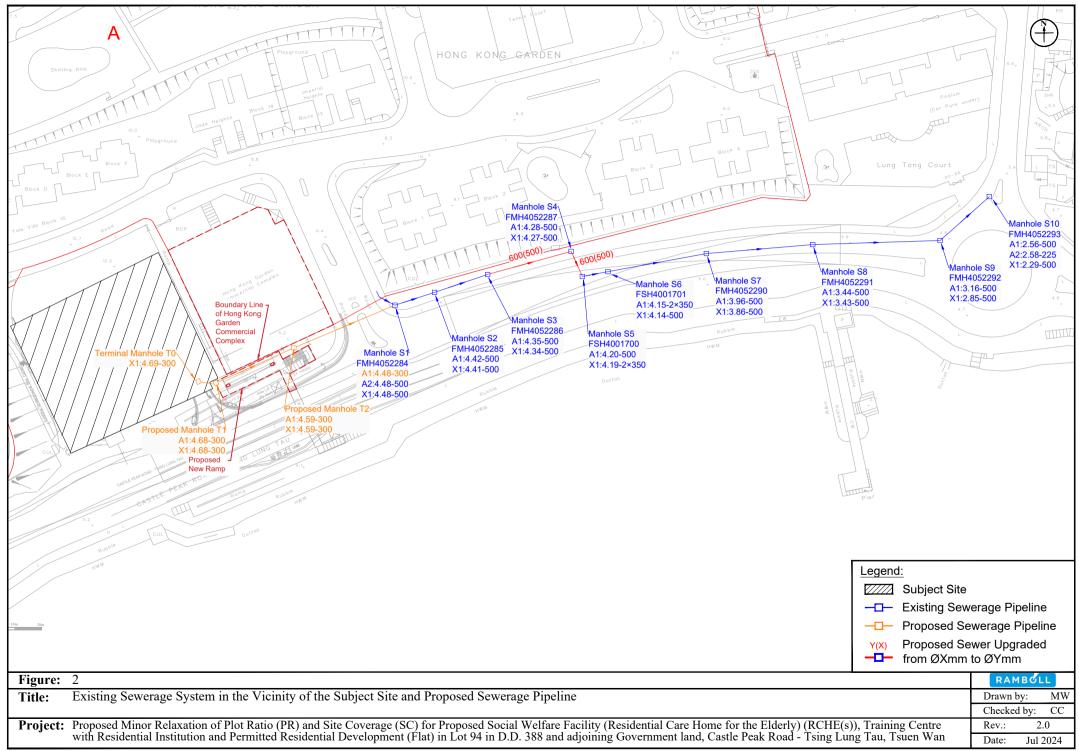


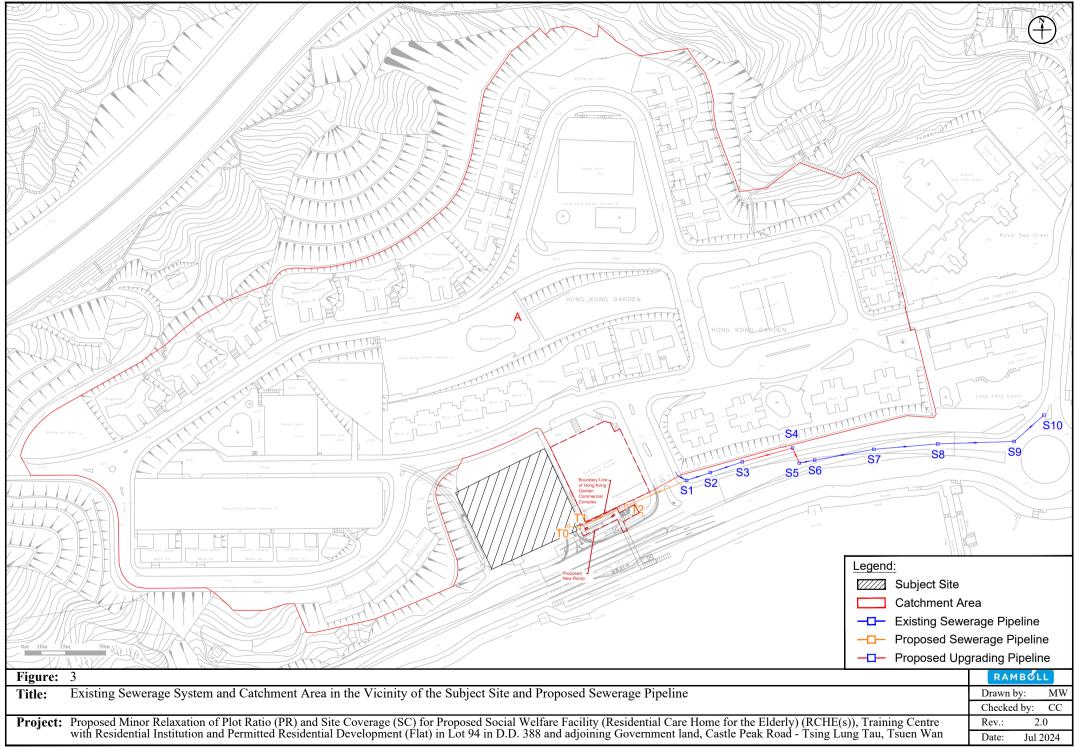
**Figures** 

SIA Report





