寄件者: Louis Tse

寄件日期: 2024年12月27日星期五 17:22

收件者: tpbpd/PLAND

副本: Andrea Wing Yin YAN/PLAND; Jet Sze Jet CHEUNG/PLAND; Bon

Tang; Matthew Ng; Christian Chim; Danny Ng; Grace Wong; Kevin

Lam

主旨: [FI] S.16 Application No. A/YL-KTN/1023 - FI to address

departmental comments

附件: FI2 for A_YL-KTN_1023 (20241227).pdf

類別: Internet Email

Dear Sir,

Attached herewith the **FI** to address departmental comments of the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

Louis TSE | Town Planner R-riches Group (HK) Limited

R-riches Property Consultants Limited | R-riches Planning Limited | R-riches Construction Limited



Our Ref. : DD107 Lot 1512 & VL Your Ref. : TPB/A/YL-KTN/1023 顧問有限公司 **盈卓物業**

The Secretary,
Town Planning Board,
15/F, North Point Government Offices,
333 Java Road,
North Point, Hong Kong

By Email

27 December 2024

Dear Sir,

2nd Further Information

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone,

<u>Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Yuen Long</u>

(S.16 Planning Application No. A/YL-KTN/1023)

We are writing to submit further information to address departmental comments on the subject application (Appendix I).

Should you require more information regarding the application, please contact our Mr. Danny NG at or the undersigned at your convenience. Thank you for your kind attention.

Yours faithfully,

For and on behalf of

R-riches Property Consultants Limited

Louis TSE

Town Planner

cc DPO/FSYLE, PlanD

(Attn.: Ms. Andrea YAN

email: awyyan@pland.gov.hk

(Attn.: Mr. Jet CHEUNG

email: jsjcheung@pland.gov.hk)

Responses-to-Comments

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Yuen Long

(Application No. A/YL-KTN/1023)

(i) A RtoC Table:

	Departmental Comments	Applicant's Responses
1. (Comments of the Director of Fire Services (D of	FS)
(Contact Person: Mr. CHEUNG Wing-hei; Tel.:27	733 7737)
(a)	Based on the submitted FSI proposal, I have the following comments:	Noted and revised accordingly, please refer to the revised fire service installations (FSIs) proposal for details (Annex I).
	Schematic drawings will not be vetted at this stage and shall be omitted;	proposarior details (Almex 1).
(b)	Modified hose reel system shall be provided in accordance with the Code of Practice for Minimum Fire Service Installations and Equipment 2022;	
(c)	Sufficient directional and exit signs shall be provided in accordance with BS 5266-1:2016 and the FSD Circular Letter No. 5/2008;	
(d)	Justifications for the non-provision of smoke extraction system to Structure B1, i.e. "Smoke extraction system shall not be provided to structure B1 as the aggregate area of openable windows exceeds 6.25% of the floor area of the structure B1" shall be specified in the FS Notes 8.1;	
(e)	The sprinkler inlets shall be positioned in a prominent position on the exterior of the building;	
(f)	Based on the proposed access route, it is noted that the nearest available street fire hydrant is more than 500m away from the site. In this regard, a street fire hydrant	



system with adequate flow, pressure and size of water tank shall be provided.

2. Comments of the Chief Engineer/Mainland North, Drainage Services Department (CE/MN, DSD)

(Contact Person: Mr. Terence TANG; Tel.: 2300 1257)

(a) The DIA should be signed and certified by a qualified engineer (Registered Professional Engineer in the Civil Engineering discipline) before it is submitted to DSD for comment.

A revised drainage impact assessment report, which signed and certified by the qualified engineer, has been submitted by the applicant to review the drainage arrangements for the proposed development (Annex II).

3. Comments of the District Planning Officer/Fanling, Sheung Shui and Yuen Long East, Planning Department (DPO/FSYLE, PlanD)

(Contact Person: Ms. Andrea YAN; Tel.: 3168 4049)

(a) While the applicant claimed that the open space at the Site is to accommodate the required machineries, equipment, parking and loading/unloading facilities, more justifications/elaborations should be provided to demonstrate why an extensive open space (about 64%) is needed for a warehouse development.

The remaining uncovered area is reserved for the provision of drainage and fire safety facilities. The open area is necessary to comply with fire safety regulations and allows for safe workers pathways and emergency exits, providing clear access for emergency services.

4. Comments of the Chief Town Planner/Urban Design and Landscape, Planning Department (CTP/UD&L, PlanD)

(Contact Person: Mr. Samuel HUI; Tel.: 3565 3957)

(a) Having reviewed the Further Information, it is noted that the filling material in the eastern portion of the Site is revised from concrete to soil. 7 nos. of new trees are proposed along the southern site boundary within the Site. Please find our comments from landscape planning perspective:

Landscape Proposal (ver.001) (Annex IV)

Considering the species proposed (*Ficus microcarpa*) is large tree species that can grow up to approx. 25m high with wide crown spread and aggressive roots. The applicant is advised to revise the proposed

A revised landscape proposal has been submitted by the applicant to provide landscape mitigation measure for the proposed development (**Annex III**). A total of <u>7</u> Small tree species (i.e. *Polyspora Axillaris*), with continuous soil trench, are proposed to be planted along the southwest portion of the Site as a landscape buffer to the surrounding areas. All these new trees within the Site will be maintained by the applicant during the planning approval period.



S.16 Planning Application No. A/YL-KTN/1023

	species to small tree species to suit the site	
	context.	
(b)	The applicant is advised to revise the	
	individual tree pits proposed to a continuous	
	soil trench to facilitate growing of tree as far	
	as possible. Relevant description and	
	dimension should be reviewed.	
5. (Comments of the Director of Agriculture, Fishe	ries and Conservation (DAFC)
(Contact Person: Ms. WONG Cheuk-ling; Tel.: 2	150 6933)
(a)	The applicant has not provided information	Loam would be used for filling of land and
	on the type of soil to be filled.	pond for the proposed development.



FIRE SERVICES NOTES

- 1. HOSE REEL SYSTEM
- 1.1 MODIFIED HOSE REEL SYSTEM SHALL BE PROVIDED IN ACCORDANCE WITH THE CODE OF PRACTICE FOR MIN. FIRE SERVICE INSTALLATIONS & EQUIPMENT 2022.
- 1.2 WATER SUPPLY FOR THE MODIFIED HOSE REEL SYSTEM TO BE SINGLE END FEED FROM THE GOVERNMENT TOWN MAIN.
- 1.3 NO FIRE SERVICES INLET TO BE PROVIDED FOR THE MODIFIED HOSE REEL SYSTEM.
- 1.4 A MODIFIED HOSE REEL SYSTEM OF 2,000 LITRES WATER TANK TO BE PROVIDED FOR THE PREMISES AS INDICATED ON PLAN.
- 1.5 TWO HOSE REEL PUMPS (ONE DUTY & ONE STANDBY) SHALL TO BE PROVIDED AT FS PUMP ROOM.
- 1.6 SUFFICIENT HOSE REELS SHALL BE PROVIDED TO THE PREMISES AS INDICATED ON PLANS. HOSE REELS SHALL BE PROVIDED TO ENSURE THAT EVERY PART OF THE BUILDING CAN BE REACHED BY A LENGTH OF NOT MORE THAN 30 M OF HOSE REEL TUBING. ONE ACTUATING POINT AND ONE AUDIO WARNING DEVICE TO BE LOCATED AT EACH HR POINT.

2. SPRINKLER SYSTEM

- 2.1 THE CLASSIFICATION OF THE AUTOMATIC SPRINKLER INSTALLATION TO BE ORDINARY HAZARD GROUP 3.
- 2.2 AUTOMATIC SPRINKLER SYSTEM SHALL SUPPLIED BY A 135,000L SPRINKLER WATER TANK AND COVERED TO THE ENTIRE WAREHOUSE IN ACCORDANCE WITH LPC RULES INCORPORATING BS EN12845: 2015 AND FSD CIRCULAR LETTER 5/2020. THE SPRINKLER WATER TANK, SPRINKLER PUMP ROOM, SPRINKLER INLET AND SPRINKLER CONTROL VALVE GROUP SHALL BE AS INDICATED ON PLANS.
- 2.3 ALL INSTALLED SPRINKLER SHOULD BE PENDENT TYPE AND THE TEMPERATURE RATING OF SPRINKLER HEAD SHALL BE 68°C UNLESS OTHERWISE SPECIFIED.
- 2.4 ALL INSTALLED SPRINKLER SHOULD BE CONVENTIONAL TYPE AND THE TEMPERATURE RATING OF SPRINKLER HEAD SHALL BE 68°C UNLESS OTHERWISE SPECIFIED.
- 2.5 ALL SPRINKLER PIPE SIZE SHOULD BE Ø32MM UNLESS SPECIFY.
- 2.6 ALL SPRINKLER PIPE SIZE SHOULD BE Ø32MM UNLESS SPECIFY.
- 2.7 STORAGE BLOCK SHOULD BE SEPARATED BY AISLES NO LESS THAN 2.4M WIDE.
- 2.8 THE MAXIMUM STORAGE AREA SHALL BE 50m2 FOR ANY SINGLE BLOCK.
- 2.9 TYPE OF STORAGE METHOD FOR THE BUILDING IS AS FOLLOWS:
- i) STORAGE CATEGORY: CATEGORY (III)
- ii) STORAGE HEIGHT: NOT EXCEEDING 2.1M
- iii)STORAGE: ST1

3. FIRE ALARM SYSTEM

- 3.1 FIRE ALARM SYSTEM SHALL BE PROVIDED THROUGHOUT THE ENTIRE COVERED AREA OF WAREHOUSE IN ACCORDANCE WITH BS 5839-1: 2017 AND FSD CIRCULAR LETTER 6/2021. ONE ACTUATING POINT AND ONE AUDIO WARNING DEVICE SHOULD BE LOCATED AT EACH HOSE REEL POINT. THE ACTUATION POINT SHOULD INCLUDE FACILITIES FOR HOSE REEL PUMP START AND AUDIO / VISUAL WARNING DEVICE INITIATION.
- 3.2 AN ADDRESSABLE TYPE FIRE ALARM PANEL TO BE PROVIDED AND LOCATED IN FRONT OF THE MAIN ENTRANCE OF WAREHOUSE ON G/F.
- 3.3 AN ADDRESSABLE TYPE FIRE ALARM PANEL TO BE PROVIDED AND LOCATED IN FRONT OF THE MAIN ENTRANCE OF WAREHOUSE ON G/F.
- 4. EMERGENCY LIGHTING
- 4.1 SUFFICIENT EMERGENCY LIGHTING SHALL BE PROVIDED THROUGHOUT THE COVERED AREA OF WAREHOUSE IN ACCORDANCE WITH BS 5266-1:2016 AND BS EN 1838:2013 AND FSD CIRCULAR LETTER 4/2021.
- 4.2 SELF-CONTAINED TYPE EMERGENCY LIGHTING SYSTEM COMPLYING WITH H.K.F.S.D.'S CODE OF PRACTICE AS WELL AS BS 5266-1: 2011 + BS EN 1838: 2013 WILL BE PROVIDED, AND PERMANENTLY MAINTAINED IN EFFECTIVE WORKING ORDER FROM NORMAL SUPPLY & TO BE PROVIDED.
- 4.3 EMERGENCY LIGHTING SHALL BE PROVIDED THROUGHOUT THE ENTIRE WAREHOUSE AND ALL EXIT ROUTES LEADING TO EXIT OF BUILDING.
- 5. EXIT SIGN
- 5.1 SUFFICIENT DIRECTIONAL & EXIT SIGNS SHALL BE PROVIDED IN ACCORDANCE WITH BS5266-1:2016 & THE FSD CIRCULAR LETTER NO. 5/2018.
- 5.2 SELF-CONTAINED TYPE DIRECTIONAL AND EXIT SIGNS TO ENSURE THAT ALL EXIT ROUTES FROM ANYWHERE WITHIN THE WAREHOUSE ARE CLEARLY INDICATED AS REQUIRED BY THE CONFIGURATION OF EXIT ROUTE SERVING THE BUILDING.
- 6. EMERGENCY GENERATOR
- 6.1 <u>NO</u> EMERGENCY GENERATOR TO BE PROVIDED FOR SERVING THE EMERGENCY POWER. A.C. SUPPLY SOURCE WITH SECONDARY SUPPLY SHALL FEED BEFORE MAIN SWITCH.
- 6.2 DUPLICATED POWER SUPPLIES FOR ALL FIRE SERVICES INSTALLATIONS COMPRISING A CABLE CONNECTED FROM ELECTRICITY MAINS DIRECTLY BEFORE THE MAIN SWITCH.

7 PORTABLE HAND-OPERATED APPROVED APPLIANCE

7.1 PORTABLE FIRE EXTINGUISHER WITH SPECIFIED TYPE AND CAPACITY TO BE PROVIDED AT LOCATIONS AS INDICATED ON PLANS.

8 STATIC OR DYNAMIC SMOKE EXTRACTION SYSTEM

8.1 SMOKE EXTRACTION SYSTEM SHALL NOT BE PROVIDED AS THE AGGREGATE AREA OF OPENABLE WINDOWS OF THE COMPARTMENT SHALL PROVIDE MORE THAN 6.25% OF THE FLOOR AREA OF THAT COMPARTMENT.

9 VENTILATION/AIR CONDITIONING CONTROL SYSTEM

9.1 WHEN A VENTILATION/ AIR CONDITIONING CONTROL SYSTEM TO A BUILDING IS PROVIDED, IT SHALL STOP MECHANICALLY INDUCED AIR MOVEMENT WITHIN A DESIGNATED FIRE COMPARTMENT.

