Proposed Temporary Open Storage of Construction Materials, Construction Machineries and Vehicles with Ancillary Facilities for A Period of 3 Years and Associated Filling of Pond/Land and Excavation Of Land

Various Lots in D.D. 125 And Adjoining Government Land, Ha Tsuen, Yuen Long, New Territories

Drainage Impact Assessment Report

Applicant

First Champion Limited

AMENDMENT RECORD

REVISION NO.	DESCRIPTION	PREPARED BY (Date)	REVIEWED BY (Date)	APPROVED BY (Date)
1.0	Final DIA	12/10/2023	13/10/2023	13/10/2023
2.0	Final DIA	07/11/2023	09/11/2023	09/11/2023
3.0	Final DIA	22/3/2024	25/3/2024	25/3/2024
4.0	Final DIA	2/6/2024	3/6/2024	4/6/2024

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1 Project Background

1.1 Introduction

- 1.1.1 The Applicant intends to develop a open storage of construction materials, construction machineries and vehicles with ancillary facilities for a period of 3 years and associated filling of pond, filling of land and excavation of land at various lots in D.D. 125 and adjoining Government Land in Ha Tsuen, Yuen Long, New Territories.
- 1.1.2 According to the Approved Ha Tsuen Fringe Outline Zoning Plan ("OZP") No. S/YL-HTF/12, the application site currently falls within "Green Belt" ("GB") zone. A planning permission for the proposed open storage of construction materials, construction machineries and vehicles as well as the proposed filling of pond, filling of land and excavation of land, are required on application to the Town Planning Board ("TPB") under Section 16 of the Town Planning Ordinance.
- 1.1.3 In order to assess possible drainage impact may be generated from the proposed development, a Drainage Impact Assessment ("DIA") is conducted to support this Section 16 planning application.
- 1.2 Objective of the Assessment
- 1.2.1 The objectives of this DIA are to assess the potential drainage impact that may be generated from the proposed development and recommend the mitigation measures, if necessary, to alleviate the impacts.

2 Site Description

- 2.1 Description of Existing Environment
- 2.1.1 The area of the application site is about 41,569m² and is located at Ha Tsuen, Yuen Long District. Existing site levels ranging from +17.9mPD to +29.1mPD.
- 2.2 Existing Baseline Conditions
- 2.2.1 According to the site inspection conducted in September 2023, the site is currently a vacant land overgrown with weeds and different tree groups. Moreover, several ditches/watercourses were found next to the Site, which are connected to surrounding catchments to South China Sea. The location of the Site is shown on Drawing No. PLAN 1 in **Appendix A**.
- 2.3 Proposed Development Scheme
- 2.3.1 The site is proposed to be a temporary open storage of construction materials, construction machineries and vehicles with ancillary facilities for a period of 3 years and associated filling of pond/land and excavation of land. A proposed master layout plan with Drawing No. PLAN 6 is enclosed in **Appendix A**.
- 2.3.2 The following uses or facilities will be provided:
 - · Open Storage of Vehicles;
 - · Open Storage of Construction Materials;
 - Open Storage of Construction Machineries.

3 Methodology

- 3.1 Assessment Method
- 3.1.1 Rational Method is used to estimate the peak runoff from the catchment according to "Stormwater Drainage Manual Planning, Design and Management" (SDM). The peak runoff is given by the following expression:

$$Q_p = 0.278 \, \text{C i A}$$

Where Q_p = peak runoff in m³/s

C = runoff coefficient (dimensionless)

i = rainfall intensity in mm/hr
 A = catchment area in km²

3.1.2 According to the Stormwater Drainage Manual, the runoff coefficient C is considered below:

Table 1: Runoff Coefficients

	I
Surface Characteristics	Runoff Coefficient
Asphalt	0.70 - 0.95
Concrete	0.80 - 0.95
Brick	0.70 - 0.85
Grassland (Heavy Soil)	
Flat	0.13 - 0.25
Steep	0.25 - 0.35
Grassland (Sandy Soil)	
Flat	0.05 - 0.15
Steep	0.15 - 0.20

3.1.3 The rainfall intensity i is determined by using the Gumbel Solution:

$$i = a/(td + b)c$$

Where i = extreme mean intensity in mm/hr

td = duration in minutes (*td*≤240)

a, b, c = storm constants given in the table below

Table 2: Storm Constants for Different Return Periods of HKO Headquarters (based on Table 3a of SDM)

Return Period T(years)	2	5	10	20	50	100	200
а	499.8	480.2	471.9	463.6	451.3	440.8	429.5
b	4.26	3.36	3.02	2.76	2.46	2.26	2.05
С	0.494	0.429	0.397	0.369	0.337	0.316	0.295

3.1.4 The Brandsby William's Equation is used to determine the time of concentration etc.

$$t_o = 0.14465L/(H^{0.2}A^{0.1})$$

Where t_0 = time of concentration of a natural catchment (min.);

A = catchment area (m^2);

H = average slope (m per 100m), measured along the line of natural flow, from the summit of the catchment to the point under consideration;

L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m)

3.1.5 The Manning's Equation is used to determine the capacity of U-channel and Stream:

$$V = \frac{R^{\frac{1}{6}}}{n} \sqrt{Rs}$$

where V = mean velocity (m/s)

R = hydraulic radius (m)

n = Manning coefficient (s/m^{1/3})

s = hydraulic gradient (energy loss per unit length due to friction)

3.1.6 The application site is proposed to be temporary open storage of construction materials, construction machineries and vehicles with ancillary facilities for a period of 3 years and associated filling of pond/land and excavation of land. Rainfall increase due to climate change is not adopted in the runoff assessment in Appendix B.

4 Existing Drainage

- 4.1 Existing Drainage Routes and Arrangements
- 4.1.1 The Site is located almost immediately adjacent to (to the north of) a substantial (in the order of 2m wide) natural streamcourse which serves a large upstream catchment, leading up to Yuen Tau Shan. The overall catchment is shown on Drawing No. DIA1 in **Appendix A**. The overall catchment measures approximately 41,569m².
- 4.1.2 Within the Site, there are no apparent main drainage systems, with runoff generally passing overland from South to North, towards the main natural streamcourse as indicated on Drawing No. DIA1 in **Appendix A**.
- 4.1.3 There are no flooding blackspots in the vicinity of the Site and there is no history of flooding in the area (apart from the natural pond within the Site).
- 4.1.4 There are no known Ecologically Important Streams/Rivers in the catchment in which the Site is located.

5 Drainage Impact Assessment (DIA)

- 5.1 Project Site
- 5.1.1 The site is proposed to be concrete paved. The proposed site levels are ranging from +22.0 mPD to +26.0 mPD. There would be additional concrete paving area compared to the existing situation, with a resultant increase in runoff. The increase is quantified and discussed in Section 5.4. The proposed site catchment is shown on Drawing No. DIA2 in **Appendix A**
- 5.1.2 According to the topographical data and the existing drainage facilities on the surveys map obtained from Lands Department, there is an External Catchment located at the adjacent to the project site. The runoff from the External Catchment will flow from the Eastern and Southern Boundary of the Project Site, this extra runoff will potentially further drain into the proposed drainage system. As such, runoff arising from the External Catchment should be considered in this DIA using Rational Method.
- 5.1.3 Three main catchment areas were identified based on the proposed site layout plan including the local natural upstream catchment, external catchment adjacent to the proposed site and the proposed site itself. The runoffs are further collected into the existing public open rectangular channel next to the Site. The proposed site condition is shown on Drawing No. PLAN 11 and PLAN 12 in **Appendix A**.

