寄件者: 寄件日期:	Louis Tse 2024年09月03日星期二 16:13
收件者:	tpbpd/PLAND
副本:	Andrea Wing Yin YAN/PLAND; Olivia Lam Yan NG/PLAND; Bon
主旨:	Tang; Matthew Ng; Christian Chim; Danny Ng; Grace Wong [Supersede][FI] S.16 Application No. A/YL-KTN/1004 - FI to address
	departmental comments
附件:	FI5 for A_YL-KTN_1004 (20240903).pdf
類別:	Internet Email

Dear Sir,

Attached herewith the FI to <u>supersede</u> our previous submissions dated <u>02/09/2024</u> (below email) to address departmental comments on the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

Louis TSE | Town Planner R-riches Group (HK) Limited

R-riches Property Consultants Limited | R-riches Planning Limited | R-riches Construction Limited

寄件者: Louis Tse 寄件日期: 2024 年 9 月 2 日 上午 11:23 收件者: Town Planning Board <tpbpd@pland.gov.hk> 副本: awyyan@pland.gov.hk <awyyan@pland.gov.hk>; olyng@pland.gov.hk <olyng@pland.gov.hk>; Bon Tang Matthew Ng · Christian Chim · Ch

主旨: [FI] S.16 Application No. A/YL-KTN/1004 - FI to address departmental comments

Dear Sir,

Attached herewith the further information to address departmental comments of the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

Louis TSE | Town Planner R-riches Group (HK) Limited

R-riches Property Consultants Limited | R-riches Planning Limited | R-riches Construction Limited



Our Ref. : DD107 Lot 1291 Your Ref. : TPB/A/YL-KTN/1004

The Secretary, Town Planning Board, 15/F, North Point Government Offices, 333 Java Road, North Point, Hong Kong



By Email

3 September 2024

Dear Sir,

5th Further Information

Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Lot 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories

(S.16 Planning Application No. A/YL-KTN/1004)

We are writing to submit further information to address departmental comments of the subject application (Appendix I).

Should you require more information regarding the application, please contact our Mr. Danny NG at (or the undersigned at your convenience. Thank you for your kind attention.

Yours faithfully,

For and on behalf of **R-riches Property Consultants Limited**

Louis TSE **Town Planner**

cc DPO/FSYLE, PlanD

8

(Attn.: Ms. Andrea YAN (Attn.: Ms. Olivia NG

email: awyyan@pland.gov.hk email: olyng@pland.gov.hk

))



Responses-to-Comments

Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Lot 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories

(Application No. A/YL-KTN/1004)

- (i) A revised plan showing the filling of land at the application site (the Site) is provided. The existing site levels range from +12.4mPD to +13.1mPD. The whole Site is proposed to be filled with concrete of not more than 0.4m, with site level ranges from +12.8mPD to +13.3mPD, in order to facilitate a flat surface for site formation and circulation area (Plan 1 and Annex I).
- (ii) A RtoC Table:

	Departmental Comments	Applicant's Responses		
1.	Comments of the Chief Engineer/Mainland	North, Drainage Services Department (CE/MN,		
	DSD)			
	(Contact Person: Mr. Terence TANG; Tel.: 2300 1257)			
(a)	The 300UC in Appendix D Section does not	Please note the 300UC in Section 1 of		
	tally with the size given in drainage	"Appendix D – Section" is UC3 which 300mm U-		
	proposal.	channel as proposed in Figure 3 (Annex II).		
		Please also note that the existing ground level		
		information is updated as per the most		
		updated available information. Please refer to		
		the page 2 of Appendix B.		
		The proposed levels are updated accordingly in		
		Figure 3, Figure 4 and Appendix D (highlighted		
		in yellow).		



Proposed operating hours 擬議營運時間 Mondays to Saturdays from 09:00 to 18:00, no operation on Sunday and public holiday					
(d)	Any vehicular acce the site/subject buildi 是否有車路通往地 有關建築物?	SS to ng? 盤/	 ✓ There is an existing access. (please indicate the street name, where appropriate) 有一條現有車路。(請註明車路名稱(如適用)) Accessible from Fung Kat Heung Road via Mei Fung Road and a local access □ There is a proposed access. (please illustrate on plan and specify the width) 有一條擬議車路。(請在圖則顯示,並註明車路的闊度) 		
(e)	 INO 否 Impacts of Development Proposal 擬議發展計劃的影響 (If necessary, please use separate sheets to indicate the proposed measures to minimise possible adverse impacts or give justifications/reasons for not providing such measures.如需要的話,請另頁註明可盡量減少可能出現不良影響的 措施,否則請提供理據/理由。) 				
(i) (ii)	Does the development proposal involve alteration of existing building? 擬議發展計劃是 否包括現有建築 物的改動? Does the development proposal involve the operation on the right? 擬議發展是否涉 及右列的工程?	Yes 是 □ No 否 ✓ Yes 是 ✓	 Please provide details 請提供詳情 (Please indicate on site plan the boundary of concerned land/pond(s), and particulars of stream diversion, the extent of filling of land/pond(s) and/or excavation of land) (請用地盤平面圖顯示有關土地/池塘界線,以及河道改道、填塘、填土及/或挖土的細節及/或範圍) Diversion of stream 河道改道 ✓ Filling of pond 填塘 Area of filling 填塘面積		
(iii)	Would the development proposal cause any adverse impacts? 擬議發展計劃會 否造成不良影 響?	On environme On traffic 對 On water supp On drainage On slopes 對 Affected by sl Landscape Im Tree Felling Visual Impact Others (Please	nt 對環境 Yes 會 □ No 不會 交通 Yes 會 □ No 不會 ly 對供水 Yes 會 □ No 不會 對排水 Yes 會 □ No 不會 對排水 Yes 會 □ No 不會 No 不會 No No 不會 No No No No No No No No		

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Lot 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Y.L., N.T.