LEGEND (FOR LAYOUT PLAN)

HOSE REEL W/ LOCKABLE GLASS FRONTED NOZZLE BOX, STRIKER, C/W FIRE ALARM BELL & BREAK GLASS UNIT

-) 150mm FIRE ALARM BELL
- BREAK GLASS UNIT
- SPRINKLER HEAD
- © FLOW SWITCH
 - MONITORED GATE VALVE

S P P	SPRINKLER ZONE SUBSIDIARY CONTROL VALVE ASSEMBI
Ç DŞ ŞT	FLOW SWITCH, TEST GATE VALVE AND DRAIN VALVE

- M GATE VALVE
- NON RETURN VALVE
- ♥ VORTEX INHIBITOR
- BALL FLOAT VALVE
 PRESSURE SWITCH
- —— SPRINKLER PIPE
- ---- HOSE REEL PIPE
- |⊗| SPRINKLER CONTROL VALVE SET
- SPRINKLER / F.S. INLET
- F.E_{Co2} 5Kg CO2 TYPE FIRE EXTINGUISHER
- 4Kg DRY POWDER TYPE FIRE EXTINGUISHER
- PUMP
- 150mm WATER ALARM GONG
- EMERGENCY LIGHTING
- EXIT SIGN
- FAP FIRE ALARM PANEL
- PUMP CONTROL PANEL
- SELF-CONTAINED EMERGENCY FLUORESCENT LIGHTING UNIT
- ---- F. S. INSTALLTION
- 💢 FLASH LIGHT

ABBREVIATION

- SPR. SPRINKLER
- F.H. FIRE HYDRANT
- H.R. HOSE REEL
- F.E. FIRE EXTINGUISHER
- CO₂ CARBON DIOXIDE
- L.P.C. LOSS PREVENTION COUNCIL
- F.S.I. FIRE SERVICES INSTALLATION
- H/L HIGH LEVEL
- M/L MID LEVEL
- L/L LOW LEVEL
- F/A FROM ABOVE
- F/B FROM BELOW
- T/A TO ABOVE
- T/B TO BELOW
- u/g UNDERGROUND
- F.S. FIRE SERVICES

LEGEND (FOR SCHEMATIC DIAGRAM)

HOSE REEL W/ LOCKABLE GLASS FRONTED NOZZLE BOX, STRIKER, C/W FIRE ALARM BELL & BREAK GLASS UNIT

- 为 150mm FIRE ALARM BELL
- BREAK GLASS UNIT
- FAST RESPONSE TYPE SPRINKLER HEAD
- FLOW SWITCH
- MONITORED GATE VALVE
- SPRINKLER ZONE SUBSIDIARY CONTROL VALVE ASSEMBLY INCLUDES ZONE SUBSIDIARY CONTROL VALVE, FLOW SWITCH, TEST GATE VALVE AND DRAIN VALVE
 - - NON RETURN VALVE
 - VORTEX INHIBITOR
 - BALL FLOAT VALVE
 - P PRESSURE SWITCH
 - PRESSURE GAUGE WITH COCK
 - \$\frac{1}{2}\text{A.A.V.} AUTOMATIC AIR VENT WITH COCK
- SPRINKLER / HOSE REEL PIPE

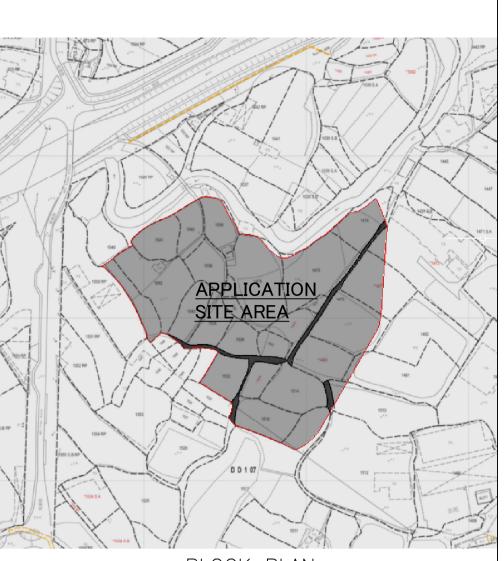
 SPRINKLER CONTROL VALVE SET
- LEVEL SWITCH (HIGH LEVEL SIGNAL & LOW LEVEL SIGNAL)
- □ FLEXIBLE CONNECTOR
- → ← CHECK METER POSITION
- ⊢l PLUG
- Y−STRAINER
- SPRINKLER / F.S. INLET
- SPRINKLER PROVING PIPE
- F. S. INSTALLTION
- PUMP SET

DRAWING LIST

- DRAWING NO DESCRIPTION
- YL-KTN1023-FS01 FS NOTES, LEGEND, ABBREVIATIONS AND
 - DRAWING LIST
- YL-KTN1023-FS02 FIRE SERVICES INSTALLATION LAYOUT PLAN
 - G/F LAYOUT PLAN
- YL-KTN1023-FS03 SCHEMATIC DIAGRAM FOR SPRINKLER SYSTEM
 YL-KTN1023-FS04 SCHEMATIC DIAGRAM FOR HOSE REEL SYSTEM

COLOUR CODE

PIPE SIZES	COLOUR
ø25mm	LIGHT GREEN
ø32mm	RED
ø40mm	PURPLE
ø50mm	YELLOW
ø65mm	BLUE
ø80mm	GREEN
ø100mm	LIGHT BROWN
ø150mm	DEEP BROWN



BLOCK PLAN

1	TPB RESUBMISSION	20-11-2024	LH		
0	TPB SUBMISSION	05-08-2024	LH		
REV	DESCRIPTION	DATE	BY		
FSI CONTRACTOR					

East Power Engineering Limited



Flat A, 7/F., Hop Shing Commercial Building 41 Chi Kiang Street, Tokwawan, Kowloon Fax.: 2394-3772 Tel.: 2397-3238

PROJECT

PROPOSED TEMPORARY WAREHOUSE (EXCLUDING DANGEROUS GOODS GODOWN) WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF LAND AND POND AT VARIOUS LOTS IN D.D. 107 AND ADJOINING GOVERNMENT LAND, FUNG KAT HEUNG, KAM TIN, YUEN LONG, NEW TERRITORIES

DRAWING TITLE

FS NOTES, LEGEND, ABBREVIATIONS AND DRAWING LIST

		INITIAL	DESIGNATION	DATE
	DRAWN BY	HY	Eng.T	05-08-2024
	DESIGNED BY	HY	Eng.T	05-08-2024
	CHECKED BY	СМ	PM	05-08-2024
	APPROVED BY	_	_	_
	PROJECT NO.	A_YL-KTN_1023		

PAPER SIZE

DRAWING NO.

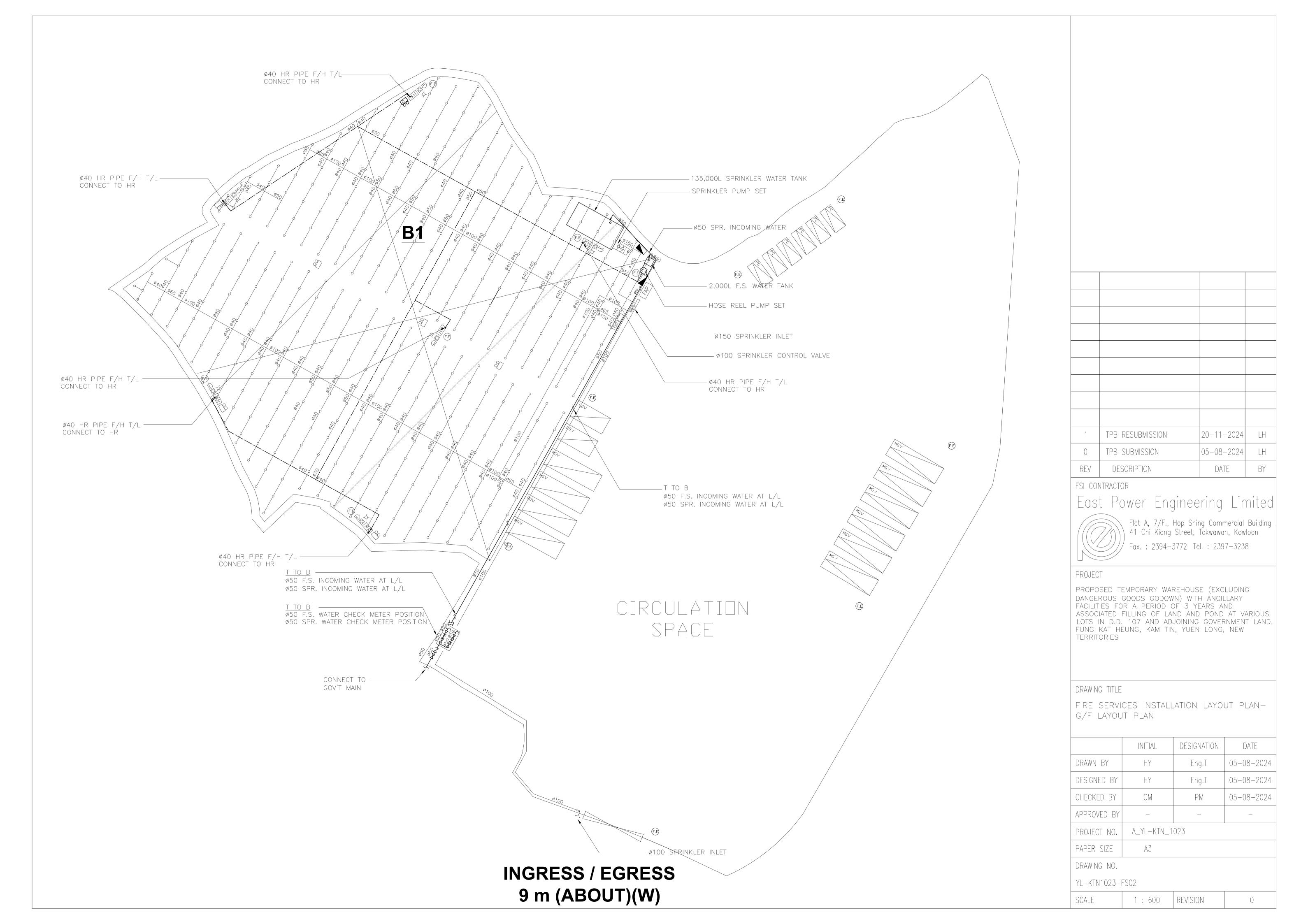
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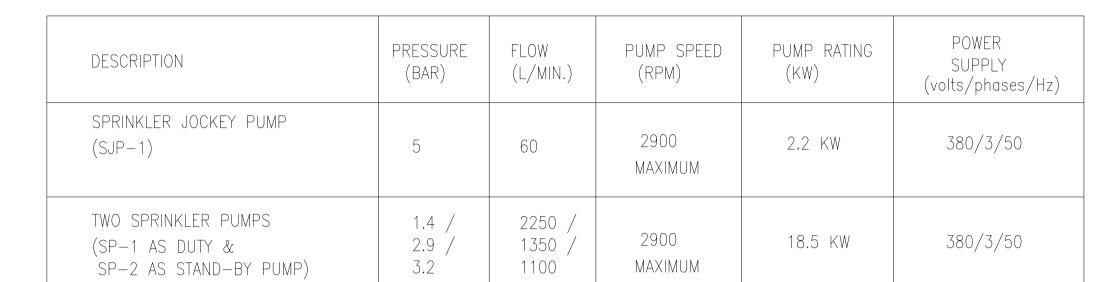
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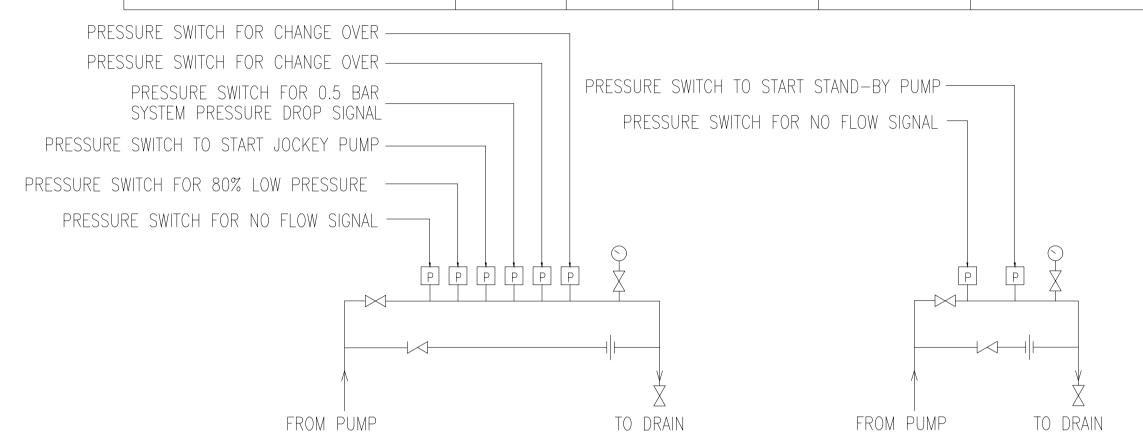
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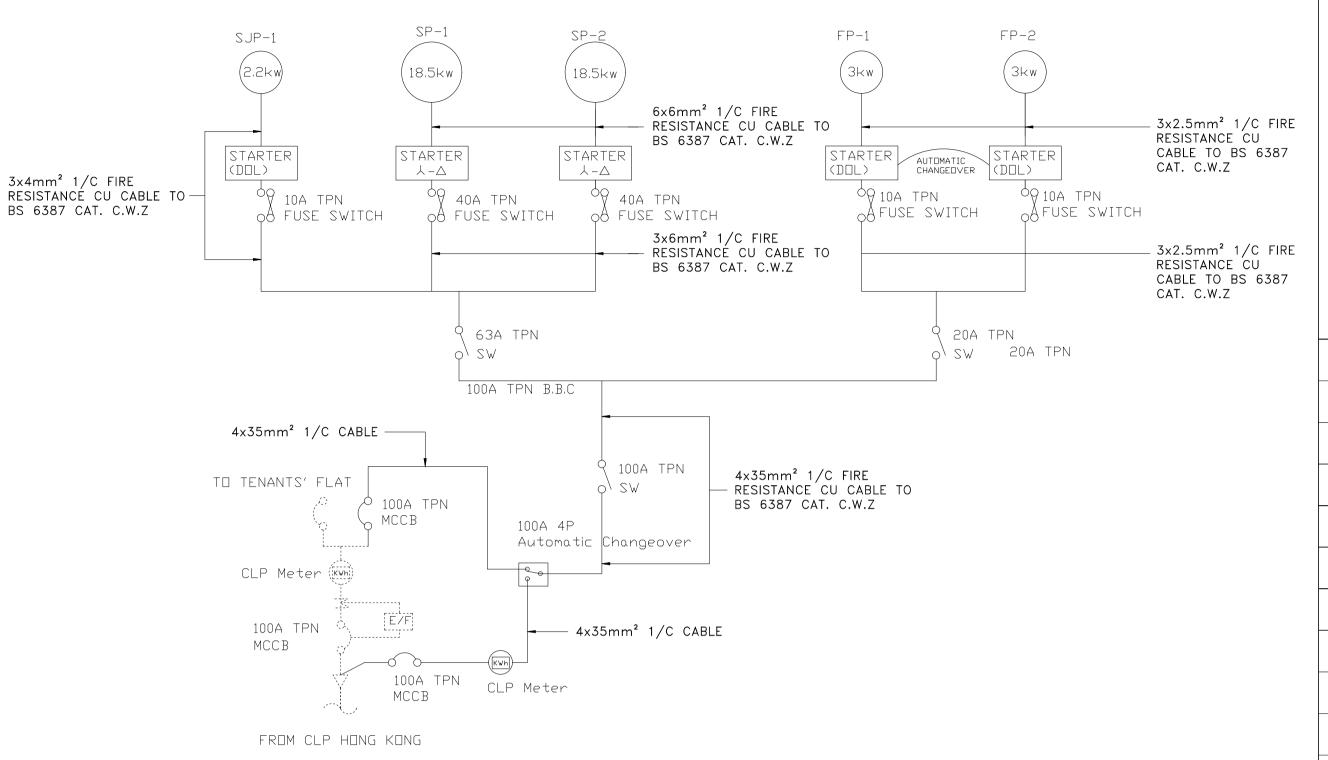
PUMP SCHEDULE