5.2 Proposed Drainage Arrangement

5.2.1 The Site currently receives runoff from the External Catchment and this will continue after the proposed development as shown in Drawing No. DIA2. The runoff is expected to be widespread (rather than at discrete locations), U-channels will be proposed to collect the internal and external drainage. The flow capacities of the proposed U-channel are calculated using the Chart for the Rapid Design of Channels. Runoff from corresponding Site Catchments (calculated based on a return period of 50 years), the capacity estimation and checking for the proposed U-channels are included in **Appendix C**. The overall drainage is proposed to discharge into Existing 6m open rectangular channel. The exact arrangement(s) for the U channels will be determined during later stages of Project implementation.

5.3 Assessment Assumptions

5.3.1 Runoff coefficient of C=0.25 is adopted for the naturally vegetated hillsides and steep vegetated soil. For the Proposed Development, the whole site coverage will be paved with impervious concrete, runoff coefficient of C = 0.95 is adopted. However, the development is proposed to introduce a range of different materials for various parts of the Site and different runoff coefficients are adopted. Details of the runoff assessment, please refer to the calculation in **Appendix B**.

5.4 Drainage Impact Assessment

- 5.4.1 The Site currently receives runoff from the local upstream catchment and this will continue after the proposed development. The runoff is expected to be widespread (rather than at discrete locations), U-channels are proposed for both Existing Site Catchment and the External Catchment. For the drainage system, flow capacities of the proposed U-channel are calculated using the Chart for the Rapid Design of Channels. A proposed UC is designed to collect the runoff collected by the U channels and discharge to the existing open rectangular channel next to the site. The preliminary drainage layout and capacity checking of the internal UC are shown in **Drawing No. DIA3** in **Appendix A** and **Appendix C** based on the runoff assessment.
- 5.4.2 The 1 in 50-year peak discharge from the Site alone will increase from 0.439 to 1.561 m³/s, i.e. an increase of 1.122 m³/s. The existing and future runoff flows from the proposed site, external and overall catchment are presented in **Appendix B**. It is understood that the proposed development would cause additional flow to the public drainage system. The overall drainage flow is estimated to be 7.630 m³/s for 1 in 50-year peak discharge. To avoid adverse drainage impact on the existing natural stream, the capacity of Existing 6m open rectangular channel is calculated using Manning Equation, please refer to the calculation shown in **Appendix D**. The design capacity of the Existing 2.5m open rectangular Channel is found to be 24.456m³/s. Therefore, the existing drainage system is adequate to cater the additional flow from the proposed development.

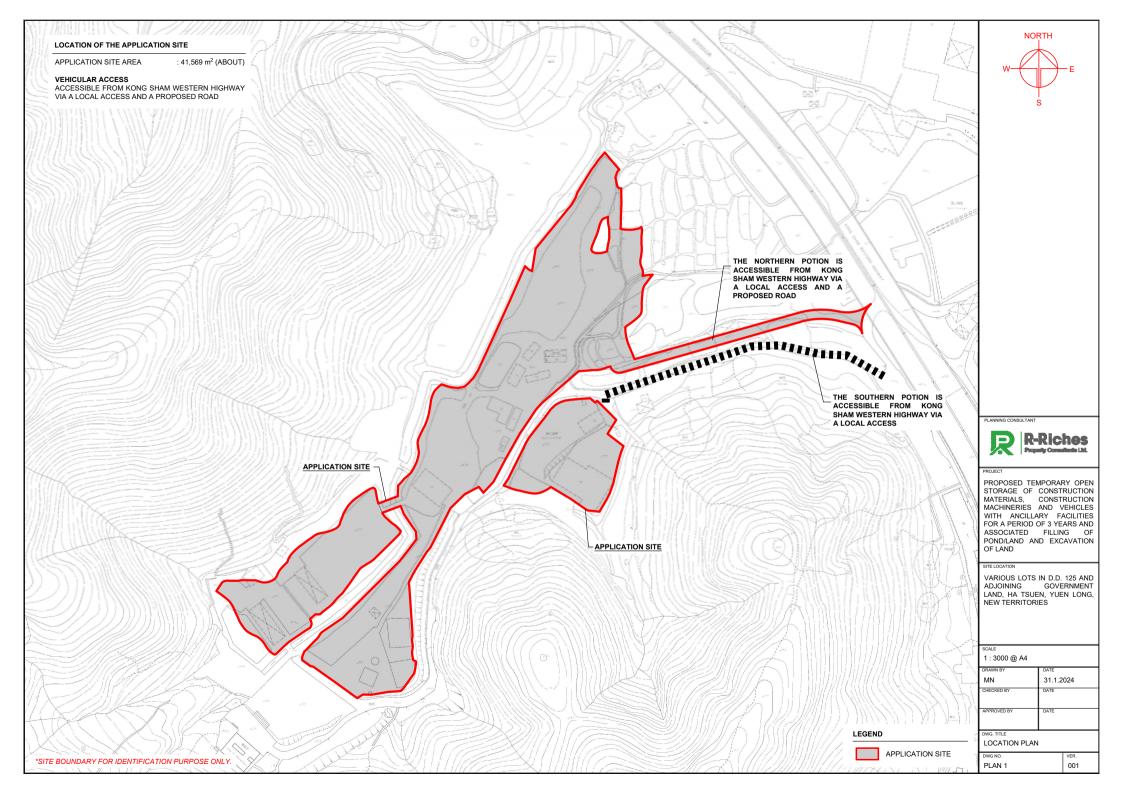
6 CONCLUSION

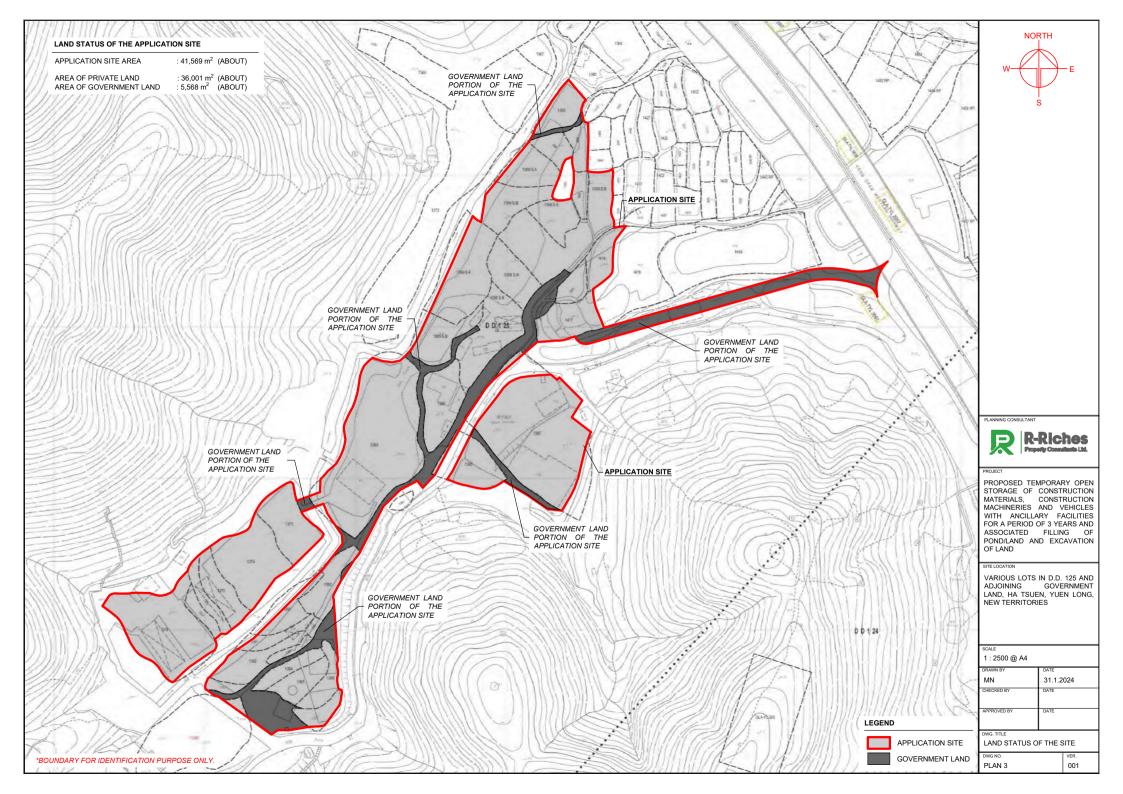
- 6.1.1 The Project Proponent will be responsible for the construction and ongoing maintenance of the drainage facilities. The runoff to the existing natural stream during rainstorm would be discharged by means of 300mm, 900mm U-channels and 1200mm pipe connecting to existing 2.5m open rectangular channel.
- 6.1.2 The proposed development will result in slightly greater runoff than the existing Site comparing to the design capacity of the Existing 6m rectangular Channel. The incremental runoff before and after the development was estimated using the rational method. The existing 2.5m rectangular Channel is adequate to collect the incremental runoff during the heavy rainstorm. As a result, no adverse drainage impact to the existing drainage system is anticipated after the development of the Site with 50 year return period.
- 6.1.3 This DIA Report presents the initial findings regarding drainage impact and indicative drainage layout. A qualified engineer should be engaged by the Applicant of the Proposed Development to review and provide detailed designs for the internal Site drainage layout. A "Drainage Proposal" including detailed designs based on calculations and quantitative assessments shall be prepared by the qualified engineer and submitted to DSD, for their review and approval prior to the commencement of work.

