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PLANNING APPLICATION FOR PROPOSED COMPREHENSIVE DEVELOPMENT SCHEME TO INCLUDE WETLAND RESTORATION PROPOSAL AND PROPOSED FILLING OF PONDS/LAND AND EXCAVATION OF LAND IN "OU(CDWRA)" ZONE AT VARIOUS LOTS IN D.D. 104, NORTH OF KAM POK ROAD EAST, POK WAI, YUEN LONG, NEW TERRITORIES

DRAINAGE IMPACT ASSESSMENT



Planning Application for Proposed Comprehensive Development Scheme to include Wetland Restoration Proposal and Proposed Filling of Ponds/Land and Excavation of Land in "OU(CDWRA)" Zone at Various Lots in D.D. 104, North of Kam Pok Road East, Pok Wai, Yuen Long, New Territories

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1. INTRODUCTION

- 1.1.1 The applicant proposes to develop the Application Site at various lots in DD104, north of Kam Pok Road East, Yuen Long, into a residential development cum wetland restoration area. The zoning of the Application Site is "Other Specified Uses Comprehensive Development to include Wetland Restoration Area" (OU(CDWRA)) on the approved Nam Sang Wai Outline Zoning Plan S/YL-NSW/8. A S16 application is required for the proposed development.
- 1.1.2 Ramboll Hong Kong Limited has been commissioned by the Project Proponent to conduct the said Drainage Impact Assessment (DIA) for the proposed development under this application. Architectural drawings and technical information of the Development Site were largely provided respectively by the project architect and other project team members.
- 1.1.3 The Application Site is also the subject of a previous planning application under the application no. A/YL-NSW/290 and a DIA report (R7192) was previously submitted in support of that planning application (Previous DIA). Since then, the layout plan of proposed development has been further reviewed taking into account the concerns of AFCD with respect to the layout of proposed wetland restoration area. Compared to the previous scheme in Previous DIA, the application boundary in current application remains the same. Thus, this DIA serves as an update to the above-mentioned previous submitted DIA report based on the current revised development scheme as requested by AFCD.

1.2 Application Site and its Environs

- 1.2.1 The Application Site is about 51,073 m², and it is immediate southeast of an existing low-rise residential development, Man Yuen Chuen and north of the Kam Pok Road East. The Application Site is partly occupied by abandoned ponds and partly by soil ground.
- 1.2.2 **Figure 1.1** shows the location of the Application Site and the environs.

1.3 Proposed Development

- 1.3.1 The original development scheme comprises a total of 114 units in 108 housing blocks of 3 to 5-storey high (i.e. 89 in the form of 2- to 4-storey on top of 1-level of communal basement carpark and 25 in 2-storey on top of 1-level of carport), two 2-storeys clubhouses, an underground sewage pumping station (SPS) and a proposed wetland restoration area (WRA). To respond to AFCD's comments, amendments to the original MLP are hence required. As a result, the proposed Wetland Restoration Area (WRA) has been increased and there is a net reduction in total no. of units have been reduced to 90 units in 84 housing blocks varying from 2-storeys to 4-storeys on top of 1-level of carport.
- 1.3.2 The indicative revised Master Layout Plan (MLP) and sections of the Proposed Development are included in planning statement.



2. DRAINAGE IMPACT ASSESSMENT ("DIA")

2.1 Scope of Work

2.1.1 The aim of this study is to assess the changes to runoff from the Application Site as a result of the proposed development and the potential impacts on the existing drainage system and surrounding areas. Drainage Record Plans from the Drainage Services Department (DSD) were obtained for the information of drainage impact assessment.

2.2 Assessment Criteria and Methodology

- 2.2.1 The assessment standard complies with the Stormwater Drainage Manual (2022 Edition) published by DSD (DSD SDM). The Site is located within an urban drainage branch system and a 1 in 50 years return storm has therefore been adopted for the DIA.
- 2.2.2 The catchment runoff has been calculated using the "Rational Method", as outlined in the DSD SDM:

Q = 0.278 C i A

| Where | Q | = | peak runoff in m ³ /s |
|-------|---|---|------------------------------------|
| | С | = | runoff coefficient (dimensionless) |
| | i | = | rainfall intensity in mm/hr |
| | Α | = | catchment area in km |
| | | | |

