Traffic Impact Assessment Final Report 17<sup>th</sup> September 2024

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# **CONTENTS**

<u>C</u> H	<u>CHAPTER</u>		
1.	INTRODUCTION Background Structure of Report	1	
2.	EXISTING SITUATION The Subject Site The Road Network Existing Traffic Flows Performance of the Surveyed Junctions Existing Road-Based Public Transport Services Existing Pedestrian Flow Operational Performance of the Surveyed Pedestrian Facilities	2	
3.	THE PROPOSED DEVELOPMENT Proposed Development Internal Transport Facilities Swept Path Analysis Car Lift Analysis Contingency Plan of Car Lift	7	
4.	TRAFFIC IMPACT Design Year Traffic Forecasting Committed and Planning Developments in the Vicinity Traffic Generated by the Proposed Development Construction Traffic Generated by the Proposed Development 2031 Traffic Flows 2031 Junction Capacity Analysis Pedestrian Forecasting Pedestrian Generated by the Proposed Development 2031 Levels of Service Analysis Impact on Existing Public Transport Services	10	
5.	SUMMARY	16	
	Annex A – Junction Capacity Analysis Annex B – Swept Path Analysis Annex C – Car Lift Analysis Annex D – Project Programme		

# **TABLES**

# NUMBER

2.1	AADT for ATC Core Station 6209
2.2	Existing Junction Performance
2.3	Existing Road-Based Public Transport Services near the Subject Site
2.4	Existing Operational Performance for Nearby Pedestrian Facilities
3.1	Details of Development Parameters
3.1	Provision of Internal Transport Facilities for the Proposed Development
4.1	TPEDM Data for Tsuen Wan District
4.2	Population Projections by C&SD
4.3	Annual Traffic Growth of Traffic Flows for Castle Peak Road – New Ting Kau
4.4	Details of Known Major Committed and Planned Developments Identified
4.5	Traffic Generations of the Known Major Committed and Planned Developments Identified
4.6	Adopted Trip Rates and Traffic Generation of the Proposed Development
4.7	Construction Traffic Generation of the Proposed Development
4.8	2031 Junction Performance
4.9	Results of Pedestrian Generation Surveys
4.10	Adopted Pedestrian Generation
4.11	Operational Performance for Nearby Pedestrian Facilities

## **FIGURES**

#### **NUMBER**

- 1.1 Location of the Subject Site and Area of Influence
- 2.1 Locations of Surveyed Junctions
- 2.2 Existing Layout of Surveyed Junctions Castle Peak Road Ting Kau / Castle Peak Road New Ting Kau (J01), and Castle Peak Road Ting Kau / Ting Yat Road (J02)
- 2.3 Locations of Surveyed Pedestrian Facilities
- 2.3 Adjusted Existing Peak Hour Traffic Flows
- 3.1 Proposed Internal Transport Layout
- 3.2 Schematic Section of the Proposed Development
- 4.1 2031 Traffic Flows without the Proposed Development
- 4.2 2031 Traffic Flows with the Proposed Development
- 4.3 2031 Traffic Flows with Construction Traffic for the Proposed Development

## 1.0 INTRODUCTION

## **Background**

- 1.1 The Subject Site is located at Lot 453 in DD 399, Ting Kau, the New Territories. At present, it is occupied by a 2-storey single-family house which is known as *La Casetta* (the "Existing House"). **Figure 1.1** shows the location of the Subject Site.
- 1.2 The Owner intends to reconstruct the Existing House into a new 3-storey single-family house with a carport (hereinafter "the Proposed Development").
- 1.3 Against this background, CKM Asia Limited, a traffic and transportation planning consultancy firm, was commissioned to conduct this Traffic Impact Assessment ("TIA") in support of the S16 planning application for the Proposed Development. This report presents the findings and recommendations of the TIA for the Proposed Development.

## Structure of Report

1.4 The report is structured as follows:

Chapter One - Gives the background of the project; Chapter Two - Describes the existing situation;

Chapter Three - Provides details on the Proposed Development and the

access arrangement;

Chapter Four - Describes the traffic impact analysis; and

Chapter Five - Gives the overall conclusion.

### 2.0 EXISTING SITUATION

## The Subject Site

- 2.1 The Subject Site is located on the hillside of Ting Kau at the eastern end of Ting Yat Road, and to the immediate north of Castle Peak Road New Ting Kau.
- 2.2 At present, the Subject Site is occupied by a 2-storey single-family house built at base level of +42.4mPD, which is some 13m higher than the street level of Ting Yat Street at +29.6mPD. The Existing House is only accessible on foot via a series of staircase through a narrow street frontage of approximately 4.5m at the cul-de-sac of Ting Yat Road
- 2.3 In addition, the Existing House is located behind and above a retaining structure along its frontage at Castle Peak Road New Ting Kau (Tsuen Wan bound), which is located at around +24.8mPD, i.e. some 17.6m below. Footbridge NF390 and its landing are also situated below and along the site frontage. Hence, vehicular access to/from the Subject Site is not available at present.
- 2.4 **Photo 2.1** below shows the existing site condition in relation to the abutting road network.

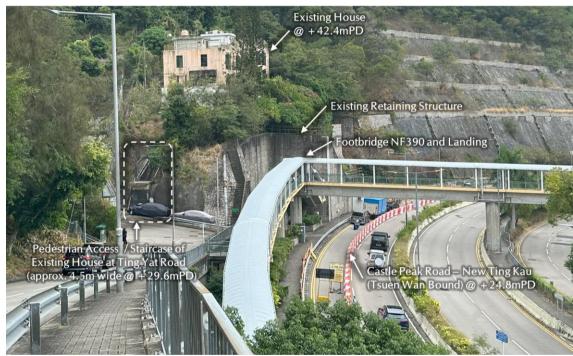


Photo 2.1 – Existing Condition of the Subject Site

#### The Road Network

- 2.5 Ting Yat Road is a local road of single 2 carriageway standard. It connects to Castle Peak Road Ting Kau to the west, and terminates as a cul-de-sac to the east abutting the Subject Site. It only serves several houses, including the Existing House, and the Private Water Works Road owned by the Water Supplies Department ("WSD").
- 2.6 The section of Castle Peak Road New Ting Kau fronting the Existing House is a Rural Road classified by Transport Department, and is of dual 2 carriageway 2-lane standard.

- 2.7 Whereas, the section of Castle Peak Road Ting Kau to the south of the Existing House is of a single carriageway 2 lane road serving the Ting Kau Village, which connects with Castle Peak Road New Ting Kau at both ends.
- 2.8 To the east and to the west, Castle Peak Road Ting Kau continues as of dual 2 carriageway 2-lane road extending towards Tsuen Wan and Sham Tseng respectively providing regional access.

## **Existing Traffic Flows**

- 2.9 To quantify the existing traffic flows in the vicinity, manual classified counts were conducted on Wednesday 17<sup>th</sup> January 2024, during the AM and PM peak periods at the following junctions:
  - J01 Castle Peak Road Ting Kau / Castle Peak Road New Ting Kau, and
  - J02 Castle Peak Road Ting Kau / Ting Yat Road / Open Car Park
- 2.10 **Figure 2.1** shows the locations of the surveyed junctions, and **Figure 2.2** shows their existing layouts.
- 2.11 The traffic counts were classified by vehicle types to enable traffic flows in passenger car units ("pcu") to be calculated. The AM and PM peak hours identified from the surveys are between 0730 and 0830 hours, and between 1730 and 1830 hours respectively.
- 2.12 Reference is made to the Annual Traffic Census ("ATC") published by Transport Department, and found that the closest ATC station is 6209 Castle Peak Road Tsuen Wan, Ting Kau & Sham Tseng. The annual average daily traffic ("AADT") information for this ATC station is found in **Table 2.1**.

TABLE 2.1 AADT FOR ATC CORE STATION 6209

Station	6209		
Road	Castle Peak Road - Tsuen Wan, Ting Kau &		
_	Sham Tseng		
From	Tuen Mun Road		
То	Sham Tseng		
2017	11,520		
2018	11,110		
2019	11,000		
2020	10,770		
2021	11,360		
2022	10,070		
Average Annual Growth (2017 to 2021)	-0.4%		

- 2.13 It should be noted that the AADT for 2022 is the lowest in since 2017, of which may have been impacted by the COVID-19 pandemic. Hence, the AADT for 2022 are shown above for <u>reference only</u>, and excluded in determining the average annual growth.
- 2.14 As shown **Table 2.1,** the historical AADT of Castle Peak Road Tsuen Wan, Ting Kau & Sham Tseng has an average annual growth rate of -0.4%, between 2017 and 2021.

- 2.15 With reference to the 2021 ATC, traffic flow in January is around 6% lower than the average month, and it is also around 10% lower than the highest monthly average being occurred in December.
- 2.16 Moreover, the daily traffic flow on a typical Wednesday is around 0.5% lower than the average daily traffic flow, and is around 5% lower than the highest weekday daily average traffic flow in 2021 occurred on a Friday.
- 2.17 To be conservative, a factor of 1.155 (i.e.  $(1+10\%) \times (1+5\%) = 1.155$ ) is applied to the surveyed flows, and **Figure 2.3** presents the adjusted existing peak hour traffic flows after considering the seasonal adjustments.

#### Performance of the Surveyed Junctions

2.18 The performance of the surveyed junctions is calculated based on the existing traffic flows presented in **Figure 2.3**, and the analyses were undertaken using the methods outlined in Volume 2 of Transport Planning and Design Manual ("TPDM") published by Transport Department. **Table 2.2** presents the analysis results and the detailed calculations are found in **Annex A**.

TABLE 2.2 EXISTING JUNCTION PERFORMANCE

Ref.	Junction	Type of Junction	Parameter	AM Peak Hour	PM Peak Hour
J01	Castle Peak Road – Ting Kau / Castle	Roundabout	RFC	0.197	0.264
	Peak Road – New Ting Kau				
J02	Castle Peak Road – Ting Kau / Ting Yat	Priority	RFC	0.03	0.04
	Road / Open Car Park				

Note: RFC – Ratio of Flow to Capacity

2.19 The above results indicate the surveyed junctions operate with capacities.

## **Existing Road-Based Public Transport Services**

The Existing House is accessible to road-based public transport services with the nearest bus stops located less than 100m away. **Table 2.3** summarises details of the available services, and **Figure 2.4** shows their locations.

TABLE 2.3 EXISTING ROAD-BASED PUBLIC TRANSPORT SERVICES NEAR THE SUBJECT SITE

Route	Routings	Frequency	Remark
KMB 48P	Fo Tan Chun Estate → Tsing Lun Tau	2 per AM, and 2 per PM	(1)
	Tsing Lun Tau → Fo Tan Chun Estate	3 per AM and 2 per PM	(1)
KMB 53	Tsuen Wan (Nina Tower) ↔ Yuen Long (YOHO Mall)	25 - 35	
KMB 234B	Tsuen Wan West Station ↔ Sham Tseng (Sea Crest Villa)	12 - 30	
KMB 234C	Sham Tseng → Kwun Tong (Tsui Ping North Estate)	4 per AM	(1)
		2 per AM	(2)
	Kwun Tong (Tsui Ping North Estate) → Sham Tseng	2 per PM	(1)
KMB 234D	Tsing Lun Tau → Kwun Tong (Tsui Ping North Estate)	3 per AM	(1)
	Kwun Tong (Tsui Ping North Estate) → Tsing Lun Tau	2 per PM	(1)
KMB N252	Mei Foo → Tuen Mun (Sam Shing Estate)	2 per night	(3)

Note: KMB – Kowloon Motor Bus

GMB – Green Minibus

RMB – Red Minibus

- Service information as of 22<sup>nd</sup> March, 2024.
- Limited services during specific hours only.
- (1) Monday to Friday only, except public holidays.
- (2) Saturday only, except public holidays.
- (3) Overnight service.

TABLE 2.3 EXISTING ROAD-BASED PUBLIC TRANSPORT SERVICES NEAR THE SUBJECT SITE (CONTINUED)

Route	Routings	Frequency	Remark
LWB A38	Tsuen Wan (Allway Gardens) ↔ Airport	30 - 60	
GMB 96	Tsing Lung Tau ↔ Tsuen Wan (Hoi Pa Street)	6 - 25	
GMB 96C	Bellagio ↔ Tsuen Wan Staiton	15 - 30	
GMB 96M	Tsing Lung Tau ↔ Tsuen Wan Staiton	10 - 25	
RMB AN3	So Kwun Wat → Tai Wai Station	1 per AM, and	(1)
		1 per PM	
	Tai Wai Station → So Kwun Wat	1 per PM	(1)
RMB	Yuen Long / Tuen Mun ↔ Jordan Road (Parkes Street)	n/a	
RMB	Mong Kok (Reclamation Street) → Yuen Long / Tuen Mun	n/a	

Note: KMB – Kowloon Motor Bus

GMB - Green Minibus

RMB - Red Minibus

- Service information as of 22<sup>nd</sup> March, 2024.
- Limited services during specific hours only.
- (1) Monday to Friday only, except public holidays.
- (2) Saturday only, except public holidays.
- (3) Overnight service.

## **Existing Pedestrian Flow**

- 2.21 Pedestrian counts were conducted at following nearby pedestrian facilities, i.e. footpath and footbridges:
  - FP01 Ting Yat Road
  - FP02 Castle Peak Road New Ting Kau (Northern Footpath)
  - FP03 Castle Peak Road Ting Kau (Southern Footpath)
  - FP04 Footbridge NF390 across Castle Peak Road New Ting Kau / Ting Kau
  - FP05 Ramp between FP02 and FP04
  - FP06 Staircase between FP02 and FP04
  - FP07 Ramp between FP03 and FP04
  - FP08 Staircase between FP04 and FP04
- 2.22 **Figure 2.5** shows locations of the above pedestrian facilities.