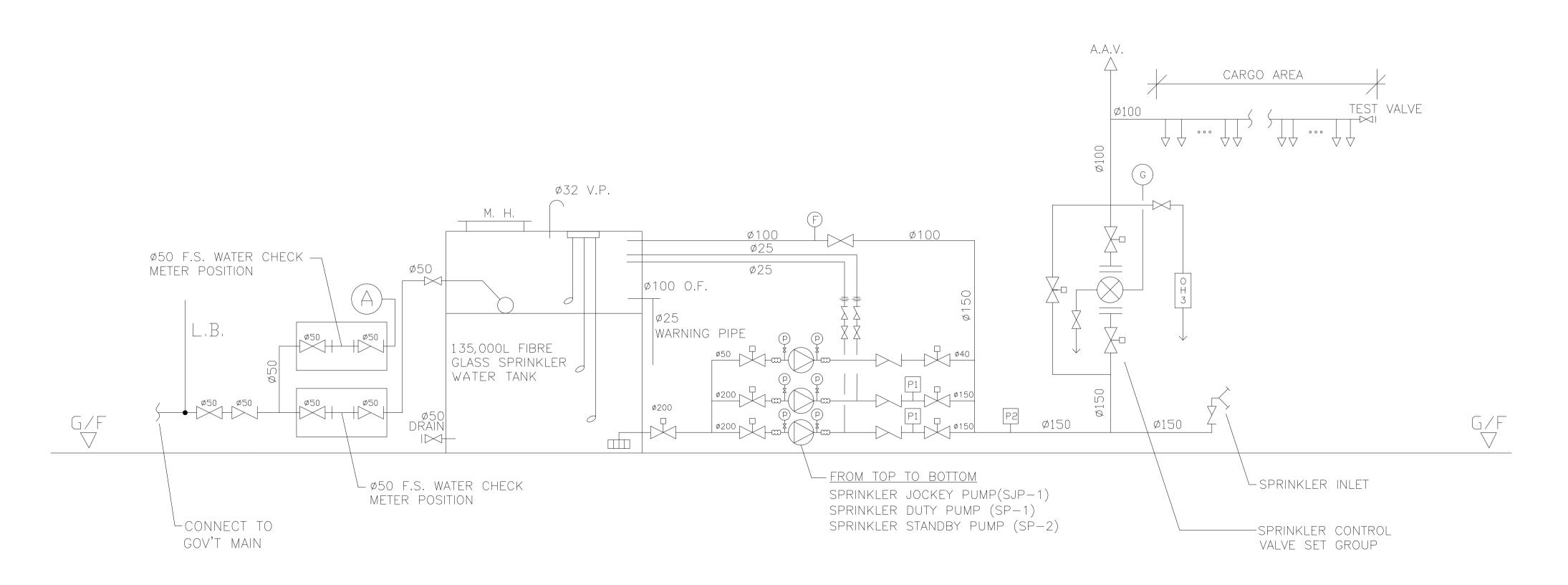


DETAIL ARRANGEMENT FOR 'P2'

DETAIL ARRANGEMENT FOR 'P1'



POWER DISTRIBUTION DIAGRAM FOR SPRINKLER AND FIRE SERVICE PUMPS



SCHEMATIC DIAGRAM FOR SPRINKLER SYSTEM

1	TPB RESUBMISSION	20-11-2024	LH
0	TPB SUBMISSION	05-08-2024	LH
REV	DESCRIPTION	DATE	BY

FSI CONTRACTOR

East Power Engineering Limited



Flat A, 7/F., Hop Shing Commercial Building
41 Chi Kiang Street, Tokwawan, Kowloon

Fax.: 2394-3772 Tel.: 2397-3238

PROJECT

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DRAWING TITLE

SCHEMATIC DIAGRAM FOR SPRINKLER SYSTEM

INITIAL	DESIGNATION	DATE
HY	Eng.T	05-08-2024
HY	Eng.T	05-08-2024
CM	PM	05-08-2024
_	_	_
A_YL-KTN_1023		
A3		
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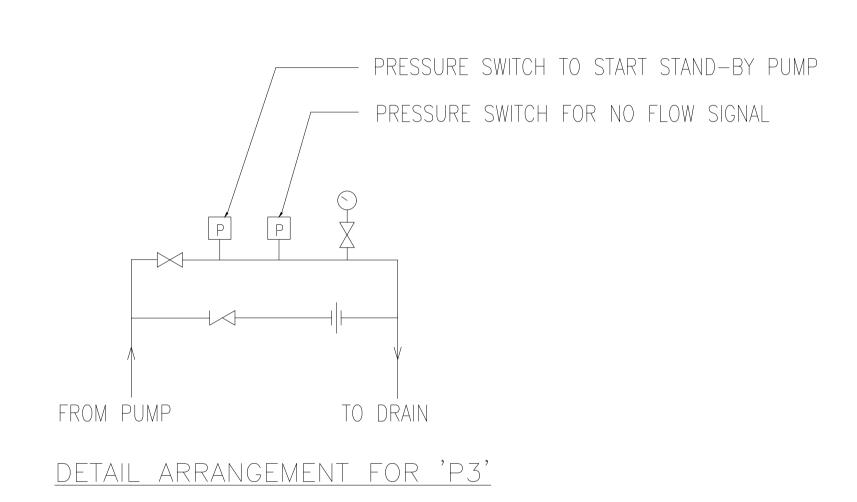
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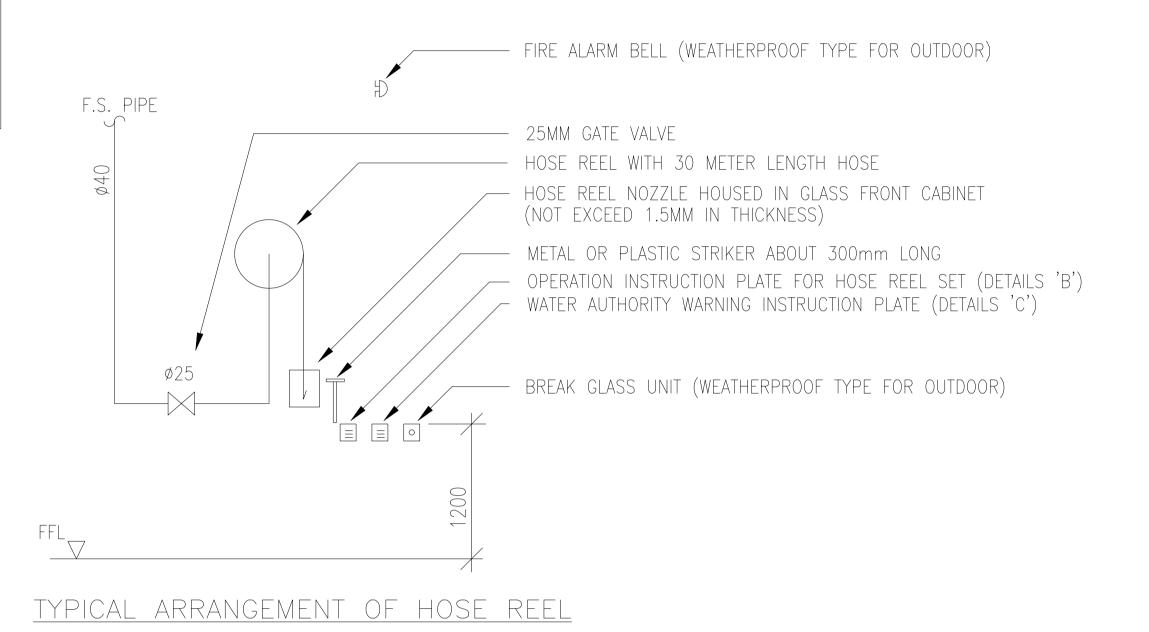
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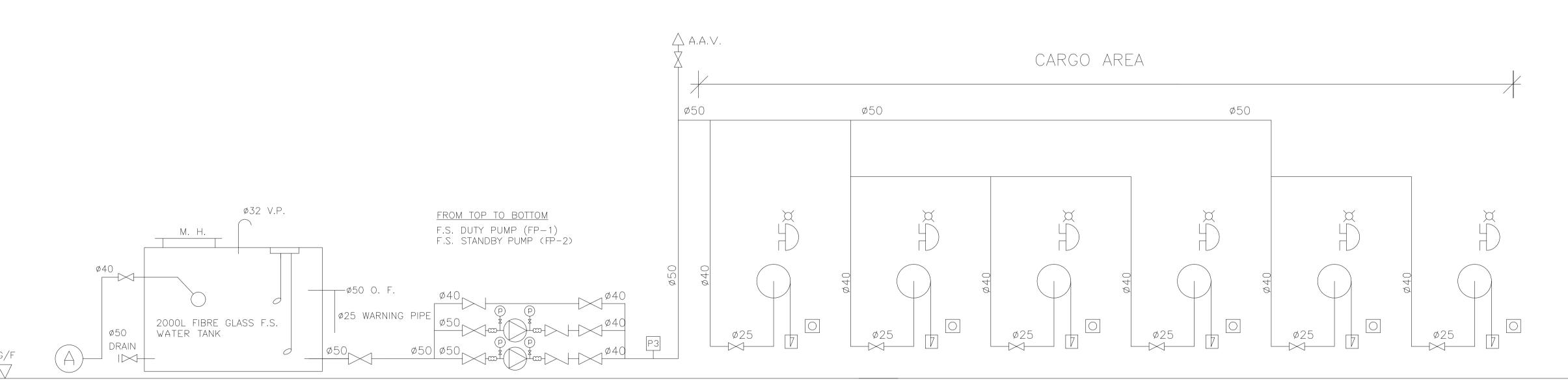
SCALE N. T. S. REVISION

PUMP SCHEDULE

DESCRIPTION	PRESSURE (BAR)	FLOW (L/MIN.)	PUMP SPEED (RPM)	PUMP RATING (KW)	POWER SUPPLY (volts/phases/Hz)
TWO FIRE SERVICES PUMPS (FP-1 AS DUTY & FP-2 AS STANDBY PUMP)	5	60	2900 MAXIMUM	2.2KW	380/3/50







SCHEMATIC DIAGRAM FOR HOSE REEL SYSTEM

1	TPB RESUBMISSION	20-11-2024	LH
0	TPB SUBMISSION	05-08-2024	LH
REV	DESCRIPTION	DATE	BY
	170 1 07 0 0		

FSI CONTRACTOR

East Power Engineering Limited



Flat A, 7/F., Hop Shing Commercial Building 41 Chi Kiang Street, Tokwawan, Kowloon Fax.: 2394-3772 Tel.: 2397-3238

PROJECT

PROPOSED TEMPORARY WAREHOUSE (EXCLUDING DANGEROUS GOODS GODOWN) WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF LAND AND POND AT VARIOUS LOTS IN D.D. 107 AND ADJOINING GOVERNMENT LAND, FUNG KAT HEUNG, KAM TIN, YUEN LONG, NEW TERRITORIES

DRAWING TITLE

YL-KTN1023-FS04

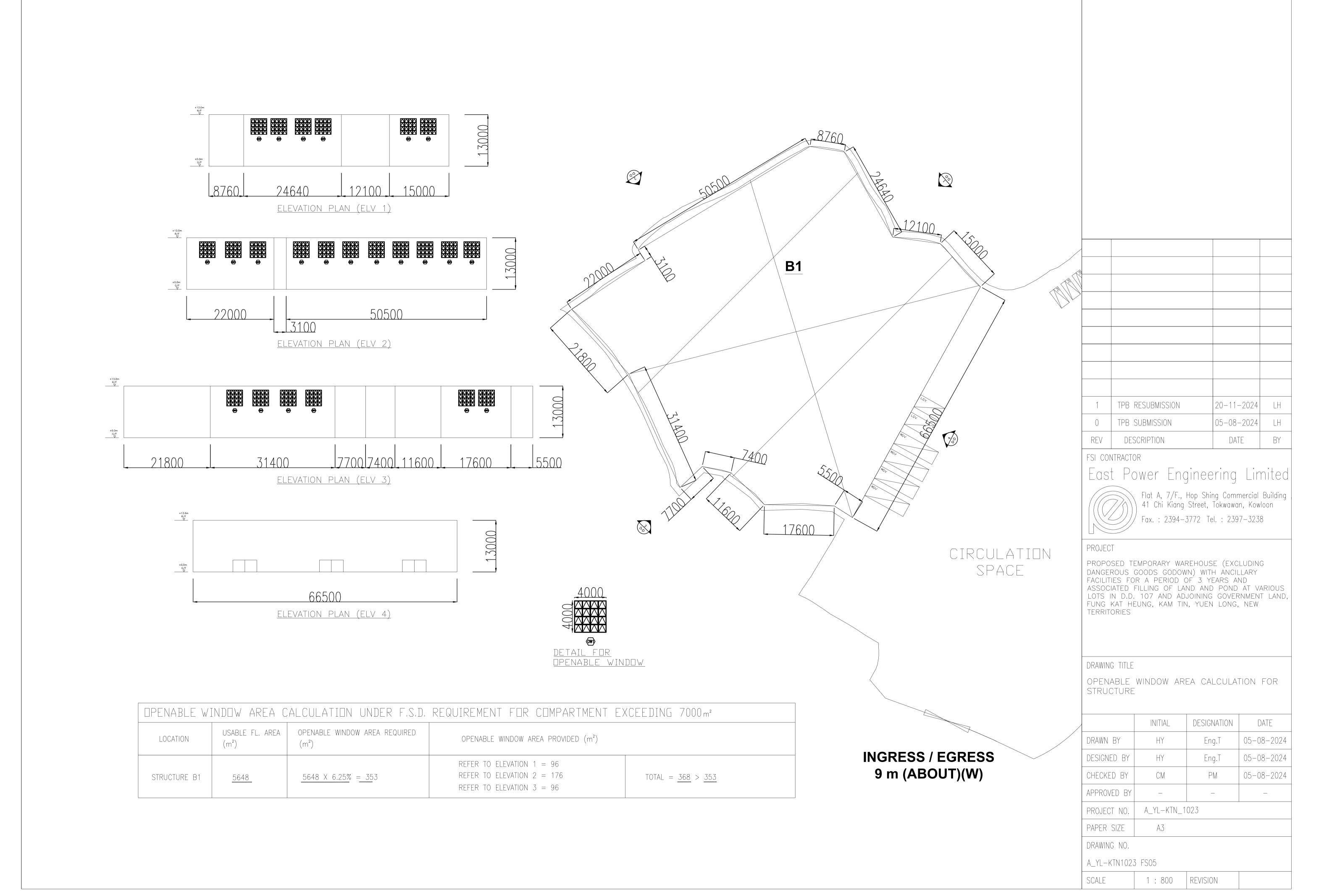
SCALE

SCHEMATIC DIAGRAM FOR HOSE REEL SYSTEM

	INITIAL	DESIGNATION	DATE
DRAWN BY	HY	Eng.T	05-08-2024
DESIGNED BY	HY	Eng.T	05-08-2024
CHECKED BY	CM	PM	05-08-2024
APPROVED BY	_	_	_
PROJECT NO.	A_YL-KTN_1	023	
PAPER SIZE	A3		
DRAWING NO.			