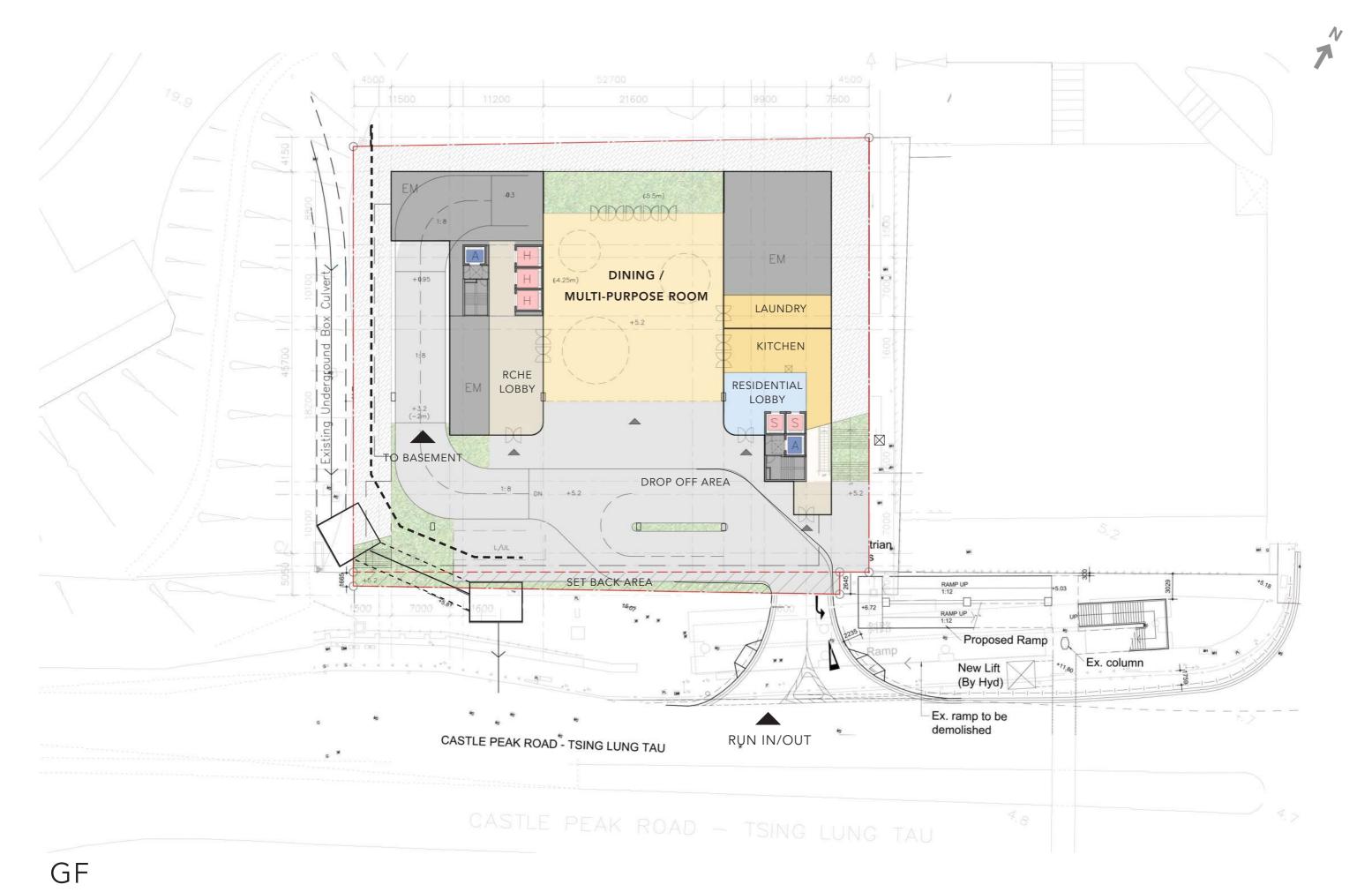




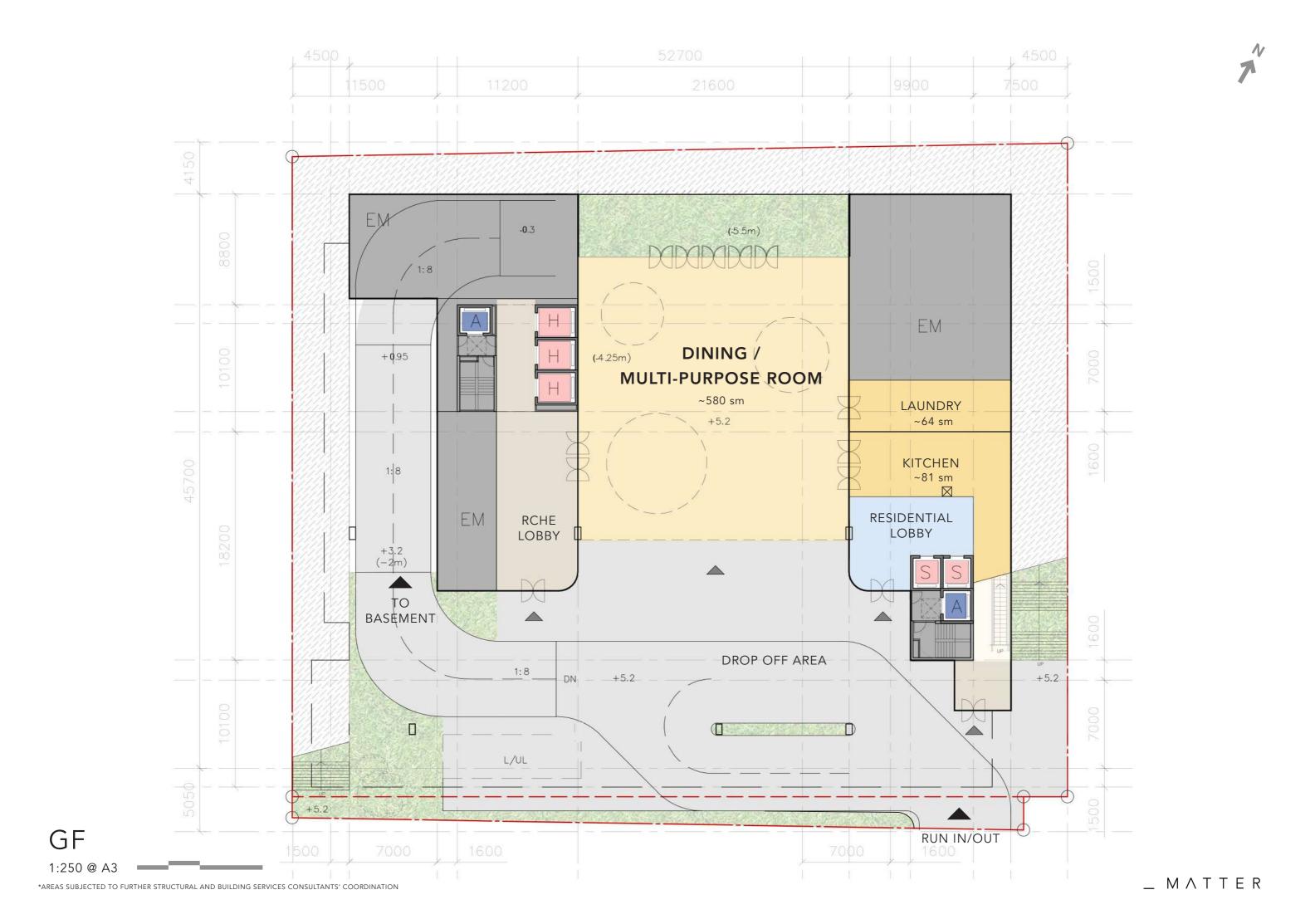
**Appendix 1** Indicative MLP of the Proposed Scheme