- 2.2.3 The existing Site consists of a group of abandoned ponds of around 4.9 ha in total. According to the latest development layout, the proposed development comprises houses. In the site inspection, it has been noted that parts of the Application Site have been paved concrete and pantry, unpaved soft landscape around intermixed among fishponds. Runoff coefficient of 0.95 has been adopted for the paved areas and 0.35 has been adopted for unpaved areas (i.e. soft landscape). A runoff coefficient of 0.35 has been adopted for fishponds in the existing scenario and a coefficient of 1.0 has been adopted for fishponds and the proposed WRA in the proposed scenario. A summary of catchment characteristics is provided in **Appendix 2.1**.
- 2.2.4 The design criteria for flood level depends on a combination of rainstorm event and tidal level, as well as the catchment characteristics. With reference to the Table 11 of DSD SDM, the determination of flood level is provided in **Table 2.1**.

| Flood Level Return Period Scenarios | Rainfall Return Period (year) | Sea Level Return Period (year) |
|--|----------------------------------|-----------------------------------|
| 50-years A (Case I) | 50 | 10 |
| 50-years B (Case II) | 10 | 50 |

Table 2.1Determination of Flood Level



2.2.5 With the reference to Table 28 of DSD SDM, as extracted in **Table 2.2** below, rainfall increase and sea level rise due to climate change has been considered in the calculations. As 1 in 50 years return storm is adopted for the assessment, the mid-21st century value has been considered.

Table 2.2 Rainfall Increase and Sea Level Rise due to Climate Change

| | Rainfall Increase | Sea Level Rise (m) |
|---|-------------------|--------------------|
| Mid-21 st Century (2041 – 2060) | 11.1% | 0.20 |

2.3 Existing Drainage and Condition of the Application Site

- 2.3.1 According to the Drainage Record Plans obtained from DSD, there is no public drainage in the immediate vicinity of the Site. There is a drainage ditch to the west of the side, which runs northwards along the eastern perimeter of Ma Yuen Chuen. The drainage ditch turns westward before discharging into Ngau Tam Mei Drainage Channel.
- 2.3.2 Based on the ground levels shown on the basemaps, surface runoff within the Site runs westwards, entering the abovementioned drainage ditch. For the ponds outside Site to the north and northeast of the Site, and the construction site and open storage area to the east of the site, the existing ground level indicates that surface runoff would run westwards as well then enter the above-mentioned drainage ditch. For the Hong Kong and China Gas Company Limited Yuen Long Open Yard to the south of the Site, runoff runs towards the east and south into another drainage ditch, which discharges to Ngau Tam Mei Drainage Channel. For Ma Yuen Chuen and the open storage area to the west of the Site, based on site observation, there are gullies along Kam Pok Road collecting runoff from these areas. The runoff then discharges to Ngau Tam Mei Drainage Channel as well.
- 2.3.3 The existing drainage system in the vicinity of the Application Site and the direction of runoff flow are shown in **Figure 2.1**. Sections of the existing drainage ditch to the west of the Site are shown in **Appendix 5.1**. For the purpose of this study, this DIA has been prepared based on a proposed new drainage system for the proposed development as described in below paragraphs. During later detailed design stage, availability of the existing drainage system as shown in **Figure 2.1** and its maintenance responsibility to cater for discharge from the proposed development, will be further reviewed where appropriate.

2.4 Proposed Development and Drainage System

- 2.4.1 A set of 800mm peripheral drain is proposed along the east of the application site boundary to divert the surface runoff from Catchments C2 and C3 and to maintain the flow as its original flow direction.
- 2.4.2 A set of peripheral drains are proposed across the Site to collect stormwater runoff from the Proposed Development, Catchment C2 and Catchment C3. It runs to the proposed standard sized drains (at **Appendix 2.3**).
- 2.4.3 New drainage pipes are proposed along Kam Pok Road East towards west and then to Kam Pok Road. The storm water will be directly discharge to the Ngau Tam Mei Channel.
- 2.4.4 The designated location of the outlet has avoided the existing retaining walls and the cycle track underpass.