Drainage Appraisal



Drainage Appraisal

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Drainage Appraisal

1. Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) to use Lot 1291 (Part) in D.D. 107, Fung Kat Heung, Kam Tin, Yuen Long, New Territories (the Site) for 'Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond' (Proposed Development).
- 1.1.2 This Drainage Proposal is to support the planning application for the proposed use.

1.2 The Site

- 1.2.1 The Application Site at Kam Tin North has an area of about 6,968 m². The site is currently an unused grassland with temporary structures and a small dried pond. The site location plan is shown in **Figure 1**.
- 1.2.2 The existing ground level of the site is approx. +12.4 mPD to +13.1 mPD and it is intended to fill to +12.8 mPD to +13.3 mPD. The ground level is gently falling from east to west.
- 1.2.3 There is an existing approx.. 7m width channel about 50m at the south of the site. Existing Drainage Plan and Site Photo of existing 7m width channel are shown in **Figure 2** for reference.
- 1.2.4 Proposed Development Layout plan is shown in **Appendix B** for reference.

Drainage Appraisal

2. Development Proposal

2.1 The Proposed Development

2.1.1 The total site area is approximately 6,968 m². The indicative development schedule is summarized in **Table 1** below for technical assessment purpose. Catchment plan with external catchment is shown in **Figure 4**.

Proposed Development	
Total Site Area (m ²)	6,968
Paved Area (m ²)	6,968
Assume all proposed site area as paved area	
after development for assessment purpose	
Table 1 Key Development Decemeters	

Table 1 - Key Development Parameters

3. Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this DIA. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2– Design Return Periods under SDM

3.1.2 The proposed village drainage system intended to collect runoff from the internal site and upper catchment to discharge to existing approx. 7m width channel at the south of the site. 1 in 10 years return period is adopted for the drainage design.

Drainage Appraisal

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Headquarters Rainfall Zone. Therefore, for 10 years return period, the following values are adopted.

а	=	471.9
b	=	3.02
с	=	0.397

2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278$ CiA

where	Q_p	=	peak runoff in m ³ /s
	С	=	runoff coefficient (dimensionless)
	i	=	rainfall intensity in mm/hr
	А	=	catchment area in km ²

3. The run-off coefficient (C) of surface runoff are taken as follows:

•	Paved Area:	C = 0.95
•	Unpaved Area:	C = 0.35

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

- V = velocity of the pipe flow (m/s)
- S_f = hydraulic gradient
- n = manning's coefficient
- R = hydraulic radius (m)
- 5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:

$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V	=	velocity of the pipe flow (m/s)
S _f	=	hydraulic gradient
k_{f}	=	roughness value (m)
v	=	kinematics viscosity of fluid
D	=	pipe diameter (m)
R	=	hydraulic radius (m)

Drainage Appraisal

4. Proposed Drainage System

- 4.1.1 Proposed drainage system are designed for collection of runoff from the application site and external catchment at the north-east. It is proposed to discharge to existing approx. 7m channel at south of the development. The alignment, size and gradient of the proposed drains are shown in **Figure 3**. The catchment plan is shown in **Figure 4**.
- 4.1.2 Where any hoarding or wall to be erected, 100mm separation opening from ground level to be provided along the hoarding/wall.
- 4.1.3 The design calculations of proposed drains are shown in **Appendix A**.
- 4.1.4 The reference standard drawings of drains are shown in **Appendix C**.
- 4.1.5 Design checking of existing downstream approx. 7m channel is shown in **Appendix E**.

5. Conclusion

- 5.1.1 A drainage appraisal has been conducted for the Proposed Development. The surface runoff from the Application Site will be collected by the proposed drains and discharged to the existing channel at south.
- 5.1.2 With the proposed drainage system, it is anticipated that there will be no significant drainage impact to the area after the implementation of the development.

- End of Text -

FIGURES





LEGEND:

	Combined Manhole
ъ	Overflow (Combined)
-	Pipe (Combined)
	Interface Valve Chamber
	Sewer Manhole
	Oil / Petrol Interceptor
S	Overflow (Sewer)
-	Pipe (Sewer)

н	Tapping Point (Sewer)	н	Tapping Point (Storm)
	Sewer Terminal Manhole	٥	Storm Water Terminal Manhole
•	Catchpit	7223	Tunnel Protection Zone (100m / 200m)
↦	Inlet	7223	Tunnel Protection Zone (General Range)
•	Storm Water Manhole		Tunnel / Box Culvert (Sewer)
+-(Outlet	8008	Tunnel / Box Culvert (Storm)
_	Pipe (Storm)		
-	Sand Trap	•••	EXISTING U CHANNEL