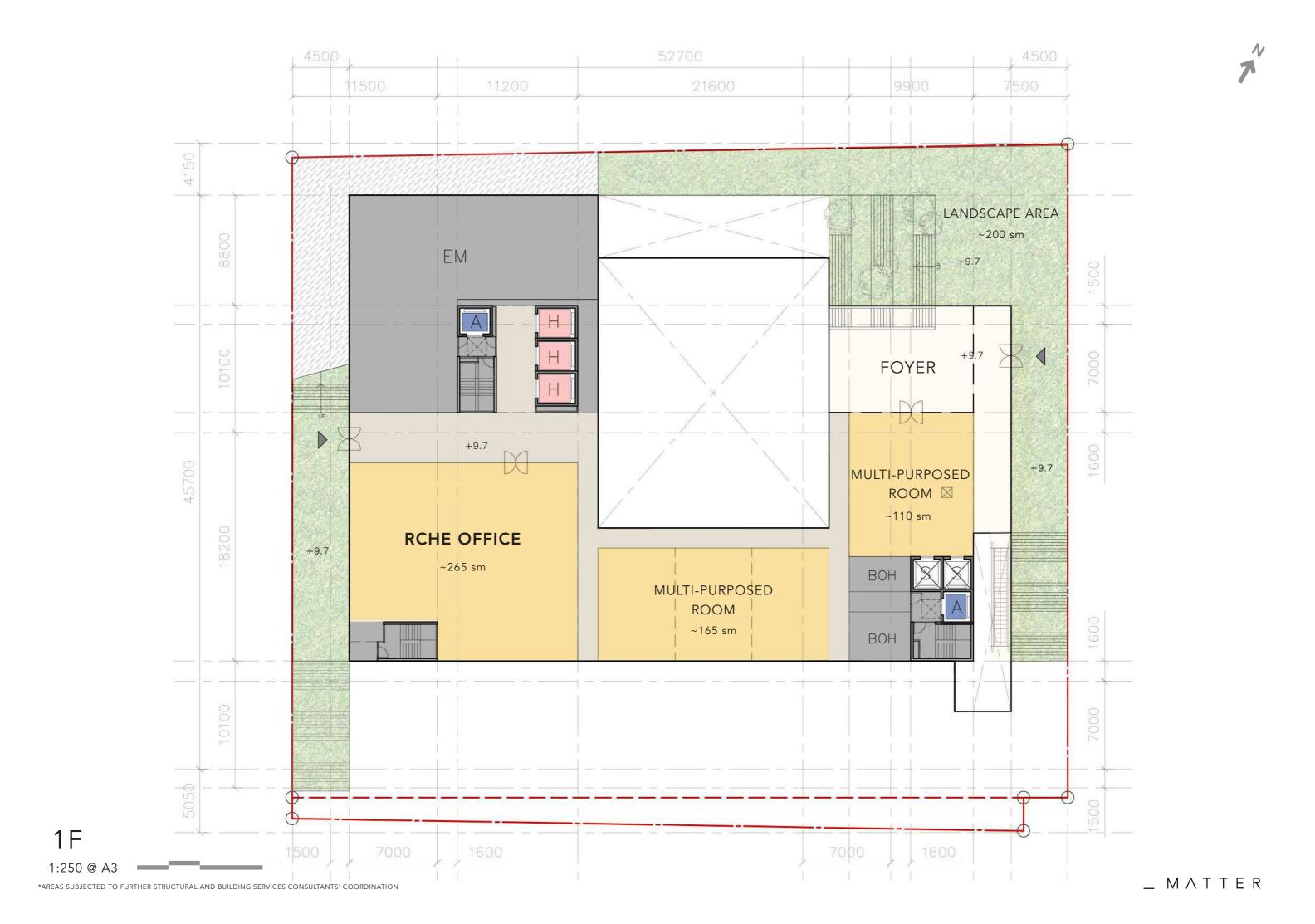


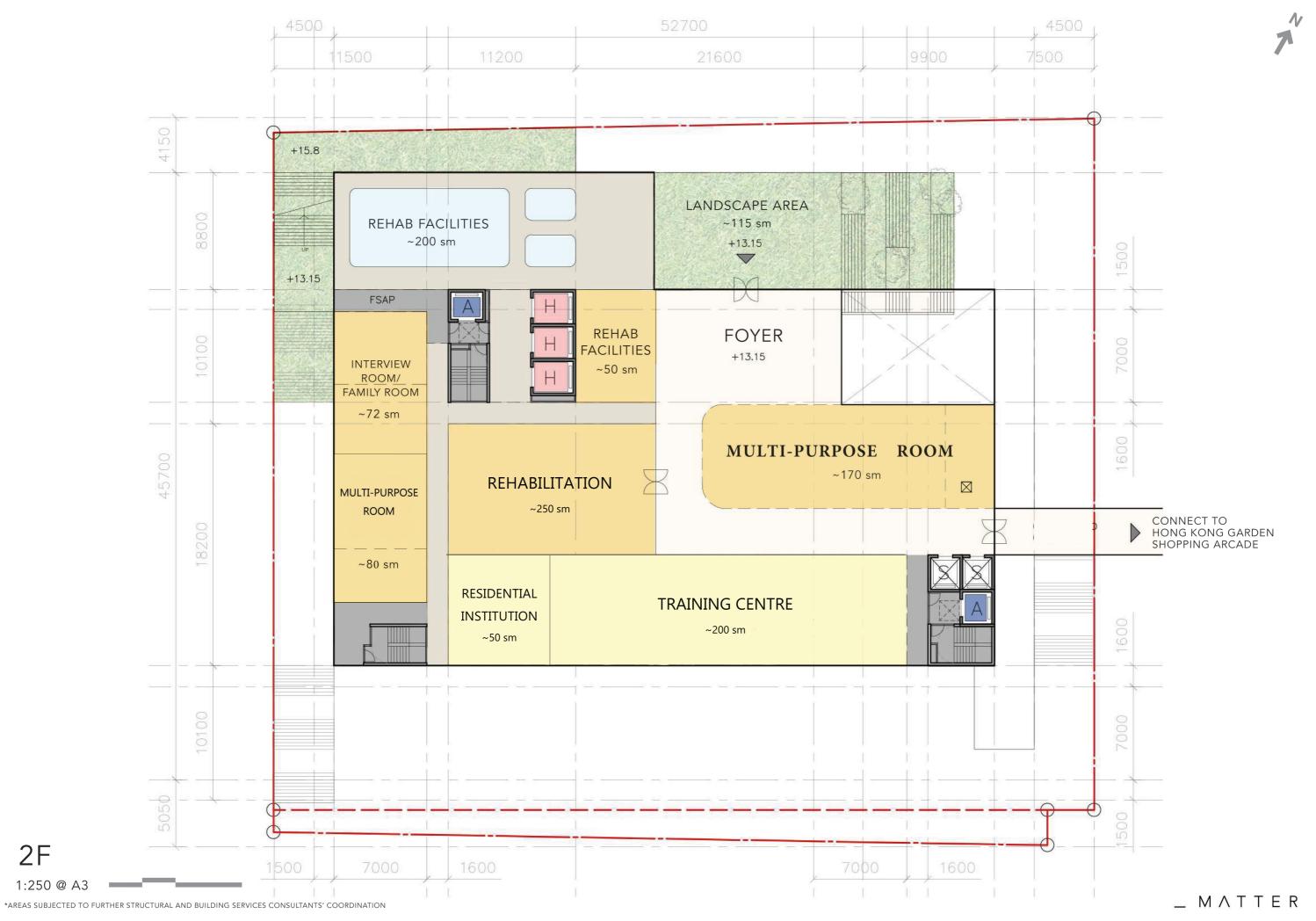
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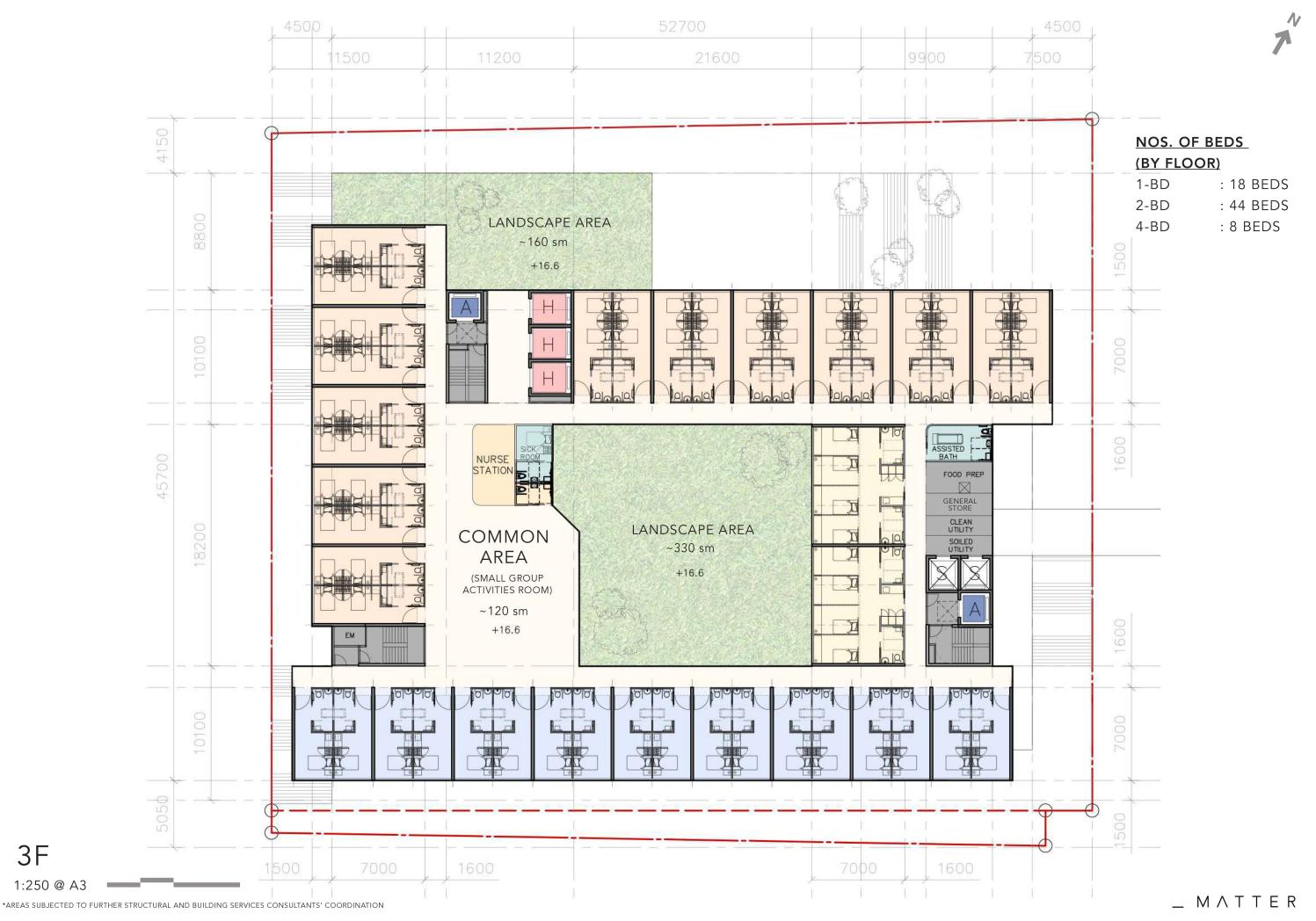