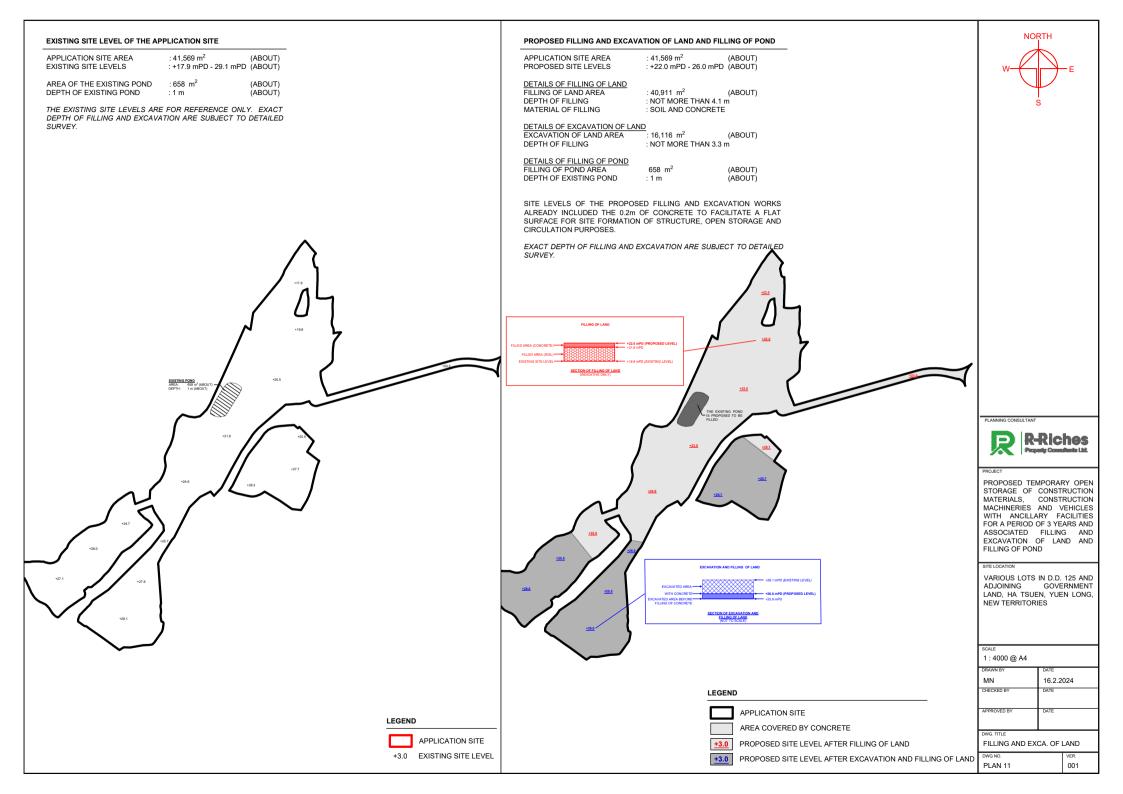
June 2024

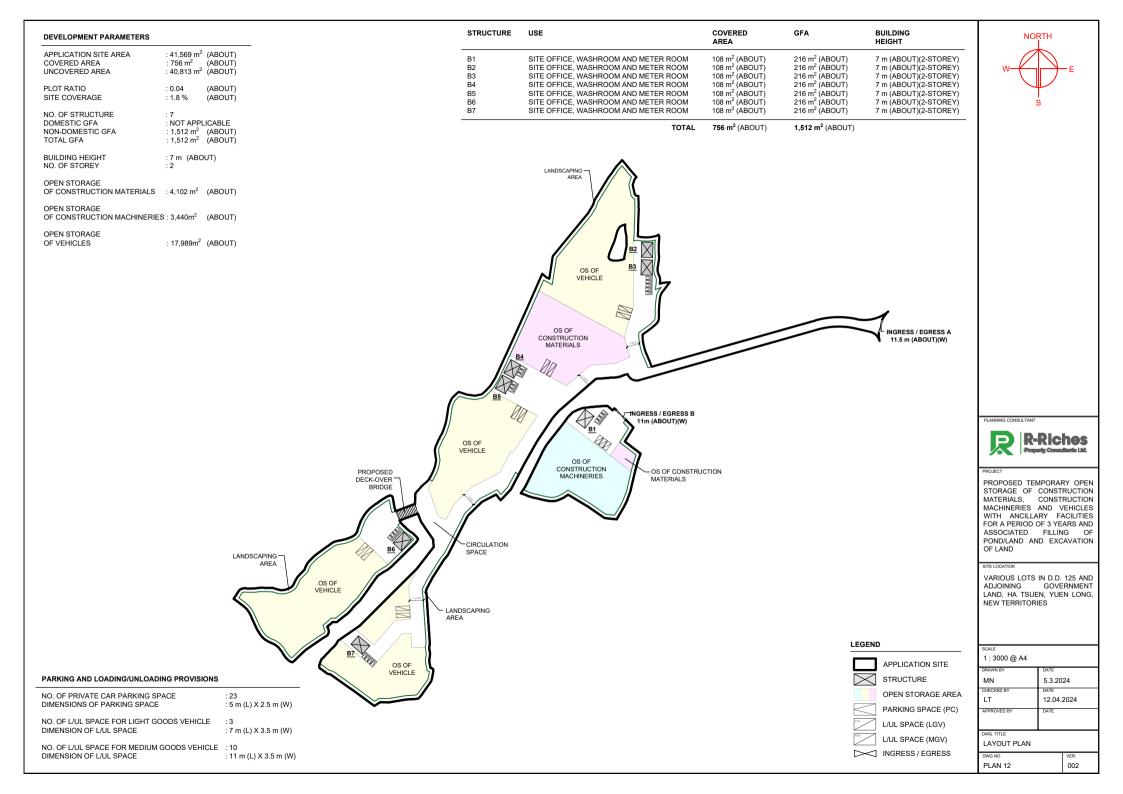
Appendix A

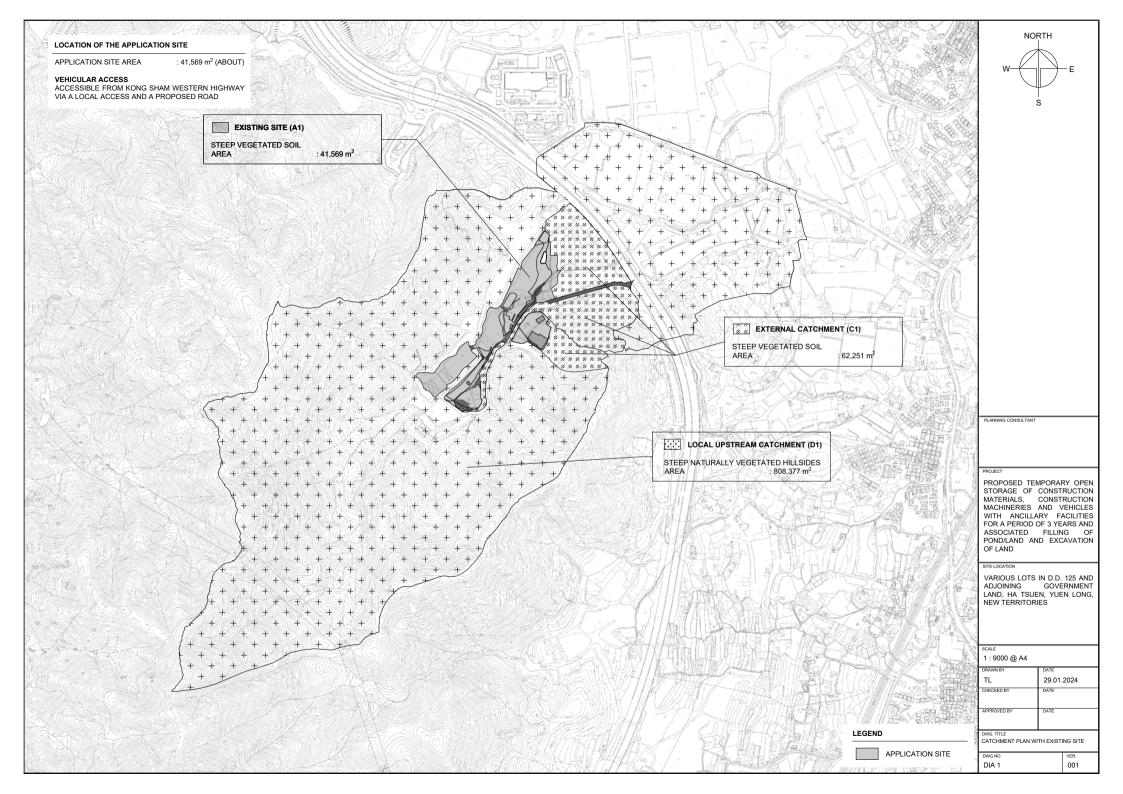
Drawings

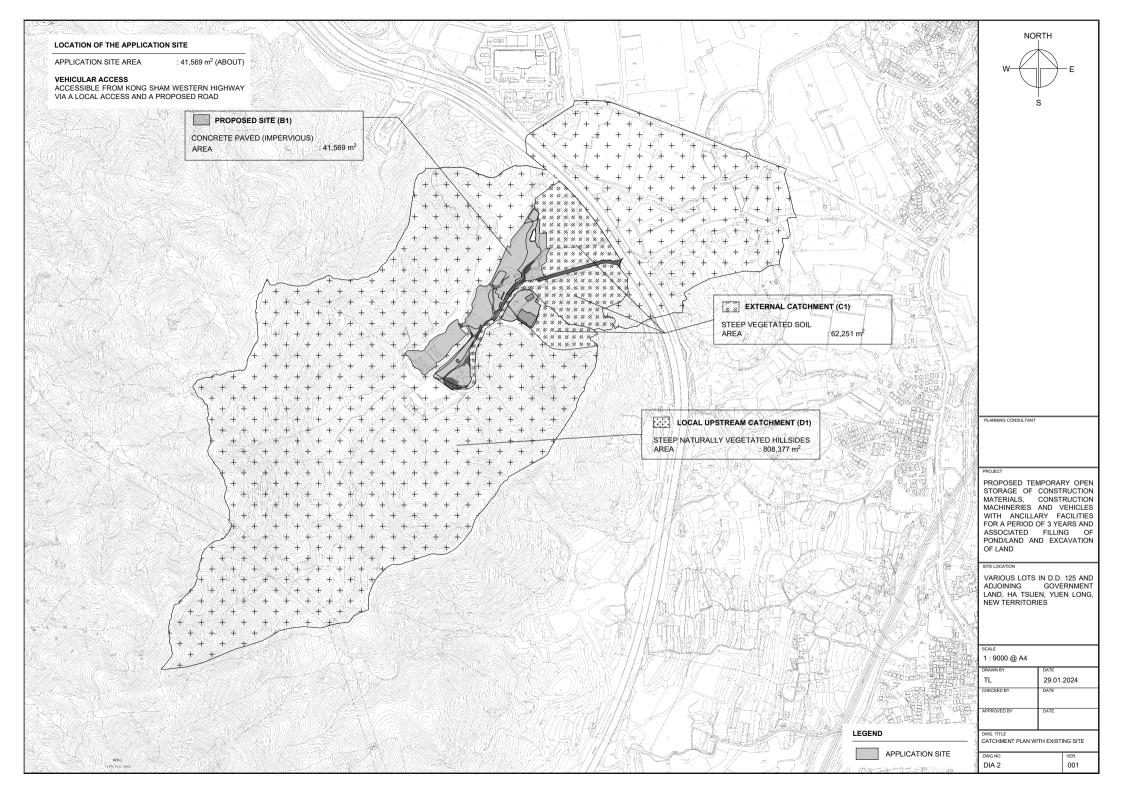


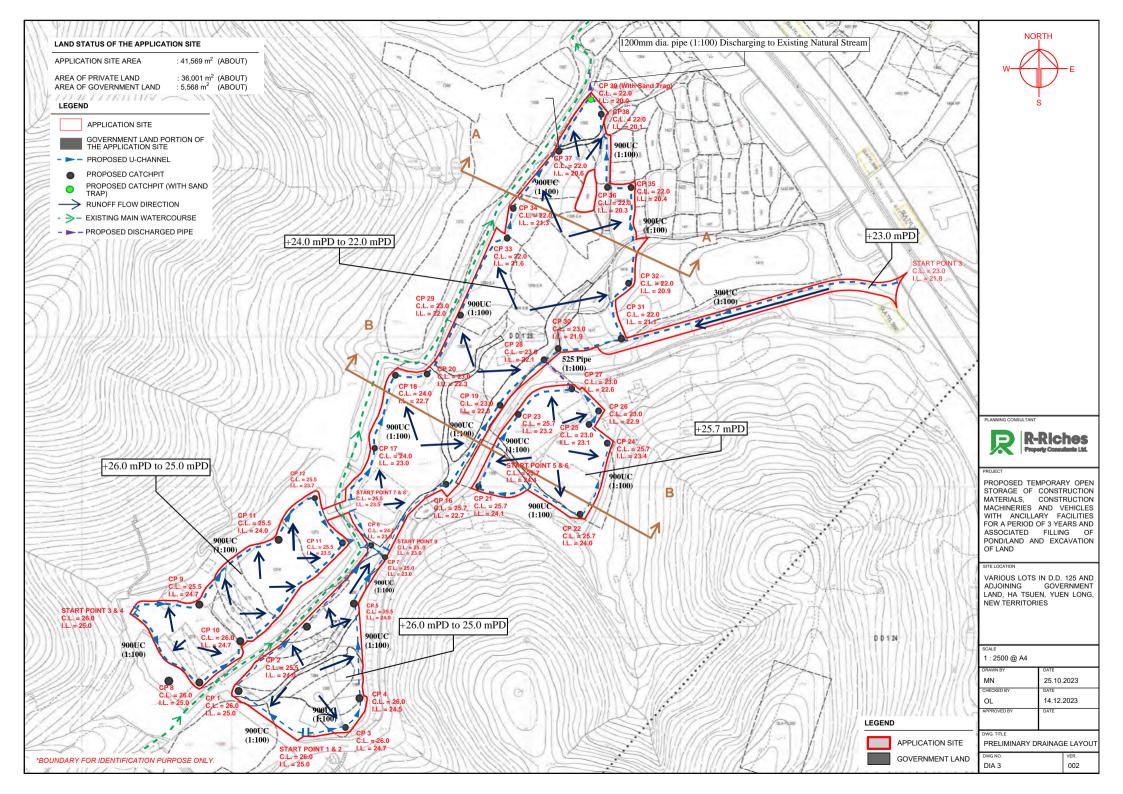


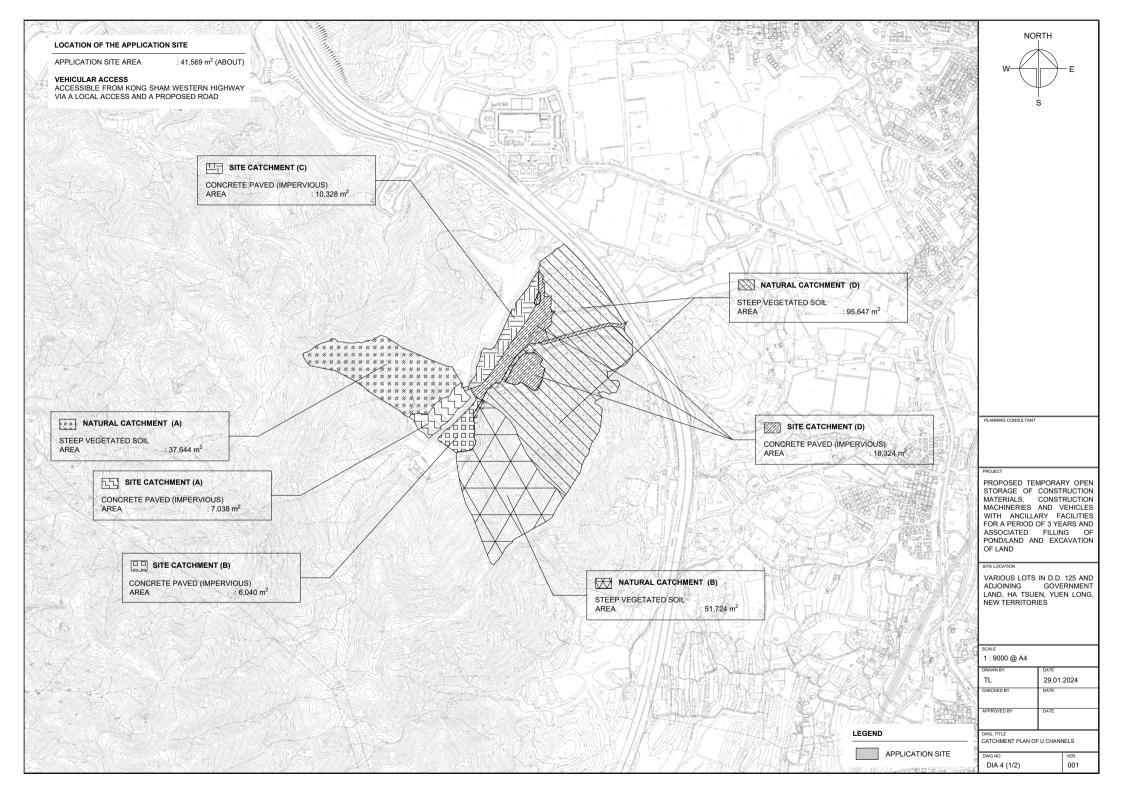














APPLICATION SITE AREA : 4

: 41,569 m² (ABOUT)

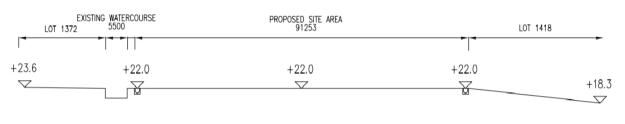
PROPOSED SITE LEVEL

: +22mPD - +26mPD (ABOUT)

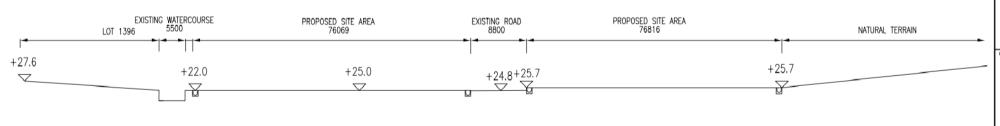
VEHICULAR ACCESS

ACCESSIBLE FROM KONG SHAM WESTERN HIGHWAY VIA A LOCAL ACCESS AND A PROPOSED ROAD





SECTION A-A



SECTION B-B

PLANNING CONSULTANT

PROJECT

PROPOSED TEMPORARY OPEN STORAGE OF CONSTRUCTION MACHINERIES AND VEHICLES WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF PONDILAND AND EXCAVATION OF LAND

SITE LOCATION

VARIOUS LOTS IN D.D. 125 AND ADJOINING GOVERNMENT LAND, HA TSUEN, YUEN LONG, NEW TERRITORIES

SCALE

DRAWN BY	DATE
TL	22.03.2024
CHECKED BY	DATE
APPROVED BY	DATE
DATE OF THE PARTY OF	

001

CROSS SECTION

DWG NO.
DIA 4 (2/2)

Appendix B

Runoff Calculations

Runoff Estimation

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM. The parameters and assumptions refer to section 3.

The local upstream catchment comprises mainly naturally vegetated hillsides; C = 0.25