Appendix

U Channel 1 (Zone A1 + B1)						
Runoff Estimation						
Design Return Period	Т	1 in	10	vears		
Paved Area	360 + 1990 x 0.3 =		957	(m2)		
Unpaved Area	1990 x 0.7 =		1393	(m2)		. a
Total Equivalent Area	957 x 0.95 + 1393 x 0.35 =		1397	(m2)		$_{*}$ $l = \frac{1}{(t_{*} + h)^{c}}$
Rainfall Intensity, I *			206	mm/hr		(a + b)
Design Discharge Rate, Q	0.278 x 1397 x 206 / 1000000 =		0.080	m3/s		
U Channel						
Channel Size			375	(mm)		
Gradient		1 in 🔤	200			
Area	$\pi \times 0.38^2 / 8 + 0.38 \times 0.38 / 2 =$		0.126	(m2)		
Wetted Perimeter	$\pi \times 0.38 / 2 + 0.38 / 2 \times 2 =$		0.964	(m)		
R	0.126 / 0.964 =		0.130	(m)		
Velocity			1.30	m/s		
Capacity			0.163	m3/s		
Utilization	0.08 / 0.163	=	49.22	%	ОК	(less than 90%, for 10% siltation allowand
U Channel 2 (Zone [A1 + B1]	+A4)					
Runoff Estimation						
Design Return Period	1	1 in	10	vears		
Paved Area	$957 + 2243 \times 1 =$		3200	(m2)		
Unpaved Area	1393 =		1393	(m2)		a
Total Equivalent Area	$3200 \times 0.95 + 1393 \times 0.35 =$		3528	(m2)		$* i = \frac{u}{1 + 1 + 1}$
Rainfall Intensity 1*	6200 X 0100 Y 1000 X 0100 =		206	mm/hr		$(t_d + b)^c$
Design Discharge Rate, O	0 278 x 1393 x 206 / 1000000 -		0 202	m3/s		
U Channel						
Channel Size			450	(mm)		
Gradient		1 in	200			
Area	$\pi \times 0.45^2 / 8 + 0.45 \times 0.45 / 2 =$		0.181	(m2)		
Wetted Perimeter	$\pi \times 0.45 / 2 + 0.45 / 2 \times 2 =$		1.157	(m)		
R	0.181 / 1.157 =		0.156	(m)		
Velocity			1.47	m/s		
Capacity			0.265	m3/s		
Utilization	0.202 / 0.265	=	76.45	%	ОК	(less than 90%, for 10% siltation allowand
U Channel 3 (Zone B2)						
Runoff Estimation		,				
Design Return Period		1 in	10	years		
Paved Area	2366 x 0.3 =		710	(m2)		
Unpaved Area	2366 x 0.7 =		1656	(m2)		a
Total Equivalent Area	710 x 0.95 + 1656 x 0.35 =		1254	(m2)		* $i = \frac{1}{(t + b)^{c}}$
Rainfall Intensity, I *			206	mm/hr		$(l_d + b)^{\circ}$
Design Discharge Rate, Q	0.278 x 1254 x 206 / 1000000 =		0.072	m3/s		
U Channel (Half round to U)	1		000	(
U Channel (Half round to U) Channel Size	<u> </u>		300	(mm)		
U Channel (Half round to U) Channel Size Gradient		1 in	300 200	(mm)		
U Channel (Half round to U) Channel Size Gradient Area	π x 0.3 ² /8 + 0.3 x 0.3/2 =	1 in	300 200 0.080	(mm) (m2)		
U Channel (Half round to U) Channel Size Gradient Area Wetted Perimeter	$\begin{aligned} &\pi \times 0.3^{A}2 \ /8 \ + \ 0.3 \times 0.3/2 \ = \\ &\pi \times 0.3 \ / \ 2 \ + \ 0.3/2 \ \times 2 \ = \end{aligned}$	1 in	300 200 0.080 0.771	(mm) (m2) (m)		
U Channel (Half round to U) Channel Size Gradient Area Wetted Perimeter R	$\pi \times 0.3^{2}/8 + 0.3 \times 0.3/2 = \\\pi \times 0.3/2 + 0.3/2 \times 2 = \\0.08/0.771 =$	1 in	300 200 0.080 0.771 0.104	(mm) (m2) (m) (m)		
U Channel (Half round to U) Channel Size Gradient Area Wetted Perimeter R Velocity	$\begin{aligned} &\pi \times 0.3^{3} / 2 / 8 \ + \ 0.3 \times 0.3 / 2 \ = \\ &\pi \times 0.3 / 2 + 0.3 / 2 \times 2 \ = \\ &0.08 / \ 0.771 \ = \end{aligned}$	1 in	300 200 0.080 0.771 0.104 1.12	(mm) (m2) (m) (m) m/s		
U Channel (Half round to U) Channel Size Gradient Area Wetted Perimeter R Velocity Capacity	$\pi \times 0.3^{2} / 8 + 0.3 \times 0.3 / 2 = \\\pi \times 0.3 / 2 + 0.3 / 2 \times 2 = \\0.08 / 0.771 =$	1 in	300 200 0.080 0.771 0.104 1.12 0.090	(mm) (m2) (m) (m) m/s m3/s		
U Channel (Half round to U) Channel Size Gradient Area Wetted Perimeter R Velocity Capacity	$\begin{aligned} &\pi \times 0.3^{A}2 / 8 \ + \ 0.3 \times 0.3 / 2 \ = \\ &\pi \times 0.3 / 2 + 0.3 / 2 \times 2 \ = \\ &0.08 / 0.771 \ = \end{aligned}$	1 in	300 200 0.080 0.771 0.104 1.12 0.090	(mm) (m2) (m) (m) m/s m3/s		