- 2.4.5 Locations of existing drainage system for the proposed development (S1 to S27) are shown in **Appendix 5.1** and in **Figure 2.1**. Calculation of drainage discharge under existing condition is shown in **Appendix 2.2**. The boundary conditions of the application site provided by DSD is shown in **Appendix 4.1**.
- 2.4.6 Upon the completion of the Proposed Development, the Site would consist of residential blocks, clubhouse, driveways/EVAs, and a WRA. Except the WRA, the other areas will be paved. With the adoption of Rational Method, the WRA is considered as paved area under a conservative perspective in the hydraulic analysis/calculation. Consideration has been given to the rainfall intensity and sea level rise due to climate change effect when calculating the drainage discharge under backwater effect, which are demonstrated in **Appendix 2.3** and **Appendix 3.1**. The drainage discharge still works under backwater effect. Hydraulic checking of peripheral drain is provided in **Appendix 2.4**.
- 2.4.7 The indicative location of peripheral drains within the Application Site, and the proposed drains are indicated in **Figure 3.1**.



3. OVERALL CONCLUSION

- 3.1.1 A residential development cum wetland restoration area is proposed for the Application Site at various lots in DD104, north of Kam Pok Road, Yuen Long. The potential drainage impact has been quantitatively addressed.
- 3.1.2 The Proposed Development will increase the runoff generated from the Application Site. Runoff from the Proposed Development and nearby Catchments to the east of the Site will be conveyed to the proposed drainage system, and then discharging into Ngau Tam Mei Drainage Channel.
- 3.1.3 Temporary drainage measures shall be implemented to ensure that the flooding conditions will not be worsened during construction. Periodic inspection by the Authorized Person or his representative will be carried out during construction.
- 3.1.4 With the implementation of the above proposed drainage measures and temporary drainage works during construction, if any, the Proposed Development at the Application Site is technically feasible, having no insurmountable impact from drainage point of view.



Figures







\\APHKFPS3\DRIVE Q\PROJECTS\FDBNPWWREA00\04 DELIVERABLES\02 DIA\01 FIGURE\R7192_V4.0\SOURCE\FIGURE 2.DWG



File : Y:\Daily Work\20230911A\Figure3.2.dgn

| | NAM | |
|------------|-------------|-----------|
| | Drawn by: | CL |
| | Checked by: | NH |
| of Land in | Rev: | 6.1 |
| | Date: | Dec.,2023 |







\APHKFPS3\DRIVE Q\PROJECTS\FDBNPWWREA00\04 DELIVERABLES\03 SIA\01 FIGURE\R7191 V6.1\SOURCE\FIGURE 2.1 PROPOSED SEWER SCENARIO 2 220812.DWG

Appendix 1.1 Indicative Master Layout Plan of the Proposed Development

Please refer to the Planning Statement



Appendix 2.1 Summary of Catchment Characteristic



Summary of Catchment Characteristic

| | Catchment | Area | Coverage | Runoff Coefficient |
|------------------|--|---------------------------|-------------------------|-----------------------|
| Fxisting | Pond | 49.000.0 | 96% | 0.35 |
| (Site) | Unpaved pond bund / footpath | 2.073.3 | 4% | 0.35 |
| | Total | 51,073.3 | | 0.00 |
| Future (Site) | Paved area Wetland Restoration Area (WRA) Uncovered horizontal planting area, grass paver | 20,582 24,702 5,789 | 40.3% 48.4% 11.3% | 0.95 1.00 0.35 |
| | Total | 51,073.3 | | |
| C1 | Pond | 2191.3 | 79% | 1.00 |
| | Unpaved pond bund / footpath | 593.5 | 21% | 0.35 |
| | Total | 2,784.8 | | |
| C2 | Construction site for village houses | 4365.1 | 40% | 0.95 |
| | Open storage area | 6,547.6 | 60% | 0.35 |
| | Total | 10,912.7 | | |
| C3 | Pond | 5,954.1 | 81% | 1.00 |
| | Unpaved pond bund / footpath | 942.4 | 13% | 0.35 |
| | Village houses | 497.7 | 7% | 0.95 |
| | Total | 7,394.2 | | |
| C4 | Pond | 15,359.8 | 70% | 1.00 |
| | Unpaved pond bund / footpath | 6,657.0 | 30% | 0.35 |
| | Total | 22,016.8 | | |
| C5 | Pond | 626.5 | 2% | 1.00 |
| | Village houses / paved area | 30,333.1 | 96% | 0.95 |
| | Landscaping area | 660.0 | 2% | 0.35 |
| | Total | 31,619.6 | | |
| C6 | Pond | 734.0 | 29% | 1.00 |
| | Temporary structure / paved area | 1,824.5 | 71% | 0.95 |
| | Total | 2,558.5 | | |
| C7 | Pond | 537.2 | 86% | 1.00 |
| | Temporary structure / paved area | 86.2 | 14% | 0.95 |
| | Total | 623.4 | | |
| C8 | Unpaved area | 340.3 | 20% | 0.35 |
| | Paved area | 1,355.0 | 80% | 0.95 |
| | Total | 1,695.3 | | |