#### Operational Performance of the Surveyed Pedestrian Facilities

2.23 Levels of Service ("LOS") analysis for the AM and PM peak hour was conducted. To be conservative, a 'dead area' of 0.5m is assumed on either side when calculating the "Effective Width". The LOS grading is as per TPDM Vol 6, Section 10.4, and the analysis results are summarised in **Table 2.4.** 

TABLE 2.4 EXISTING OPERATIONAL PERFORMANCE FOR NEARBY PEDESTRIAN FACILITIES

Footpath	Measured Width (m)	Effective Width (m)	2-Way Pedestrian Flow (ped/hour)		Flow Rate [LOS] (ped/m/min)	
			AM Peak	PM Peak	AM Peak	PM Peak
FP01	1.5m	0.5m	5	5	0.3 [A]	0.3 [A]
FP02	2.8m	1.8m	15	15	0.2 [A]	0.2 [A]
FP03	1.5m	1.0m	15	15	0.3 [A]	0.3 [A]
FP04	3.0m	2.0m	15	15	0.2 [A]	0.2 [A]
FP05	3.0m	2.0m	10	10	0.1 [A]	0.1 [A]
FP06	2.5m	1.5m	5	5	0.1 [A]	0.1 [A]
FP07	2.5m	1.5m	5	5	0.1 [A]	0.1 [A]
FP08	2.0m	1.0m	10	10	0.3 [A]	0.3 [A]

Note: Due to the small amount of pedestrian observed, 2-way pedestrian flow is rounded-up to nearest 5.

FP01 - Ting Yat Road

FP02 - Castle Peak Road – New Ting Kau (Northern Footpath)
FP03 - Castle Peak Road – Ting Kau (Southern Footpath)

FP04 - Footbridge NF390 across Castle Peak Road - New Ting Kau / Ting Kau

FP05 - Ramp between FP02 and FP04 FP06 - Staircase between FP02 and FP04 FP07 - Ramp between FP03 and FP04 FP08 - Staircase between FP04 and FP04

2.24 The above results indicate the surveyed footpaths and footbridge operate with capacities.

## 3.0 THE PROPOSED DEVELOPMENT

## **Proposed Development**

3.1 The Proposed Development is a new single-family house, reconstructing from the Existing House. **Table 3.1** summarises the development parameters.

TABLE 3.1 DETAILS OF DEVELOPMENT PARAMETERS

Item	Parameters	
Site Area	Approximately 773m <sup>2</sup>	
Existing and Proposed Use	Domestic – Single-Family House	
Proposed Plot Ratio	0.748	
Proposed Gross Floor Area	Approximately 578 m <sup>2</sup>	
Planned Population	No more than 10 people	

## **Internal Transport Facilities**

- 3.2 Internal transport facilities for the Proposed Development are to be provided in accordance to the recommendation of the Hong Kong Planning Standards and Guidelines ("HKPSG").
- 3.3 **Table 3.2** compares the HKPSG recommended internal transport provision and the proposed provision.

TABLE 3.2 PROVISION OF INTERNAL TRANSPORT FACILITIES FOR THE PROPOSED DEVELOPMENT

Use	HKPSG Recommendation	Proposed Provision					
	Car Parking Space						
Residential [a]	Number of space = GPS $x$ R1 $x$ R2 $x$ R3, where:	4 nos., including					
	Global Parking Standard (GPS) = 1 space per 4 – 7 flats	- <b>3 nos.</b> @ 5.0m (L) x					
	R1 = 7 for flat size of $> 160$ m <sup>2</sup>	2.5m (W) x min. 2.4m					
	R2 = 1.0 for development outside 500m of rail station	(H)					
	R3 = 1.3 for domestic plot ratio (PR) $\leq$ 1.00	- <b>1 no.</b> @ 5.0m (L) x 3.5m					
	•	(W) x min. 2.4m (H)					
	Minimum						
	$\overline{(1 \div 7 \times 7 \times 1.0 \times 1.3)} = 1.3$ , say 2 nos.	= HKPSG Maximum, OK					
	Maximum						
	$\overline{(1 \div 4 \times 7} \times 1.0 \times 1.3) = 2.3$ , say <b>3 nos.</b>						
Visitor	For private residential developments with 75 units or less						
[b]	per block, the visitor car parking provision will be						
	determined by Transport Department on a case-by-case						
	basis.						
	1 space for 30 units or less, = 1 no.						
Total	Minimum = 2 + 1 = 3  nos.						
[a] + [b]	Maximum = $3 + 1 = 4$ nos.						
	Motorcycle Parking Space						
Residential	1 space per 100 – 150 flat	<b>1 no.</b> @ 2.4m (L) x 1.0m					
		(W) x min. 2.4m (H)					
	<u>Minimum</u>						
	$1 \div 150 = 0.01$ , say 1 no.	= HKPSG Maximum, OK					
	Maximum	·					
	$1 \div 100 = 0.01$ , say 1 no.						

TABLE 3.2 PROVISION OF INTERNAL TRANSPORT FACILITIES FOR THE PROPOSED DEVELOPMENT (CONTINUED)

Use	HKPSG Recommendation	Proposed Provision	
	Loading / Unloading Bay		
Residential	Minimum of 1 loading / unloading bay for goods vehicles within the site for every 800 flats or part thereof, subject to a minimum of 1 bay for each housing block.  Since the Proposed Development is a house, not a residential block, the HKPSG recommendation on loading / unloading bay is opined not applicable.		

- 3.4 The proposed internal transport layout is found in **Figures 3.1**, and a schematic section of the Proposed Development is shown in **Figure 3.2**.
- 3.5 It should be noted that to overcome the significant difference in vertical levels between the proposed carport at level +44mPD and the street level of Ting Tat Road at +29.6mPD, a car lift is proposed, which will lift the car between the 2 levels. In connection, an internal driveway will be constructed from Ting Yat Road by craving out the existing slope.
- 3.6 Goods vehicle loading / unloading will be conducted within the Proposed Development at the new internal driveway at the Ting Yat Road level, indicated in **Figure 3.2**, which is presently carried out on Ting Yat Road. Demand on loading / unloading for a single-family house is expected to be negligible, and the Owner will schedule the time of delivery, so that it will not interfere with ingress and egress of his/her private cars to and from the carport.
- 3.7 Feasibility on provision of a goods vehicle loading / unloading was reviewed, but was found impractical. This is due to severe constraints at the Subject Site, including narrow and small site area at Ting Yat Road level, and significant level difference between the carport level, and the vehicular access at Ting Yat Road. Hence, use of the new private internal driveway for occasional loading / unloading is found to be the most suitable solution to avoid loading / unloading activities to be carried out on public roads.
- 3.8 In addition, the proposed internal transport provision in the Proposed Development is considered a merit from traffic engineering viewpoint when comparing to the Existing House, which has no internal transport facilities and access is for pedestrians only. Moreover, the Proposed Development provides barrier-free-access from Ting Yat Road, which is currently absent from the Existing House.

#### **Swept Path Analysis**

3.9 Swept path analyses using CAD-based simulation program were conducted to ensure ease of vehicle manoeuvring within the Proposed Development. No manoeuvring issues are encountered and the swept path analysis drawings are found in **Annex B**.

#### Car Lift Analysis

- 3.10 Car lift analysis was conducted and found that the probability of more than 1 vehicle arriving at the same time when the car lift is occupied is 0.1%, which is acceptable from traffic engineering viewpoint. Detail calculation, which is based on the Queuing Theory, is found in **Annex C**.
- 3.11 In addition, the new internal ramp from Ting Yat Road to the car lift is some 13m long, and is capable to hold 2 private cars in queue at the same time. Hence, queuing of entering vehicles for the Proposed Development to be extended onto public roads is not expected.

## Contingency Plan of Car Lift

- 3.12 In case of mechanical breakdown, the Owner will immediately contact the maintenance contractor. Usually, the car lift maintenance team is committed to arrive as soon as possible.
- 3.13 The Owner will also immediately notify all other family members about the mechanical breakdown, and tell them to make their own alternative travel and parking arrangement due to the suspension of the car lift service.
- 3.14 In case if a car is trapped in the car lift, the Owner will also immediately contact the Fire Service Department via 999 emergency call for assistance.
- 3.15 To minimize the potential of mechanical breakdown, regular maintenance will be scheduled routinely, and the works will be performed during the off-peak period.

### 4.0 TRAFFIC IMPACT

## **Design Year**

4.1 The Proposed Development is expected to be completed in 2028, and the design year adopted in this TIA is 2031, i.e. 3 years after completion. For reference, a tentative project programme prepared by the Project Architect is attached in Annex D.

#### **Traffic Forecasting**

- 4.2 The 2031 traffic flows used for the junction analysis are produced with reference to the following:
  - (i) 2031 traffic flows derived with reference to 2019-based Base District Traffic Model ("BDTM") purchased from Transport Department;
  - (ii) Traffic generated by other identified planned and committed developments in the vicinity; and
  - (ii) Traffic generated by the Proposed Development.

## Alternative methods of traffic forecasting

- 4.3 Beside the method stated in paragraph 4.2, alternative traffic forecasting by growth factor method was also reviewed, including the following:
  - (a) historic AADT from the ATC, which is illustrated in Table 2.1
  - (b) "2019-based Territorial Population and Employment Data Matrix" ("TPEDM") published by Planning Department ("PlanD"), and
  - (c) "Hong Kong Population Projections for 2022 2046" published by Census and Statistics Department ("C&SD").
- 4.4 Information from the above sources (b) and (c) are summarised in **Tables 4.1 and 4.2** respectively.

TABLE 4.1 TPEDM DATA FOR TSUEN WAN DISTRICT

Item	Year 2019	Year 2026	Year 2031	An	nual Growth R	ate
				2019 - 2026	2026 - 2031	2019 - 2031
Population	293,700	279,450	249,400	-0.7%	-2.3%	-1.4%
Employment	165,000	166,100	160,650	0.1%	-0.7%	-0.2%
TOTAL	458,700	445,550	410,050	-0.4%	-1.7%	-0.9%

Source: "2019-based Territorial Population and Employment Data Matrix," Planning Department,
December 2021

TABLE 4.2 POPULATION PROJECTIONS BY C&SD

Year	Hong Kong Resident Population (in Thousands)
2024	7,526.8
2031	7,820.2
Average Annual Growth (2024 – 2031) =	+0.6%

Source: "Hong Kong Population Projections for 2022 – 2046," Census and Statistics Department, 15<sup>th</sup> August 2023.

4.5 **Table 4.1** shows that the overall population and employment is reduced by 0.4% to 1.7% per annum from 2019 to 2031. According to **Table 4.2**, the overall population of Hong Kong is expected to increase by +0.6% per annum from 2024 to 2031.

<sup>&</sup>lt;a href="https://www.pland.gov.hk/pland\_en/resources/info\_serv/statistic/tpedm19.html">https://www.pland.gov.hk/pland\_en/resources/info\_serv/statistic/tpedm19.html</a>

<sup>&</sup>lt;a href="https://gia.info.gov.hk/general/202308/15/P2023081500308">https://gia.info.gov.hk/general/202308/15/P2023081500308">https://gia.info.gov.hk/general/202308/15/P2023081500308</a> 428784 1 1692085684250.pdf>

4.6 Based on the 2031 traffic flow produced as stated in paragraph 4.2, the average AM and PM Peak hour annual growths are 6.1% and 2.7% (the "adopted annual growths") respectively, and the calculations are presented in **Table 4.3**.

TABLE 4.3 ANNUAL TRAFFIC GROWTH OF TRAFFIC FLOWS FOR CASTLE PEAK ROAD – NEW TING KAU

Case		ffic Flows for New Ting Kau (pcu/hr)
	AM Peak Hour	PM Peak Hour
Existing (2024) [a]	902	1,023
2031 without Proposed Development [b]	1,368	1,234
Average Annual Growth Rate (Note 1)	+6.1%	+2.7%

Note (1) Average Annual Growth =  $([b]/[a])^{1/(2031-2024)} - 1$ 

4.7 **Table 4.3** shows that the 2031 traffic flows derived with reference to the 2019-based BDTM resulted in higher than those in **Tables 4.1** and **4.2**, which is considered more conservative.

## Committed and Planned Developments in the Vicinity

4.8 Other known major committed and planned developments in vicinity of the Proposed Development have been identified and summarised in **Table 4.4**.

TABLE 4.4 DETAILS OF KNOWN MAJOR COMMITTED AND PLANNED DEVELOPMENTS IDENTIFIED

Ref.	Location	Use	Development Parameter
A.	Starter Home for Hong Kong Residents Project by	Subsidised	About 1,940 flats
	Hong Kong Housing Society in Yau Kam Tau	Sale Flats	
B.	Proposed Private Housing Development in Yau	Private	About 613 flats
	Kam Tau north of Tuen Mun Road	Housing	
C.	Proposed Private Housing Development in Yau	Private	About 490 flats
	Kam Tau near Po Fung Terrace	Housing	
D.	Lot No. 407 in DD. 399, Ting Kau, Tsuen Wan	Private	1 residential house with
	West	Housing	about 911m <sup>2</sup> GFA
E.	Lot 162RP (Part) in DD. 399 and Adjoining	Private	1 residential house with
	Government Land, Ting Kau, Tsuen Wan West	Housing	about 435m <sup>2</sup> GFA
F.	400 Castle Peak Road - Ting Kau	Private	1 residential house with
		Housing	about 487m <sup>2</sup> GFA
G.	Lots 99, 100, 101R.P., 110R.P., 171C and 183 in	Private	About 56 flats
	D.D. 390 and Adjoining Government	Housing	
	Land, Sham Tseng, Tsuen Wan		
Н.	Lot No. 408 in DD. 399 & the Extension Thereto,	Private	1 residential house with
	Ting Kau, Tsuen Wan, N.T.	Housing	about 450m² GFA
1.	House 117, Ting Kau Village, Tsuen Wan, New	Retail	About 113m <sup>2</sup> GFA
	Territories		

4.9 Traffic generations of the above identified major committed and planned developments in vicinity are summarized in **Table 4.5**, and this traffic generation are added to the trip ends of the 2031 BDTM, and then assigned onto the road network, including within the AOI.