Design Return Period Priod Area T(1 + 1755 = 1 in 10 years Uppaved Area T(0 + 1755 = 1 in 10 years Uppaved Area 1056 = 1056 (m2) 2658 (m2) years 2 default Area 100 (m2) 2658 (m2) 200 (m2) years 2 default Area 0.278 x 2921 x 208 / 1000000 = 0.118 (m2) years 2 default Area 1 m 200 (m2) years years 2 default Area 1 m 200 (m2) years years 2 default Area 1 m 200 (m2) years years 2 default Area 1 m 200 (m2) years years 2 default Area 1 m 10 (m2) years years 2 default Area 1 m 10 (m2) years years 2 default Area 1 m 10 (m2) years years 2 default Area 2 default Area 2 default Area years years 2 default Area 2 default Area 2 default Area years years 2 default Area 2 default Area	Runoff Estimation						
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Design Return Period		1 in	10	vears		
$ \begin{array}{c} 1 & 1 & 0 & 0 & 0 \\ 1 & 1 & 0 & 0 & 0 & 0 \\ 2 & 265 \times 0.55 + 1565 \times 0.35 & 2231 & (m_1^2) \\ 2 & 265 \times 0.55 + 1565 \times 0.35 & 2231 & (m_1^2) \\ 2 & 265 \times 0.55 + 1565 \times 0.35 & 2231 & (m_1^2) \\ 2 & 265 \times 0.55 + 1565 \times 0.35 & 2231 & (m_1^2) \\ 2 & 265 \times 0.55 + 1565 \times 0.35 & 2231 & (m_1^2) \\ 2 & 205 & m_1^{NB} \end{array} $ $ \begin{array}{c} t = \frac{a}{(t_a + b)^2} \\ t = \frac{a}{(t_a + b)^2$	Design Retain Fonda	710 + 1755 =		2465	(m2)		
The definition and the set of th		1656 -		1656	(m2)		<i>a</i>
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	T-t-L Suivelent Area	1050 = 2465 × 0.05 + 1656 × 0.25 =		2021	(1112)		* i = <u>u</u>
Understelling Comments of the second se	Total Equivalent Area	2405 X U.95 + 1050 X U.55 =		2921	(m∠)		$(t_d + b)^c$
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Rainfall Intensity, I*	0.070 + 2024 + 200 / 4000000		206	mm/hr		
U Channel Channel Size Gradenti Area Im 450 200 (rm) 0.181 Wited Perimeter If x 0.45/2/8 + 0.45 x 0.45/2 = 1 0.187 0.181 (rm) 0.181 Weited Perimeter 0.181/1.157 = 0.157 0.17 (rm) 0.181/1.157 = 0.17 0.161 Weited Perimeter 0.181/1.157 = 0.167 0.167 (rm) 0.265 rm 3/9 OK U Channel 5 (Zone A2 + [A3 + B2]) Runoff Estimation 0.168 / 0.265 = 6.3.1 % % OK (less than 90%, for 10% sittation allowance) U Channel 5 (Zone A2 + [A3 + B2]) Runoff Estimation 2465 + 2210 x 1 = 1 fm 10 years 4675 x 0.95 + 1656 x 0.35 = 5021 (rm2) 4675 x 0.95 + 1656 x 0.35 = 5021 (rm2) 4675 x 0.95 + 1056 x 0.35 = 5021 (rm2) 4675 x 0.95 + 1056 x 0.452 z = 0.161 (rm2) 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr , $t = \frac{a}{(t_d + b)^c}$ U Channel Channel Size Area Tx 0.45/2/8 + 0.452 x 0.452 = 1.55 (rm2) 0.181 / 1.157 (rm) 0.181 / 1.157 = 2.07 mis 2.07 mis 2	Design Discharge Rate, Q	0.278 X 2921 X 2067 1000000 =		0.168	m3/s		
U Channel Im 450 (mm) Gradenti Area $\pi \times 0.45^{1}2/8 + 0.45 \times 0.452 = 1$ 1.16 (m2) Wated Perimeter $\pi \times 0.45^{1}2/8 + 0.45 \times 0.452 = 1$ 1.16 (m2) Wondoly $0.181 / 1.15^{7} = 1$ 1.17 (m) Capacity 0.181 / 1.15^{7} = 1 1.17 (m) UBLization 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% siltation allowance) ULItation 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% siltation allowance) ULItation 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% siltation allowance) ULItation 0.465 + 2210 x 1 = 1 in 10 years Total Equivalent read 2.465 + 2210 x 1 = 1.16 106 (m2) Design Rotum Period 2.475 x 0.25 + 0.45 x 0.452 = 1.050 (mn) .17 1.050 (m) Design Rotum Period $\pi x 0.45^{1}2 / 8 + 0.45 x 0.452 = 1$ 1.060 (m2) .18 + 1.162 .18 + 1.162 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
$\begin{array}{c} \text{Latitude 12826} \\ \text{Tr} x 0.45^{1/2}/8 + 0.45 \times 0.452 = 1 \\ \text{Webs} \\ \text{Webs} \\ \text{Velocity} \\ \text{Capacity} \\ \text{Velocity} \\ \text{Capacity} \\ \text{Ullization} \\ \text{Ullization} \\ \text{Unamel Size} \\ \frac{1}{2} \times 0.45^{1/2}/8 + 0.45 \times 0.452 = 2 \\ 0.156 \\ 0.285 \\ 0.275 \\ 0.285 \\ 0.275 \\ 0.285 \\ 0.275 \\ 0.285 \\ 0.275 \\ 0.275 \\ 0.285 \\ 0.275 \\ $	U Channel			150	(
Gradient Area Rest Weaking $T \propto 0.452 \ 8 + 0.45 \ x 0.452 \ 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 2 = 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 + 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 + 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 + 0 \\ T \propto 0.457 \ 2 + 0.452 \ 2 + 0 \\ T \propto 0.457 \ 2 + 0.457 \ 2 + 0.457 \ 2 + 0.457 \ 2 + 0.457 \ 4 + 0.57 \\ T \propto 0.457 \ 2 + 0.457 \ 2 + 0.457 \ 4 + 0.$	Channel Size		4.54	450	(mm)		