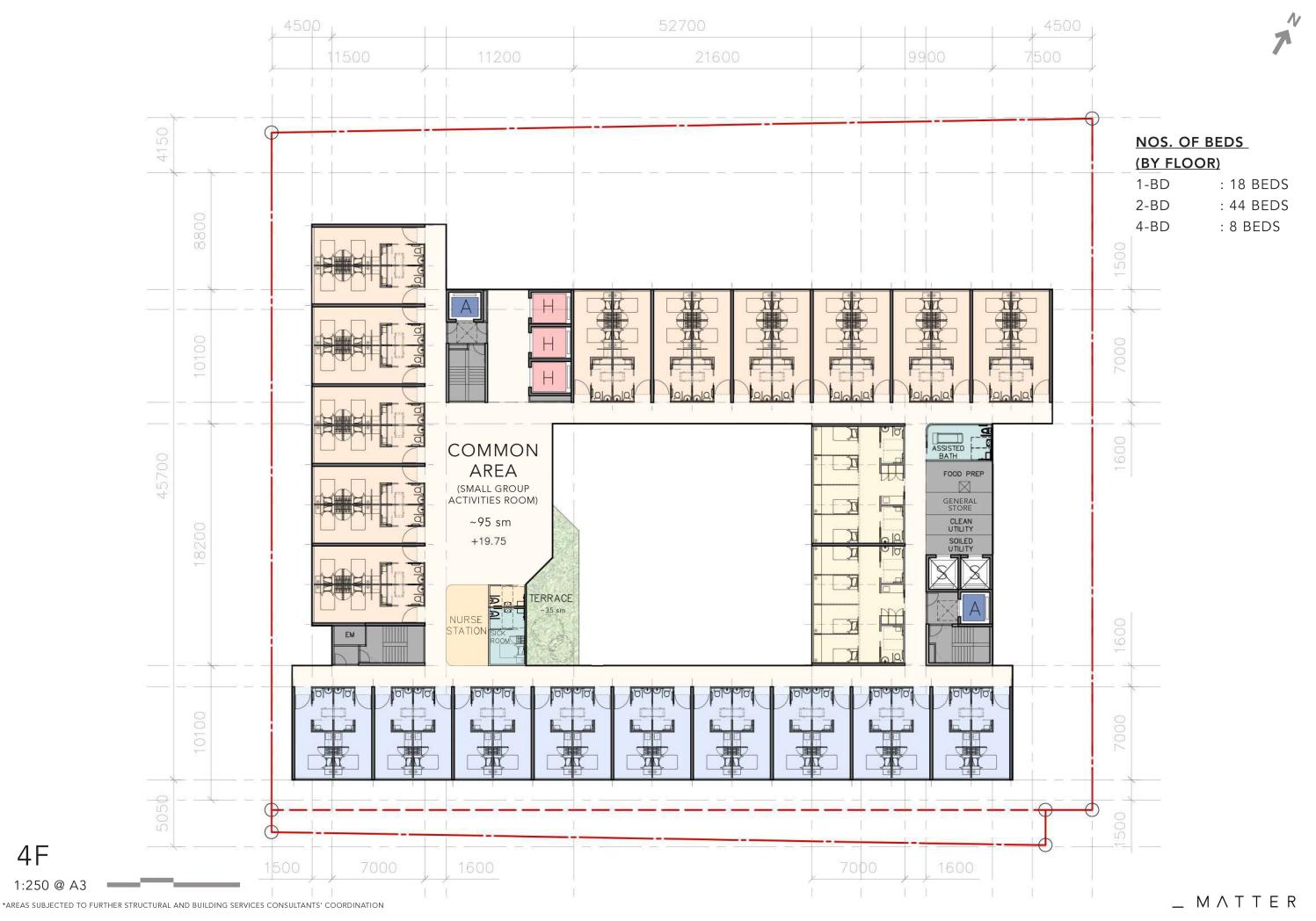
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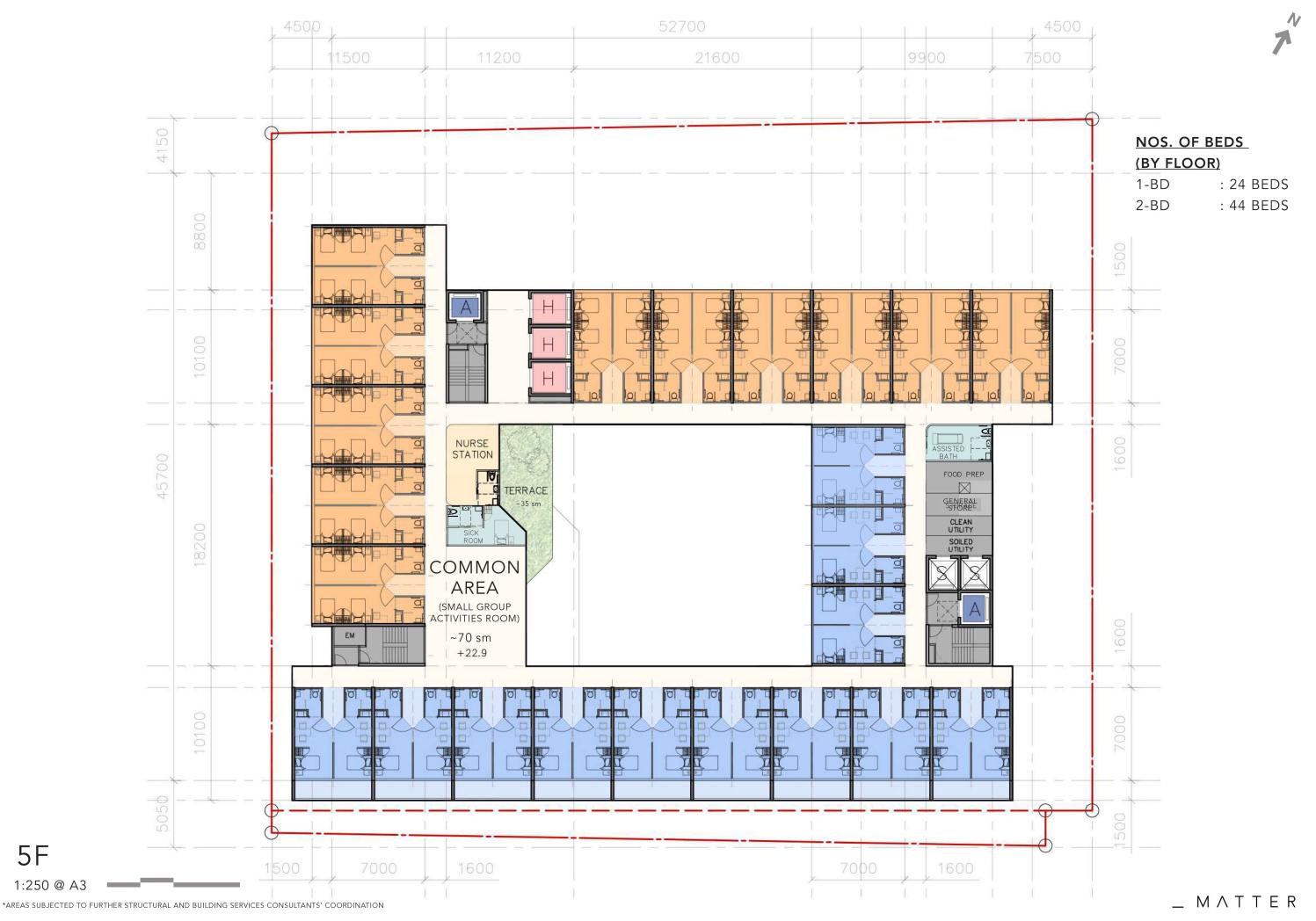