REVISION

N. T. S.



Excel Link Development Limited

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories

Drainage Impact Assessment (Section 16 Planning Application No. A/YL-KTN/1023)



Document No. V1094/01 Issue 2

December 2024



V1094/01 Issue 2 December 2024

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories

Drainage Impact Assessment (Section 16 Planning Application No. A/YL-KTN/1023)

Approved for Issue by:

Kenny W K Lam RPE (Civil)

FW0275905

Position:

Deputy Managing Director

Date:

17 December 2024

Excel Link Development Ltd 205A Sik Kong Tsuen Ha Tsuen, Yuen Long New Territories

Mannings (Asia) Consultants Ltd 5/F, Winning Commercial Building 46-48 Hillwood Road Tsim Sha Tsui Kowloon

Drainage Impact Assessment (Section 16 Planning Application No. A/YL-KTN/1023)

Issue	Prepared by	Reviewed by	Date
1	BH	BLE	23 Sep 2024
2	BH	BLE	17 Dec 2024

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3.0	Current Flooding Susceptibility and Proposed Drainage	
4.0	Changes to the Drainage Characteristics and Potential Drainage Impact	
5.0	Drainage Impact Mitigation Measures	
6.0	Monitoring Requirements	
7.0		
7.0	Conclusion	I

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Appendix A: Drawing

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V1094/004	Drainage Layout Plan – Structure Roofing
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V1094/007	Typical Details of Drainage

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Abbreviations

D.D.	Demarcation District
DSD	Drainage Services Department
SDM	Stormwater Drainage Manual

V1094/01 Issue 2 Drainage Impact Assessment



1.0 Introduction

- 1.1 This submission presents the drainage impact assessment of the proposed temporary warehouse (excluding dangerous goods godown) with ancillary facilities for a period of 3 years and associated filling of land and pond at various lots in D.D. 107 and the adjoining government land at Fung Kat Heung, Kam Tin, Yuen Long, New Territories ("Site")
- The Site has an area of about 15,822m² and it is currently covered in grassland with few 1.2 temporary structures. A 1-storey structure is proposed at the Site for temporary warehouse with total GFA of about 5,648 m². The general layout plan and cross sections of the Site are shown on the Drawing Nos. V1094/001 and V1094/002 enclosed in Appendix A.
- 1.3 Due to the concerns of possible drainage impact arising from the change of uses, Mannings (Asia) Consultants Limited (MACL) was appointed by the Excel Link Development Limited to undertake a Drainage Impact Assessment (DIA) to demonstrate the acceptability of drainage impact upon the surrounding environment.



2.0 **Design Methodology and Assumptions**

Design Code

- 2.1 The below design codes are to be followed for this design assessment:
 - Stormwater Drainage Manual (DSD) Fifth Edition, January 2018;
 - Stormwater Drainage Manual (DSD) Corrigendum No. 1/2022;
 - Stormwater Drainage Manual (DSD) Corrigendum No. 1/2024;
 - Stormwater Drainage Manual (DSD) Corrigendum No. 2/2024;
 - BS 5911 Code of Practice for Precast Concrete Pipe Design
 - **DSD Standard Drawings**

Design Parameters

2.2 **Design Parameters**

Runoff Coefficient a)

Table 2-1 Runoff Coefficients

Surface Characteristic	Runoff Coefficient, C
Roof of Structure	1.00
Grassland (heavy soil Flat), unpaved area	0.25

Roughness Coefficient for pipe flow $k_s = 3$

b) Minimum Pipeline Cover and Manhole Spacing Requirements

Table 2-1 Minimum Pipeline Cover and Manhole Spacing Requirements

Minimum pipeline cover			
In Roads	0.9 m		
In footways and verges	0.45 m		
Manhole spacing requirements			
D < 675 mm	80 m		
675 < D < 1050	100 m		
D > 1050	120 m		

Bedding factors c)

-	Granular bedding	: 1.9
-	Plain concrete bedding	: 2.6
-	Reinforced concrete bedding with allowance	: 3.4
	for minimum steel area	
_	Concrete Surround	. 4 5



d) Design Flow Velocity

- Minimum : 1 m/s

- Maximum : 3 m/s (desirable)

: 6 m/s (absolute)

Return Period

As mentioned in Para. 1.2, the Site is currently covered in grassland with few temporary structures. According to Table 10 in Stormwater Drainage Manual (DSD) - Fifth Edition, January 2018, the recommended design return periods based on flood levels of "Intensively Used Agricultural Land" is 2-5 years.

2.4 In order to provide a conservative drainage impact assessment, return period of 1 in 50 years is adopted under this assessment.

Analysis Method

2.5 Description of Analysis Method

a) Rational method is to be adopted for calculation of the peak runoff. The formula is extracted from Section 7.5.2(a) of Stormwater Drainage Manual (SDM) which is to estimate the stormwater runoff as shown below:

$$Q_p = 0.278 \text{ CiA}$$

Where $Q_p = peak runoff in m3/s$

C = runoff coefficient (dimensionless)

i = rainfall intensity in mm/hr

A = catchment area in km^2

b) 10% reduction of the flow area is allowed taken into account of the decomposition of siltation as per DSD's SDM 2018.

c) The time of concentration used for determining the duration of the design storm is considered by the time of entry and the time of flow,

$$t_c = t_o + t_f \hspace{1cm} t_f = L/V \label{eq:tc}$$

where t_0 =inlet time (time taken for flow from the remotest point to

reach the most upstream point of the urban drainage system)

 $t_f = flow time$

L = Length of drain

V = flow velocity



e) The time of entry or time of flow in the hinterland is calculated using the Bransby William's Equation.

$$t_e = \frac{0.14465L}{A^{0.1}H^{0.2}}$$

Where $t_e = time of concentration (min)$

L = catchment length (m) A = catchment area (m2)

H = average catchment slope (m/100m)

f) The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship.

$$i = a/(t_d+b)^c$$

Where i = extreme mean intensity in mm/hr

 t_d = duration in minutes (td<240), and

a,b,c =storm constants given in table 3 of SDM as below

Table 2-2 Storm Constant of SDM – Corrigendum No.1/2024

Return Period T (years)	50
a	505.5
b	3.29
С	0.355

g) Colebrook-White Equation is used in hydraulic design for pipe flow.

$$V = -\sqrt{(32gRs)}\log\left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{(32gRs)}}\right)$$

Where:

V = mean velocity (m/s)

g = gravitational acceleration (m/s^2)

R = hydraulic radius (m) D = pipe diameter (m)

 $k_s \qquad = equivalent \ sand \ roughness \ (m)$

v = kinematic viscosity of fluid (m^2/s)

s = frictional slope (energy gradient due to frictional loss)



3.0 Current Flooding Susceptibility and Proposed Drainage

Current Site Condition and Flooding Susceptibility

- 3.1 The topography of the Site is generally flat and currently situated with levels ranging from +4.2 mPD to +5.2 mPD. In general, the direction of existing surface runoff flows from north to south. Since the ground levels of the Site are generally higher than the existing surrounding area, flooding susceptibility of the Site is considered as low.
- 3.2 Catchment plan before development are shown in **Drawing No. V1094/005** in **Appendix A**.

Proposed Development

3.3 A 1-storey structure is proposed at the Site for temporary warehouse as stated in Para. 1.2. After completion of the project, the finished ground level of the Site will be raised to approximately +5.7 mPD to +6.2 mPD. Part of the unpaved areas is proposed to be occupied by a new covered structures whilst the remaining unpaved area would be unchanged in regards of the finished surface and continued to be an opened space area. In addition, some of these unpaved opened areas are proposed to be served as access road and parking spaces. A layout plan of the proposed development with **Drawing No. V1094/001** is enclosed in **Appendix A**.

Proposed Drainage

- According to the site survey and observation, there are two existing outfalls located at the north of the Site flowing from south to north and connecting to a 7m wide open channel. The runoff from the Site after development will be discharged into this open channel through the two existing outfall pipes. The photo records of the existing drainage are presented in **Appendix C**.
- 3.5 The drain on the roof of the proposed structure (i.e. Catchment Area C) will fall to two edges as shown in catchment plan in **Drawing No. V1094/006** in **Appendix A**. Then, the runoff would drain a 525mm dia. downpipe which is mounted on the structure with a 90-degree bend connected to the 525mm dia. elevated drainage pipe to manhole MH3. The proposed 525mm dia. elevated drainage pipe are installed at the side of the existing footbridge across the northern stream of the Site. The 525mm dia. drainage pipe are embedded underground from MH3 to outfall 2. Drainage layout plan and details of drainage are shown in **Drawing Nos. V1094/004** and **V1094/007** in **Appendix A**.
- 3.6 The 375mm U-channels are proposed at south of the Site. The runoff from the unpaved area in the Site (i.e. Catchment Area Nos. D) would be collected by u-channels and drain to outfall 1 by gravity via 450mm dia. drainage pipes. Drainage layout plan and details of drainage are shown in **Drawing Nos. V1094/003 and V1094/007** in **Appendix A**.
- 3.7 Calculation of the proposed drainage are presented in Section 4 and enclosed in **Appendix B**.



3.8 The proposed U-channels and drainage pipes are designed to have sufficient capacities for the estimated runoff from the unpaved area and structure roofing in the Site. Details of the calculation are enclosed in **Appendix B**.

Changes in Land Use and Planned Drainage Works in Adjacent Area

- 3.9 It is noted that changes of land use might happen at the adjacent area of south-east of the Site. The layout plans of the proposed works and the proposed drainage works for the adjacent area are attached in **Appendix D** for information.
- 3.10 Since the surface runoff of the adjacent area will be collected and discharged to an existing drainage system near Shui Mei Road as shown in **Drawing No. V1094/101**, **103** and **104** in **Appendix D**, no drainage impact to the Site in this report is anticipated.



4.0 Changes to the Drainage Characteristics and Potential Drainage Impact

Changes in Land Use and Surface Runoff Characteristics

4.1 The Site is currently covered in grassland with few temporary structures. After completion of the project, the Site will remain as an unpaved area except the proposed structure. Runoff coefficient are shown in Table 2-1 under Para. 2.2.

Changes to Surface Runoff Hydrographs

4.2 Changes in land use from unpaved area to paged area would lead to higher and faster surface runoff. However, considering the scale of the proposed development is relatively small, the changes to surface runoff hydrographs is considered as negligible.

Changes in Flood Storage

4.3 No flood storage was found near the Site.

Changes in Timing of Peak runoff

4.4 Changes of time of concentration of Outfall 1 and Outfall 2 before and after development are summarized in below table. The calculation is attached in **Appendix B**.

Outfall	Time of concentration (min)		
	Before Development	After Development	
Outfall 1	24.39	22.20	
Outfall 2	19.86	19.99	

Hydraulic Bankfull Capacity of the Proposed Drainage System

- 4.5 The proposed drainage system mentioned in Para. 3.3 to Para 3.5 are designed to have sufficient capacity to cater the flow from the Site. Detailed calculation is attached in **Appendix B**.
- 4.6 The design runoff, capacity and utilization of the U-channels are summarized in below table.

Proposed U-Channel	Design Runoff (m³/s)	Capacity (m ³ /s)	Utilization
UC1 to UC2	0.119	0.146	0.82
UC3 to UC7	0.115	0.146	0.79



4.7 The design runoff, capacity and utilization of the 375mm gutter mounted on structure are summarized in below table.

Proposed Gutter	Design Runoff (m ³ /s)	Capacity (m ³ /s)	Utilization
375mm Gutter	0.181	0.230	0.78

4.8 The design runoff, capacity and utilization of the proposed pipes are summarized in below table.

Proposed Pipe	Design Runoff	Capacity	Utilization
	(m^3/s)	(m^3/s)	
To Outfall 1:			
CP7 to MH1	0.114	0.156	0.73
MH1 to MH2	0.114	0.156	0.73
MH2 to Outfall 1	0.113	0.156	0.72
To Outfall 2:			
450mm Elevated Pipe	0.202	0.241	0.84
525mm Downpipe	0.360	1.195	0.30
MH3 to MH4	0.358	0.528	0.68
MH4 to Outfall 2	0.358	0.446	0.80

Changes in Peak Runoff and Peak Velocity at Critical Locations (Outfalls)

Below table shows the comparison of the peak runoff and peak velocity of the two outfalls before and after the development. Detailed calculation is attached in **Appendix B**.

	Outfall 1		Outfall 2	
	Peak Runoff (m³/s)	Peak Velocity (m/s)	Peak Runoff (m³/s)	Peak Velocity (m/s)
Before Development	0.269	1.079	0.014	1.052
After Development	0.330	1.079	0.274	1.052

Potential Drainage Impact to Existing Drainage System

- 4.10 The proposed drainage systems are proposed to discharge to two existing outfalls as mentioned in Para. 3.3. Flows to the two outfalls will be increased.
- 4.11 For the existing drainage system, the two existing outfall pipes located at the north of the Site are checked. Both outfall pipes have sufficient capacities to cater for the additional runoff upon completion of the proposed development. The estimated runoffs and capacities after development are summarized in Table 4-1.

Table 4-1 Estimated Runoff and Capacities of Existing Drainage

Existing Drainage	Estimated runoff (m³/s)	Capacity (m ³ /s)	Utilization
Outfall 1	0.330	0.429	0.77
Outfall 2	0.274	0.418	0.65



Temporary Drainage during Construction

4.12 According to the site survey and observation, there is no existing drainage system in the Site. Therefore, no existing drainage system would be affected during the construction. Temporary drainage is considered not necessary.

Details of Works to Existing Drainage System

4.13 Proposed drainage systems are connecting to Outfall 1 and Outfall 2 as shown in **Drawing No. V1094/003** and **004** in **Appendix A**.

Potential Drainage Impacts to Other Land Users

4.14 All runoff in the Site will be collected and drain to existing drainage system as stated in Para. 3.3, no drainage impact to other land users is anticipated.



5.0 Drainage Impact Mitigation Measures

- 5.1 As discussed in Para. 4.11 and 4.13, no existing drainage system would be affected and no drainage impact to other land users is anticipated. Therefore, Mitigation measures is considered no necessary.
- 5.2 The Contractor should monitor during the construction to ensure that there is no adverse drainage impact to the nearby drainage systems and adjacent land users.