Area Tr 2.0 45% 24 + 0.45 × 0.45% 2 = 0.151 (m2) Tr 2.0 45% 24 + 0.45 × 0.45% 2 = 0.157 (m) 0.161 / 1.157 = 0.156 (m) 0.265 m3/s Ublication 0.168 / 0.265 = 6.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 6.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 6.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 6.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 0.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 0.3.31 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.168 / 0.265 = 0.021 (m2) Draped Area Unpaved Area Tr 2.045 × 2.05 + 1656 x 0.35 = 0.021 (m2) Draped Area Tr 2.045 × 2.05 / 1000000 = 0.268 mm/hr Oceaned Size Gradient Area R N 0.181 / 1.157 = 0.156 (m) O.181 / 1.157 = 0.156 (m) O.181 / 1.157 = 0.156 (m) O.181 / 1.157 = 0.375 m3/s Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 0.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 0.94 % OK (less than 90%, for 10% sitiation allowance) Ublication 0.288 / 0.375 = 0.95 % 0.05 % 0.055 = 0.071 mm/hr Design Return Pendo Design	Gradient		1 in 📃	200	()		
We the definition of the set of	Area	$\pi \times 0.45^{2}/8 + 0.45 \times 0.45/2 =$	(0.181	(m2)		
R 0.181 / 1.157 = 0.156 (m) Qapacity 0.265 m34 Ublication 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) Ublication 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) Ublication 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) Ublication 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) Ublication 2465 + 2210 x 1 = 1 in 10 years (m) Uppaced Area 1056 = 1.056 (m2) 208 mm/hr Design Batum Period 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr 0.155 (m) Offamel Size 0.151 (n.157 = 0.156 (m) 0.375 (m2) 0.455 (m2) Gradent max 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% silitation allowance) Ublication 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% silitation allowance) Ublication <	Wetted Perimeter	$\pi \times 0.45 / 2 + 0.45 / 2 \times 2 =$		1.157	(m)		
Velocity 1.47 m/s Utilization 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) Utilization 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% silitation allowance) U Channel 5 (Zone A2 + [A3 + B2]) Runoff Estimation 1 10 years Paved Area 2465 + 2210 x 1 = 1 10 years Unpaved Area 2465 + 2210 x 1 = 1 100 years Table Equivalent Area 2465 + 2210 x 1 = 1 656 = (m2) Stall Equivalent Area 2465 x 0.35 = 5021 x (m2) . $l = \frac{a}{(t_d + b)^c}$ Channel Sce 1 1 0 0.181 (m2) . $l = \frac{a}{(t_d + b)^c}$ Velocity 2.07 ms 2.07 ms . $l = \frac{a}{(t_d + b)^c}$ Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siliation allowance) Utilization 0.288 / 0.375 = 787.5 (m2) . . $l = \frac{a}{(t_d + b)^c}$ Utilization	R	0.181 / 1.157 =	(0.156	(m)		
Capacity 0.265 m3/s Utilization 0.168 / 0.265 = 63.31 % OK (less than 90%, for 10% sitiation allowance) U Channel 5 (Zone A2 + [A3 + B2]) Runoff Estimation .	Velocity			1.47	m/s		
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U Channel 5 (Zone A2 + [A3 + B2]) Runoff Estimation Design Return Period Paved Area Unpaved Area Unpaved Area Channel Scharge Rate, 0 Distign Return Period Paved Area Uppaved Area Channel Size Gradent Area Wetted Perimeter R Ublization 0.288 / 0.375 = 76.54 OK (less than 90%, for 10% siltation allowance) Vilization 0.288 / 0.375 = 76.54 Vilization 0.288 / 0.375 = 76.54 Vilization 0.288 / 0.375 = 76.54 Vilization 0.288 / 0.375 = 7875 Uppaved Area Uppavel Area Using Network Period Paved Area Using Network Period Paved Area Uppavel Area Uppavel Area Uppavel Area Uppavel Area Uppave	Utilization	0.168 / 0.265	=	63.31	%	ОК	(less than 90%, for 10% siltation allowance)
O Channel S L 2010 P A2 + 153 + 52 (1) Canance 100 P A2 + 153 + 52 (1) Design Return Period Paved Area 2465 + 2210 x 1 = 1 in 10 years Unpaved Area 10656 = 11 in 10 years Rainfall Intensity, 1* 2465 + 2210 x 1 = 1666 (m2) 4675 x 0.95 + 1666 x 0.35 = 5021 (m2) Addition Area 4675 x 0.95 + 1666 x 0.35 = 5021 (m2) 206 mm/hr . i = $(t_d + b)^c$ Channel 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr . . i = $(t_d + b)^c$ Channel 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr . . . i = $(t_d + b)^c$ Channel Size 1 in 100 .	U Channel 5 (Zone A2 + [
Kurring upper diversion 1 in 10 years Paved Area 2465 + 2210 x1 = 1 in 10 years Dasign Return Period 2465 + 2210 x1 = 4675 (m2) 4675 (m2) Total Equivalent Area Ad55 x 0.95 + 1656 x 0.35 = 5021 (m2) (m2) Adfailal Intensity, 1* Design Discharge Rate, Q CZ78 x 5021 x 206 / 1000000 = 0.288 mm/hr U Channel U Channel T x 0.45 ¹ /2 + 0.45/2 x 2 = 0.181 (m2) mm/r Gradient T x 0.45 ¹ /2 + 0.45/2 x 2 = 0.181 (m2) mm/r R Q.278 x 50.4 x 0.45/2 = 0.181 (m2) mm/r Velecity Q.181 / 1.157 = Q.207 m/s Q.375 m3/s Utilization Q.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) U Channel 6 (Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) Empand Area Mm/r Paved Area Q.278 x 80.48/2 = Q.781 mm/hr Uppaved Area 4675 + 1303 = 206 (m2) mm/r e = $\frac{a}{(t_d + b)^c}$ i = $\frac{a}{(t_d + b)^c}$ UDial Equivalent Area Not Sita 200 / 1000000 = 0.781 mm/hr	Dunoff Estimation	<u>A3 + B2)</u>					