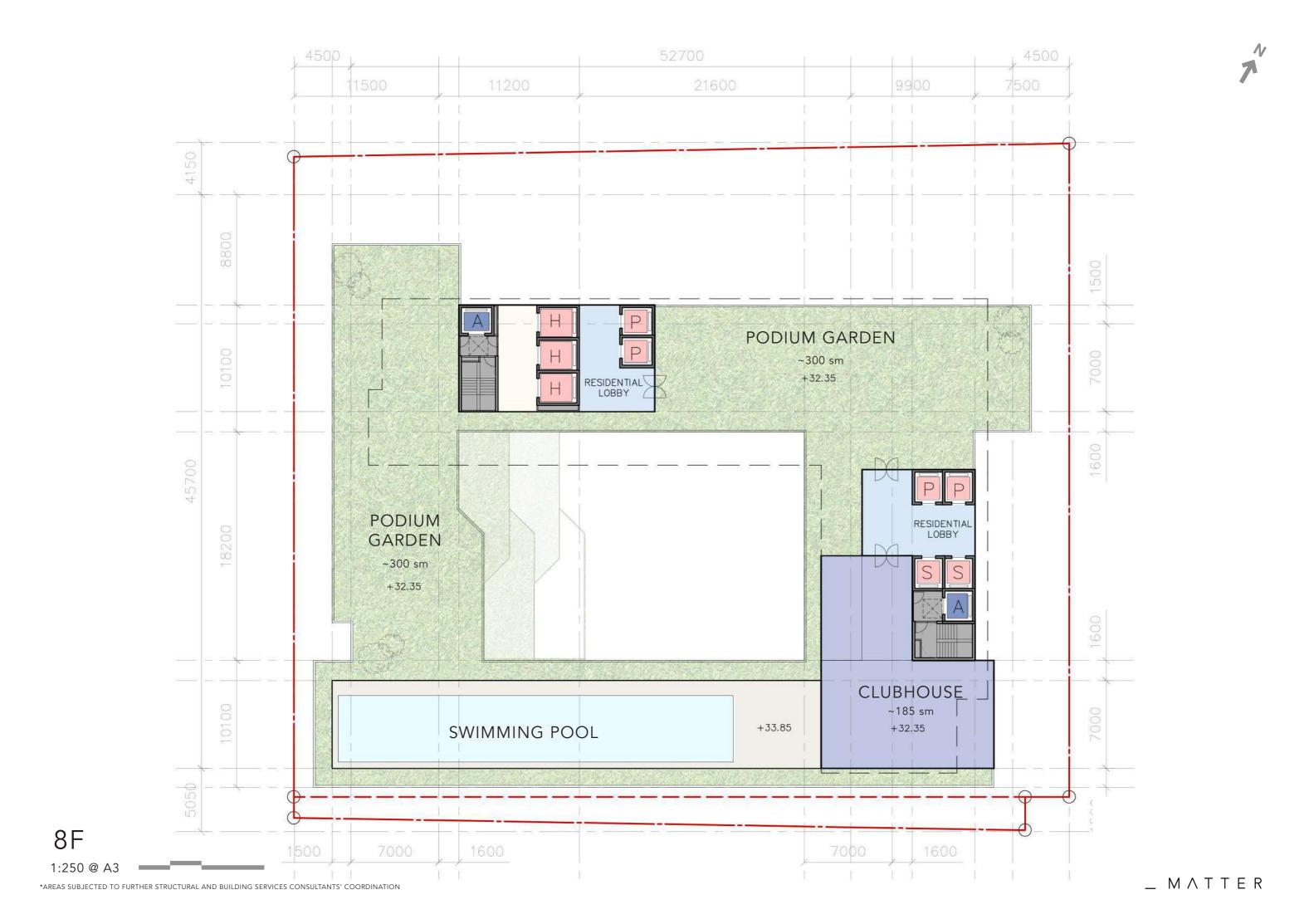






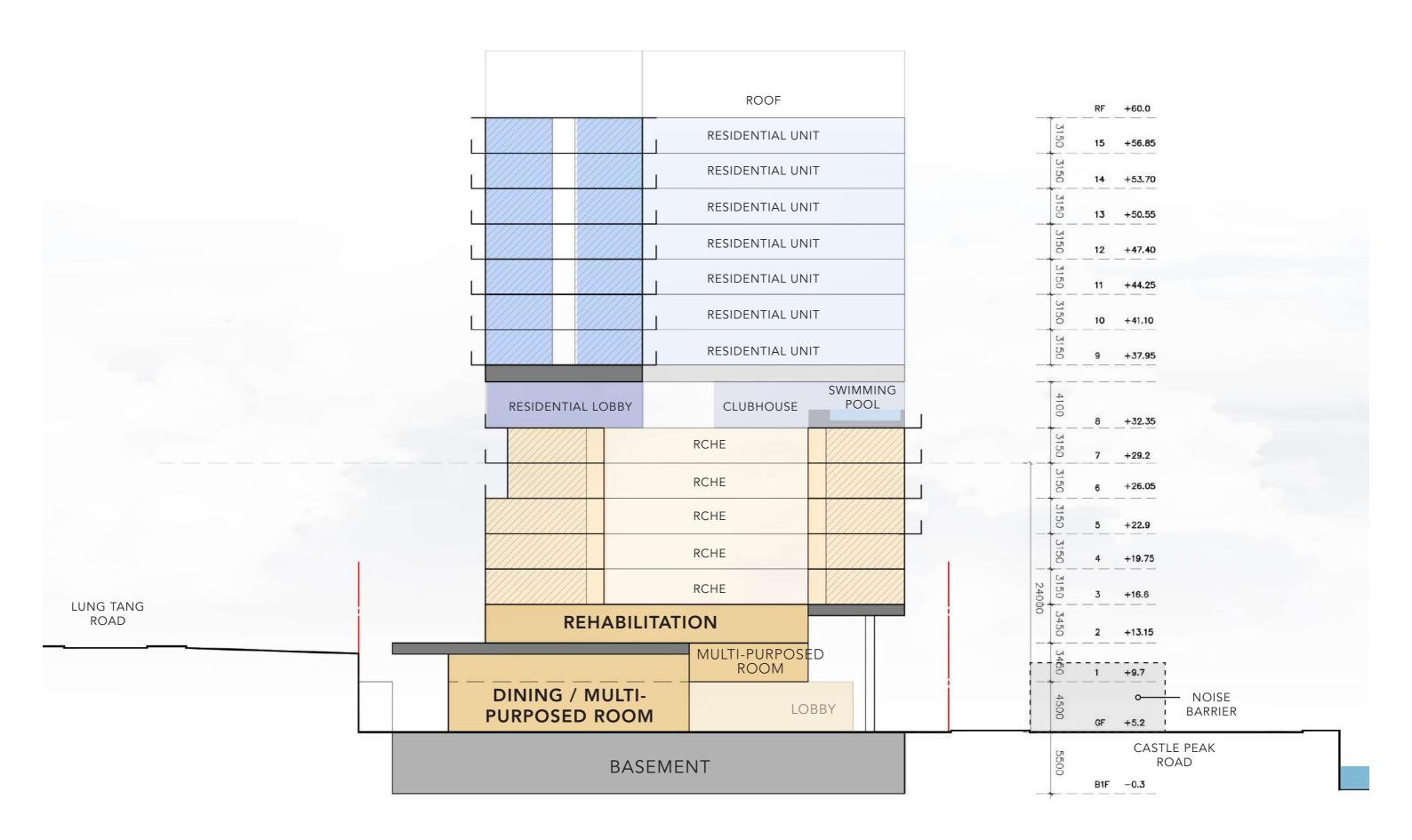


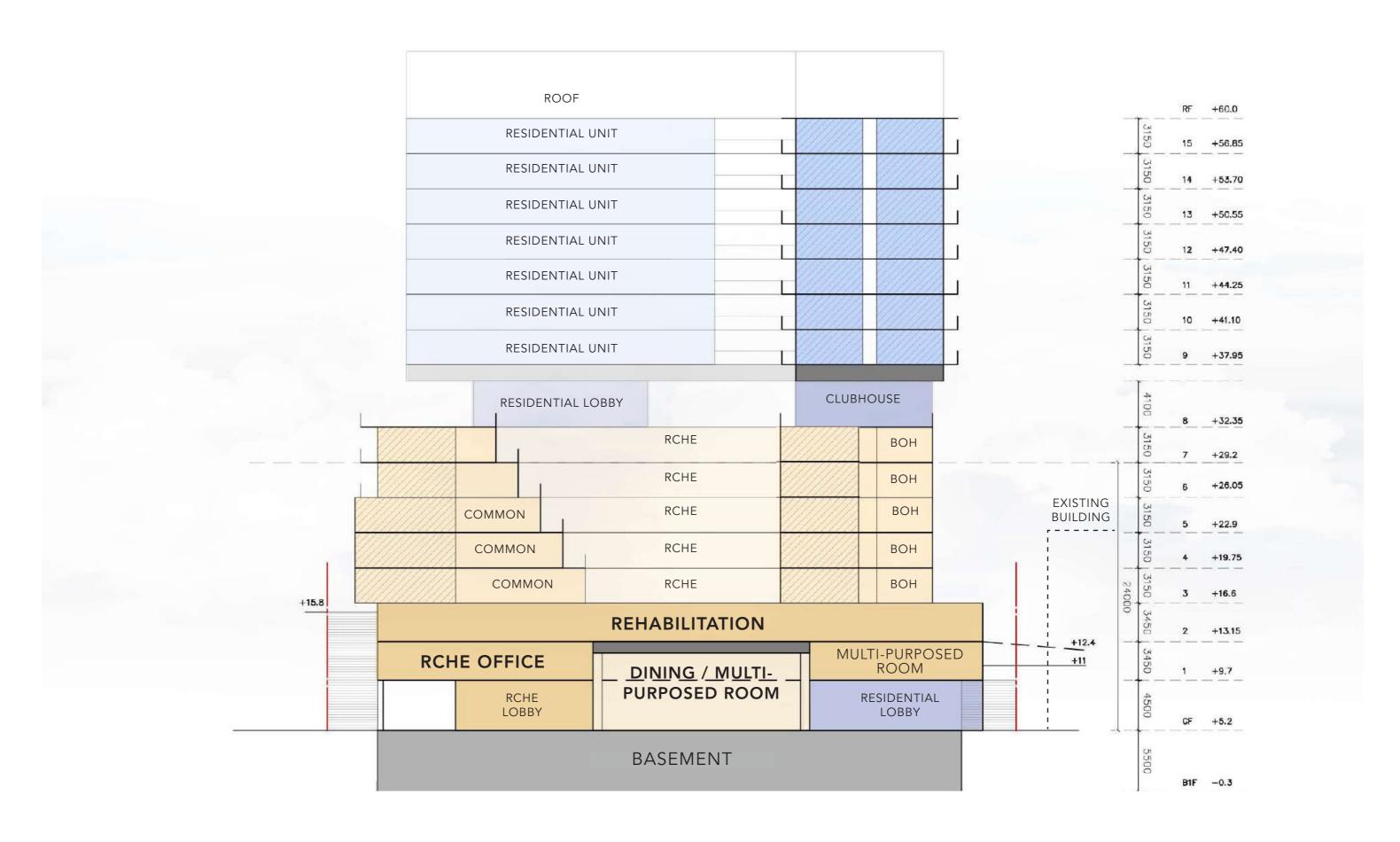














# Table 1 Calculation for Sewage Generation Rate of the Proposed Development at the Subject Site

| Residential Tower                             |   |   |
|---|---|---|
| Total number of residential units             | = | 112 units   |
| Total number of residents                     | = | 302 people (2021 Population Census: Average Household Size of 2.7 in Tsuen Wan DC)                  |
| Design flow                                   | = | 0.27 m <sup>3</sup> /person/day (Private R2 in Table T-1 of GESF)                                   |
| Sewage Generation rate                        | = | 81.6 m <sup>3</sup> /day  |
| Clubhouse                                     |   |   |
| Assumed Area                                  | = | $347 \text{ m}^2$   |
| Assumed Area Assumed floor area per employee  | = | 30.3 m <sup>2</sup> per worker (refer to Table 8 of CIFSUS - Community, Social & Personal Services) |
| Total number of employees                     | = | 11 employees  |
| Design flow for commercial activities         | = | 0.28 m³/employee/day (refer to Table T-2 of GESF - J11)   |
| Sewage Generation rate                        | = | 3.2 m <sup>3</sup> /day   |
| RCHE  |   |   |
| Resident                                      |   |   |
| Total number of places                        | = | 320 beds  |
| Design flow                                   | = | 0.19 m <sup>3</sup> /person/day (Institutional and Special Class in Table T-1 of GESF)              |
| Sewage Generation rate (Residence)            | = | <b>60.8</b> m <sup>3</sup> /day   |
| <u>Staff</u>                                  |   |   |
| Total number of employees                     | = | 33 employees (refer to Code of Practice for Residential Care Homes (Elderly Persons))               |
| Design flow for commercial activities         | = | 0.28 m <sup>3</sup> /employee/day (refer to Table T-2 of GESF - J11)                                |
| Sewage Generation rate                        | = | <b>9.2</b> $m^3/day$  |
| Total   | = | <b>70.0</b> $\text{m}^3/\text{day}$   |
| Training Centre                               |   |   |
| Assumed Area                                  | = | $200 \text{ m}^2$   |
| Assumed floor area per employee               | = | 30.3 m <sup>2</sup> per worker (refer to Table 8 of CIFSUS - Community, Social & Personal Services) |
| Total number of employees                     | = | 7 employees   |
| Design flow for commercial activities         | = | 0.28 m <sup>3</sup> /employee/day (refer to Table T-2 of GESF - J11)                                |
| Sewage Generation rate                        | = | <b>2.0</b> $\text{m}^3/\text{day}$  |
| Residential Institution                       |   |   |
| Assumed Area                                  | = | $50 \text{ m}^2$  |