6.0 Monitoring Requirements

Monitoring During Construction

- 6.1 Monitoring of the drainage system is required during construction to ensure that there are no adverse impacts which may result in flooding or deterioration in the water quality.
- 6.2 Monitoring shall include:
 - any siltation or blockages in channels, slit traps or sediment basins;
 - checking the drainage is performing in accordance with the design;
 - checking for damage; and
 - visual inspection of any high sediment levels
- 6.3 The detailed requirements of drainage monitoring should be as shown in the following table:

Table 6.1 – Detailed Requirements for Drainage Monitoring

Type / location of monitoring	Minimum Frequency	Action by
Prepare method statements	Before the start of any works	Contractor
	that could impact on drainage	
Inspect existing drainage systems	Daily, Weekly, Before every	Contractor
and all construction drainage	rainstorm warning	
systems for blockages or breakages		
	After every rainstorm	Contractor
Inspect sedimentation basins and	Daily, Weekly, Before every	Contractor
silt traps	rainstorm warning	
	After every rainstorm	Contractor



7.0 Conclusion

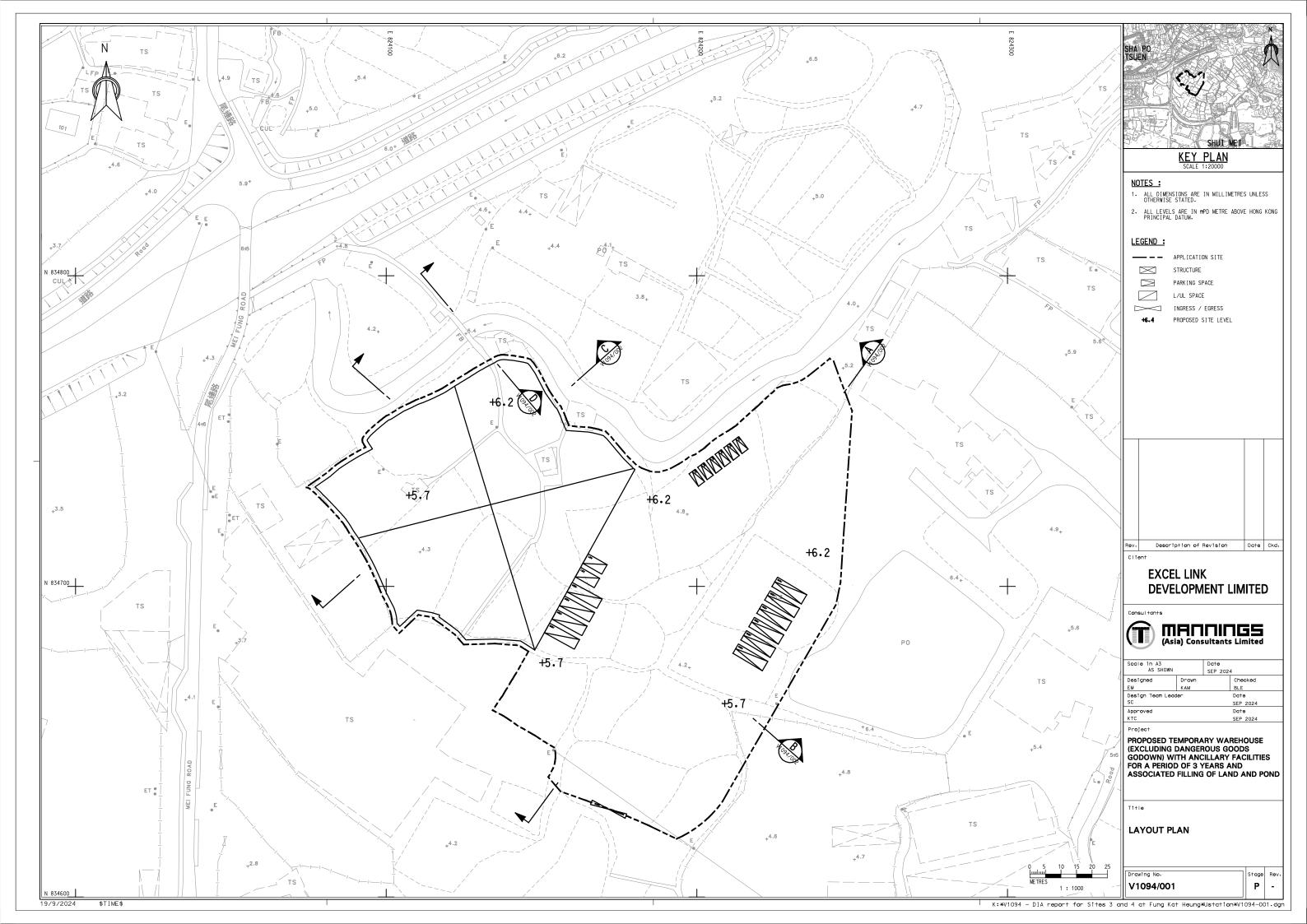
7.1 A Drainage Impact Assessment has been conducted for the proposed land use changes in Fung Kat Heung. The existing drainage system has been checked for the updated runoff from the catchment area and based on our assessment, the existing drainage system would provide sufficient capacity to cater for this additional stormwater. No adverse drainage impact shall be aroused due to the development.