The existing site comprises mainly steep vegetated soil; C = 0.25
The Proposed Site comprise Heavy Goods Vehicle Parking Space and Container Vehicle Parking Space (Concrete Paved Area (Impervious); C=0.95

Area of the Development Site = 41,569 m^2

Area of the local upstream catchment = 614983 m²
The Site is proposed to be "Proposed Temporary O

	f 3 Years a	nd Associa	ated Filling		age of Construc and Excavation		rials, Construction M	lachineries	and Vehi	cles with An	cillary Facili	ties	
Existing Site Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
41569	535	29.7	17.9	2.21	22.81	22.81	HKO headquarters	0.25 0.95	41569 0	151.98 151.98	0.44 0.00	0.439	26345
Proposed Sit	te Catchme	ent (B1)								•			
Catchment (m ²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
41569	535	26	22	0.75	28.32	28.32	HKO headquarters	0.25 0.95	0 41569	142.21 142.21	0.00 1.56	1.561	93674
External Cate	chment (C	1)		'		'				•			
Catchment (m ²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
61330	395	25	22	0.76	20.05	20.05	HKO headquarters	0.25 0.95	61330 0	158.03 158.03	0.67 0.00	0.674	40415
Existina Site	Catchmen	nt (A1) + E	xternal Ca	tchment (C1)		'				•			
Catchment (m ²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