| Assumed floor area per employee               | = | 30.3 m <sup>2</sup> per worker (refer to Table 8 of CIFSUS - Community, Social & Personal Services) |
| Total number of employees                     | = | 2 employees   |
| Design flow for commercial activities         | = | 0.28 m <sup>3</sup> /employee/day (refer to Table T-2 of GESF - J11)                                |
| Sewage Generation rate                        | = | $0.6  \mathrm{m}^{3}/\mathrm{day}$  |
| Rehab Facilities                              |   |   |
| Assumed Area of Swimming Pool                 | = | $200 \text{ m}^2$   |
| Average Depth of Water                        | = | 1.2 m (ordinary assumption)   |
| Volume of Swimming Pool (Ordinary Assumption) | = | $240 \text{ m}^3$   |
| Turnover Rate                                 | = | 4 hr  |
| Required Surface Loading Rate of Filter       | = | $60 \text{ m}^3/\text{m}^2/\text{hr}$   |
| Filter Areas required                         | = | $1.0 \text{ m}^2$   |
| Adopted Surface Loading Rate of Filter        | = | $50 \text{ m}^3/\text{m}^2/\text{hr}$   |
| Adopted Filter Area                           | = | $1.2 \text{ m}^2$   |
| Backwash Duration                             | = | $3 \min/d$  |
| Backwash flow rate                            | = | $30 \text{ m}^3/\text{m}^2/\text{hr}$   |
| Design flow for Swimming Pool Backwashing     | = | $1.8 \text{ m}^3/\text{day}$  |
| Design flow for Swimming Pool Backwashing     | = | 10.0 litre/sec  |
| Swimming Pools (Outdoor)                      |   |   |
| Assumed Area of Swimming Pool                 | = | 167 m <sup>2</sup>  |
| Average Depth of Water                        | = | 1.2 m <sub>2</sub>  |
| Volume of Swimming Pool (Ordinary Assumption) | = | $200 \text{ m}^3$   |
| Turnover Rate                                 | = | 6 hr  |
| Required Surface Loading Rate of Filter       | = | $50 \text{ m}^3/\text{m}^2/\text{hr}$   |
| Filter Area required                          | = | $0.7 \text{ m}^2$   |
| Backwash Duration                             | = | 3 min/d   |
| Backwash flow rate                            | = | $30 \text{ m}^3/\text{m}^2/\text{hr}$   |
| Design flow for Swimming Pool Backwashing     | = | $1.0 \text{ m}^3/\text{day}$  |
| Design flow for Swimming Pool Backwashing     | = | 5.6 litre/sec   |
| Total Flow from Proposed Development          |   | 242.2 3/4   |
| Flow Rate (without Catchmnet Inflow Factor)   | = | $218.2 \text{ m}^3/\text{day}$  |
| Catchment Inflow Factor                       | = | 1.10 Catchment Inflow Factor for Tsing Yi in Table T-4 of GEFS                                      |
| Flow Rate (with Catchment Inflow Factor)      | = | <b>240.0</b> m <sup>3</sup> /day  |
| Contributing Population                       | = | 889 people  |
| Peaking factor                                | = | 8 Refer to Table T-5 of GESF for population <1,000 incl. stormwater allowance                       |