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Paved Area Draved Area Total Equivalent Area Rainal Intensity, 1* Design Discharge Rate, Q U Channel U D U Channel U D U 	Design Return Period		1 in	10	years		
Unpared Area Total Equivalent Area Raintall Intensity, I* Design Discharge Rate, Q $\frac{1656 = 4675 \times 0.95 + 1656 \times 0.35 = 5021 (m2)}{0.278 \times 50.21 \times 206 / 1000000 = 0.288 mm/hr}$ $\cdot i = \frac{a}{(t_d + b)^c}$ $\cdot i = \frac{a}{(t_d + b)^c}$ $\cdot i = \frac{a}{(t_d + b)^c}$ $t = \frac{a}{(t_d + b)^c}$	Paved Area	2465 + 2210 x 1 =		4675	(m2)		
Tatal Equivalent Area Rainal Intensity, I * Design Discharge Rate, Q	Unpaved Area	1656 =		1656	(m2)		, a
Raintall Intensity, 1* 208 mm/hr (value) Design Discharge Rate, Q 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr UChannel 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr UChannel 1 100 (mm) Area $\pi x 0.45^{52}/8 + 0.45 x 0.45/2 = 0.181 (m2) R 0.181 / 1.157 = 0.186 (m) Velocity 0.181 / 1.157 = 2.07 mis Capacity 0.375 m3/s OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Disgin Return Period 4675 + 3200 = 1 in 10 years Paved Area 7875 x 0.95 + 3049 x 0.35 = 85.48 (m2) . i = (t_d + b)^c Unpaved Area 0.278 x 85.48 x 0.068/2 = $	Total Equivalent Area	4675 x 0.95 + 1656 x 0.35 =		5021	(m2)		$* i = \frac{1}{(t+h)^c}$
Design Discharge Rate, Q 0.278 x 5021 x 206 / 1000000 = 0.288 mm/hr U Output Output Output Output Main of the second s	Rainfall Intensity, I *			206	mm/hr		$(u_d + u)^{-1}$
U Channel Channel Size T x 0.45^2/8 + 0.45 x 0.45/2 = (mm) Gradient T x 0.45^2/8 + 0.45 x 0.45/2 = (n 111) Area T x 0.45/2 + 0.45/2 x 2 = (n 181) (m2) Weted Perimeter T x 0.45/2 + 0.45/2 x 2 = (n 181) (m2) R 0.181/1.157 = 2.07 m/s Capacity 0.375 m3/s OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Upgaved Area 4675 + 3200 = 1 in 100 years 3049 (m2) 3049 (m3)<	Design Discharge Rate, Q	0.278 x 5021 x 206 / 1000000 =	(0.288	mm/hr		
Uninnel 450 (mm) Gradient 1 in 100 (mm) Gradient $\pi \times 0.45^{5/2}/8 + 0.45 \times 0.45/2 = 0.181$ (mm) Wetted Perimeter $\pi \times 0.45^{5/2}/8 + 0.45 \times 0.45/2 = 0.156$ (mm) R 0.181 / 1.157 = 0.156 (mm) 0.181 / 1.157 = 0.375 m3/s Utilization 0.288 / 0.375 = 76.94 % V Channel 5(Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) R Runoff Estimation 1 in 10 years (n2) Design Return Period 4675 + 3200 = 1 in 7875 (n2) (n2) Paved Area 1656 + 1393 = 3049 (n2) . Total Equivalent Area 7875 x 0.95 + 3049 x 0.35 = 8648 (n2) . Rinfall Intensity. 1* 0.278 x 8548 x 206 / 1000000 = 0.781 mm/hr . . U Channel C U Channel Size 1 in 200 .	11 Ob - must						
Gradient 1 in 100 1 Area $\pi \times 0.455^{12}/8 + 0.45 \times 0.45/2 = 1.157$ (m) 0.181 Wetted Perimeter $\pi \times 0.45^{12}/8 + 0.45/2 \times 2 = 1.157$ (m) 0.181 R 0.181 / 1.157 = 0.156 0.181 Velocity 0.375 m3/s Utilization 0.288 / 0.375 = 76.94 W OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 Velocity 0.375 m3/s OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Uppaved Area 4675 + 3200 = 1 in 7875 (m2) (m2) issee mm/hr issee mm/hr issee mm/hr Dial Equipalent Area 7875 x 0.95 + 3049 x 0.35 = 8548 k (m2) 0.781 mm/hr isse	Channel Size		_	450	(mm)		
Area $\pi \times 0.45^2/8 + 0.45 \times 0.45/2 = 0.181$ (m2) $\pi \times 0.45/2 + 0.45/2 \times 2 = 1.157$ (m) 0.181 / 1.157 = 0.156 (m) 0.181 / 1.157 = 0.375 m3/s Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Upper data 1 in 10 years 200 mm/hr Design Return Period 4675 + 3200 = 18775 (m2) 7875 (m2) Unpaved Area 1666 + 1393 = 3049 x 0.35 = 8548 (m2) 7875 x 0.95 + 3049 x 0.35 = 206 mm/hr $i = \frac{a}{(t_d + b)^c}$ UChannel Size T x 0.68^2/8 + 0.68 x 0.68/2 = 0.781 mm/hr $i = \frac{a}{(t_d + b)^c}$ UChannel Size T x 0.68^2/8 + 0.68 x 0.68/2 = 0.407 (m2) $i = \frac{0.234 (m)}{1.32 m/s}$ Velocity 0.407 / 1.735 = 0.234 (m) $i = 0.421 (m3)$	Gradient		1 in	100	` '		