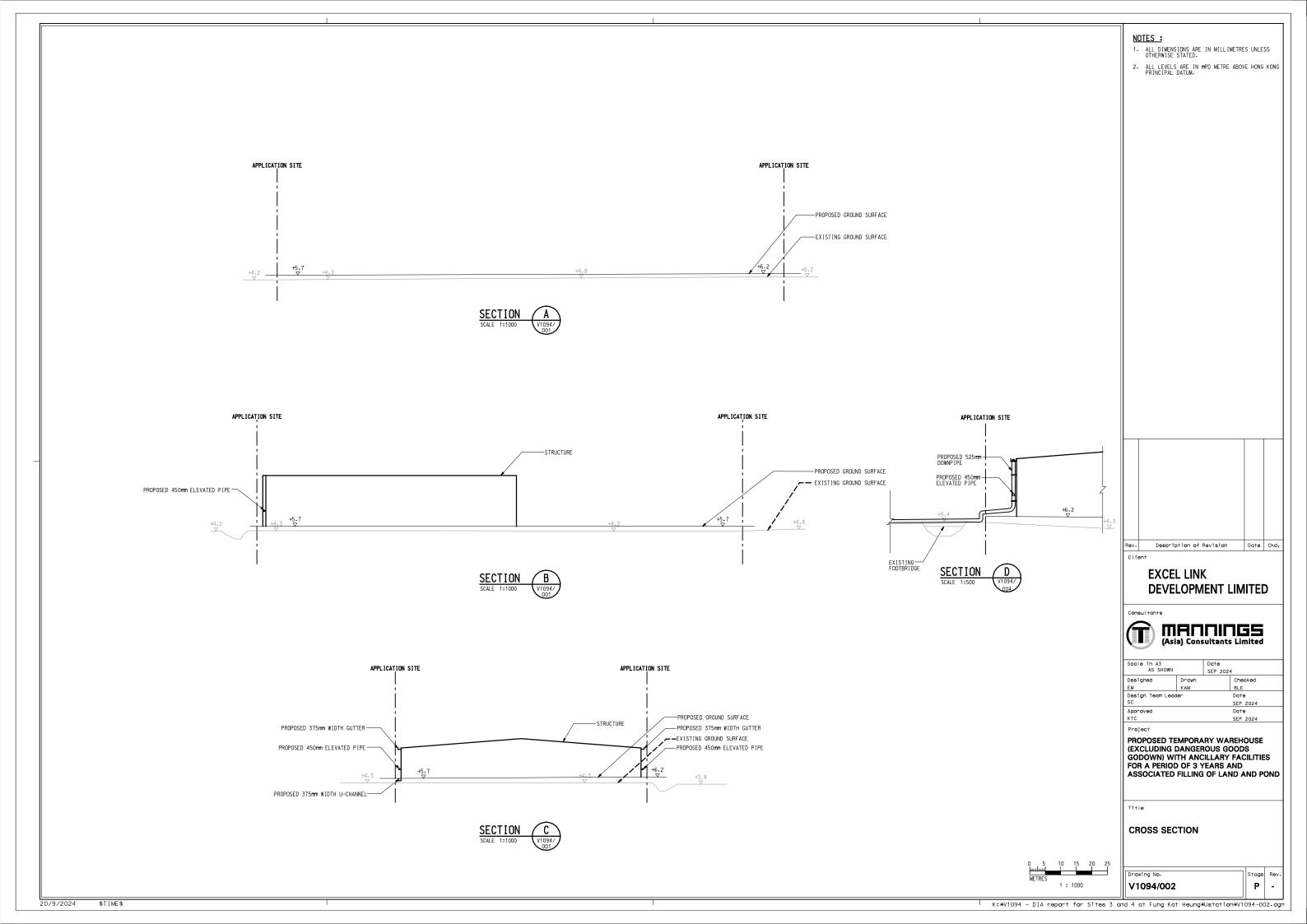


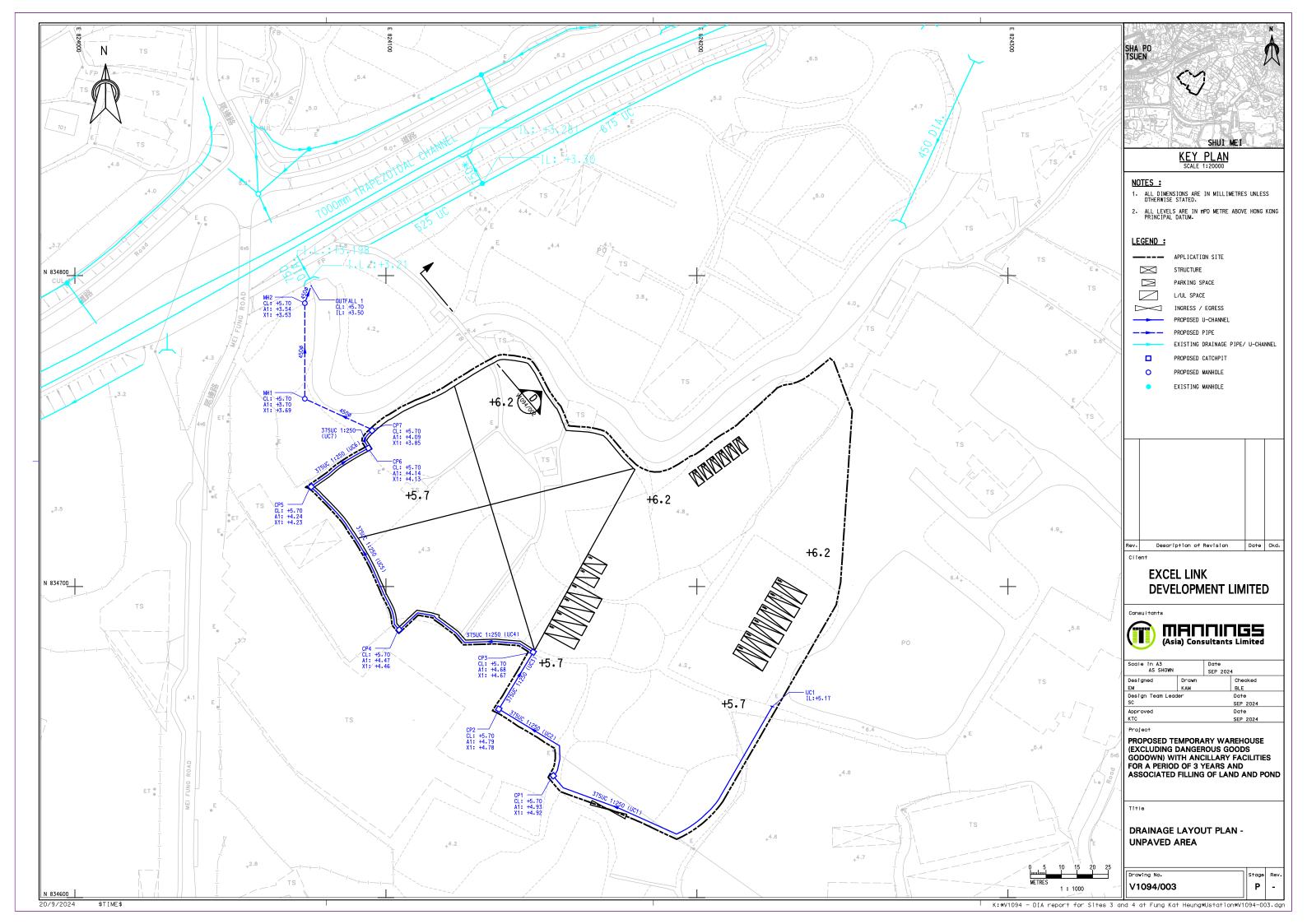
Appendix A

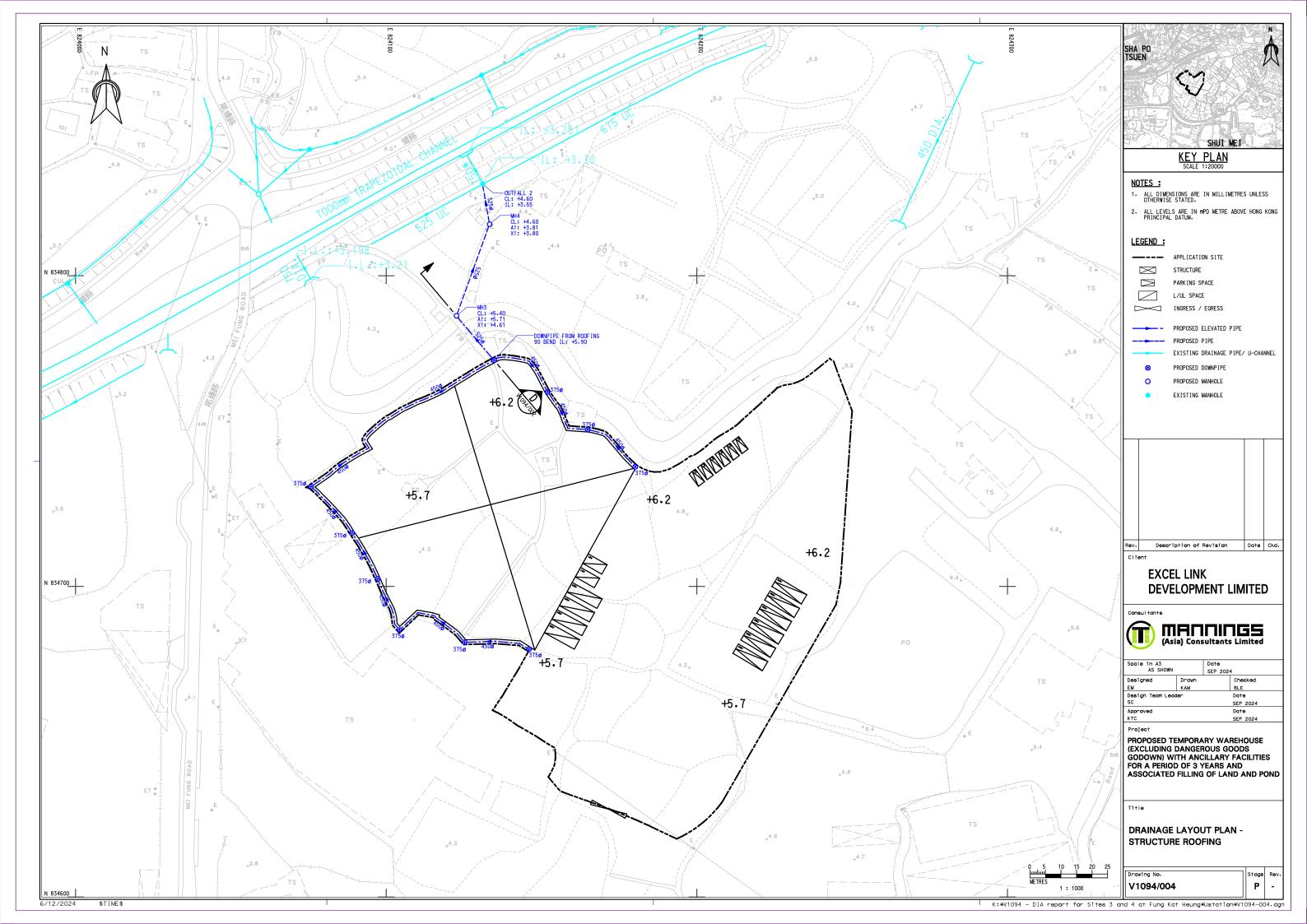
Drawings

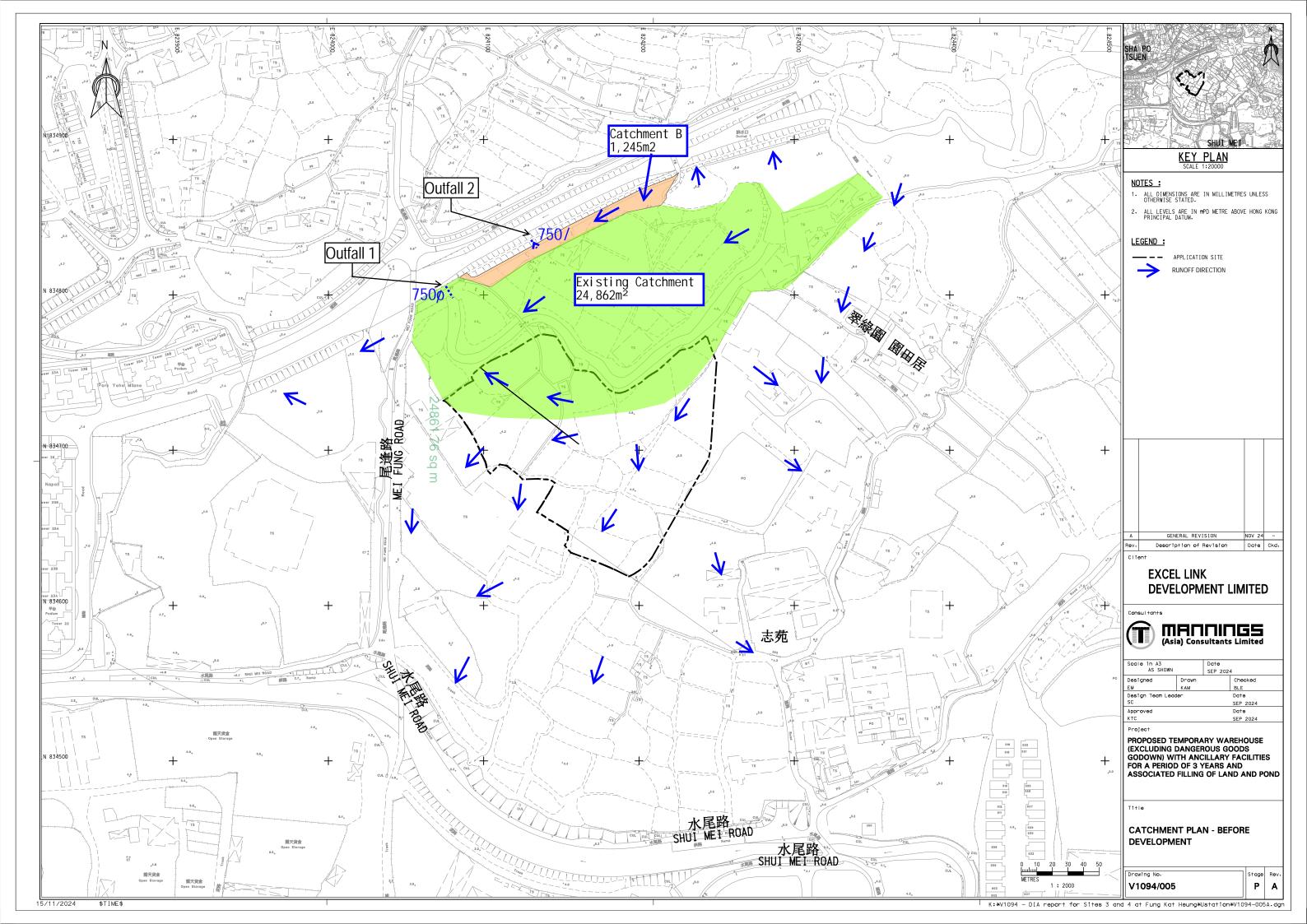
V1094/01 Issue 2 Drainage Impact Assessment

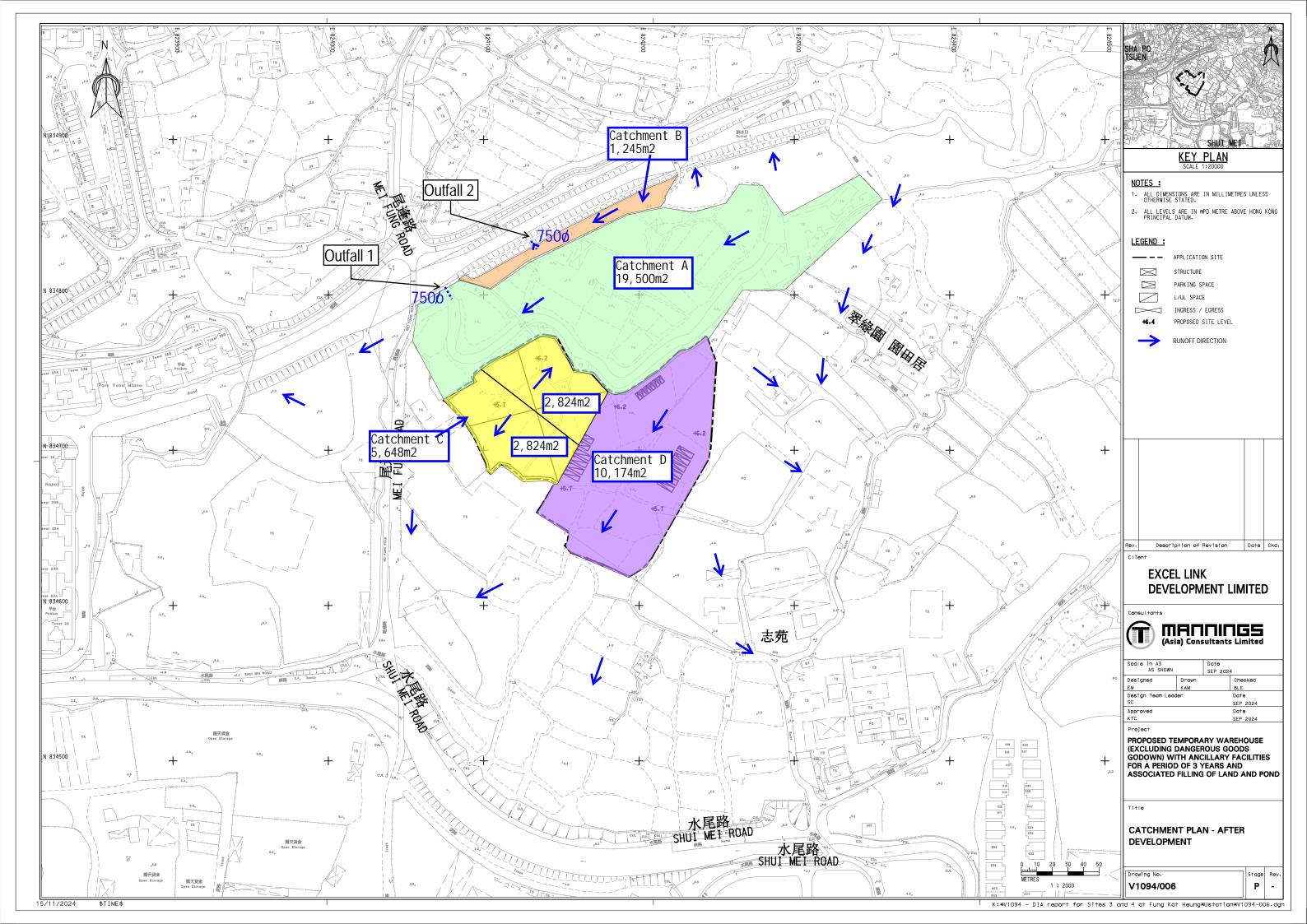


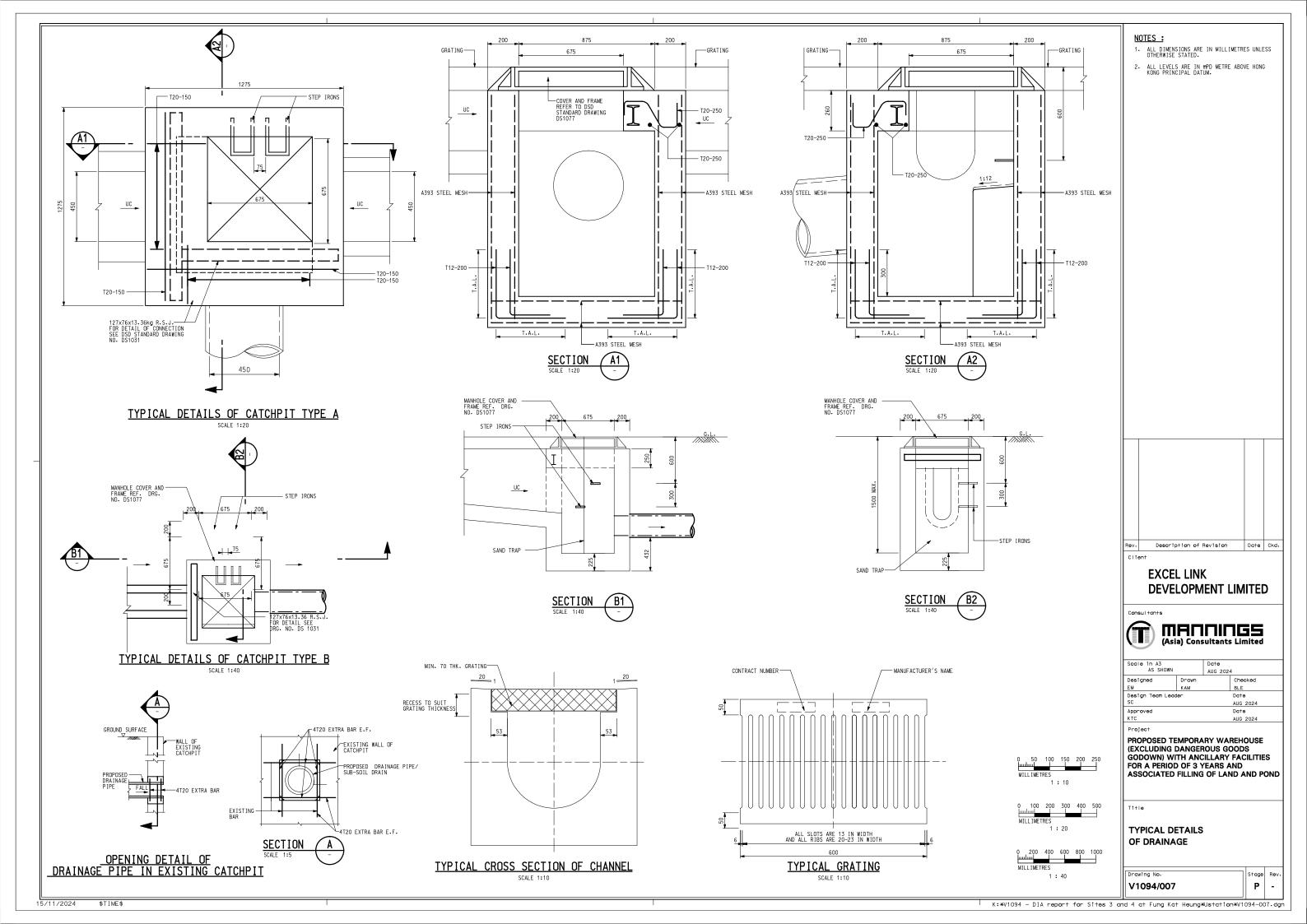














Appendix B

Design Calculations



Existing Scenario

V1094/01 Issue 2 Drainage Impact Assessment

Mannings (Asia) Consultants Ltd.	Job No.		Sheet No.	Rev.
					•
Calculation Sheet		Member / Location			
Job Tilte:	Proposed Temporary Warehouse(Excluding Dangerous Goods Godown) with	Drg. Ref.			
	Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond				
	and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land,				
	Fung Kat Heung, Kam Tin Yuen Long, New Territories	Made By	NHL	Date	Chd.

The drainage design is referring to DSD's SDM 2018 & Corrigendum No. 1/2022 and Corrigendum No. 1/2024 1 in 50 year design return period is taken.