Appendix 2.2 Drainage Impact Assessment Calculations under Existing Condition



Appendix 2.2 Drainage Impact Assessment Calculations under Existing Condition

Note:

1) Colebrook-White's equation is adopted for full-bore pipe velocity calculation; Manning's equation is adopted for non-circular features.

| | | | | | | | | | | | Runoff from | Full Flow | Full Flow | Cumulative | |
|------|--------|---------|---------|-------------|-------------|--------------|------------|-----------|------------|------------------|-------------|------------------|---------------------|-------------|-------------|
| | | US I.L. | DS I.L. | | | Bottom Width | | Top Width | Side Slope | | Catchment | Velocity | Capacity | Runoff | |
| From | То | (mPD) | (mPD) | No. of pipe | Shape | (m) | Height (m) | (m) | (m/m) | Inflow Catchment | (m^{3}/s) | (m/s) | (m ³ /s) | (m^{3}/s) | Utilization |
| S1 | S2 | 2.96 | 2.55 | 1 | Trapezoidal | 1.47 | 0.70 | 2.54 | 1.53 | C1, C6, C7 | 0.293 | 7.221 | 8.771 | 8.771 | 3.3% |
| S2 | S3 | 2.55 | 2.48 | 1 | Trapezoidal | 2.38 | 1.27 | 4.64 | 1.78 | - | 0.000 | 4.207 | 14.878 | 14.878 | 2.0% |
| S3 | S4 | 2.48 | 2.54 | 1 | Trapezoidal | 2.40 | 1.17 | 3.89 | 1.27 | - | 0.000 | 4.106 | 14.349 | 14.349 | 2.0% |
| S4 | S5 | 2.54 | 2.41 | 1 | Trapezoidal | 0.61 | 0.91 | 5.15 | 2.49 | C8 | 0.104 | 3.405 | 2.718 | 2.718 | 14.6% |
| S5 | S6 | 2.41 | 2.49 | 1 | Trapezoidal | 1.44 | 1.16 | 5.08 | 1.57 | - | 0.000 | 3.971 | 9.035 | 9.035 | 4.4% |
| S6 | S7 | 2.49 | 2.44 | 1 | Trapezoidal | 1.06 | 1.13 | 4.51 | 1.53 | - | 0.000 | 2.974 | 5.445 | 5.445 | 7.3% |
| S7 | S8 | 2.44 | 2.41 | 1 | Trapezoidal | 1.02 | 1.01 | 3.87 | 1.41 | - | 0.000 | 2.022 | 3.191 | 3.191 | 12.4% |
| S8 | S9 | 2.41 | 2.40 | 1 | Trapezoidal | 1.03 | 0.53 | 2.92 | 1.78 | - | 0.000 | 0.929 | 0.588 | 0.588 | 67.4% |
| S9 | S10 | 2.40 | 2.39 | 1 | Trapezoidal | 1.44 | 0.68 | 4.63 | 2.35 | - | 0.000 | 1.055 | 1.117 | 1.117 | 35.5% |
| S10 | S11 | 2.39 | 2.37 | 1 | Trapezoidal | 1.49 | 0.72 | 4.63 | 2.18 | C2, C3, Site | 1.800 | 1.599 | 1.886 | 1.886 | 116.5% |