TABLE 4.5 TRAFFIC GENERATIONS OF THE KNOWN MAJOR COMMITTED AND PLANNED DEVELOPMENTS IDENTIFIED

Ref.	Development Parameters	Adopted Trip Rates from TPDM (unit) AM Peak Hour PM Peak Hour			Traffic Genertation (pcu / hr) AM Peak Hour PM Peak Hour				
		Gen.	Att.	Gen.	Att.	Gen.	Att.	Gen.	Att.
Α.	1,940 Subsidised Sale Flats @ Average Flat Size = 50m <sup>2</sup>	0.0761		0.0350 nr/flat)	0.0451	148	112	68	68
В.	613 Flats Private Housing  @ Average Flat Size = 70m <sup>2</sup>	0.1021	0.0709	0.0415 nr/flat)	0.0464	63	44	26	29
C.	490 Flats Private Housing @ Average Flat Size = 70m <sup>2</sup>	0.1021	0.1021 0.0709 0.0415 0.0464 (pcu/hr/flat)				35	21	23
D.	1 residential house (911m <sup>2</sup> GFA)	0.3896	0.3896   0.3423   0.4970   0.3598   (pcu/hr/house)			1	1	1	1
E.	1 residential house (435m² GFA)	0.3896	0.3423	0.4970 /house)	0.3598	1	1	1	1
F.	1 residential house (487m² GFA)	0.3896		0.4970 /house)	0.3598	1	1	1	1
G.	56 Flats Private Housing @ Average Flat Size = 80m <sup>2</sup>	0.1379	0.1379   0.0905   0.0563   0.0689   (pcu/hr/flat)			8	6	4	4
Н.	1 residential house (450m² GFA)	0.3896   0.3423   0.4970   0.3598   (pcu/hr/house)			1	1	1	1	
l.	About 113m <sup>2</sup> Retail GFA	0.3307	0.3342 ocu/hr/10	0.3839 0m <sup>2</sup> GF		1	1	1	1

Note: Gen – Generation

- A. Starter Home for Hong Kong Residents Project by Hong Kong Housing Society in Yau Kam Tau
- B. Proposed Private Housing Development in Yau Kam Tau north of Tuen Mun Road

Att. - Attraction

- C. Proposed Private Housing Development in Yau Kam Tau near Po Fung Terrace
- D. Lot No. 407 in DD. 399, Ting Kau, Tsuen Wan West
- E. Lot 162RP (Part) in DD. 399 and Adjoining Government Land, Ting Kau, Tsuen Wan West
- F. 400 Castle Peak Road Ting Kau
- G. Lots 99, 100, 101R.P., 110R.P., 171C and 183 in D.D. 390 and Adjoining Government
- H. Land, Sham Tseng, Tsuen Wan
- I. Lot No. 408 in DD. 399 & the Extension Thereto, Ting Kau, Tsuen Wan, N.T.
- J. House 117, Ting Kau Village, Tsuen Wan, New Territories

#### Traffic Generated by the Proposed Development

4.10 Traffic generation of the Proposed Development is calculated based on the trip rates for "Private Housing: Low-Density / R(C)" with an average flat size of 300m<sup>2</sup>; and to be conservative, the upper limit trip rates are adopted. **Table 4.6** summarises the trip rates adopted and the calculated traffic generation.

TABLE 4.6 ADOPTED TRIP RATES AND TRAFFIC GENERATION OF THE PROPOSED DEVELOPMENT

ltem	AM Pea	ak Hour	PM Peak Hour		
	Generation Attraction		Generation	Attraction	
Trip Generation Rates (pcu/hour/flat)					
Private Housing: Low-Density / R(C)	0.3896	0.3423	0.4970	0.3598	
with an average flat size of 300m <sup>2</sup>					
Traffic Generation (pcu/hour)					
Proposed Development	1 1		1	1	
(1 house with about 531m <sup>2</sup> GFA)	2 (2-	way)	2 (2-way)		

- 4.11 **Table 4.6** shows that the Proposed Development is expected to generate only 2 pcu/hour (2-way) during the AM and PM peak hours respectively.
- 4.12 In addition, to err of the high side, the traffic generation associated with the Existing House is not deducted from the 2031 traffic analysis.

## Construction Traffic Generated by the Proposed Development

4.13 In view scale of the Proposed Development is small, the associated construction traffic is expected to be negligible. Conservatively, 2 construction vehicles trips per hour by goods vehicles are assumed, and **Table 4.7** summarizes the details.

TABLE 4.7 CONSTRUCTION TRAFFIC GENERATION OF THE PROPOSED DEVELOPMENT

ltem	AM Pea	ak Hour	PM Peak Hour		
	Generation	Attraction	Generation	Attraction	
Construction Vehicles	2 veh [5 pcu]				
(1  veh = 2.5  pcu)	4 veh [10 p	cu] (2-way)	4 veh [10 p	cu] (2-way)	

4.14 **Table 4.7** shows that the Proposed Development, conservatively, is expected to generate 10 pcu/hour (2-way) during the AM and PM peak hours respectively.

#### 2031 Traffic Flows

4.15 Year 2031 traffic flows for the following cases are derived:

2031 without the Proposed
Development [A] (Figure 4.1)

2031 with the Proposed
Development [B] (Figure 4.2)

2031 with the Construction
Traffic of the Proposed
Development [C] (Figure 4.3)

= 2031 traffic flows derived with reference to
BDTM + Traffic generated by other known
major committed and planned developments
(Table 4.7)

= [A] + Traffic generated by the Proposed
Development (Table 4.4)

= [A] + Construction Traffic generated (Table 4.5)

Development [C] (Figure 4.3)

4.16 **Figures 4.1, 4.2** and **4.3** show the 2031 peak hour traffic flows for the above 3 cases respectively.

#### 2031 Junction Capacity Analysis

4.17 2031 junction capacity analyses for the cases without and with the Proposed Development, and with the construction traffic of the Proposed Development are analyzed. **Table 4.8** summarises the results, and the detailed calculations are presented in **Annex A**.

TABLE 4.8 2031 JUNCTION PERFORMANCE

Ref.	Junction	Type of Junction / Parameter	Without the Proposed Development		With Prop Develo		With Construction Traffic	
			AM Peak				AM Peak	
			Hour	Hour	Hour	Hour	Hour	Hour
J01	Castle Peak Road – Ting	Round-	0.372	0.332	0.372	0.332	0.372	0.332
	Kau / Castle Peak Road	about						
	– New Ting Kau	/ RFC						
J02	Castle Peak Road – Ting	Priority	0.03	0.04	0.03	0.04	0.03	0.04
	Kau / Ting Yat Road /	/ RFC						
	Open Car Park							

Note: RFC – Ratio of Flow to Capacity

4.18 The above results indicate that the operational traffic generated by the Proposed Development, as well as the construction traffic generated, will have no impact to the junctions analysed, and is acceptable from traffic engineering viewpoint.

## **Pedestrian Forecasting**

- 4.19 Year 2031 pedestrian flows are produced: (i) with reference to the 2024 observed pedestrian flows, and (ii) the expected growth of 0.6% per annum from 2024 to 2031 in reference with the change in population projection by C&SD presented in **Table 4.2**.
- 4.20 Hence, Year 2031 pedestrian flows are derived as follows:

2031 without the Proposed = Existing Pedestrian Flows + Expected Growth Development [A] = Existing Pedestrian Flows + Expected Growth of Pedestrian Flows from 2022 to 2032 in reference with the 2019-based TPEDM

2031 with the Proposed = [A] + Pedestrian Generation of the Proposed Development [B] Development

## Pedestrian Generated by the Proposed Development

4.21 Pedestrian generation of the Proposed Development is estimated based a pedestrian generation survey carried out at the adjacent existing residential developments also located at the end of Ting Yat Road, and the survey results are summarized in **Table 4.9**.

TABLE 4.9 RESULTS OF PEDESTRIAN GENERATION SURVEYS

Surveyed Locations		ak Hour s / hour)	PM Peak Hour (persons / hour)		
	Generation	Attraction	Generation	Attraction	
The Wonderland (Lot 439 in DD399)	3	1	1	2	
Vodana Court (Lot 461 in DD399)	1	1	1	1	

- 4.22 **Table 4.9** shows the peak hour pedestrian generation and attraction of similar existing residential located nearby are no more than 2 to 3 people respectively.
- 4.23 Hence, to err on the high side, pedestrian generation and attraction for the Proposed Development are assumed to be 5 people in either direction for analysis purpose, which are summarized in **Table 4.10**.

TABLE 4.10 ADOPTED PEDESTRIAN GENERATION

ltem		ak Hour s / hour)	PM Peak Hour (persons / hour)		
	Generation	Attraction	Generation	Attraction	
Proposed Development	5 5		5 5		
	10 (2	-way)	10 (2-way)		

#### 2031 Levels of Service Analysis

4.24 2031 Levels of Services ("LOS") analyses for the cases without and with the Proposed Development are analyzed, and **Table 4.11** summarises the results.

**TABLE 4.11** OPERATIONAL PERFORMANCE FOR NEARBY PEDESTRIAN **FACILITIES** 

Footpath	Measured Width	Effective Width	Pro		out the evelopm	nent	With the Proposed Development			
	(m)	(m)	2-V	Vay	Flo	ow .	2-V	Vay	Flo	ow
			Pede		Ra	ıte	Pede			ate
			Flo		_	DS]	Flo			OS]
			(ped/	hour)	(ped/n	n/min)	(ped/	hour)	(ped/r	n/min)
			AM	PM	AM	PM	AM	PM	AM	PM
			Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak
			Hour	Hour	Hour	Hour	Hour	Hour	Hour	Hour
FP01	1.5m	0.5m	10	10	0.3 [A]	0.3 [A]	15	15	0.5 [A]	0.5 [A]
FP02	2.8m	1.8m	20	20	0.2 [A]	0.2 [A]	25	25	0.2 [A]	0.2 [A]
FP03	1.5m	1.0m	20	20	0.3 [A]	0.3 [A]	25	25	0.4 [A]	0.4 [A]
FP04	3.0m	2.0m	20	20	0.2 [A]	0.2 [A]	25	25	0.2 [A]	0.2 [A]
FP05	3.0m	2.0m	15	15	0.1 [A]	0.1 [A]	20	20	0.2 [A]	0.2 [A]
FP06	2.5m	1.5m	10	10	0.1 [A]	0.1 [A]	15	15	0.2 [A]	0.2 [A]
FP07	2.5m	1.5m	10	10	0.1 [A]	0.1 [A]	15	15	0.2 [A]	0.2 [A]
FP08	2.0m	1.0m	15	15	0.3 [A]	0.3 [A]	20	20	0.3 [A]	0.3 [A]

Note: Due to the small amount of pedestrian, 2-way pedestrian flow is rounded-up to nearest 5.

FP01 -

Castle Peak Road – New Ting Kau (Northern Footpath) Castle Peak Road – Ting Kau (Southern Footpath) FP02 -

FP03 -

FP04 -Footbridge NF390 across Castle Peak Road - New Ting Kau / Ting Kau

FP05 -Ramp between FP02 and FP04 FP06 -Staircase between FP02 and FP04 FP07 -Ramp between FP03 and FP04 FP08 -Staircase between FP04 and FP04

The above results indicate that the pedestrian generated by the Proposed 4.25 Development will have no impact to the pedestrian facilities analysed, and is acceptable from traffic engineering viewpoint.

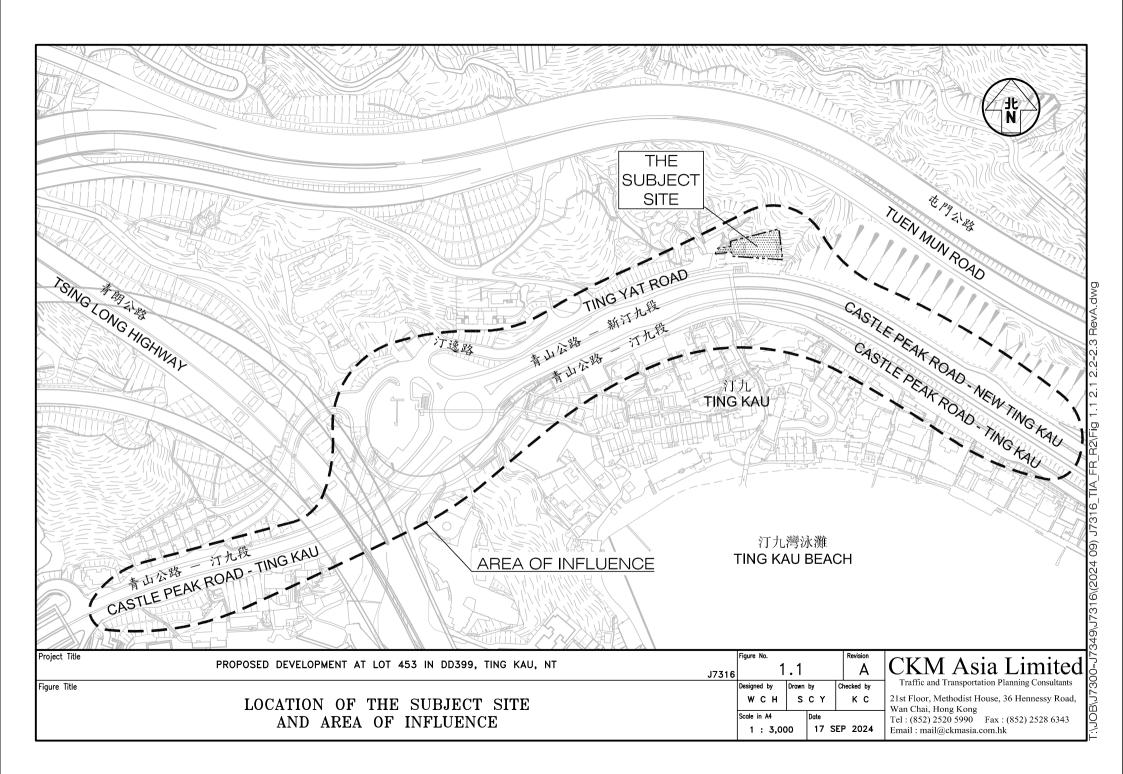
## Impact on Existing Public Transport Services