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM. Qp = 0.278 CiA

Where Qp = peak runoff in m³/s I = rainfall intensity in mm/hr A = catchment area in km²

Runoff Estimation for Outfall 1 (Existing Scenario)

		(=::::9 :						
	Natural Catch. (m²)	Longest flow path (m)	(m per	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA (m³/s)
Existing Catchment	24862	160	0.005	24.27	0.25	24862	155.75	0.269

Runoff Estimation for Outfall 2 (Existing Scenario)

rtanion Lounnation for	Outlan E	(=^	300man.o,					
	Natural Catch. (m ²)	Longest flow path (m)	(m per	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA (m³/s)
Existing Catchment	1245	100	0.006	19.73	0.25	1245	166.02	0.014

Draninage Impact Assessment

Existing Scenario

		Catchme	ent Area		Nominal	Gradi	ient, S _f	Roughness		Time of	Rainfall	50 year		50 year	Total		Adjusted		r Level	Inver	t Level
Outfall (Ex	cisting Pipe)	Increment (m²)	Accu. (m²)	Length (m)	Diameter (mm)	(%)	1 in	Coefficient (m)	Coefficient Velocity (m/s)		Duration (min)	Intensity (mm/hr)	Runoff Coeff.	Runoff (m³/s)	Flow (m³/s)		Capacity > Total Flow ?	From	To (mPD)	From (mPD)	To (mPD)
Outfall 1 (Check	ing Existing Pipe)											•								
Outfall 1	Existing Pipe	24862	24862	ρ	750	0.3	400.0	3.0	1.079	24.39	24.39	155.50	0.25	0.269	0.269	0.429	Yes	4.30	5.20	3.21	3.19
Odtiali i	Laisting ripe	0	0	· ·	730	0.5	+00.0	5.0	1.073	24.00	24.00	155.50	1.00	0.000	0.203	0.423	163	4.50	3.20	J.Z I	3.19
Outfall 2 (Check	ing Existing Pipe)																			
Outfall 2	Existing Pipe	1245	1245	Ω	750	0.2	421.1	3.0	1.052	19.86	19.86	165.70	0.25	0.014	0.014	0.418	Yes	4.30	5.20	3.30	3.28
Outrail 2	Lxisting ripe	0	0	O	730	0.2	421.1	5.0	1.032	19.00	19.00	103.70	1.00	0.000	0.014	0.410	163	4.50	3.20	3.30	3.20

Mean Velocity is calculated by Colebrook- White equation

Where:

V = Mean Velocity (m/s)
R = Hydraulic Diameter (m)

Ks =Surface Roughness (m)

V =Kinematic viscosity (kg/ms)

Sf =Slope of Hydraulic Gradient

 $g = Gravity (m/s^2)$

The Roughness Coefficient Ks is assumed to be 3 for concrete.

Peak Runoff is estimated using rational method according to SDM.

The Roughness Coefficient Ks is assumed to be 3 for concrete, 0.06 for uPVC pipe.

 $\overline{V} = -\sqrt{32gRS_f} \log \left[\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$



Proposed Drainage System

Mannings (Asia)	Consultants Ltd.	Job No.	Sheet No.	Rev.
Calculation Sheet		Member / Location		
Job Tilte:		Drg. Ref.		
	Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond			
	and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government			
	Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories	Made By NHL	Date	Chd.

The drainage design is referring to DSD's SDM 2018 & Corrigendum No. 1/2022 and Corrigendum No. 1/2024 1 in 50 year design return period is taken.

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM. Qp = 0.278 C i A Where $Qp = \text{peak runoff in m}^3/\text{s}$

I = rainfall intensity in mm/hr

A = catchment area in km²

Runoff Estimation

U-Channel	Natural Catch. (m²)	Longest flow path (m)	Gradient (m per 100m)	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	Length of U-Channel (m)	t _f = L/v (min)	tc = to + t _f (min)	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA (m³/s)
UC1 to UC 2	10174	130	0.014	17.48	90	1.29	18.77	0.25	10174	168.55	0.119
UC3 to UC 7	0	-	-	-	160	2.30	21.07	0.25	10174	162.72	0.115

Runoff Estimation (Structure Roofing)

Roofing	Natural Catch. (m²)	Longest flow path (m)	Gradient (m per 100m)	to (min) = 0.14465L/ (H ^{0.2} A ^{0.1})	Length of Ditch (m)	t _f = L/v (min)	tc = to + t _f (min)	Runoff coeff.	Total Catch. Area (m²)	50 year Intensity (mm/hr)	50 year design runoff = 0.278CiA (m³/s)
Roofing Gutter	2824	26	0.010	4.27	180	1.63	5.90	1	2824	229.99	0.181

Mannin	gs (Asia) Consultants Ltd.	Job No.	Sheet No.	Rev.	
Calculation	on Sheet	Member / Location			
Job Tilte:	Proposed Temporary Warehouse(Excluding Dangerous Goods Godown) with Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond	Drg. Ref.			
	and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories	Made By	NHL	Date	Chd.

Checking of Capacity (UC1 and UC2)

Input Data



0.1875

0.1875 0.375

Flow capacity, Q

$$Q = \frac{A \times r^{2/3} \times s^{1/2}}{n}$$

cross sectional area of flow (m²) where Α

0.125536 m²

hydraulic radius (m)

slope of the water surface or the linear hydraulic head loss (m/m)

Manning coefficient of roughness

Hydraulic radius

wetted perimeter (m) 0.96 m

0.13 *m*

Slope

$$s = 0.004 \text{ m/m}$$

Manning coefficient of roughness

$$n = 0.014$$

Therefore,

Q =
$$0.146 \text{ m}^3/\text{s}$$
 > Design runoff, OK!

$$V = Q/A = 1.16 \text{ m/s}$$

Mannin	gs (Asia) Consultants Ltd.	Job No.	Sheet No.	Rev.	
Calculation	on Sheet	Member / Location			
Job Tilte:	Proposed Temporary Warehouse(Excluding Dangerous Goods Godown) with Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond	Drg. Ref.			
	and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories	Made By	NHL	Date	Chd.

Checking of Capacity (UC3 to UC7)

Input Data



0.1875

0.1875

Flow capacity, Q

$$Q = \frac{A \times r^{2/3} \times s^{1/2}}{n}$$

where A = cross sectional area of flow (m^2)

0.125536 m²

0.375

r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

n = Manning coefficient of roughness

Hydraulic radius

p = wetted perimeter (m) = 0.96 m

r = 0.13 m

Slope

$$s = 0.004 \text{ m/m}$$

Manning coefficient of roughness

$$n = 0.014$$

Therefore,

Q =
$$0.146 \text{ m}^3/\text{s}$$
 > Design runoff, OK!

$$V = Q/A = 1.16 \text{ m/s}$$

Mannin	gs (Asia) Consultants Ltd.	Job No.	Sheet No.	Rev.	
Calculation	on Sheet	Member / Location			
Job Tilte:	Proposed Temporary Warehouse(Excluding Dangerous Goods Godown) with Ancillary Facilities for A Period of 3 Years and Associated Filling of Land and Pond	Drg. Ref.			
	and in "Agriculture" Zone, Various Lots in D.D. 107 and Adjoining Government Land, Fung Kat Heung, Kam Tin Yuen Long, New Territories	Made Bv	NHL	Date	Chd.

Checking of Capacity (Gutter)

Input Data

Width of UC = 0.375 m Height of UC = 0.375 m Design Runoff = 0.181 m $^3/s$ ($Q_{discharge}$)

0.1875

Flow capacity, Q

$$Q = \frac{A \times r^{2/3} \times s^{1/2}}{n}$$

where A = cross sectional area of flow (m²)

0.125536 m²

0.375

r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

m = Manning coefficient of roughness

Hydraulic radius

p = wetted perimeter (m) = 0.96 m

r = 0.13 m

Slope

$$s = 0.010 \text{ m/m}$$

Manning coefficient of roughness

$$n = 0.014$$

Therefore,

Q =
$$0.230 \text{ m}^3/\text{s}$$
 > Design runoff, OK!

V = Q/A = 1.84 m/s

Stormwater Drainage Design

Man	nhole	Catchm	ent Area		Nominal	Grad	ient, S _f	Roughness		Time of	Rainfall	50 year		50 year	Total		Adjusted	Cove	r Level	Inver	rt Level
From	То	Increment (m ²)	Accu. (m²)	Length (m)	Diameter (mm)	(%)	1 in	Coefficient (m)	Velocity (m/s)	Flow (min)	Duration (min)	Intensity (mm/hr)	Runoff Coeff.	Runoff (m³/s)	Flow (m³/s)	Capacity (m³/s)	Capacity > Total Flow ?	From (mPD)	To (mPD)	From (mPD)	To (mPD)
Unpaved Area							•														
CP7	MH 1	0	10174 0	30	450	0.5	200.0	3.0	1.093	0.46	21.53	161.65	0.25 1.00	0.114 0.000	0.114	0.156	Yes	5.70	4.30	3.85	3.70
MH 1	MH 2	0	10174 0	30	450	0.5	200.0	3.0	1.093	0.46	21.98	160.61	0.25 1.00	0.114 0.000	0.114	0.156	Yes	4.30	4.30	3.69	3.54
MH 2	Outfall 1	0	10174 0	6	450	0.5	200.0	3.0	1.093	0.09	22.08	160.40	0.25 1.00	0.113 0.000	0.113	0.156	Yes	4.30	5.20	3.53	3.50
Checking Existing F	Pipe			•												•	•				
Outfall 1	Existing Pipe	19500 0	29674 0	8	750	0.3	400.0	3.0	1.079	0.12	22.20	160.13	0.25 1.00	0.330 0.000	0.330	0.429	Yes	4.30	5.20	3.21	3.19
Structure Roofing	ng	-	_			ı															
Structure Roofir 450 Elevated Pipe	1	0	0	45	450	0.5	200.0	0.06	1.683	0.45	3.45	256.83	0.25	0.000	0.202	0.241	Yes	6.20	5.20		
		2824	2824										1.00	0.202							
525 DOWNPIPE	MH 3	0 5648	0 5648	18	525	1.1	94.7	0.06	2.730	0.11	6.01	229.02	0.25	0.000	0.360	0.532	Yes	5.20	5.40	5.90	5.71
MH 3	MH 4	0	0 5648	32	525	2.5	40.0	3.0	2.709	0.20	6.21	227.32	0.25 1.00	0.000 0.357	0.357	0.528	Yes	5.40	4.60	4.61	3.81
MH 4	Outfall 2	0	0 5648	14	525	1.8	56.0	3.0	2.289	0.10	6.31	226.46	0.25 1.00	0.000 0.356	0.356	0.446	Yes	4.60	4.60	3.80	3.55
Checking Existing F	Pipe																				
Outfall 2	Existing Pipe	1245 0	1245 5648	8	750	0.2	421.1	3.0	1.052	0.13	19.99	165.38	0.25 1.00	0.014 0.260	0.274	0.418	Yes	4.30	5.20	3.30	3.28

Mean Velocity is calculated by Colebrook- White equation

Where:

 \overline{V} =Mean Velocity (m/s)

R =Hydraulic Diameter (m)

Ks =Surface Roughness (m)

V =Kinematic viscosity (kg/ms)

Sf =Slope of Hydraulic Gradient

g =Gravity (m/s²)

The Roughness Coefficient Ks is assumed to be 3 for concrete.

Peak Runoff is estimated using rational method according to SDM.

The Roughness Coefficient Ks is assumed to be 3 for concrete, 0.06 for uPVC pipe.

 $\overline{V} = -\sqrt{32gRS_f} \log \left[\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$