| S11 | S12 | 2.37 | 2.27 | 1 | Trapezoidal | 1.60 | 0.76 | 3.90 | 1.51 | - | 0.000 | 3.715 | 5.343 | 5.343 | 41.1% |
| S12 | S13 | 2.27 | 2.28 | 1 | Trapezoidal | 0.98 | 1.10 | 5.26 | 1.95 | - | 0.000 | 1.227 | 1.878 | 1.878 | 117.0% |
| S13 | S14 | 2.28 | 2.26 | 1 | Trapezoidal | 1.44 | 1.67 | 7.06 | 1.68 | - | 0.000 | 2.332 | 8.527 | 8.527 | 25.8% |
| S14 | S15 | 2.26 | 2.25 | 1 | Trapezoidal | 0.44 | 0.32 | 0.76 | 0.50 | C4 | 0.178 | 0.646 | 0.201 | 0.201 | 1183.0% |
| S15 | S16 | 2.25 | 2.20 | 1 | Trapezoidal | 0.40 | 0.36 | 0.70 | 0.42 | - | 0.000 | 1.502 | 0.615 | 0.615 | 386.3% |
| S16 | S17 | 2.20 | 2.00 | 1 | Trapezoidal | 0.50 | 0.48 | 2.44 | 2.02 | - | 0.000 | 3.188 | 1.016 | 1.016 | 233.9% |
| S17 | S18 | 2.00 | 1.73 | 1 | Trapezoidal | 0.61 | 0.54 | 3.03 | 2.24 | - | 0.000 | 4.075 | 1.686 | 1.686 | 140.9% |
| S18 | S19 | 1.73 | 1.61 | 1 | Trapezoidal | 0.62 | 0.94 | 5.32 | 2.50 | - | 0.000 | 2.855 | 2.406 | 2.406 | 98.7% |
| S19 | S20 | 1.61 | 1.59 | 1 | Trapezoidal | 2.24 | 1.09 | 6.94 | 2.16 | - | 0.000 | 1.972 | 5.313 | 5.313 | 44.7% |
| S20 | S21 | 1.59 | 1.60 | 1 | Trapezoidal | 1.84 | 0.99 | 6.08 | 2.14 | - | 0.000 | 1.328 | 2.725 | 2.725 | 87.2% |
| S21 | S22 | 1.60 | 1.54 | 1 | Trapezoidal | 1.13 | 0.63 | 6.55 | 4.30 | - | 0.000 | 2.345 | 1.697 | 1.697 | 140.0% |
| S22 | S23 | 1.54 | 1.46 | 1 | Trapezoidal | 2.05 | 0.98 | 7.16 | 2.61 | - | 0.000 | 3.781 | 8.091 | 8.091 | 29.4% |
| S23 | S24 | 1.46 | 1.23 | 1 | Trapezoidal | 0.60 | 1.11 | 6.23 | 2.54 | - | 0.000 | 5.153 | 5.342 | 5.342 | 44.5% |
| S24 | S25 | 1.23 | 1.87 | 1 | Trapezoidal | 1.64 | 1.22 | 6.82 | 2.12 | - | 0.000 | 11.207 | 27.253 | 27.253 | 8.7% |
| S25 | S26 | 1.87 | 1.98 | 1 | Trapezoidal | 2.61 | 1.17 | 5.01 | 1.03 | - | 0.000 | 5.357 | 21.158 | 21.158 | 11.2% |
| S26 | S27 | 1.98 | 1.32 | 1 | Trapezoidal | 1.89 | 1.57 | 9.32 | 2.37 | - | 0.000 | 14.301 | 51.600 | 51.600 | 4.6% |
| S27 | Outlet | 1.32 | 1.58 | 1 | Trapezoidal | 3.23 | 2.96 | 8.49 | 0.89 | C5 | 2.087 | 19.690 | 344.174 | 344.174 | 1.3% |