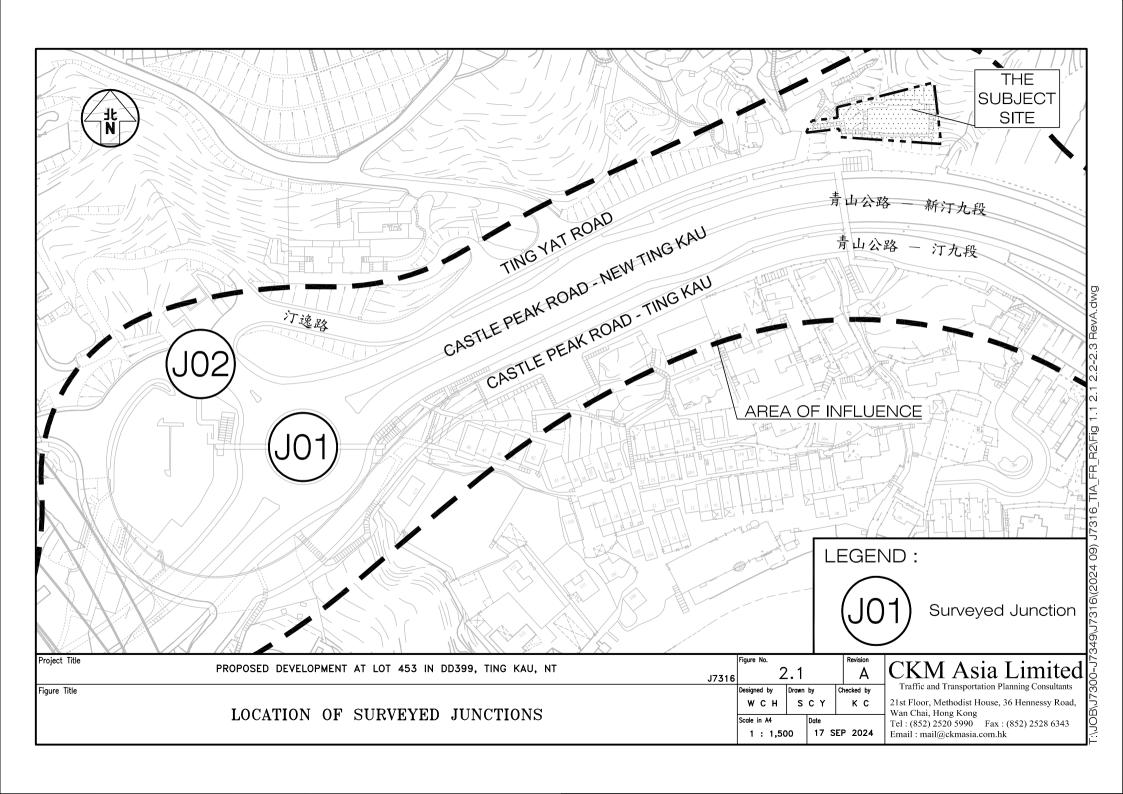
4.26 The Proposed Development is reconstruction of an existing single-family house with similar population of no more than 10 people. Hence, in view the Proposed Development has no change from the Existing House in terms of both land use and population, impact on the existing public transport service is not anticipated.

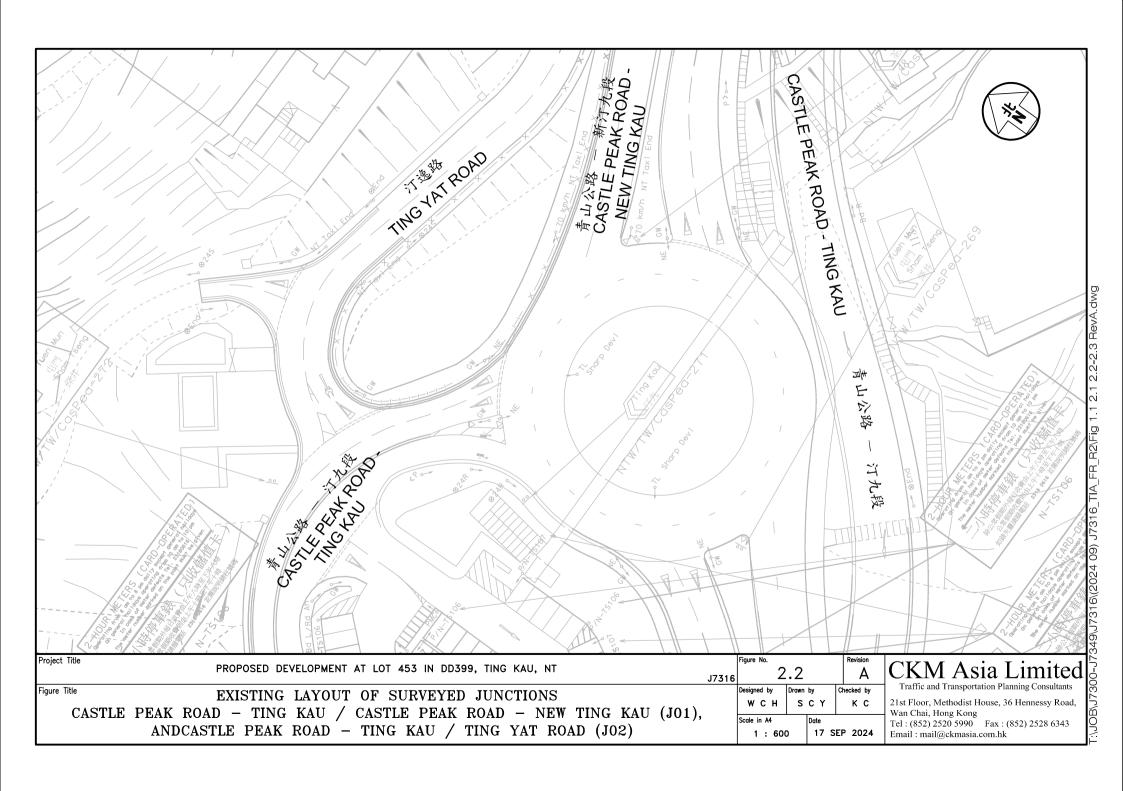
### 5.0 SUMMARY

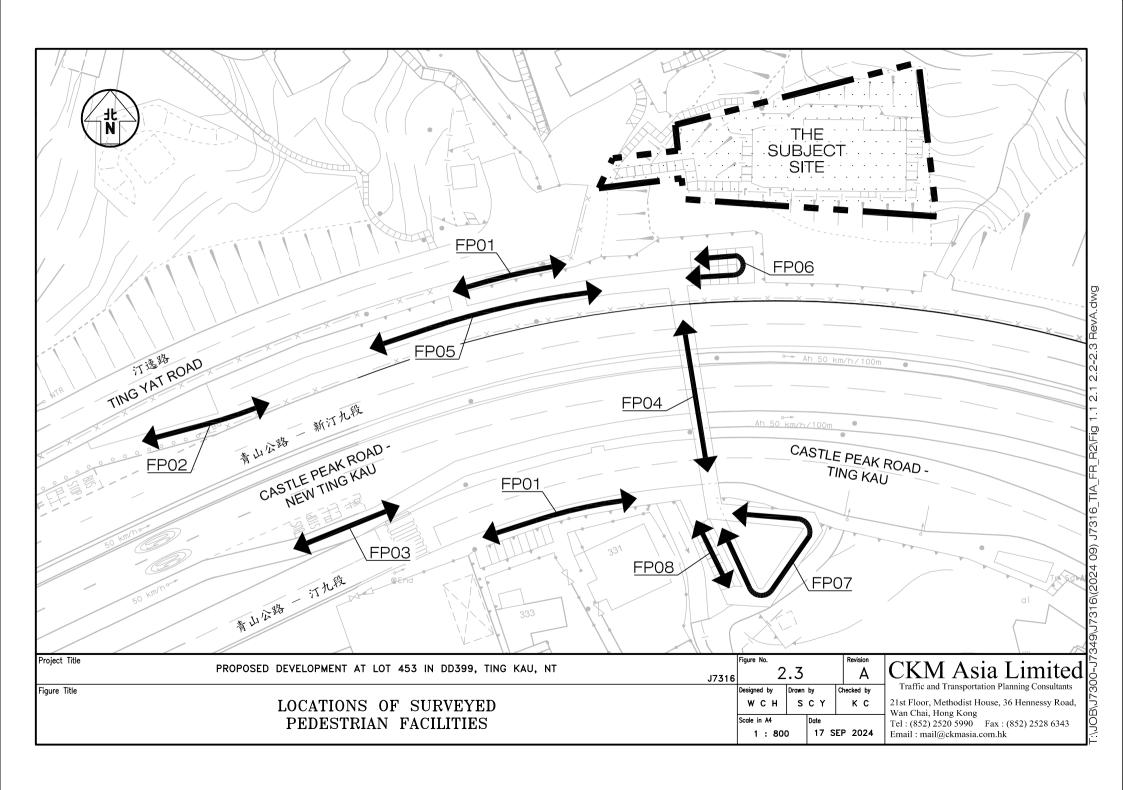
- 5.1 The Proposed Development is located at Lot 453 in DD.399 Castle Peak Road, Ting Kau, the New Territories. It is a 3-storey single-family house including a carport with some 579 m<sup>2</sup> GFA.
- 5.2 The proposed internal transport facilities to be provided at the Proposed Development satisfy the recommendation of the HKPSG, and are enhanced from the Existing House.
- 5.3 Manual classified counts were conducted at junctions located in the vicinity in order to establish the existing traffic flows during the AM and PM peak hours.
- 5.4 The design year 2031 traffic flows were derived with reference to 2031 traffic flows from the BDTM. The traffic generations associated with other known future developments in the vicinity are also taken into account.
- 5.5 The year 2031 capacity analysis concluded that the junctions analysed have sufficient capacity to accommodate the expected traffic growth and the traffic generated by the Proposed Development.
- 5.6 Pedestrian counts were conducted at nearby pedestrian facilities during the AM and PM peak hour to establish the existing pedestrian flows, of which are then forecasted to establish the year 2031 pedestrian flows, including the pedestrian generation of the Proposed Development.
- 5.7 Levels of Service analyses concluded that the pedestrian facilities have sufficient capacity to accommodate the expected pedestrian growth and pedestrian generated by the Proposed Development.
- 5.8 Hence, it can be concluded that the Proposed Development will result in no adverse traffic and pedestrian impact to the road and pedestrian network in the vicinity of the Subject Site, and is acceptable from traffic engineering viewpoint.