Appendix C

Site Photos

V1094/01 Issue 2 Drainage Impact Assessment

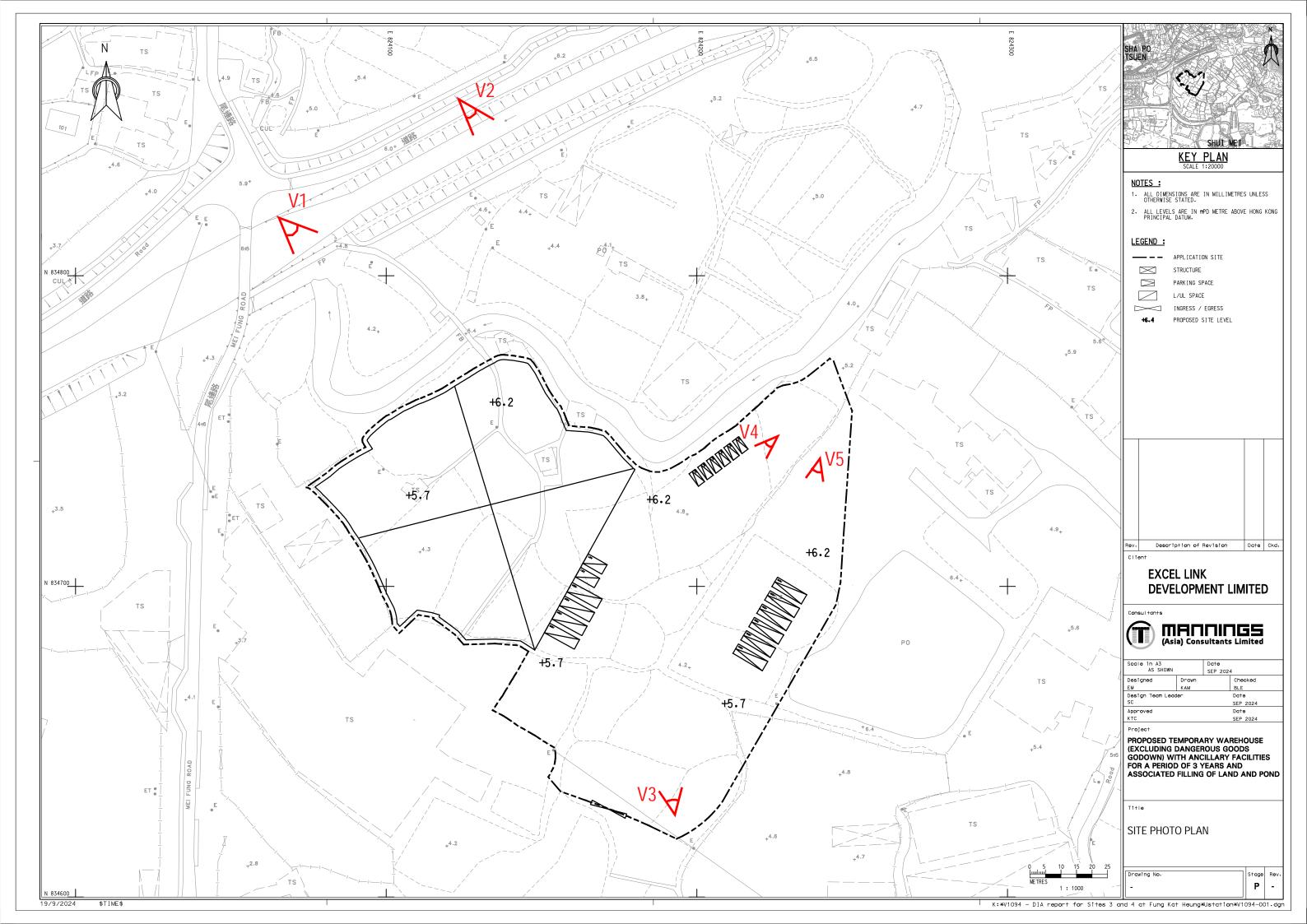




Photo V1



Photo V2





Photo V3



Photo V4





Photo V5

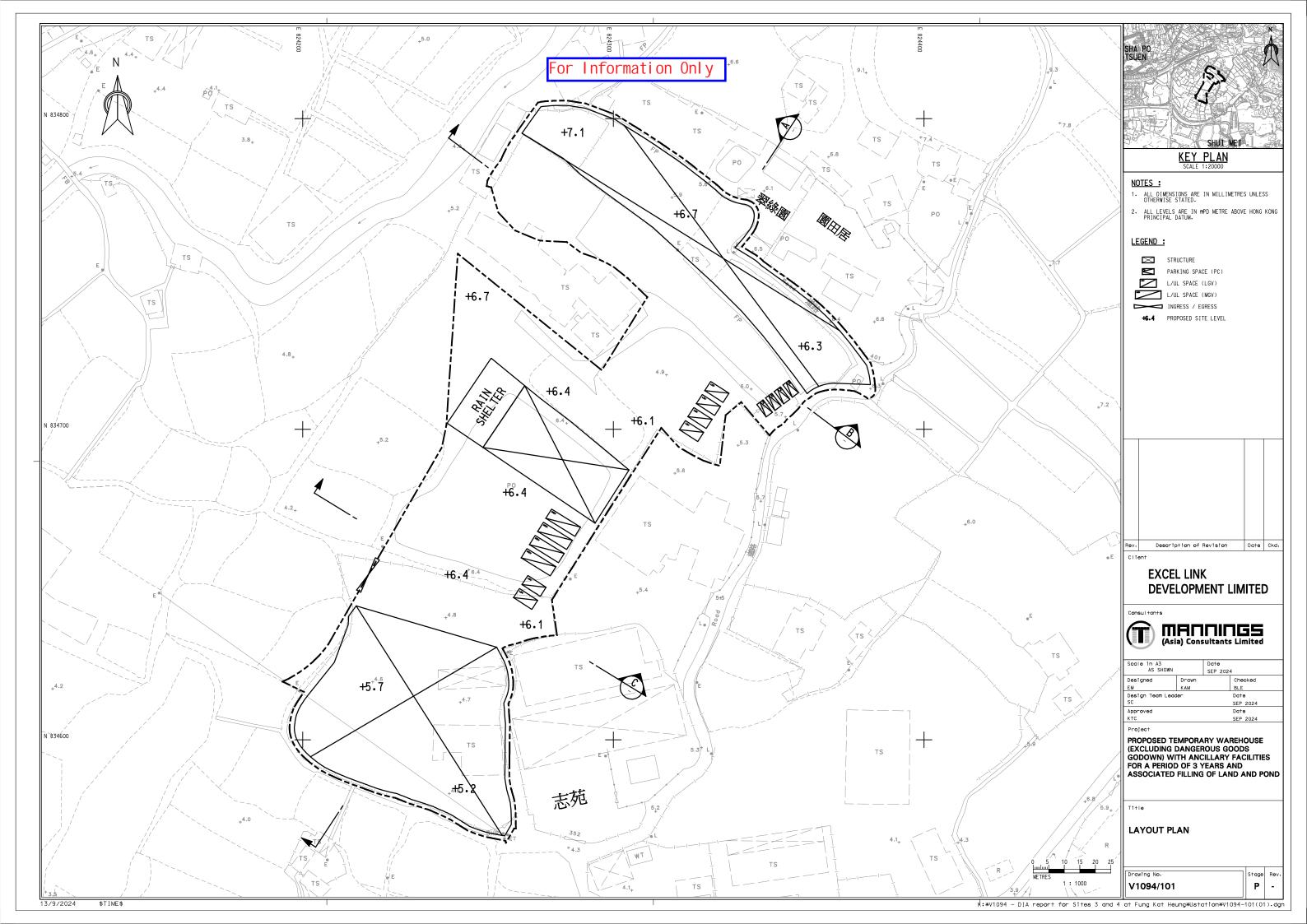


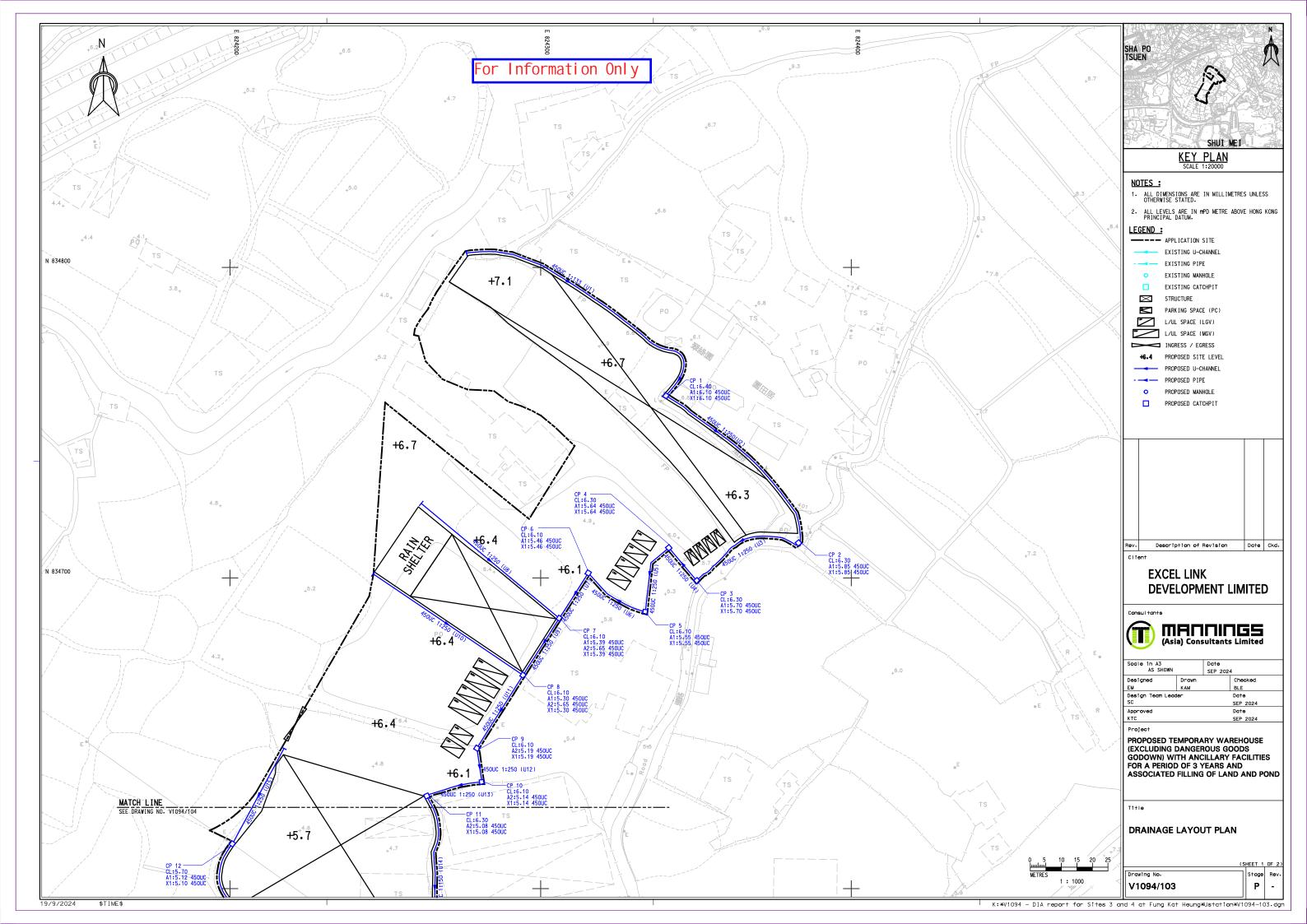


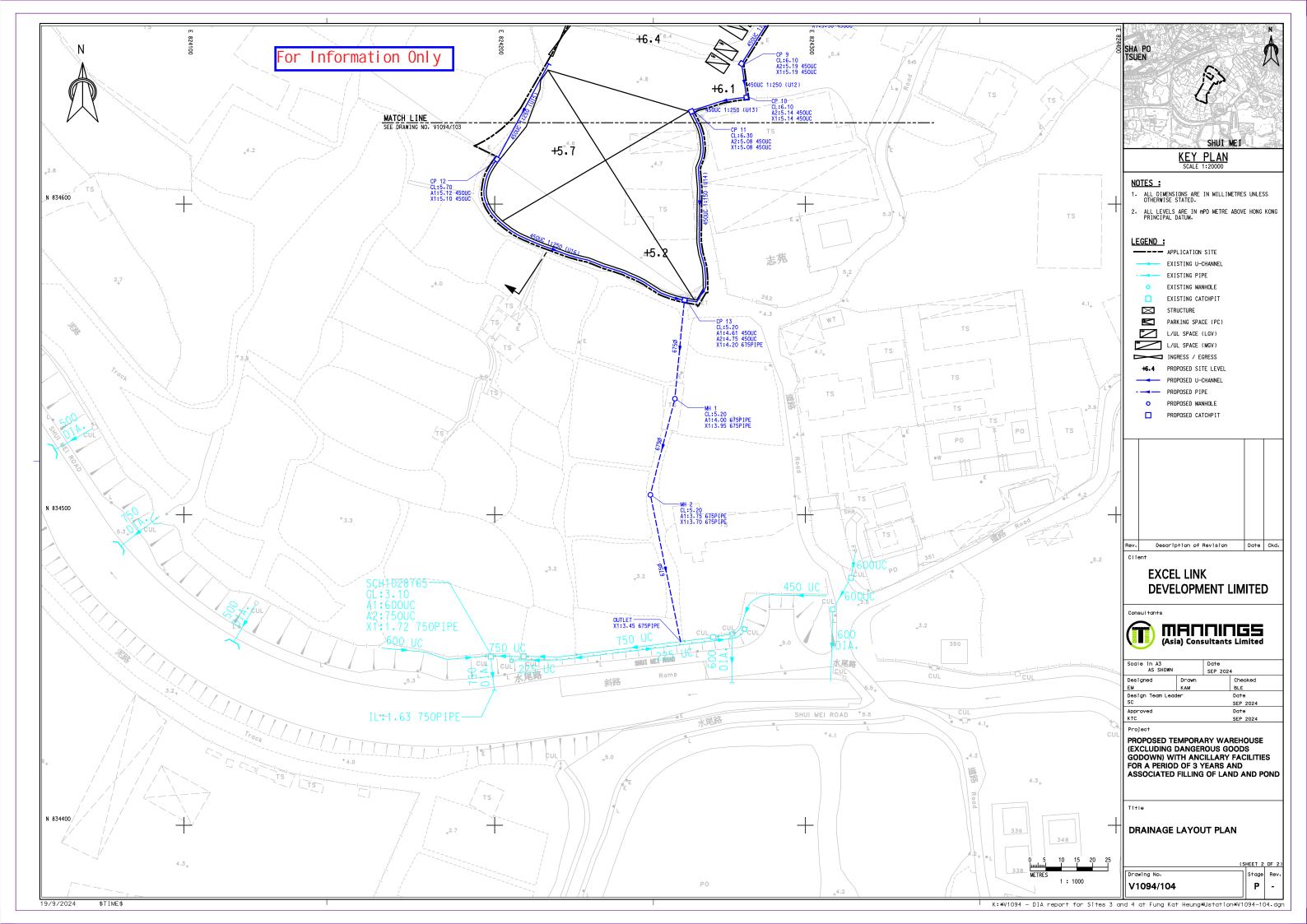
Appendix D

Layout Plans of Future Development for Adjacent Area

V1094/01 Issue 2 Drainage Impact Assessment

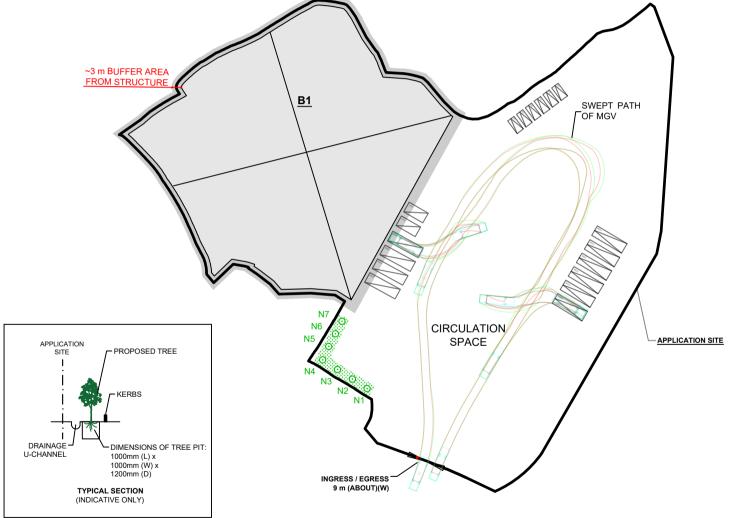






LANDSCAPE PROPOSAL	45.000 2	(ADQUE)	 STRUCTURE	USE	COVERED AREA	GFA	BUILDING HEIGHT
APPLICATION SITE AREA COVERED AREA UNCOVERED AREA	: 15,822 m ² : 5,648 m ² : 10,174 m ²	(ABOUT) (ABOUT) (ABOUT)	B1	WAREHOUSE (EXCLUDING D.G.G.) SITE OFFICE AND WASHROOM	5,648 m ² (ABOUT)	5,648 m ² (ABOUT)	13 m (ABOUT)(1-STOREY)
NO. OF NEW TREES WILL BE PL SPECIES OF NEW TREES HEIGHT OF NEW TREES SPACING OF NEW TREES DIMENSION OF SOIL TRENCH	: POLÝSF : NO LES : NOT LE	1 TO N7) PORA AXILLARIS S THAN 2.75 m SS THAN 4 m V) X 1.2 m (D)		TOTAL	<u>5,648 m² (ABOUT)</u>	<u>5,648 m² (ABOUT)</u>	
					`		







PROPOSED WAREHOUSE (EXCLUDING DANGEROUS GOODS GODOWN) WITH ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF LAND AND POND

LEGEND

APPLICATION SITE

PARKING SPACE (PC)

MEDIUM GOODS VEHICLE SWEPT PATH OF VEHICLE PROPOSED NEW TREES

CONTINUOUS SOIL TRENCH

L/UL SPACE (LGV)

L/UL SPACE (MGV)

STRUCTURE

VARIOUS LOTS IN D.D. 107 AND ADJOINING GOVERNMENT LAND, FUNG KAT HEUNG, KAM TIN, YUEN LONG, NEW TERRITORIES

CALE	
: 1200 @ A4	
RAWN BY	DATE
CAWN BY	DATE
T	16.7.2024
EVISED BY	DATE
I VISED BT	DATE
PPROVED BY	DATE
-FROVED B1	DATE
NG. TITLE	

LANDSCAPE PROPOSAL

ANNEX III 001

THE APPLICANT WILL MAINTAIN TREES IN GOOD CONDITION DURING THE PLANNING APPROVAL PERIOD.

- THE APPLICANT WILL REPLACE TREES WHICH ARE DYING OR DEAD DURING THE PLANNING APPROVAL PERIOD.
- THE APPLICANT WILL PROVIDE ADEQUATE IRRIGATION FOR TREES.