Remark: Please refer to Figure 2.1 for the location of existing drainage system, and Appendix 5.1 for the locations of S1 to S27.

Appendix 2.3 Drainage Impact Assessment Calculations under Proposed Condition



Appendix 2.3 Drainage Impact Assessment Calculations under Proposed Condition

Note:

1) Colebrook-White's equation is adopted for full-bore pipe velocity calculation; Manning's equation is adopted for non-circular features.

| | | | | | | | | | | | | Runoff from | Flow | Full Flow | Cumulative | |
|------|--------|---------|---------|-------------|----------|--------------|------------|-----------|------------|---------------|------------------|-------------|----------|---------------------|------------|-------------|
| | | US I.L. | DS I.L. | | | Bottom Width | | Top Width | Side Slope | | | Catchment | Velocity | Capacity | Runoff | |
| From | То | (mPD) | (mPD) | No. of pipe | Shape | (m) | Height (m) | (m) | (m/m) | Pipe Size (m) | Inflow Catchment | (m^{3}/s) | (m/s) | (m ³ /s) | (m^3/s) | Utilization |
| S1 | S2 | 1.94 | 1.89 | 1 | Circular | - | - | - | - | 1.60 | C2, C3, Site | 3.262 | 2.619 | 4.739 | 3.262 | 68.8% |
| S2 | S3 | 1.89 | 1.58 | 1 | Circular | - | - | - | - | 1.50 | - | 0.000 | 2.411 | 3.835 | 3.262 | 85.0% |
| S3 | S4 | 1.58 | 1.18 | 1 | Circular | - | - | - | - | 1.50 | - | 0.000 | 2.448 | 3.894 | 3.262 | 83.8% |
| S4 | S5 | 1.18 | 0.85 | 1 | Circular | - | - | - | - | 1.45 | | 0.000 | 2.375 | 3.530 | 3.262 | 92.4% |
| S5 | S6 | 0.85 | 0.50 | 1 | Circular | - | - | - | - | 1.45 | - | 0.000 | 2.450 | 3.641 | 3.262 | 89.6% |
| S6 | S7 | 0.50 | 0.15 | 1 | Circular | - | - | - | - | 1.45 | - | 0.000 | 2.422 | 3.599 | 3.262 | 90.6% |
| S7 | S8 | 0.15 | -0.08 | 1 | Circular | - | - | - | - | 1.45 | - | 0.000 | 2.500 | 3.716 | 3.262 | 87.8% |
| S8 | Outlet | -0.08 | -0.21 | 1 | Circular | - | - | - | - | 1.45 | - | 0.000 | 2.475 | 3.678 | 3.262 | 88.7% |

Remark: Please refer to Figure 3.1 for the location of proposed drainage system.

Appendix 2.4 Hydraulic Checking of Peripheral Drain (Most Critical Section)

Appendix 2.4 Hydraulic Checking of Pheripherial Drain for most cirtical section

Note:

1) Colebrook-White's equation is adopted for full-bore pipe velocity calculation; Manning's equation is adopted for non-circular features.



| m nt | Full Flow Velocity (m/s) | Full Flow Capacity (m ³ /s) | Cumulative Runoff (m ³ /s) | Utilization |
|---------|--------------------------------|--|---|-------------|
| | 2.093 | 0.947 | 0.575 | 60.8% |

Most critical section

Appendix 3.1 Calculation of Backwater Effect



Appendix 3.1 Backwater Analysis for the Proposed Application Site

| Appendix 3.1 |
|--------------|
|--------------|

| | Size | 9 | | Hydraulics | | | | | | | | | |
|-------------------------|-------|--------|--------|------------|---------------------|-------------|--------------------|----------|-------|---------|-----------------|-------------|----------------------|
| Туре | Width | Height | Length | Radius | Flow | Velocity | Friction slope | Friction | Inlet | /Outlet | Water Level (1) | Ground | Check ⁽²⁾ |
| | (m) | (m) | (m) | (m) | (m ³ /s) | (m/s) | (S _f) | Loss | к | Loss | (mPD) | Level (mPD) | |
| | | | | | | | | | | | 4.563 | | |
| | | | | | | | | | 1 | 0.0011 | 4.56 | | |
| Proposed Circular Drain | 1.60 | 1.60 | 9.92 | 0.40 | 0.296666667 | 0.147549895 | 0.000016621 | 0.000 | | | 4.56 | 4.0 | |
| Proposed Circular Drain | 1.50 | 1.50 | 66.84 | 0.38 | 0.296666667 | 0.167878992 | 0.000023449 | 0.002 | | | 4.57 | 4.0 | |
| Proposed Circular Drain | 1.50 | 1.50 | 83.68 | 0.38 | 0.296666667 | 0.167878992 | 0.000023449 | 0.002 | | | | | |
| Proposed Circular Drain | 1.45 | 1.45 | 70.26 | 0.36 | 0.296666667 | 0.179656472 | 0.000028097 | 0.002 | | | 4.57 | 4.0 | |
| Proposed Circular Drain | 1.45 | 1.45 | 70.04 | 0.36 | 0.296666667 | 0.179656472 | 0.000028097 | 0.002 | | | | | |
| Proposed Circular Drain | 1.45 | 1.45 | 71.67 | 0.36 | 0.890000000 | 0.538969415 | 0.000252872 | 0.018 | | | 4.59 | 3.1 | |
| Proposed Circular Drain | 1.45 | 1.45 | 44.20 | 0.36 | 0.840000000 | 0.508690234 | 0.000225257 | 0.010 | | | | | |
| Proposed Circular Drain | 1.45 | 1.45 | 25.49 | 0.36 | 0.840000000 | 0.508690234 | 0.000225257 | 0.006 | | | 4.57 | 3.1 | |
| | | | | | | | | | 0.5 | 0.0008 | 4.57 | 5.1 | ОК |