102899	535	26	22	0.75	25.86	25.86	HKO headquarters	0.25 0.95	102899 0	146.25 146.25	1.05 0.00	1.046	62754
Proposed Sit	te Catchme	ent (B1) +	External (Catchment (C1)	•	•						
Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)

Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
102899	535	26	22	0.75	25.86	25.86	HKO headquarters	0.25	61330	146.25	0.62	2.229	133737
102033	555	20	22	0.75	25.00	20.00	i il Co ricadquarters	0.95	41569	146.25	1.61	2.223	100707

Existing Site	Catchmen	t (A1) + E	xternal Ca	tchment (C1)	+ The Local Up	stream (Catchment (D1)						
Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
911276	1473	307.7	22	19.40	29.85	29.85	HKO headquarters	0.25	911276	139.89	8.86	8.860	531597
311270	14/3	307.7		13.40	29.00	29.00	i into ricauquarters	0.95	0	139.89	0.00	0.000	001097

Proposed Sit	Proposed Site Catchment (B1) + External Catchment (C1) + The Local Upstream Catchment (D1)													
Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)	
911276	1473	307.7	22	19.40	29.85	29.85	HKO headquarters	0.25	864827	139.89	8.41	9.944	596649	
								0.95	41569	139.89	1.54			

Appendix C

Calculation of Drainage Capacity of U
Channels

Runoff Estimation of the U-channels

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM. The parameters and assumptions refer to section 3.