Wetted Perimeter $\pi \times 0.45/2 + 0.45/2 \times 2 =$ 1.157 (m) R 0.181 / 1.157 = 0.156 (m) Velocity 0.375 m3/s Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Design Return Period A4 + B1] + [A2 + A3 + B2]) Runoff Estimation years years Design Return Period 1 in 10 years 7875 (m2) (m2) (less than 90%, for 10% siltation allowance) Unpaved Area 4675 + 3200 = 1 in 10 years years (less than 90%, for 10% siltation allowance) Unpaved Area 1656 + 1393 = 3049 (m2) (m2) (less than 90%, for 10% siltation allowance) Unpaved Area 7875 x 0.95 + 3049 x 0.35 = 8548 (m2) (less than 90%, for 10% siltation allowance) Urbannel 7875 x 0.95 + 3049 x 0.35 = 0.266 mm/hr . i Channel Size 1 in 200 0.407 (m2) . i i	Area	$\pi \times 0.45^{2} / 8 + 0.45 \times 0.45 / 2 =$	1	0.181	(m2)		
Interference Interference <thinterference< th=""> Interferenc <thi< td=""><td>Wetted Perimeter</td><td>$\pi \times 0.45 / 2 + 0.45 / 2 \times 2 =$</td><td></td><td>1.157</td><td>(m)</td><td></td><td></td></thi<></thinterference<>	Wetted Perimeter	$\pi \times 0.45 / 2 + 0.45 / 2 \times 2 =$		1.157	(m)		
Velocity 2.07 m/s Capacity 0.375 m/s Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Up channel 6 (Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) Runoff Estimation	R	0.181/1.157 =	i.	0.156	(m)		
Clapacity Discrete mark Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Upgated Area 1 in 10 years 7875 (m2) 7875 (m2) 98.04 99.05 99.06 99.06 90.07 <	Velocity			2.07	m/s		
Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) Utilization 0.288 / 0.375 = 76.94 % OK (less than 90%, for 10% siltation allowance) U Channel 6 (Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) Runoff Estimation . . i $a = \frac{1}{7875}$. . i $a = \frac{1}{10}$. . i $a = \frac{1}{10}$. . i $a = \frac{1}{10}$. . . $a = \frac{1}{(t_d + b)^c}$. . . $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$ $a = \frac{1}{(t_d + b)^c}$. </td <td>Capacity</td> <td></td> <td>,</td> <td>0 375</td> <td>m2/s</td> <td></td> <td></td>	Capacity		,	0 375	m2/s		
Utilization $0.288 / 0.375$ = 76.94 % OK (less than 90%, for 10% siltation allowance) U Channel 6 (Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) Runoff Estimation . . .	Сараску			0.375	1113/5		
U Channel 6 (Combined: Zone [A1 + A4 + B1] + [A2 + A3 + B2]) Runoff Estimation Design Return Period Paved Area 4675 + 3200 = 1 in 10 years Dynaved Area 4675 + 3200 = 7875 (m2) Unpaved Area 1656 + 1393 = 3049 (m2) Total Equivalent Area 7875 x 0.95 + 3049 x 0.35 = 8548 (m2) Rainfall Intensity, 1* 0.278 x 8548 x 206 / 1000000 = 0.781 mm/hr U Channel U Channel Size Gradient m x 0.682 / 8 + 0.68 x 0.68/2 = 0.407 (m2) Area m x 0.682 / 8 + 0.68 x 0.68/2 = 0.407 (m2) Wetted Perimeter m x 0.68 / 2 + 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 0.491 m3/s 0.491 m3/s	Utilization	0.288 / 0.375	=	76.94	%	OK	(less than 90%, for 10% siltation allowance)
Understand Image: Constraint of the system of	U Channel 6 (Combined: Z	Zone [<u>A1 + A4 + B1] +[A2 + A3 + B</u>	2])				
Design Return Period 1 in 10 years Paved Area 4675 + 3200 = 7875 (m2) Unpaved Area 1656 + 1393 = 3049 (m2) Total Equivalent Area 7875 x 0.95 + 3049 x 0.35 = 8548 (m2) Rainfall Intensity, I* 206 mm/hr $(t_d + b)^c$ Design Discharge Rate, Q 0.278 x 8548 x 206 / 1000000 = 0.781 mm/hr Channel Size 1 in 200 Gradient $\pi x 0.68^{n} 2/8 + 0.68 x 0.68/2 = 0.407 (m2) Wetted Perimeter \pi x 0.68/2 + 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 0.491 m3/s 1.92 m/s $	Runoff Estimation						
Paved Area $4675 + 3200 =$ 7875 (m2) Unpaved Area 1656 + 1393 = 3049 (m2) Total Equivalent Area 7875 x 0.95 + 3049 x 0.35 = 8548 (m2) Rainfall Intensity, I * 206 mm/hr 1675 (mm) Design Discharge Rate, Q 0.278 x 8548 x 206 / 1000000 = 0.781 mm/hr U Channel Size 1 in 200 Gradient 1 in 200 Area $\pi x 0.68^{4}/8 + 0.68 x 0.68/2 = 0.407 (m2) Wetted Perimeter \pi x 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 0.491 m3/s $	Design Return Period		1 in	10	years		
Unpaved Area 1656 + 1393 = 3049 (m2) Total Equivalent Area 7875 x 0.95 + 3049 x 0.35 = 8548 (m2) Rainfall Intensity, 1* 206 mm/hr Design Discharge Rate, Q 0.278 x 8548 x 206 / 1000000 = 0.781 mm/hr U Channel Channel Size Gradient 1 in 200 Area $\pi x 0.68^2/8 + 0.68 x 0.68/2 = 0.407 (m2) Wetted Perimeter \pi x 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 0.491 m3/s 1.92 m/s $	Paved Area	4675 + 3200 =		7875	(m2)		
Total Equivalent Area $7875 \times 0.95 + 3049 \times 0.35 =$ 8548 $(m2)$ Rainfall Intensity, 1* 206 mm/hr Design Discharge Rate, Q $0.278 \times 8548 \times 206 / 1000000 =$ 0.781 mm/hr U Channel Channel Size Gradient 1 in 200 Area $\pi \times 0.68^{n}/2 / 8 + 0.68 \times 0.68/2 =$ 0.407 (m2) Wetted Perimeter $\pi \times 0.68 / 2 + 0.68 / 2 \times 2 =$ 1.735 (m) R $0.407 / 1.735 =$ 0.234 (m) Velocity 0.491 m3/s m3/s	Unpaved Area	1656 + 1393 =		3049	(m2)		а