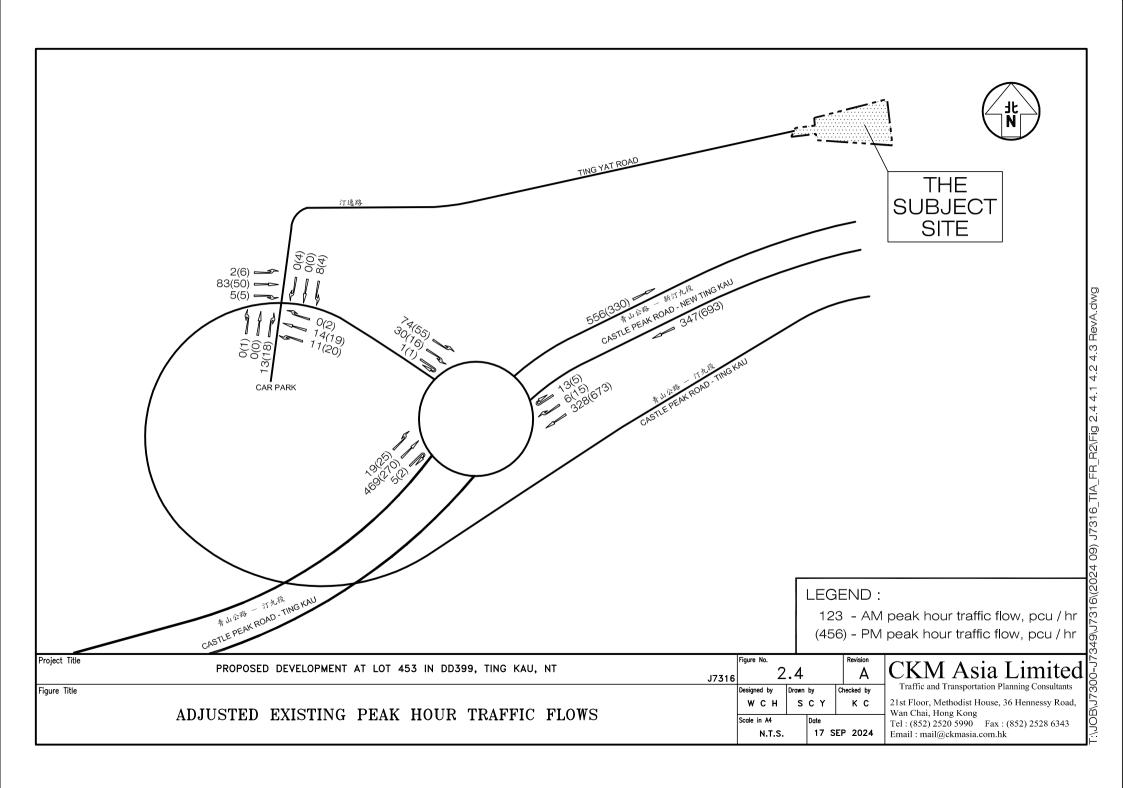


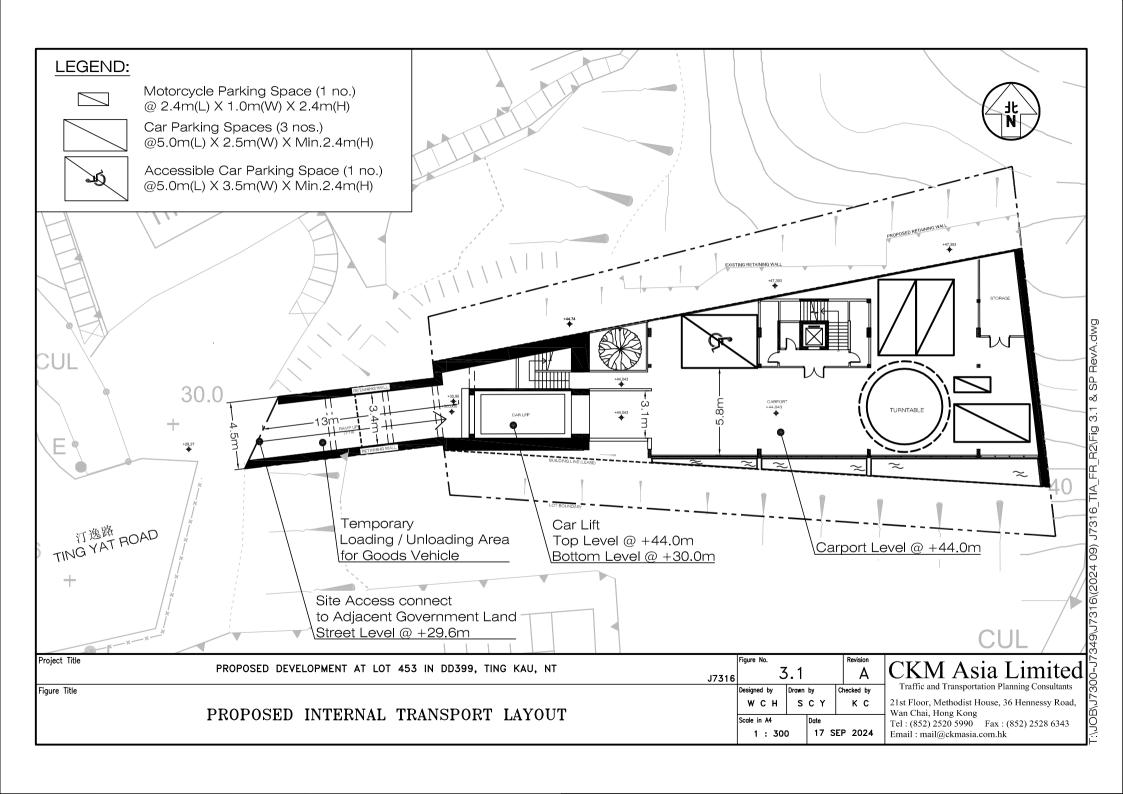


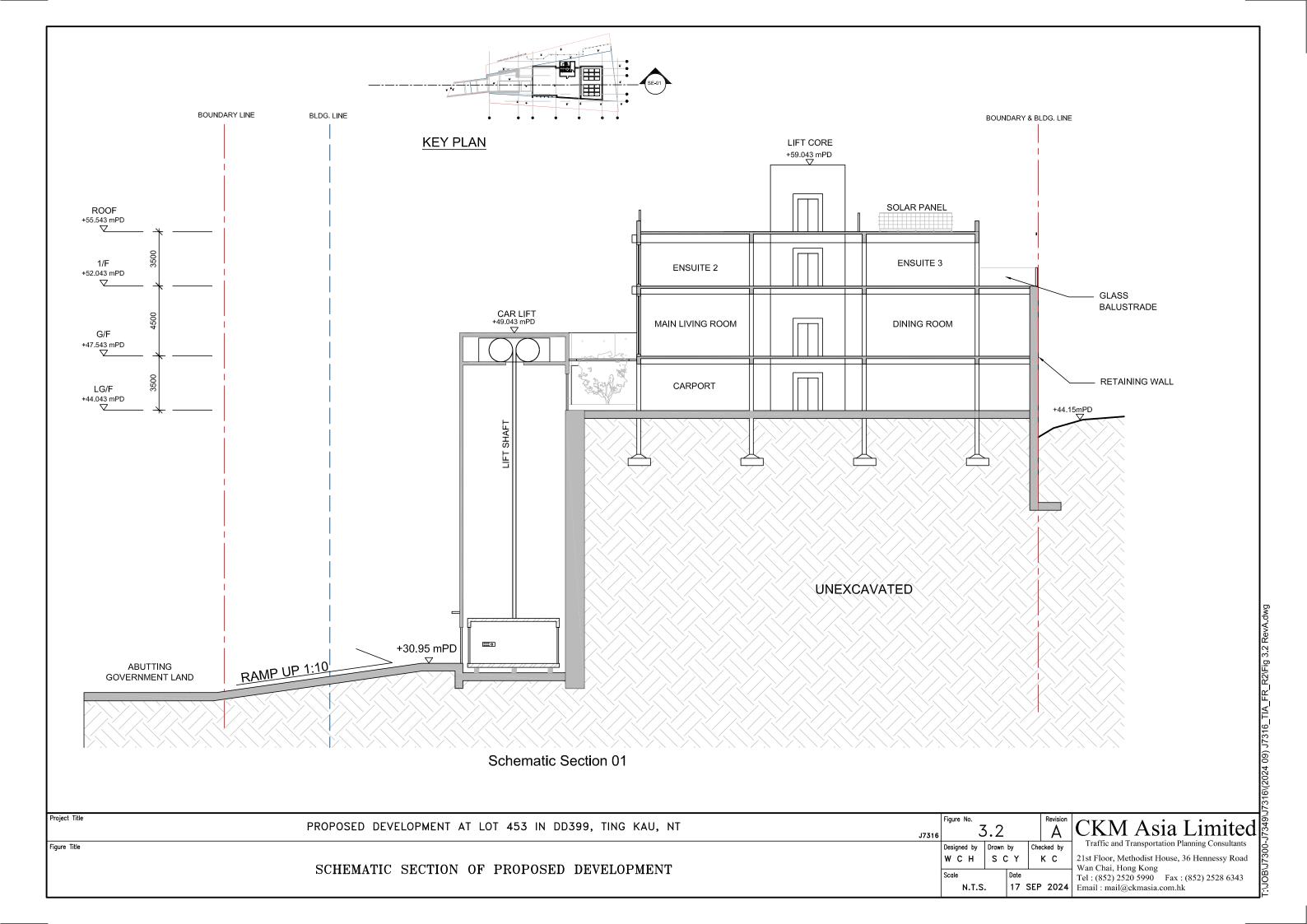


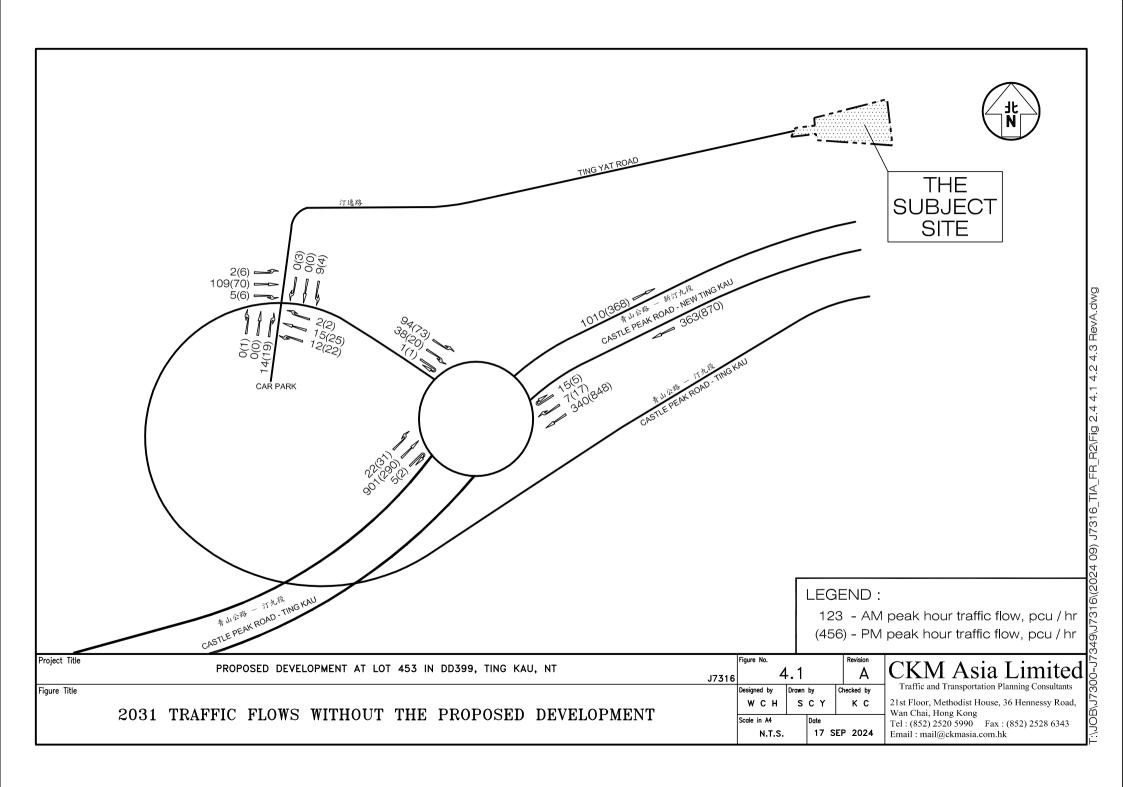


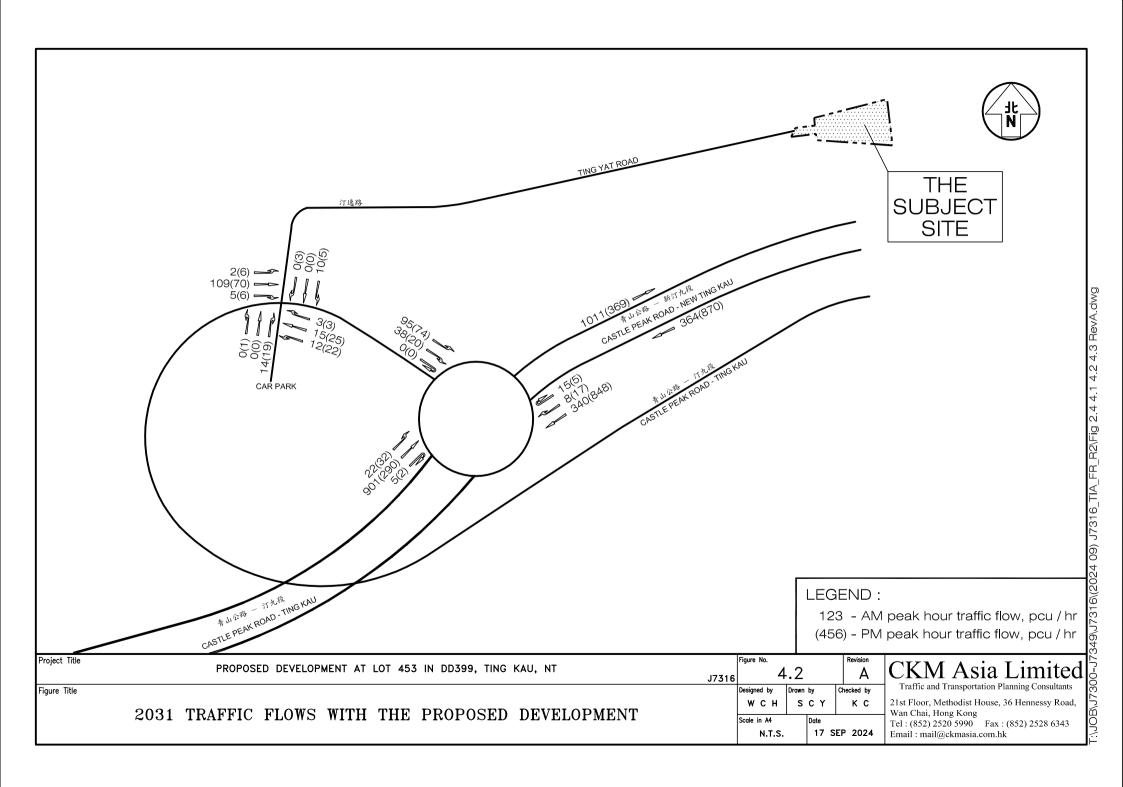


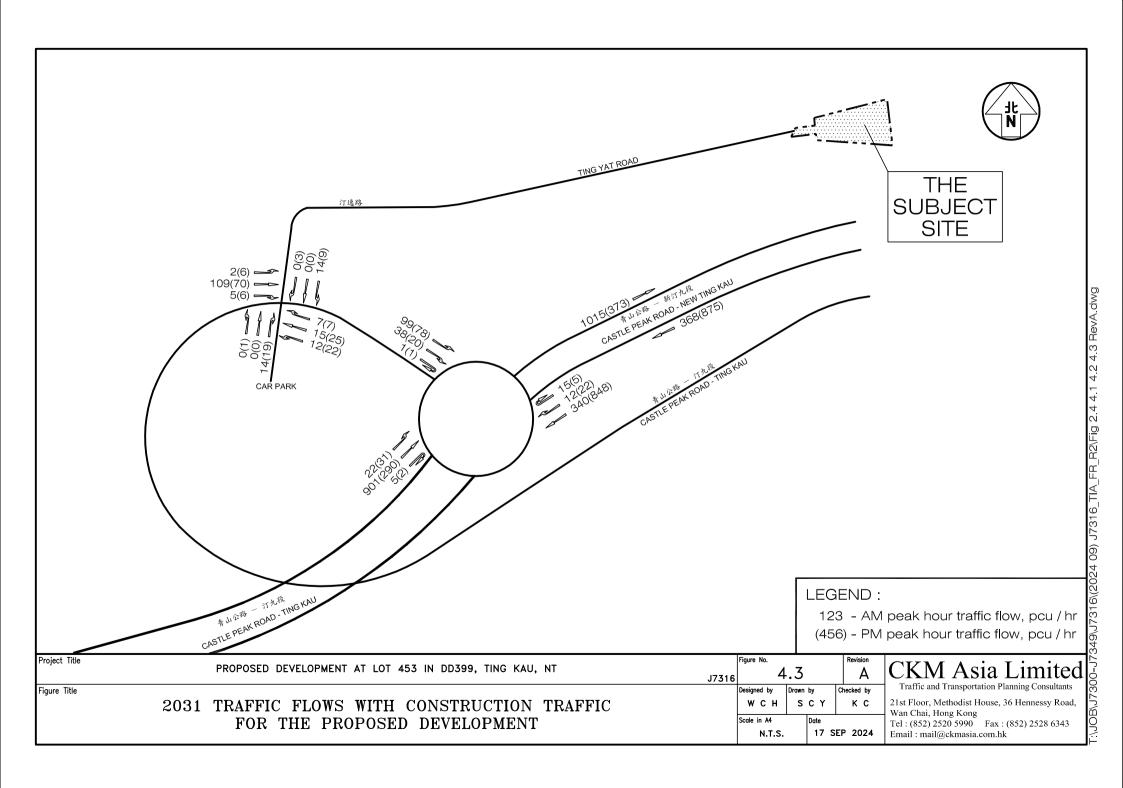














# **Roundabout Analysis**

Junction:	Castle Pea	k Road - Ting Kau / Castle Peak f	Jo	b Number: <u>J7316</u>	
Scenario:	Existing Co	ndition		Page	J1- 1
Design Year:	2024	Designed By: WCH	Checked Bv: WCH	Date:	17 September 2024

#### AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	To H	Total	q <sub>c</sub>
From A	13	328	6						347	36
From B	469	5	19						493	19
From C	74	30	1						105	486
From D										
From E										
From F										
From G										
From H										
Total	556	363	26						944	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	q <sub>c</sub>
From A	5	673	15						693	19
From B	270	2	25						297	21
From C	55	16	1						73	277
From D										
From E										
From F										
From G										
From H										
Total	330	692	41						1063	