= **37.8** litre/sec

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Peak Flow

Table 2a Hydraulic Capacity of Existing Sewers at Castle Peak Road – Tsing Lung Tau, Tsuen Wan

| Cogmont | Manhole    | Manhole    | Pipe Dia. | Pipe Length | Invert Level 1 | Invert Level 2 | g       | $\mathbf{k}_{\mathrm{s}}$ | S     | v                         | V    | Area           | Q       | Estimated Capacity |
|---------|------------|------------|-----------|-------------|----------------|----------------|---------|---------------------------|-------|---------------------------|------|----------------|---------|--------------------|
| Segment | Reference  | Reference  | mm        | m           | mPD            | mPD            | $m/s^2$ | m                         |       | $\mathbf{m}^2/\mathbf{s}$ | m/s  | m <sup>2</sup> | $m^3/s$ | L/s                |
| S1-S2   | FMH4052284 | FMH4052285 | 500       | 13.8        | 4.48           | 4.42           | 9.81    | 0.00060                   | 0.004 | 0.000001                  | 1.43 | 0.20           | 0.28    | 280                |
| S2-S3   | FMH4052285 | FMH4052286 | 500       | 19.2        | 4.41           | 4.35           | 9.81    | 0.00060                   | 0.003 | 0.000001                  | 1.21 | 0.20           | 0.24    | 237                |
| S3-S4   | FMH4052286 | FMH4052287 | 500       | 30.7        | 4.34           | 4.28           | 9.81    | 0.00060                   | 0.002 | 0.000001                  | 0.95 | 0.20           | 0.19    | 187                |
| S4-S5   | FMH4052287 | FSH4001700 | 500       | 8.5         | 4.27           | 4.20           | 9.81    | 0.00060                   | 0.008 | 0.000001                  | 1.97 | 0.20           | 0.39    | 386                |
| S5-S6   | FSH4001700 | FSH4001701 | 2 x 350   | 8.0         | 4.19           | 4.15           | 9.81    | 0.00060                   | 0.005 | 0.000001                  | 1.22 | 0.19           | 0.24    | 236                |
| S6-S7   | FSH4001701 | FMH4052290 | 500       | 35.7        | 4.14           | 3.96           | 9.81    | 0.00060                   | 0.005 | 0.000001                  | 1.54 | 0.20           | 0.30    | 302                |
| S7-S8   | FMH4052290 | FMH4052291 | 500       | 38.1        | 3.86           | 3.44           | 9.81    | 0.00060                   | 0.011 | 0.000001                  | 2.28 | 0.20           | 0.45    | 448                |
| S8-S9   | FMH4052291 | FMH4052292 | 500       | 46.0        | 3.43           | 3.16           | 9.81    | 0.00060                   | 0.006 | 0.000001                  | 1.66 | 0.20           | 0.33    | 326                |
| S9-S10  | FMH4052292 | FMH4052293 | 500       | 22.9        | 2.85           | 2.56           | 9.81    | 0.00060                   | 0.013 | 0.000001                  | 2.45 | 0.20           | 0.48    | 480                |

Table 2b Hydraulic Capacity of Existing Sewers at Castle Peak Road – Tsing Lung Tau, Tsuen Wan (After Upgrading Segment S3-S5)

| Sogmont | Manhole    | Manhole    | Pipe Dia. | Pipe Length | Invert Level 1 | Invert Level 2 | g                | $\mathbf{k}_{\mathrm{s}}$ | S     | V        | V    | Area           | Q       | Estimated Capacity |
|---------|------------|------------|-----------|-------------|----------------|----------------|------------------|---------------------------|-------|----------|------|----------------|---------|--------------------|
| Segment | Reference  | Reference  | mm        | m           | mPD            | mPD            | m/s <sup>2</sup> | m                         |       | $m^2/s$  | m/s  | m <sup>2</sup> | $m^3/s$ | L/s                |
| S1-S2   | FMH4052284 | FMH4052285 | 500       | 13.8        | 4.48           | 4.42           | 9.81             | 0.00060                   | 0.004 | 0.000001 | 1.43 | 0.20           | 0.28    | 280                |
| S2-S3   | FMH4052285 | FMH4052286 | 500       | 19.2        | 4.41           | 4.35           | 9.81             | 0.00060                   | 0.003 | 0.000001 | 1.21 | 0.20           | 0.24    | 237                |
| S3-S4   | FMH4052286 | FMH4052287 | 600       | 30.7        | 4.34           | 4.28           | 9.81             | 0.00030                   | 0.002 | 0.000001 | 1.15 | 0.28           | 0.33    | 325                |
| S4-S5   | FMH4052287 | FSH4001700 | 600       | 8.5         | 4.27           | 4.20           | 9.81             | 0.00030                   | 0.008 | 0.000001 | 2.38 | 0.28           | 0.67    | 673                |
| S5-S6   | FSH4001700 | FSH4001701 | 2 x 350   | 8.0         | 4.19           | 4.15           | 9.81             | 0.00060                   | 0.005 | 0.000001 | 1.22 | 0.19           | 0.24    | 236                |
| S6-S7   | FSH4001701 | FMH4052290 | 500       | 35.7        | 4.14           | 3.96           | 9.81             | 0.00060                   | 0.005 | 0.000001 | 1.53 | 0.20           | 0.30    | 301                |
| S7-S8   | FMH4052290 | FMH4052291 | 500       | 38.1        | 3.86           | 3.44           | 9.81             | 0.00060                   | 0.011 | 0.000001 | 2.28 | 0.20           | 0.45    | 448                |
| S8-S9   | FMH4052291 | FMH4052292 | 500       | 46.0        | 3.43           | 3.16           | 9.81             | 0.00060                   | 0.006 | 0.000001 | 1.66 | 0.20           | 0.33    | 326                |
| S9-S10  | FMH4052292 | FMH4052293 | 500       | 22.9        | 2.85           | 2.56           | 9.81             | 0.00060                   | 0.013 | 0.000001 | 2.45 | 0.20           | 0.48    | 480                |

Table 2c Hydraulic Capacity of Proposed Sewers from the Terminal Manhole of the Proposed Development for Sewage generated from the Proposed Development

| Sogmont | Manhole   | Manhole    | Pipe Dia. | Pipe Length | Invert Level 1 | Invert Level 2 | g       | $\mathbf{k}_{\mathrm{s}}$ | S      | v        | V    | Area           | Q       | Estimated Capacity |
|---------|-----------|------------|-----------|-------------|----------------|----------------|---------|---------------------------|--------|----------|------|----------------|---------|--------------------|
| Segment | Reference | Reference  | mm        | m           | mPD            | mPD            | $m/s^2$ | m                         |        | $m^2/s$  | m/s  | m <sup>2</sup> | $m^3/s$ | L/s                |
| T0-T1   | -         | -          | 300       | 5.9         | 4.69           | 4.68           | 9.81    | 0.00150                   | 0.0028 | 0.000001 | 0.73 | 0.07           | 0.05    | 52                 |
| T1-T2   | -         | -          | 300       | 31.4        | 4.68           | 4.59           | 9.81    | 0.00150                   | 0.0028 | 0.000001 | 0.73 | 0.07           | 0.05    | 52                 |
| T2-S1   | -         | FMH4052284 | 300       | 39.2        | 4.59           | 4.48           | 9.81    | 0.00150                   | 0.0028 | 0.000001 | 0.73 | 0.07           | 0.05    | 52                 |

Remarks: (1) g=gravitational acceleration; k<sub>s</sub>=equivalent sand roughness; s=gradient; v=kinematic viscosity of water; V=mean velocity

- (2) Table 2a: The value of k<sub>s</sub> = 0.6mm is used for the calculation of slimed clayware sewer, poor condition (based on Table 5: Recommended roughness values in Sewerage Manual)
- (3) Table 2c: The value of k<sub>s</sub> = 0.3mm is used for the calculation of slimed polyethelyene for the proposed sewers, poor condition (based on Table 5: Recommended roughness values in Sewerage Manual)
- (4) The value of velocity (V) is referred to the Tables for the hydraulic design of pipes, sewers and channels (8th edition)

(5) Equation used: 
$$V = -\sqrt{(8gDs)}\log(\frac{k_s}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}})$$

## Table 3 Calculation for Sewage Generation Rate of the Existing Surrounding Building

# Catchment A (S1)

Hong Kong Garden Total number of units

7641 people -- (2021 Population Census: Average Household Size of 2.7 in Tsuen Wan DC) 0.27 m³/person/day -- (Private R2 in Table T-1 of GESF) Total number of residents

Design flow

Sewage Generation rate **2063.1** m<sup>3</sup>/day

<u>Retail</u>

 $2702 \text{ m}^2$ Assumed Area

Assumed floor area per employee 28.6 m<sup>2</sup> per worker -- (refer to Table 8 of CIFSUS - Retail Trade) Total number of employees 94 employees

Design flow for commercial activities 0.28 m<sup>3</sup>/employee/day -- (refer to Table T-2 of GESF - J4)