The local upstream catchment comprises mainly naturally vegetated hillsides; C = 0.25
The existing site comprises mainly steep vegetated soil; C = 0.25
The Proposed Site comprise Heavy Goods Vehicle Parking Space and Container Vehicle Parking Space (Concrete Paved Area (Impervious); C=0.95

The Site is proposed to be "Proposed Temporary Open Storage of Construction Materials, Construction Machineries and Vehicles with Ancillary Facilities for A Period of 3 Years and Associated Filling of Pond/Land and Excavation of Land", so check the 1 in 50-year Scenario.

Natural Catchment (A) + Site Catchment (A)

Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
44682	412	25	23	0.49	23.60	23.60	HKO headquarters	0.25	37644	150.41	0.39	0.673	40384
1.1002	712	-0		0.40	20.00	20.00	Tito noadquartors	0.95	7038	150.41	0.28	0.070	10004

Natural Catcl	hment (B) +	Site Cate	hment (B)	1									
Catchment (m ²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
57764	402	25	23	0.50	22.34	22.34	HKO headquarters	0.25	51724	152.95	0.55	0.794	47629
37704	702	20	23	0.50	22.54	22.04	Tinto neadquarters	0.95	6040	152.95	0.24	0.154	77029

Site Catchme	ent (C)									_			
Catchment (m ²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)
10328	334	23.5	20	1.05	18.99	18.99	HKO headquarters	0.25	0	160.60	0.00	0.438	26284
								0.95	10328	160.60	0.44	00	

Natural Catchment (D) + Site Catchment (D)															
Catchment (m²)	Flow Distance (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h ₁ -h ₂)/L x 100	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	tc = to + t _f (min)	Storm Constants	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA	50 year Total runoff (m³/s)	50 year Total runoff (L/min)		
113971	544	23	20	0.55	27.67	27.67	27.67	27.67	27.67 HKO headquarters	0.25	95647	143.24	0.95	1.645	98723
113971	344	20	20	0.00	21.01	21.01	Tito ricauquarters	0.95	18324	143.24	0.69	1.040	30723		

Referring to Drawing No. DIA3, 900UCs are adopted for the whole site,
The runoff generated from Natural Catchment (D) + Site Catchment (D) (Largest amount of runoff collected by the 900UC),
98723 (L/min) would be checked against the chart of rapid design of channels

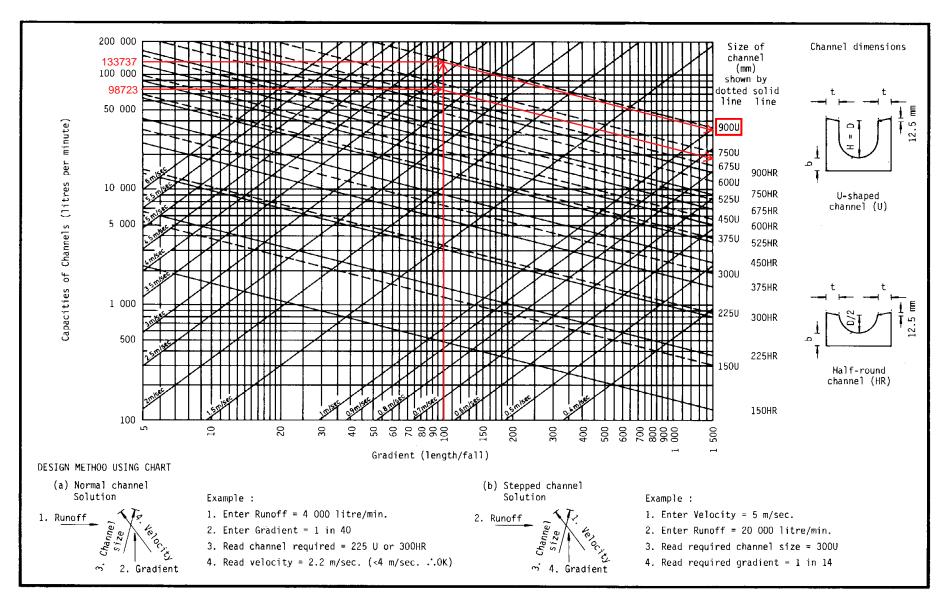


Figure 8.7 - Chart for the Rapid Design of Channels

Drainage Assessment for 1200mm Concrete Pipe discharging to Existing Open Rectangular Channel

				Gradient, S _f							
50 year Runoff Generated from the Proposed Site and External Catchment (m³/s)	Length (m)	Nominal Diameter (mm)	No. of Pipe	(%)	1 in	Velocity ⁷ (m/s)	50 year Runoff ¹ (m³/s)	Capacity (m³/s)	Utilization (%)	Pipe Capacity > Runoff ?	
2.229	6	1200	1	1.0	66.7	3.363	2.229	3.423	65%	Yes	

Mean Velocity is calculated by Colebrook- White equation

Where:

V =Mean Velocity (m/s)

R =Hydraulic Diameter (m)

Ks =Surface Roughness (m)

V =Kinematic viscosity

(kg/ms)

S =Slope of Hydraulic

Gradient

g =Gravity (m/s2)

$$\overline{V} = -\sqrt{32gRS_f} \log \left[\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$$

The Roughness Coefficient Ks is assumed to be 1.5 for Concrete (Monolithic construction against rough forms)