Note required intensity, 1* 206 mm/hr Design Discharge Rate, Q $0.278 \times 8548 \times 206 / 1000000 =$ 0.781 mm/hr U Channel 0.278 × 8548 × 206 / 1000000 = 0.781 mm/hr Gradient 1 in 200 Area $\pi \times 0.68^{\circ}2 / 8 + 0.68 \times 0.68 / 2 =$ 0.407 (m2) Wetted Perimeter $\pi \times 0.68 / 2 + 0.68 / 2 \times 2 =$ 1.735 (m) R $0.407 / 1.735 =$ 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Total Equivalent Area	$7875 \times 0.95 + 3049 \times 0.35 =$		8548	(m2)		$* i = \frac{1}{(1-i)}$
Unit of the sty, 1 200 mm/h Design Discharge Rate, Q $0.278 \times 8548 \times 206 / 1000000 =$ 0.781 mm/h U Channel Size 675 (mm) Gradient 1 in 200 1 Area $\pi \times 0.68^2 / 8 + 0.68 \times 0.68 / 2 =$ 0.407 (m2) Wetted Perimeter $\pi \times 0.68 / 2 + 0.68 / 2 \times 2 =$ 1.735 (m) R $0.407 / 1.735 =$ 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Painfall Intensity 1*	1010 X 0.00 1 0040 X 0.00 =		206	mm/hr		$(t_d + b)^c$
U Channel 0.270 × 8040 × 200 / 100000 = 0.701 mm/m Channel Size 1 in 200 Gradient 1 in 200 Area $\pi \times 0.68^2 / 8 + 0.68 \times 0.68 / 2 = 0.407 (m2) Wetted Perimeter \pi \times 0.68 / 2 + 0.68 / 2 × 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s $	Docian Discharge Rate O	0 278 x 8548 x 206 / 1000000 -	,	0 781	mm/hr		
U Channel 675 (mm) Gradient 1 in 200 Area $\pi \times 0.68^{A}2/8 + 0.68 \times 0.68/2 = 0.407$ (m2) Wetted Perimeter $\pi \times 0.68/2 + 0.68/2 \times 2 = 1.735$ (m) R $0.407/1.735 = 0.234$ (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Design Discharge Nate, Q	0.270 x 0340 x 2007 1000000 -		0.701	1111/111		
Channel Size 675 (mm) Gradient 1 in 200 Area π x 0.68 ² /8 + 0.68 x 0.68/2 = 0.407 Wetted Perimeter π x 0.68/2 x 2 = 1.735 R 0.407 / 1.735 = 0.234 Velocity 1.92 m/s Capacity 0.491 m3/s	II Channel				i		
Channel Size1 in200Gradient1 in200Area $\pi \times 0.68^{A}2/8 + 0.68 \times 0.68/2 = 0.407$ (m2)Wetted Perimeter $\pi \times 0.68/2 + 0.68/2 \times 2 = 1.735$ (m)R $0.407/1.735 = 0.234$ (m)Velocity1.92 m/sCapacity0.491 m3/s				075	(mm)		
Gradient $\pi x 0.68^{\prime} 2/8 + 0.68 x 0.68/2 = 0.407$ (m2) Area $\pi x 0.68^{\prime} 2/8 + 0.68 x 0.68/2 = 0.407$ (m2) Wetted Perimeter $\pi x 0.68/2 x 2 = 0.735$ (m) R $0.407/1.735 = 0.234$ (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Channel Size		4 in	675	(11111)		
Area π x 0.68*2/8 + 0.68 x 0.68/2 = 0.407 (mz) Wetted Perimeter π x 0.68/2 + 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s	A		1 in 🔛	200	(
Wetted Perimeter π x 0.68 / 2 + 0.68/2 x 2 = 1.735 (m) R 0.407 / 1.735 = 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Gradient			0 407	(m2)		
R 0.407 / 1.735 = 0.234 (m) Velocity 1.92 m/s Capacity 0.491 m3/s	Gradient Area	$\pi \times 0.68^{2}/8 + 0.68 \times 0.68/2 =$	1				
Velocity 1.92 m/s Capacity 0.491 m3/s	Gradient Area Wetted Perimeter	$\pi \times 0.68^2 / 8 + 0.68 \times 0.68 / 2 = \pi \times 0.68 / 2 + 0.68 / 2 \times 2 =$		1.735	(m)		
Capacity 0.491 m3/s	Gradient Area Wetted Perimeter R	$\begin{aligned} &\pi \times 0.68^{*}2 / 8 \ + \ 0.68 \times 0.68 / 2 \ = \\ &\pi \times 0.68 / 2 \ + \ 0.68 / 2 \ \times 2 \ = \\ &0.407 / \ 1.735 \ = \end{aligned}$		1.735 0.234	(m) (m)		
	Gradient Area Wetted Perimeter R Velocity	$\begin{array}{l} \pi \times 0.68^{*}2 / 8 \ + \ 0.68 \times 0.68 / 2 \ = \\ \pi \times 0.68 / 2 \ + \ 0.68 / 2 \ \times 2 \ = \\ 0.407 / 1.735 \ = \end{array}$		1.735 0.234 1.92	(m) (m) m/s		
	Gradient Area Wetted Perimeter R Velocity Capacity	$\begin{array}{l} \pi \times 0.68^{*}2 \ / 8 \ + \ 0.68 \times 0.68 \ / 2 \ = \\ \pi \times 0.68 \ / \ 2 \ + \ 0.68 \ / \ 2 \ = \\ 0.407 \ / \ 1.735 \ = \\ \end{array}$		1.735 0.234 1.92 0. <u>491</u>	(m) (m) m/s m3/s		
	Sradient rea Vetted Perimeter c elocity <u>apacity</u> tilization	π x 0.68 ² /8 + 0.68 x 0.68/2 = π x 0.68 / 2 + 0.68/2 x 2 = 0.407 / 1.735 = 0.781 / 0.491	(1.735 0.234 1.92 0.491 62.83	(m) (m) m/s m3/s	ОК	(less than 90%, for 10% siltation allowance





Appendix C - Reference Drawings





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	A	MINOR AMENDMENT.	Original Signed 04.2016			
	REF.	FORMER DRG. NO. C2406J. REVISION	Original Signed 03.2015 SIGNATURE DATE			
CATCHPIT WITH TRAP	C	CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT				
(SHEET 2 OF 2)	SCAL DATE	E 1:20 JAN 1991	drawing no. C2406 /2A			
卓越工程 建設香港	V	/e Engineer Hong	Kong's Development			















Time of Concentration for Catchement of Existing

Catalanant	Flow Distance	Highest Lowest		Gradient (per 100m)	to (min) =	tc =
Catchment		Level	Level	= (H1-H2)/L x 100	0.14465L/ (H ^{0.2} A ^{0.1})	to + tf
А	L			Н		
(m2)	(m)	(mPD)	(mPD)		(min)	(min)
1076839.86	1851	466	10	24.635	35.172	35.172