Sewage Generation rate **26.5** m<sup>3</sup>/day

<u>F&B</u>

Assumed Area  $1158 \text{ m}^2$ 

19.6 m<sup>2</sup> per worker -- (refer to Table 8 of CIFSUS - Restaurants) Assumed floor area per employee Total number of employees 59 employees Design flow for commercial activities 1.58 m<sup>3</sup>/employee/day -- (refer to Table T-2 of GESF - J10)

Sewage Generation rate **93.3** m<sup>3</sup>/day

**Swimming Pools (Outdoor)** 

Assumed Area of Swimming Pool  $338 \text{ m}^2$ Average Depth of Water 1.25 m Volume of Swimming Pool (Ordinary Assumption)  $423 \text{ m}^3$ Turnover Rate 6 hr  $50 \text{ m}^3/\text{m}^2/\text{hr}$ Required Surface Loading Rate of Filter Filter Area required  $1.4 \text{ m}^2$ 

**Backwash Duration** 3 min/d Backwash flow rate  $30 \text{ m}^3/\text{m}^2/\text{hr}$ Design flow for Swimming Pool Backwashing  $2.1 \text{ m}^3/\text{day}$ Design flow for Swimming Pool Backwashing **11.7** litre/sec

Overall Catchment A

= 2182.9 m<sup>3</sup>/day Estimated Flow Rate

Catchment Inflow Factor 1.1 Catchment Inflow Factor for Tsing Yi in Table T-4 of GEFS

Total Flow Rate = **2401.2** m<sup>3</sup>/day

Table 4a Comparision of the Hydraulic Capacity of Existing Sewers for Sewerage generated from the Proposed Development and Surrounding Catchment Areas

| Segment | Pipe Dia. | Pipe Length (m) | Gradient | Estimated<br>Capacity<br>(L/s) | ADWF<br>(m³/day) | Contributing<br>Population | Peaking<br>Factor |      | Peak Flow from the Proposed<br>Development and Catchment<br>Areas (L/s) | Contribution from the<br>Proposed Development and<br>the Surrounding Catchment<br>Areas (%) | Status |
|---------|-----------|-----------------|----------|--------------------------------|------------------|----------------------------|-------------------|------|---|---|--------|
| S1-S2   | 500       | 13.8            | 0.004    | 280                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 64.3%   | OK     |
| S2-S3   | 500       | 19.2            | 0.003    | 237                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 75.9%   | OK     |
| S3-S4   | 500       | 30.7            | 0.002    | 187                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 96.1%   | Not OK |
| S4-S5   | 500       | 8.5             | 0.008    | 386                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 46.7%   | OK     |
| S5-S6   | 2 x 350   | 8.0             | 0.005    | 236                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 76.5%   | OK     |
| S6-S7   | 500       | 35.7            | 0.005    | 302                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 59.6%   | OK     |
| S7-S8   | 500       | 38.1            | 0.011    | 448                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 40.2%   | OK     |
| S8-S9   | 500       | 46.0            | 0.006    | 326                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 55.2%   | OK     |
| S9-S10  | 500       | 22.9            | 0.013    | 480                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 37.5%   | OK     |

Table 4c Comparision of the Hydraulic Capacity of Existing Sewers for Sewerage generated from the Proposed Development and Surrounding Catchment Areas (After Upgrading Segment S3-S5)

| Segment | Pipe Dia. | Pipe Length (m) | Gradient | Estimated<br>Capacity<br>(L/s) | ADWF<br>(m³/day) | Contributing<br>Population | Peaking<br>Factor |      | Peak Flow from the Proposed<br>Development and Catchment<br>Areas (L/s) | Contribution from the<br>Proposed Development and<br>the Surrounding Catchment<br>Areas (%) | Status |
|---------|-----------|-----------------|----------|--------------------------------|------------------|----------------------------|-------------------|------|---|---|--------|
| S1-S2   | 500       | 13.8            | 0.004    | 280                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 64.3%   | OK     |
| S2-S3   | 500       | 19.2            | 0.003    | 237                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 75.9%   | OK     |
| S3-S4   | 600       | 30.7            | 0.002    | 325                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 55.4%   | OK     |
| S4-S5   | 600       | 8.5             | 0.008    | 673                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 26.8%   | OK     |
| S5-S6   | 2 x 350   | 8.0             | 0.005    | 236                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 76.5%   | OK     |
| S6-S7   | 500       | 35.7            | 0.005    | 301                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 59.8%   | OK     |
| S7-S8   | 500       | 38.1            | 0.011    | 448                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 40.2%   | OK     |
| S8-S9   | 500       | 46.0            | 0.006    | 326                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 55.2%   | OK     |
| S9-S10  | 500       | 22.9            | 0.013    | 480                            | 2641.2           | 9782                       | 5                 | 27.3 | 180.1   | 37.5%   | OK     |

Remarks: (1) The value of peaking factor = 5 is used for population 5,000-10,000 incl. stormwater allowance (refers to Table T-5 of GESF)

Table 4b Comparision of the Hydraulic Capacity of Proposed Sewers from the Terminal Manhole of the Proposed Development for Sewage generated from the Proposed Development

| Segment | Pipe Dia. | Pipe Length (m) | Gradient | Estimated<br>Capacity<br>(L/s) | ADWF<br>(m³/day) | Contributing<br>Population | H'actor | 0    | Peak Flow from the Proposed<br>Development and Catchment<br>Areas (L/s) | Contribution from the<br>Proposed Development and<br>the Surrounding Catchment<br>Areas (%) | Status |
|---------|-----------|-----------------|----------|--------------------------------|------------------|----------------------------|---------|------|---|---|--------|
| T0-T1   | 300       | 5.9             | 0.003    | 52                             | 240.0            | 889                        | 8       | 15.6 | 37.8  | 73.0%   | OK     |
| T1-T2   | 300       | 31.4            | 0.003    | 52                             | 240.0            | 889                        | 8       | 15.6 | 37.8  | 73.0%   | OK     |
| T2-S1   | 300       | 39.2            | 0.003    | 52                             | 240.0            | 889                        | 8       | 15.6 | 37.8  | 73.0%   | OK     |

Remarks: (1) The value of peaking factor = 8 is used for population <1,000 incl. stormwater allowance (refers to Table T-5 of GESF)

## Table 5 Existing & Future Flows to Tsing Lung Tau Sewage Pumping Station (TLTSPS)

**ADWF** 

1. Designed Capacity

Design Daily Flow = 240.0 1/s (DSD)

2. Existing Situation

Average Flow of 2020 =  $2581 \text{ m}^3/\text{day (DSD)}$ 

3. Future Situation (including sewerage generated by the Proposed Development)

Existing and Planned Development =  $2581 \text{ m}^3/\text{day}$ Proposed Development (ADWF) =  $240 \text{ m}^3/\text{day}$ 

Total =  $2821 \text{ m}^3/\text{day}$ 

Equivalent Contributing Population = 10448

Peaking Factor = 3.5 Population 10,000 - 25,000 including stormwater allowance

Peak Flow = 128.7 l/s

54% (of designed pump rate)

Appendix 3 Relevant Information from DSD



SIA Report

## **Lily Chow**

From: kkchoi@dsd.gov.hk

**Sent:** 17 November 2021 12:04 PM

To: Lily Chow

**Cc:** mcchung@dsd.gov.hk; kwliu@dsd.gov.hk; wfsin@dsd.gov.hk; hktung@dsd.gov.hk; Calvin Chiu; Austin Chan

**Subject:** Fw: [Internet] RE: [Internet] Request for Information of Tsing Lung Tau Sewage Pumping Station

**Attachments:** R8291\_v1.0 all 20211013.pdf

Some people who received this message don't often get email from kkchoi@dsd.gov.hk. <u>Learn why this is important</u>

## Dear Ms Chow

## We spoke.

The information required are listed below for your attention, please: -

- a) The average daily flow of Tsing Lung Tau Sewage Pumping Station in 2020 was 2,581 cubic meter
- b) The peak pumping capacity is around 240 l/s

I would like to draw your attention that the above information is for reference & used for the captioned project only. No any part of obtained information from DSD is allowed to disclose to others.

Should you have any queries, please feel free to contact me. Best Regards

K.K. CHOI AMI/TM DSD ST1/3

Tel:2491 3609/9313 2373 Fax: 2613 5709



----- Forwarded by KK CHOI/ST1/DSD/HKSARG on 17/11/2021 11:51 -----