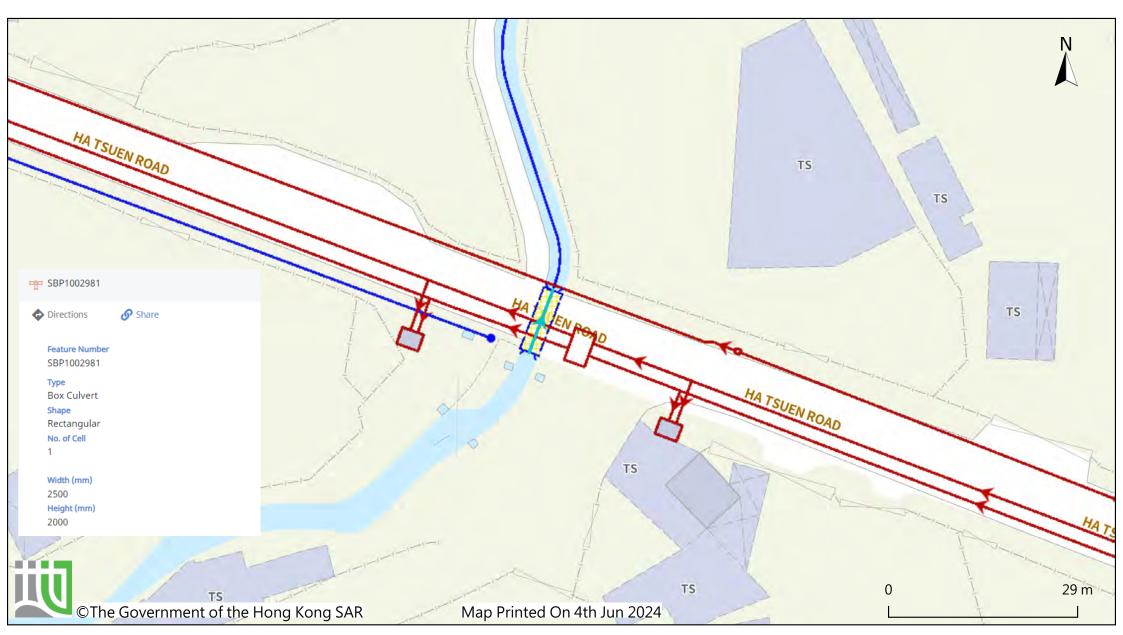
Appendix D

Calculation of Existing Open Rectangular Channel



Go to map: https://www.map.gov.hk/gm/geo:22.4460,113.9872?z=564





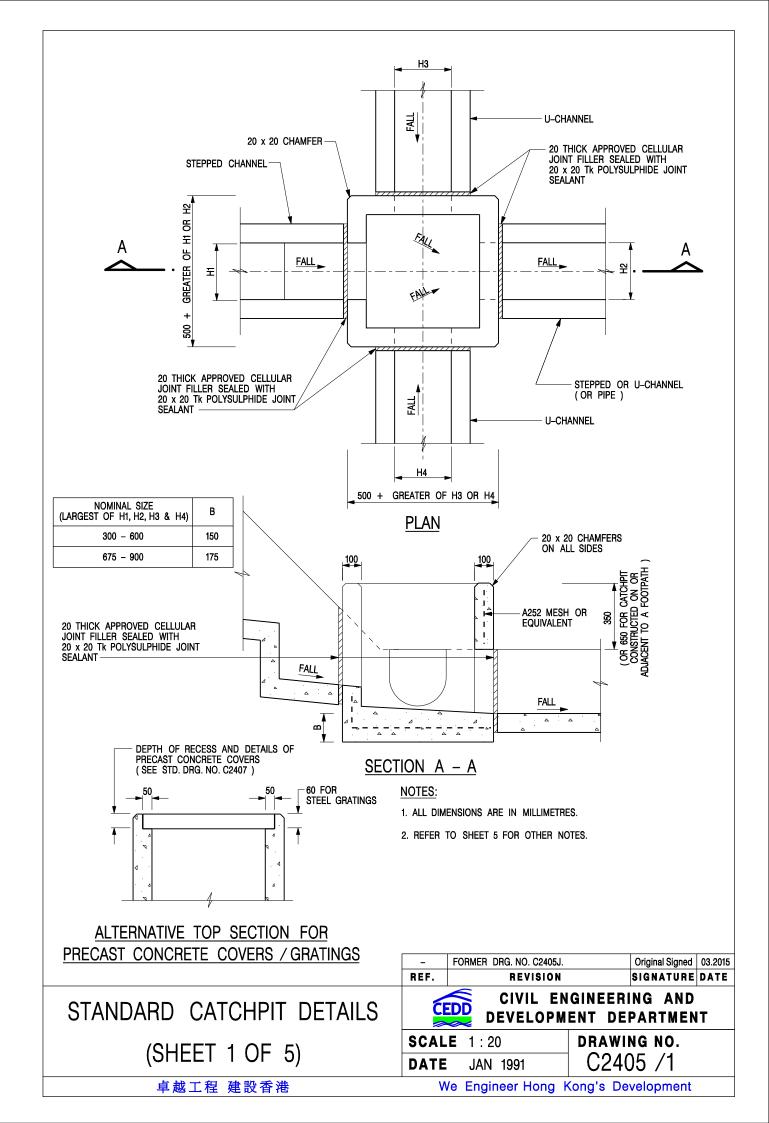
Powered by GeoInfo Map: https://www.map.gov.hk

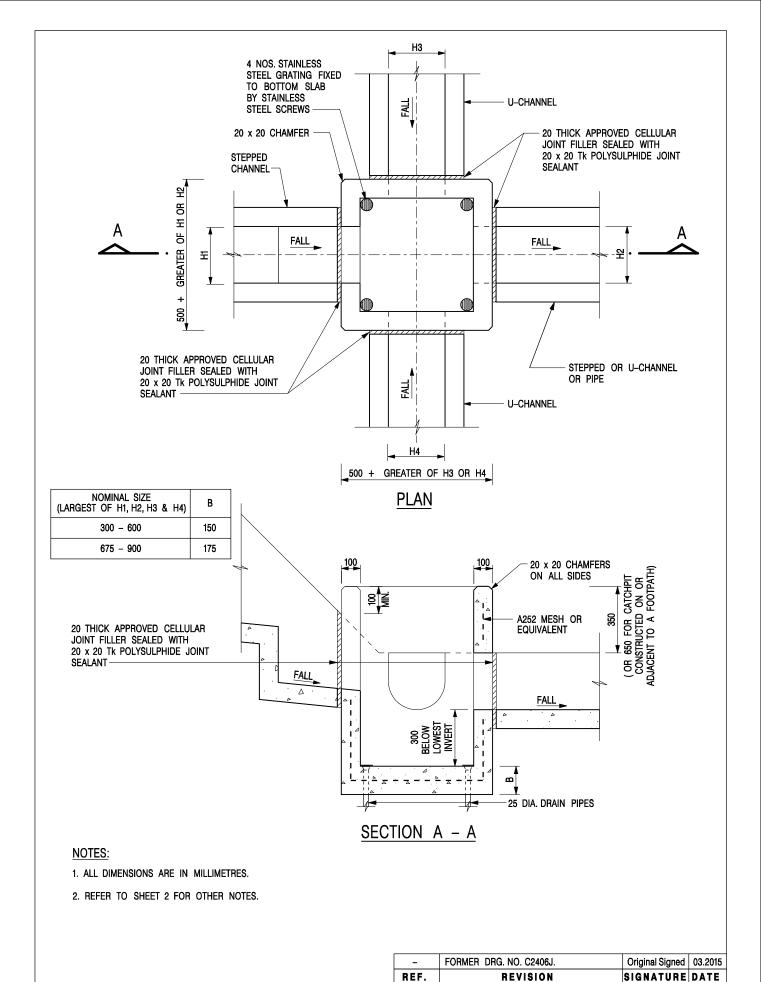
Note: The use of this map is subject to the Terms and Conditions and the IP Rights Notice of GeoInfo Map.

								Sheet No. 1	Rev. 1	
Calcul	<u>ation</u>	Sheet						Lot No. DD	125	
Job Tilte:	bb Tilte: Proposed Temporary Open Storage of Construction Materials, Construction Machineries and Vehicles with Ancillary Facilities for A Period of 3 Years and Associated Filling of Pond/Land and Excavation Of Land							-		
				Associated Filling of Pond/Land and Exo t Land, Ha Tsuen, Yuen Long, New Te		Land		Date	10/10/2023	
Chaokin	og of C				TROTICO			Date	10/10/2020	
CHECKII	ig or C	apacity of Existing R	ectangular Channe	<u> </u>						
Input Da	ata									
\^/idth of	f Eviatio	a Doctoualor Channal	- 25	m						
		ig Rectauglar Channel ng Rectauglar Channe		m						
-		enerated from the Prop								
Ü		·		m^3/s	2.0					
Flow ca	nacity	0								
i iow ca						2.5	5			
	Ω =	A x r ^{2/3}	x s 1/2	_						
	•	n		_						
where	Α	=	cross sectional a	rea of flow (m ²)	=	5	m ²			
WITEIG	A'	=		ectional area (SDM Section 9.3		5	Х	0.9		
			with 10% reduction		, =	4.5	m ²			
	r	=	hydraulic radius	(m)						
	s	=		er surface or the linear hydraulic	head los	ss (m/m)				
	n	=	Manning coefficient	ent of roughness						
Hydraul	lic radi	us								
	r	=	A'							
	р	=	wetted perimeter	(m)	=	6.50	m			
	P		wottou porimotor	(111)		0.00				
	r	=	0.69 <i>m</i>							
Slope										
Siope										
	S	=	0.010 m/m	(Gradient = 1:100)						
Mannin	~ ~~~ff	isiont of roughness								
wanning	g coem	icient of roughness								
	n	=	0.016	Fair Condition for Concrete-lin	ned chan	nels				
-										
Therefo	re,									
	Q	=	24.4559 m ³ /s	> Design flow, OK!						
				,						
Uti	lization	=	40.65%							
Free	eboard	=	1.69 m							
116	Socia	_	1.00 111							