#### Legend

Arm	Road (in clockwise order)
Α	Castle Peak Road - New Ting Kau East
В	Castle Peak Road - Ting Kau West
С	Castle Peak Road - Ting Kau North
D	
Е	
F	
G	
Н	

#### **Geometric Parameters**

Geometri	ric Parameters								
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S		
From A	10.0	7.0	20.0	20.0	50	40	0.2		
From B	10.5	7.0	20.0	20.0	50	60	0.3		
From C	10.0	6.0	20.0	10.0	50	50	0.6		
From D									
From E									
From F									
From G									
From H									

## Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_D$	= 1+0.5/(1+M)
М	$= \exp[(D-60)/10]$
<b>x</b> <sub>2</sub>	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

## Limitation

е	Entry Width	4.0 - 15.0 m
V	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
s	Sharpness of Flare	0.0 - 3.0

### Ratio-of-Flow to Capacity (RFC)

							$Q_{E}$		Entry Flow	,	RFC	
Arm	X <sub>2</sub>	M	$t_D$	K	F	$f_{c}$	AM	PM	AM	PM	AM	PM
From A	9.027	0.368	1.366	0.965	2735	0.804	2612	2625	347	693	0.133	0.264
From B	9.244	0.368	1.366	0.896	2801	0.817	2495	2494	493	297	0.197	0.119
From C	7.754	0.368	1.366	0.931	2350	0.731	1855	1998	105	73	0.057	0.036
From D												
From E												
From F												
From G												
From H												

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# **Roundabout Analysis**

Junction:	Castle Pea	k Road - Ting Kau / Ca		Job Number: J7316				
Scenario:	Without Pro	oposed Development	Pa	age J1- 2	<u>.</u>			
Design Year:	2031	Designed By:	WCH	Checked By:	WCH	Date:	17 Septem	ber 2024

#### AM Peak

Arm	To A	To B	To C	To D	To E	To F	To G	То Н	Total	q <sub>c</sub>
From A	15	340	7						362	44
From B	901	5	22						928	23
From C	94	38	1						133	921
From D										
From E										
From F										
From G										
From H										
Total	1010	383	30						1423	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	q <sub>c</sub>
From A	5	848	17						870	23
From B	290	2	31						323	23
From C	73	20	1						94	297
From D										
From E										
From F										
From G										
From H										
Total	368	870	49						1287	

#### Legend

Arm	Road (in clockwise order)
Α	Castle Peak Road - New Ting Kau East
В	Castle Peak Road - Ting Kau West
С	Castle Peak Road - Ting Kau North
D	
Е	
F	
G	
Н	

## Geometric Parameters

Geometri	offiction raidificates								
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S		
From A	10.0	7.0	20.0	20.0	50	40	0.2		
From B	10.5	7.0	20.0	20.0	50	60	0.3		
From C	10.0	6.0	20.0	10.0	50	50	0.6		
From D									
From E									
From F									
From G									
From H									

## Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{\rm c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_D$	= 1+0.5/(1+M)
М	$= \exp[(D-60)/10]$
$x_2$	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

#### Limitation

е	Entry Width	4.0 - 15.0 m
٧	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
s	Sharpness of Flare	0.0 - 3.0

### Ratio-of-Flow to Capacity (RFC)

							$Q_{E}$		Entry Flow		RFC	
Arm	<b>X</b> <sub>3</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	9.027	0.368	1.366	0.965	2735	0.804	2606	2622	362	870	0.139	0.332
From B	9.244	0.368	1.366	0.896	2801	0.817	2492	2492	928	323	0.372	0.130
From C	7.754	0.368	1.366	0.931	2350	0.731	1560	1984	133	94	0.085	0.047
From D												
From E												
From F												
From G												
From H												

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## **Roundabout Analysis**

 Junction:
 Castle Peak Road - Ting Kau / Castle Peak Road - New Ting Kau
 Job Number: J7316

 Scenario:
 With Proposed Development
 Page
 J1- 3

 Design Year:
 2031
 Designed By: WCH
 Checked By: WCH
 Date: 17 September 2024

#### AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	To H	Total	q <sub>c</sub>
From A	15	340	8						363	44
From B	901	5	22						928	24
From C	95	38	1						134	921
From D										
From E										
From F										
From G										
From H										
Total	1011	383	31						1425	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	To H	Total	q <sub>c</sub>
From A	5	848	17						870	23
From B	290	2	32						324	23
From C	74	20	1						95	297
From D										
From E										
From F										
From G										
From H										
Total	369	870	50						1289	

#### Legend

Arm	Road (in clockwise order)
Α	Castle Peak Road - New Ting Kau East
В	Castle Peak Road - Ting Kau West
С	Castle Peak Road - Ting Kau North
D	
Е	
F	
G	
Н	

#### **Geometric Parameters**

CCOIIICUIN	sometric i didireters											
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S					
From A	10.0	7.0	20.0	20.0	50	40	0.2					
From B	10.5	7.0	20.0	20.0	50	60	0.3					
From C	10.0	6.0	20.0	10.0	50	50	0.6					
From D												
From E												
From F												
From G												
From H												

#### Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{\rm c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_{D}$	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
$x_2$	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

#### Limitation

е	Entry Width	4.0 - 15.0 m
٧	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

#### Ratio-of-Flow to Capacity (RFC)

							$Q_{E}$		Entry Flow		RFC	
Arm	X <sub>4</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	9.027	0.368	1.366	0.965	2735	0.804	2606	2622	363	870	0.139	0.332
From B	9.244	0.368	1.366	0.896	2801	0.817	2492	2492	928	324	0.372	0.130
From C	7.754	0.368	1.366	0.931	2350	0.731	1560	1984	134	95	0.086	0.048
From D												
From E												
From F												
From G												
From H												

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# **Roundabout Analysis**

Junction:	Castle Peal	J	Job Number: <u>J7316</u>			
Scenario:	With Const	ruction Traffic	Page	Page J1- 4		
Design Year:	2031	Designed By:	WCH	Checked Bv: WCH	Date:	17 September 2024

#### AM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	To H	Total	qc
From A	15	340	12						367	44
From B	901	5	22						928	28
From C	99	38	1						138	921
From D										
From E										
From F										
From G										
From H										
Total	1015	383	35						1433	

#### PM Peak

Arm	To A	То В	To C	To D	To E	To F	To G	То Н	Total	qc
From A	5	848	22						875	23
From B	290	2	31						323	28
From C	78	20	1						99	297
From D										
From E										
From F										
From G										
From H										
Total	373	870	54						1297	

#### Legend

Arm	Road (in clockwise order)
Α	Castle Peak Road - New Ting Kau East
В	Castle Peak Road - Ting Kau West
С	Castle Peak Road - Ting Kau North
D	
Е	
F	
G	
Н	

#### **Geometric Parameters**

CCOIIICUI	c i arainete	,13					
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
From A	10.0	7.0	20.0	20.0	50	40	0.2
From B	10.5	7.0	20.0	20.0	50	60	0.3
From C	10.0	6.0	20.0	10.0	50	50	0.6
From D							
From E							
From F							
From G							
From H							

#### Predictive Equation $Q_E = K(F - f_cq_c)$

$Q_{E}$	Entry Capacity
$q_{\rm c}$	Circulating Flow across the Entry
K	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x <sub>2</sub>
f <sub>c</sub>	$= 0.210t_D(1+0.2x_2)$
$t_D$	= 1+0.5/(1+M)
М	$= \exp[(D-60)/10]$
<b>x</b> <sub>2</sub>	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

#### Limitation

е	Entry Width	4.0 - 15.0 m
V	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

#### Ratio-of-Flow to Capacity (RFC)

							QE		Entry Flow	1	RFC	
Arm	X <sub>4</sub>	M	$t_D$	K	F	f <sub>c</sub>	AM	PM	AM	PM	AM	PM
From A	9.027	0.368	1.366	0.965	2735	0.804	2606	2622	367	875	0.141	0.334
From B	9.244	0.368	1.366	0.896	2801	0.817	2489	2489	928	323	0.373	0.130
From C	7.754	0.368	1.366	0.931	2350	0.731	1560	1984	138	99	0.088	0.050
From D												
From E												
From F												
From G												
From H												

CKM Asia Limited J1



### **Junctions 9**

### **PICADY 9 - Priority Intersection Module**

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Filename: J7316\_TIA\_R2.j9

Path: J:\ENG\Job\J73XX\J7316 Ting Kau - Redevelopment of La Casette\Working\Jct9

Report generation date: 13/9/2024 15:46:40

»2024, AM

»2024, PM

»2031 Without, AM

»2031 Without, PM

»2031 With Development, AM

»2031 With Development, PM

»2031 With Construction, AM

»2031 With Construction, PM

#### Summary of junction performance

		AM			PM			
	Queue (PCU)	Delay (s)	RFC	LOS	Queue (PCU)	Delay (s)	RFC	LOS
				20	24			
Stream B-ACD	0.0	6.72	0.03	А	0.0	6.68	0.04	А
Stream A-BCD	0.0	0.00	0.00	А	0.0	5.88	0.00	Α
Stream D-ABC	0.0	5.46	0.01	А	0.0	6.18	0.01	А
Stream C-B	0.0	5.32	0.01	А	0.0	5.35	0.01	А
			2	031 W	Vithout			
Stream B-ACD	0.0	6.79	0.03	А	0.0	6.76	0.04	А
Stream A-BCD	0.0	6.04	0.00	А	0.0	5.87	0.00	А
Stream D-ABC	0.0	5.52	0.01	А	0.0	6.11	0.01	Α
Stream C-B	0.0	5.33	0.01	А	0.0	5.37	0.01	Α
		20	031 W	/ith D	evelopment			
Stream B-ACD	0.0	6.80	0.03	А	0.0	6.76	0.04	А
Stream A-BCD	0.0	6.05	0.01	А	0.0	5.87	0.00	Α
Stream D-ABC	0.0	5.53	0.02	А	0.0	6.03	0.01	Α
Stream C-B	0.0	5.33	0.01	А	0.0	5.37	0.01	Α
		2	031 V	Vith C	onstruction			
Stream B-ACD	0.0	6.82	0.03	А	0.0	6.79	0.04	А
Stream A-BCD	0.0	6.10	0.01	А	0.0	5.91	0.01	А
Stream D-ABC	0.0	5.57	0.02	А	0.0	5.88	0.02	Α
Stream C-B	0.0	5.34	0.01	А	0.0	5.39	0.01	А

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.



#### File summary

#### File Description

Title	J7316
Location	CPR-TK / Ting Yat Rd / Open Car Park
Site number	
Date	13/9/2024
Version	
Status	
Identifier	
Client	
Jobnumber	
Enumerator	СКМ
Description	

#### Units

	Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
Ī	m	kph	PCU	PCU	perHour	s	-Min	perMin

#### **Analysis Options**

Calculate Queue Percentiles		RFC Threshold	RFC Threshold Average Delay threshold (s) Queu	
		0.85	36.00	20.00

### **Demand Set Summary**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2024	AM	ONE HOUR	08:00	09:30	15
D2	2024	PM	ONE HOUR	17:00	18:30	15
D3	2031 Without	AM	ONE HOUR	08:00	09:30	15
D4	2031 Without	PM	ONE HOUR	17:00	18:30	15
D5	2031 With Development	AM	ONE HOUR	08:00	09:30	15
D6	2031 With Development	PM	ONE HOUR	17:00	18:30	15
D7	2031 With Construction	AM	ONE HOUR	08:00	09:30	15
D8	2031 With Construction	PM	ONE HOUR	17:00	18:30	15

#### **Analysis Set Details**

ID	Network flow scaling factor (%)
A1	100.000

2



# 2024, AM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

# **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.16	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

#### Arms

#### Arms

Arm	Name	Description	Arm type
Α	CPR-TK		Major
В	Open Car Park		Minor
С	CPR-TK		Major
D	Ting Yat Rd		Minor

#### **Major Arm Geometry**

Arm	Width of carriageway (m)	Has kerbed central reserve	Has right turn bay	Width for right turn (m)	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)
A - CPR-TK	10.30				50.0	<b>✓</b>	0.00
C - CPR-TK	10.30		✓	3.50	50.0		-

 $\textit{Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (\textit{if relevant}) are measured opposite Arm D. } \\$ 

#### **Minor Arm Geometry**

Arm	Minor arm type	Lane width (m)	Visibility to left (m)	Visibility to right (m)
B - Open Car Park	One lane	4.00	50	50
D - Ting Yat Rd	One lane	3.50	50	50



#### Slope / Intercept / Capacity

**Priority Intersection Slopes and Intercepts** 

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for A-D	Slope for B-A	Slope for B-C	Slope for B-D	Slope for C-A	Slope for C-B	Slope for C-D	Slope for D-A	Slope for D-B	Slope for D-C
1	A-D	603	-	-	-	-	-	-	0.190	0.271	0.190	-	-	-
1	B-A	570	0.084	0.213	0.213	-	-	-	0.134	0.305	-	0.213	0.213	0.107
1	B-C	721	0.090	0.227	-	-	-	-	-	-	-	-	-	-
1	B-D, nearside lane	570	0.084	0.213	0.213	-	-	-	0.134	0.305	0.134	-	-	-
1	B-D, offside lane	570	0.084	0.213	0.213	-	-	-	0.134	0.305	0.134	-	-	-
1	C-B	688	0.217	0.217	0.310	-	-	-	-	-	-	-	-	-
1	D-A	688	-	-	-	-	-	-	0.217	-	0.086	-	-	-
1	D-B, nearside lane	544	0.128	0.128	0.291	-	-	-	0.204	0.204	0.081	-	-	-
1	D-B, offside lane	544	0.128	0.128	0.291	-	-	-	0.204	0.204	0.081	-	-	-
1	D-C	544	-	0.128	0.291	0.102	0.204	0.204	0.204	0.204	0.081	-	-	-

The slopes and intercepts shown above do NOT include any corrections or adjustments.