Appendix E

Standard Details





CATCHPIT WITH TRAP (SHEET 1 OF 2)

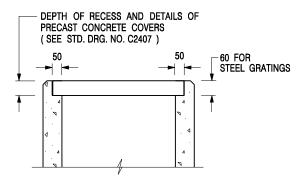
CEDD **DEVELOPMENT DEPARTMENT** SCALE 1:20 **DATE** JAN 1991

DRAWING NO. C2406 /1

CIVIL ENGINEERING AND

卓越工程 建設香港

We Engineer Hong Kong's Development



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 ℃ STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

REF.	REVISION	SIGNATURE	DATE
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
Α	MINOR AMENDMENT.	Original Signed	04.2016

CATCHPIT WITH TRAP (SHEET 2 OF 2)

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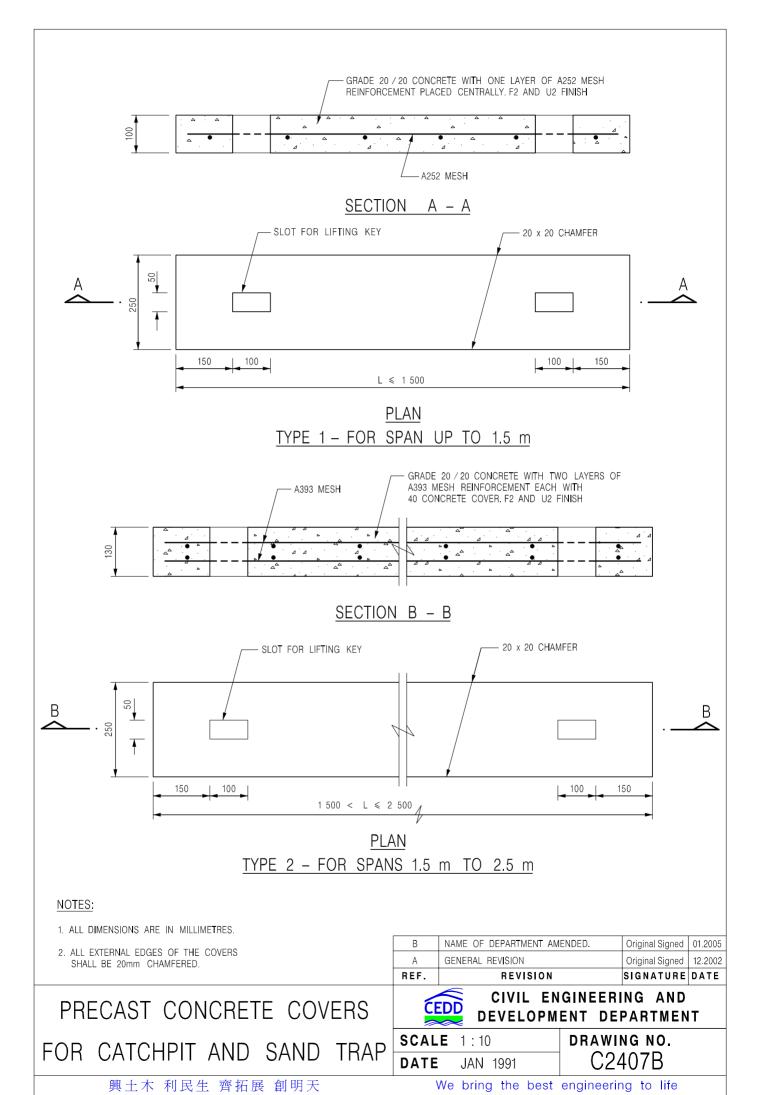


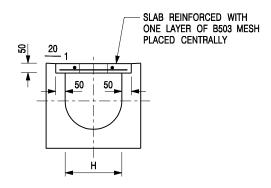
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

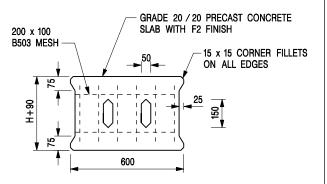
SCALE 1:20 **DATE** JAN 1991

DRAWING NO. C2406 /2A

We Engineer Hong Kong's Development





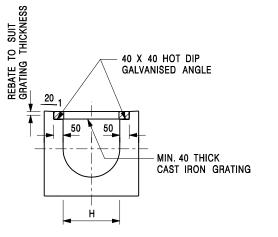


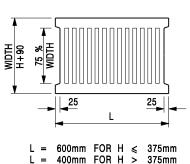
TYPICAL SECTION

PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)





TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- 3. ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

REF.	REVISION	SIGNATURE	DATE
Α	CAST IRON GRATING AMENDED.	Original Signed	12.2002
В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
D	NOTE 4 ADDED.	Original Signed	06.2008
Ε	NOTES 3 & 4 AMENDED.	Original Signed	12.2014

COVER SLAB AND CAST IRON GRATING FOR CHANNELS

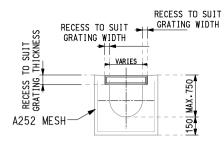


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

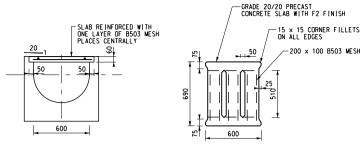
 SCALE 1:20
 DRAWING NO.

 DATE JAN 1991
 C2412E

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TYPICAL CROSS SECTION OF U-CHANNEL WITH COVER



TYPICAL SECTION

PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE STATED
- 2. DETAILS OF CATCHPIT REFER TO STANDARD DRAWING NO. C2405/1.
- 3. DETAILS OF CATCHPIT WITH TRAP STANDARD DRAWING NO. C2406/1.

PLANNING CONSULTANT

PROJECT

PROPOSED TEMPORARY OPEN STORAGE OF CONSTRUCTION MATERIALS, CONSTRUCTION MACHINERIES AND VEHICLES WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF PONDILAND AND EXCAVATION OF LAND

SITE LOCATION

VARIOUS LOTS IN D.D. 125 AND ADJOINING GOVERNMENT LAND, HA TSUEN, YUEN LONG, NEW TERRITORIES

SCALE

AS SHOWN

DRAWN BY	DATE							
TL	9.10.2023							
CHECKED BY	DATE							
APPROVED BY	DATE							

DWG. TITLE

STANDARD DETAILS OF U-CHANNEL

DWG NO. VER. SD 1 001

Appendix F

Photo Records of the Existing Water Course next to the Site

Photo Records of the Existing Water Course next to the Site

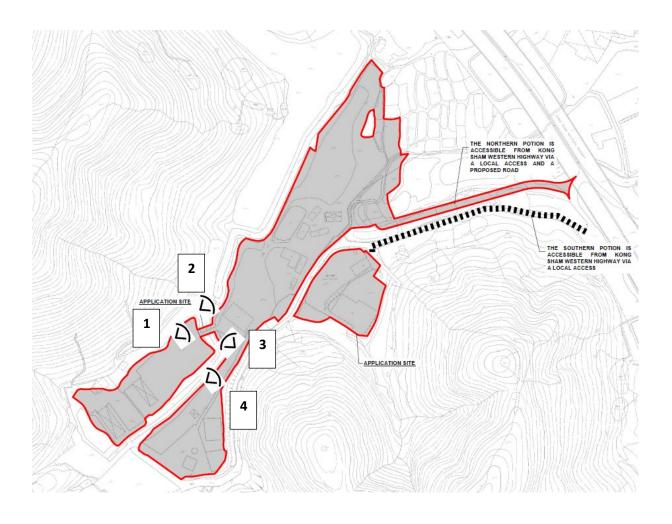


Photo Records of the Existing Water Course next to the Site