# **Traffic Demand**

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D1	2024	AM	ONE HOUR	08:00	09:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	25	100.000
B - Open Car Park		✓	13	100.000
C - CPR-TK		✓	90	100.000
D - Ting Yat Rd		✓	8	100.000

# Origin-Destination Data

Demand (PCU/hr)

	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	11	14	0	
From	B - Open Car Park	13	0	0	0	
	C - CPR-TK	83	5	0	2	
	D - Ting Yat Rd	8	0	0	0	

### **Vehicle Mix**

**Heavy Vehicle Percentages** 

	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	0	0	0	
From	B - Open Car Park	0	0	0	0	
	C - CPR-TK	0	0	0	0	
	D - Ting Yat Rd	0	0	0	0	

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



# Results

			=	
Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.03	6.72	0.0	А
A-BCD	0.00	0.00	0.0	А
A-B				
A-C				
D-ABC	0.01	5.46	0.0	А
C-D				
C-A				
С-В	0.01	5.32	0.0	А



# 2024, PM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

# **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.67	А

#### **Junction Network Options**

Driving side	Lighting	
Left	Normal/unknown	

### Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D2	2024	PM	ONE HOUR	17:00	18:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	41	100.000
B - Open Car Park		✓	19	100.000
C - CPR-TK		✓	61	100.000
D - Ting Yat Rd		✓	8	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То						
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd		
	A - CPR-TK	0	20	19	2		
From	B - Open Car Park	18	0	1	0		
	C - CPR-TK	50	5	0	6		
	D - Ting Yat Rd	4	0	4	0		



	То						
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd		
	A - CPR-TK	0	0	0	0		
From	B - Open Car Park	0	0	0	0		
	C - CPR-TK	0	0	0	0		
	D - Ting Yat Rd	0	0	0	0		

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.04	6.68	0.0	А
A-BCD	0.00	5.88	0.0	А
A-B				
A-C				
D-ABC	0.01	6.18	0.0	А
C-D				
C-A				
С-В	0.01	5.35	0.0	А



# 2031 Without, AM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

# **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.10	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

### Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D3	2031 Without	AM	ONE HOUR	08:00	09:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	29	100.000
B - Open Car Park		✓	14	100.000
C - CPR-TK		✓	116	100.000
D - Ting Yat Rd		✓	9	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То				
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd
	A - CPR-TK	0	12	15	2
From	B - Open Car Park	14	0	0	0
	C - CPR-TK	109	5	0	2
	D - Ting Yat Rd	9	0	0	0



		То				
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	0	0	0	
From	B - Open Car Park	0	0	0	0	
	C - CPR-TK	0	0	0	0	
	D - Ting Yat Rd	0	0	0	0	

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.03	6.79	0.0	А
A-BCD	0.00	6.04	0.0	А
A-B				
A-C				
D-ABC	0.01	5.52	0.0	А
C-D				
C-A				
С-В	0.01	5.33	0.0	А



# 2031 Without, PM

#### **Data Errors and Warnings**

Severity	Area	Item Description	
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

# **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.41	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

# Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D4	2031 Without	PM	ONE HOUR	17:00	18:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	49	100.000
B - Open Car Park		✓	20	100.000
C - CPR-TK		✓	82	100.000
D - Ting Yat Rd		✓	7	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То						
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd		
	A - CPR-TK	0	22	25	2		
From	B - Open Car Park	19	0	1	0		
	C - CPR-TK	70	6	0	6		
	D - Ting Yat Rd	4	0	3	0		



	То							
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd			
	A - CPR-TK	0	0	0	0			
From	B - Open Car Park	0	0	0	0			
	C - CPR-TK	0	0	0	0			
	D - Ting Yat Rd	0	0	0	0			

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.04	6.76	0.0	А
A-BCD	0.00	5.87	0.0	А
A-B				
A-C				
D-ABC	0.01	6.11	0.0	А
C-D				
C-A				
С-В	0.01	5.37	0.0	А



# 2031 With Development, AM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

### **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.15	А

#### **Junction Network Options**

Driving side	Lighting	
Left	Normal/unknown	

# Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D5	2031 With Development	AM	ONE HOUR	08:00	09:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	30	100.000
B - Open Car Park		✓	14	100.000
C - CPR-TK		✓	116	100.000
D - Ting Yat Rd		✓	10	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	12	15	3	
From	B - Open Car Park	14	0	0	0	
	C - CPR-TK	109	5	0	2	
	D - Ting Yat Rd	10	0	0	0	



	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	0	0	0	
From	B - Open Car Park	0	0	0	0	
	C - CPR-TK	0	0	0	0	
	D - Ting Yat Rd	0	0	0	0	

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.03	6.80	0.0	А
A-BCD	0.01	6.05	0.0	А
A-B				
A-C				
D-ABC	0.02	5.53	0.0	А
C-D				
C-A				
С-В	0.01	5.33	0.0	А



# 2031 With Development, PM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

### **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.44	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

# Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D6	2031 With Development	PM	ONE HOUR	17:00	18:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	49	100.000
B - Open Car Park		✓	20	100.000
C - CPR-TK		✓	82	100.000
D - Ting Yat Rd		✓	8	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	22	25	2	
From	B - Open Car Park	19	0	1	0	
	C - CPR-TK	70	6	0	6	
	D - Ting Yat Rd	5	0	3	0	



	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	0	0	0	
From	B - Open Car Park	0	0	0	0	
	C - CPR-TK	0	0	0	0	
	D - Ting Yat Rd	0	0	0	0	

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.04	6.76	0.0	А
A-BCD	0.00	5.87	0.0	А
A-B				
A-C				
D-ABC	0.01	6.03	0.0	А
C-D				
C-A				
С-В	0.01	5.37	0.0	А



# 2031 With Construction, AM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

# **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.37	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

# Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D7	2031 With Construction	AM	ONE HOUR	08:00	09:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	34	100.000
B - Open Car Park		✓	14	100.000
C - CPR-TK		✓	116	100.000
D - Ting Yat Rd		✓	14	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

	То				
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd
	A - CPR-TK	0	12	15	7
From	B - Open Car Park	14	0	0	0
	C - CPR-TK	109	5	0	2
	D - Ting Yat Rd	14	0	0	0



	То				
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd
	A - CPR-TK	0	0	0	0
From	B - Open Car Park	0	0	0	0
	C - CPR-TK	0	0	0	0
	D - Ting Yat Rd	0	0	0	0

# Results

Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.03	6.82	0.0	А
A-BCD	0.01	6.10	0.0	А
A-B				
A-C				
D-ABC	0.02	5.57	0.0	А
C-D				
C-A				
С-В	0.01	5.34	0.0	А



# 2031 With Construction, PM

#### **Data Errors and Warnings**

Severity	Area	Item	Description
Warning	Vehicle Mix		HV% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in PCUs or Vehs.

### **Junction Network**

#### **Junctions**

Junction	Name	Junction Type	Major road direction	Junction Delay (s)	Junction LOS
1	CPR-TK / Ting Yat Rd	Crossroads	Two-way	1.69	А

#### **Junction Network Options**

Driving side	Lighting
Left	Normal/unknown

# Traffic Demand

#### **Demand Set Details**

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time segment length (min)
D8	2031 With Construction	PM	ONE HOUR	17:00	18:30	15

Vehicle mix source	PCU Factor for a HV (PCU)
HV Percentages	2.00

#### **Demand overview (Traffic)**

Arm	Linked arm	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
A - CPR-TK		✓	54	100.000
B - Open Car Park		✓	20	100.000
C - CPR-TK		✓	82	100.000
D - Ting Yat Rd		✓	12	100.000

# Origin-Destination Data

#### Demand (PCU/hr)

		То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd		
	A - CPR-TK	0	22	25	7		
From	B - Open Car Park	19	0	1	0		
	C - CPR-TK	70	6	0	6		
	D - Ting Yat Rd	9	0	3	0		

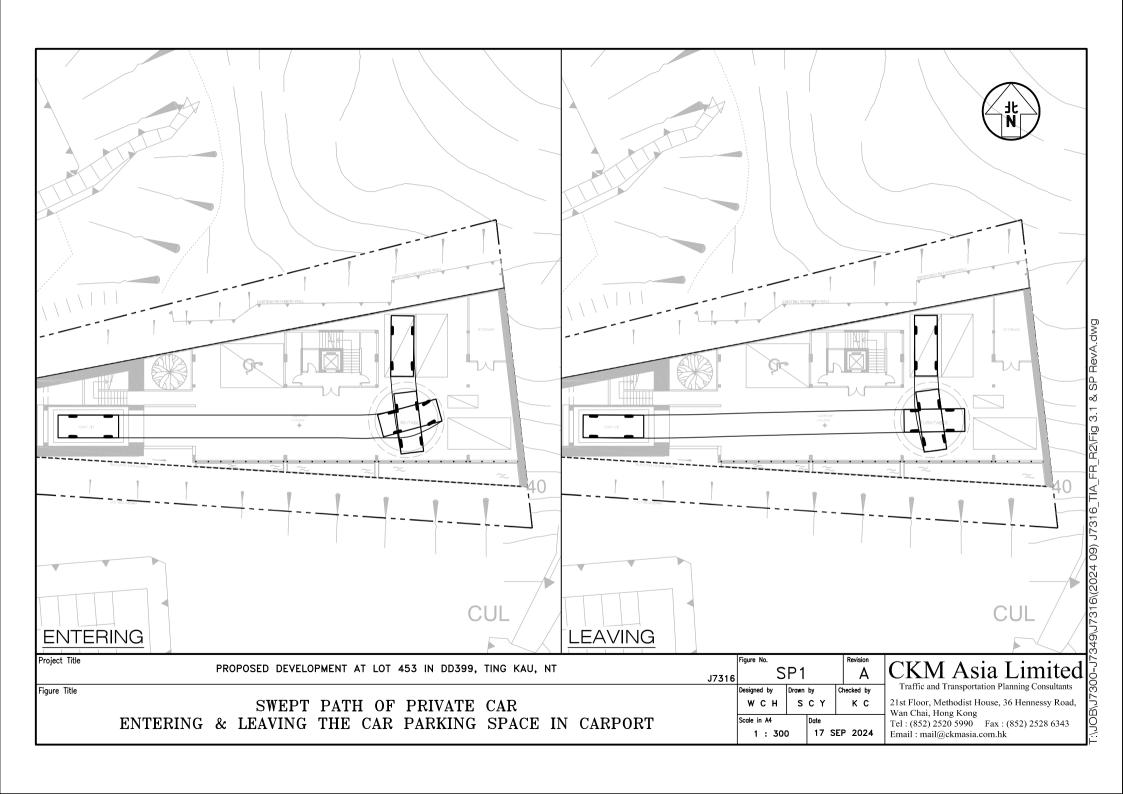


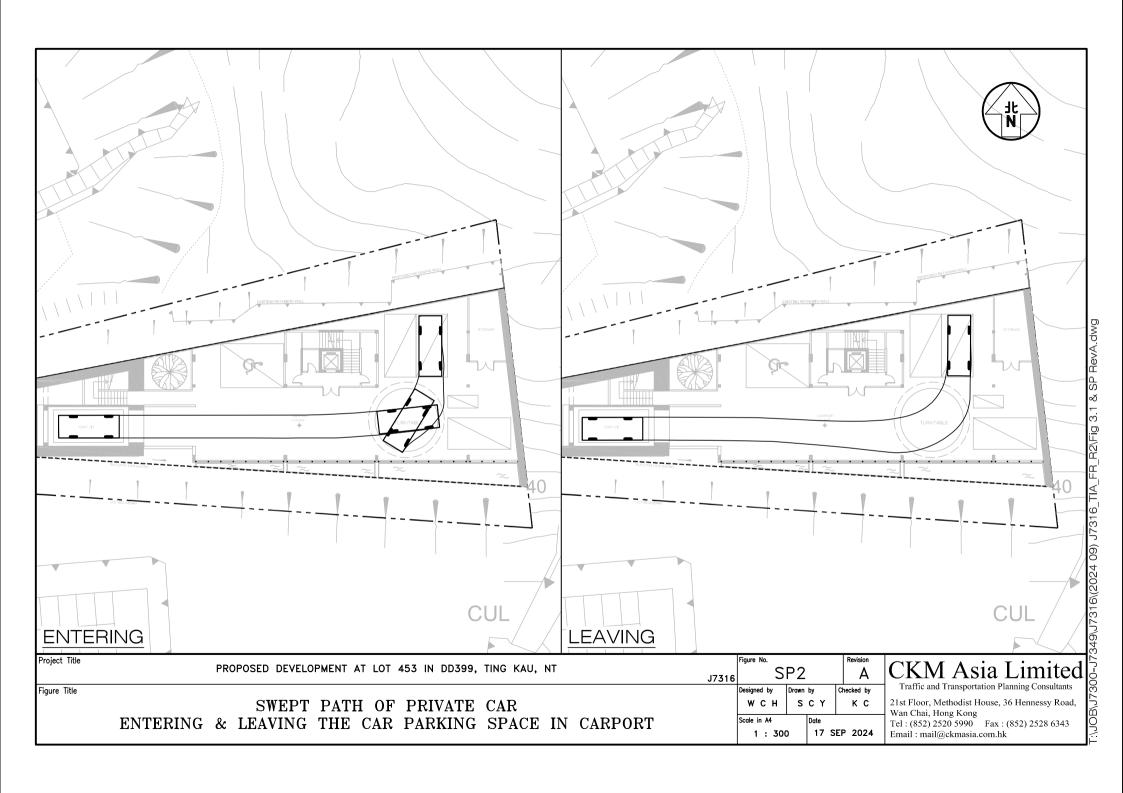
	То					
		A - CPR-TK	B - Open Car Park	C - CPR-TK	D - Ting Yat Rd	
	A - CPR-TK	0	0	0	0	
From	B - Open Car Park	0	0	0	0	
	C - CPR-TK	0	0	0	0	
	D - Ting Yat Rd	0	0	0	0	

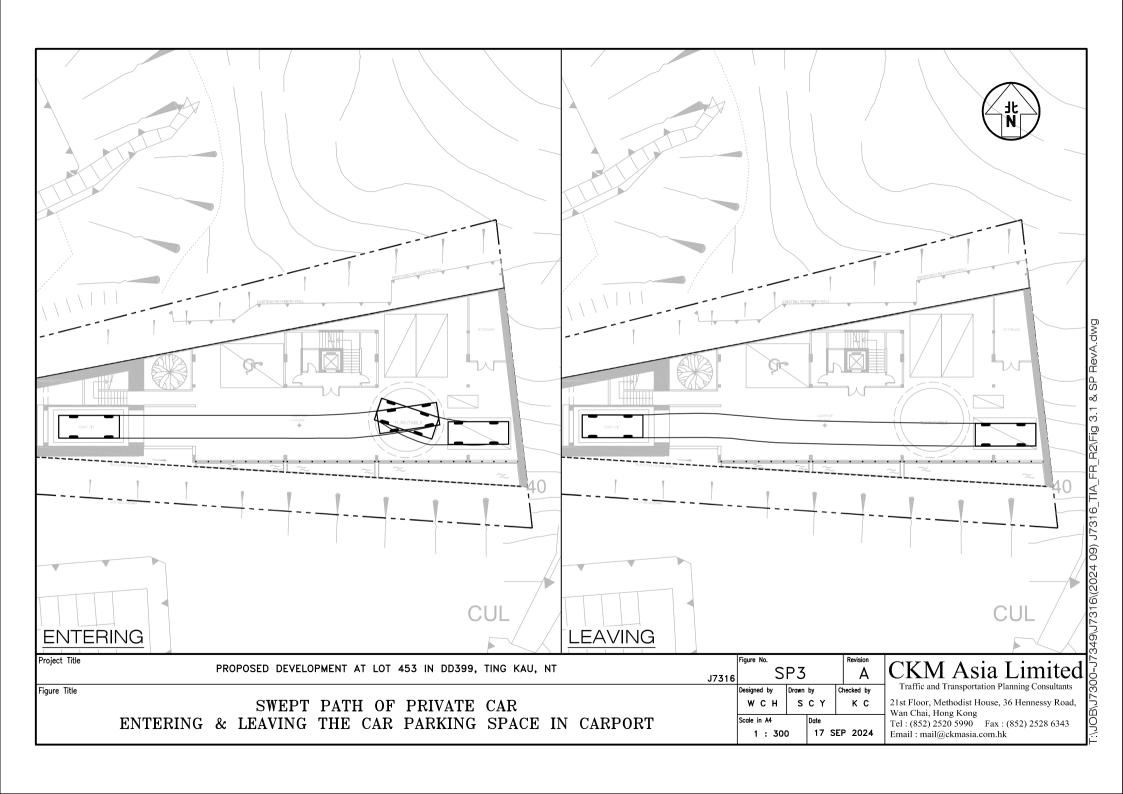
# Results

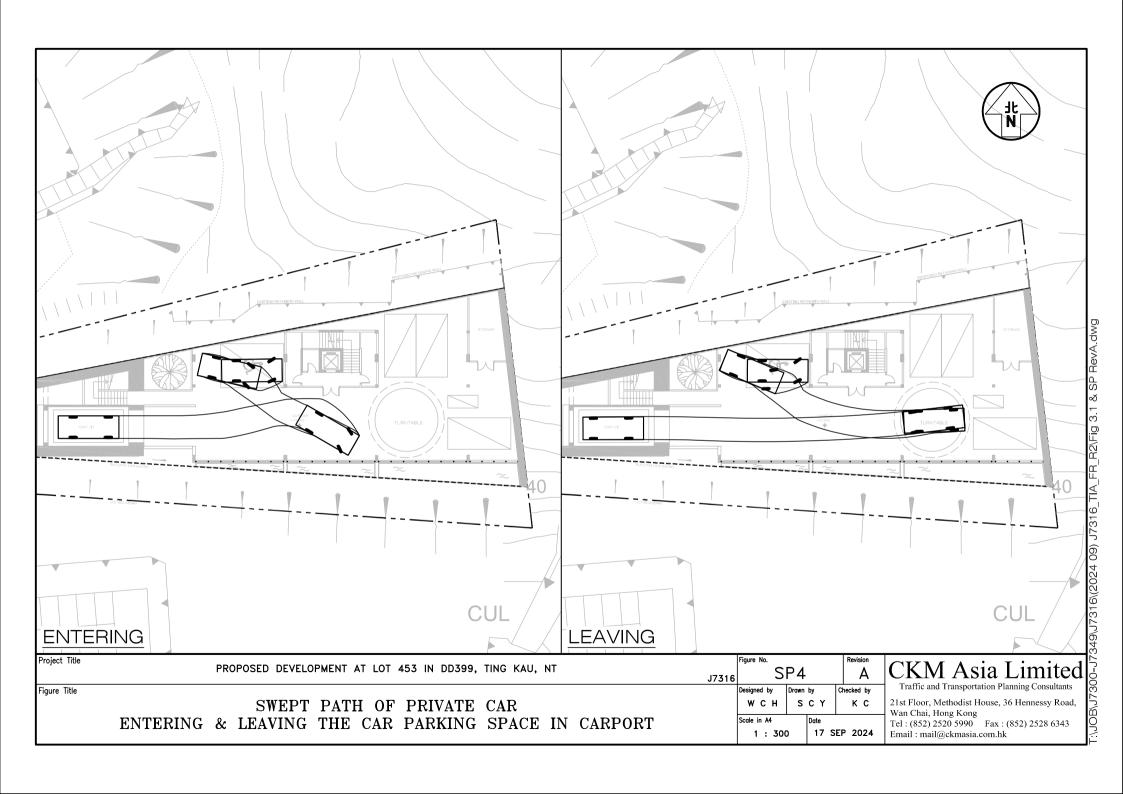
Stream	Max RFC	Max delay (s)	Max Queue (PCU)	Max LOS
B-ACD	0.04	6.79	0.0	А
A-BCD	0.01	5.91	0.0	А
A-B				
A-C				
D-ABC	0.02	5.88	0.0	А
C-D				
C-A				
С-В	0.01	5.39	0.0	А

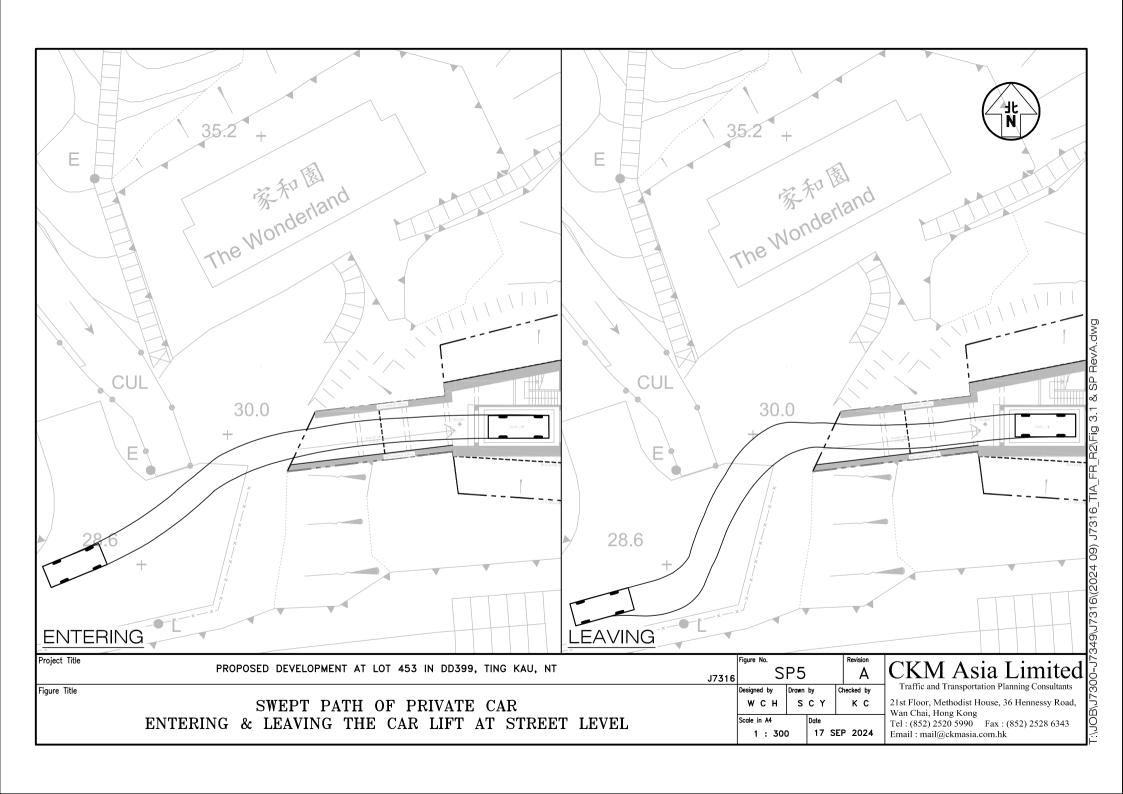


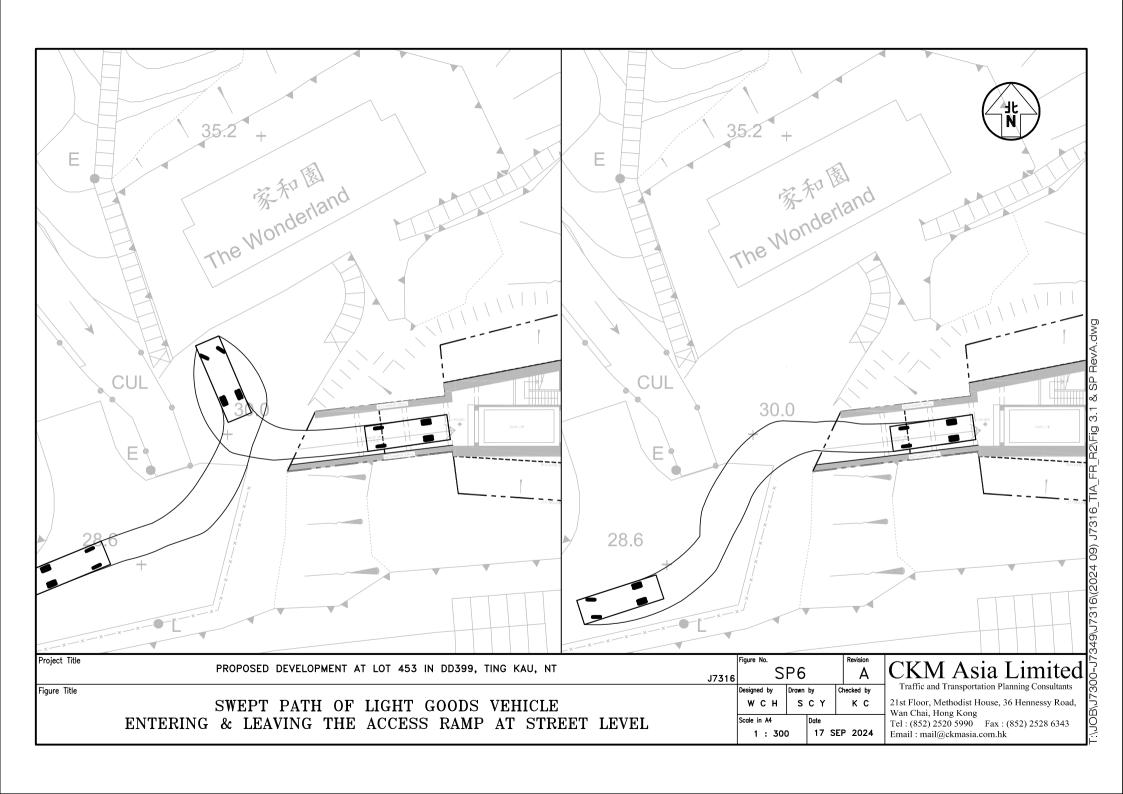














# Car Lift Analysis -

### Based on TPDM Upper Trip Attraction Rates

Job Title	Proposed Development Lot 453 in DD 339, Ting Kau	<u>Date</u>	17 Sep 2024
	port floor (m) Speed (m/s)		15.7 0.5 31.0
Lift door o Car exits I Car enters Door close Car lift trav Lift door o Car exits I	ift in forward gear on carport floor is lift in forward gear on carport floor es vels from carport floor to G/F pens ift in forward gear on G/F ift in forward gear on G/F	<u>T</u>	Time (s) 31.0 3.0 10.0 10.0 3.0 31.0 3.0 10.0 10
Peak Hou Arrival rate Cycle time	f lift servers, k r Traffic Attraction (TPDM Upper Limit) e λ (vehicles / hour) e ω (s) te μ of one lift server (vehicles / hour)		1 1.0 114.0 31.6

	Probability of	Probability of	Probability of
<u>Number</u>	<b>Exact N Cars</b>	N Cars or Less	<b>More Than N Cars</b>
of Cars N	in the Lift System	in the Lift System	in the Lift System
0	96.8%	96.8%	3.2%
1	3.1%	99.9%	0.1%
2	0.1%	100.0%	0.0%
3	0.0%	100.0%	0.0%
4	0.0%	100.0%	0.0%
5	0.0%	100.0%	0.0%
6	0.0%	100.0%	0.0%
7	0.0%	100.0%	0.0%

#### Conclusion

#### Normal Condition:

The probability of 1 additional car arriving when 1 car lift and 0 waiting spaces being occupied is 0.1% The provision of 0 waiting space is acceptable under normal condition.

Floor	Level (m)	Distance	No. of	Column C *	
	f	rom G/F (m)	spaces	Column D	Remarks
4/F		0.00		0	
3/F		0.00		0	
2/F		0.00		0	
1/F	44.00	15.70		5 78.5	4 PC + 1 MC
G/F	28.30	0.00		0	
B1		0.00		0	
B2		0.00		0	
		TOTAL		5 15.7	

#### Formula:

k is the number of lift servers.

 $\boldsymbol{\lambda}$  is the arrival rate in vehicles per 15 minutes.

 $\mu$  is the service rate of a lift server in vehicles per 15 minutes.

N  $1/N!^*(\lambda/\mu)$  summation from N=0 to N=k-1

	( , , = =	
0	1	1
1	0	1
2	0	1
3	0	1
4	0	1
5	0	1
6	0	1
7	0	1
8	0	1
9	0	1
10	0	1

Probability of having exactly zero cars in the lift system: 
$$P(0) = \frac{1}{\left[\sum_{N=0}^{k-1} \frac{1}{N!} \left(\frac{\lambda}{\mu}\right)^N\right] + \frac{1}{k!} \left(\frac{\lambda}{\mu}\right)^k \frac{k\mu}{k\mu - \lambda}}$$
 Probability of having exactly N cars in the lift system: For N < k: 
$$P(N) = \frac{1}{N!} \left(\frac{\lambda}{\mu}\right)^N P(0)$$
 For N \geq k: 
$$P(N) = \frac{1}{k!} \left(\frac{\lambda}{\mu}\right)^N P(0)$$
 k - -number of lift servers 
$$\lambda$$
 - -arrival rate 
$$\mu$$
 - -service rate



Page 1