Remark:

(1) With reference to the Appendix 4, flood level of 50B (PP with climate change) provided by DSD, the maximum water level of the Ngau Tam Mei Drainage Channel at the oulet is +4.563 mPD.

(2) According to Appendix 4, the boundary water level of Ngau Tam Mei River is already higher than the ground level, hence, the unfavourable condition is not a result of the Proposed Development. Thus, no mitigation works is proposed under this project.

Appendix 4.1 Boundary Condition provided by DSD













50A (PP with climate change)



50B (PP with climate change)

Appendix 5.1 Site Surveying of Existing Drainage Ditch





| LEGEND: Log CONTOURS E Electric Pole IV AUDIC PRE HR. ADDREVAILORS | 836670N | B33300E | $\begin{array}{c} & & & & & \\ & & & & \\$ | ⁺ 3.65 ⁺ 3.64 ⁺ 3.64 ⁺ 3.63 ⁺ 3.63 ⁺ ⁺ 3.67 ⁺ 3.67 ⁺ 3.67 ⁺ 3.67 ⁺ 3.67 | + POND | + 3.2 | PO + | | PO | + + | B36670N |
|--|---|--|---|--|--|---|---|--|-------------------|---|-----------------|
| ARTIFICIAL SLOPE SB SIGN BOARD SS SIGN BOARD SS SIGN POLE door - CHANNEL WITH FLOW DIRECTION DAGO WOOL OF CHANNEL TS TEMPORARY STRUCTURE SS SIGN POLE MANHOLE, HUTCHSON GLOBAL CROSSING LIMITED BROADBAND CUL CULVERT COLVERT COLVERT door - Dago Wool of Channel PB PILLAR BOX MANHOLE, MUTH FLOW DIRECTION Image: Coll Coll Coll Coll Coll Coll Coll Col | LEGEND: | - 130 CONTOURS | E • ELECTIC POLE | VALVE FIRE | H€L MANHOLE, HUTCHISON | ABBREVIATION: bd boulder be bern | | | | Fruit Design & F | Build Ltd. |
| OBSINCE UHANNUEL WiTH FLOW DIRCUTION PB PILAR BOX SIGN POLe Catch PIT Catch PIT Conc contraste 0 80 // 000 wide of step channel | | TS TEMPORARY STRUCTURE | SB SP SB | KI VALVE GAS | HC MANHOLE, HUTCHISON GLOBAL CROSSING LIMITED BROADBAND | BS BOUNDARY STONE CA CANOPY CUL CULVERT | SCALE 1 : 300 (SIZE | AO) | | Topoaraphical | Survey |
| OBD OBD <td>0.80 U-C CHANNEL WITH FLUW DIRECTION 0.80 WIDE OF CHANNEL ++>++> STEP CHANNEL WITH FLOW DIRECTION</td> <td>PB PILLAR BOX</td> <td>SIGN POLE</td> <td>MH MANHOLE</td> <td></td> <td>CO COLUMN CONC CONCRETE EB ELECTRIC BOX</td> <td>metres 0 6 12 18</td> <td>24 30 metres</td> <td></td> <td>Nam Sana</td> <td>Wai</td> | 0.80 U-C CHANNEL WITH FLUW DIRECTION 0.80 WIDE OF CHANNEL ++>++> STEP CHANNEL WITH FLOW DIRECTION | PB PILLAR BOX | SIGN POLE | MH MANHOLE | | CO COLUMN CONC CONCRETE EB ELECTRIC BOX | metres 0 6 12 18 | 24 30 metres | | Nam Sana | Wai |
| - GATE - KL- KERB LINE (BOTTOM) - KL- KERB LIN | 0.80 s-c 0.80 wide of step channel -·⊕·- BOUNDARY LINE & BOUNDARY POINT | — HR — HAND RAILING — HD — HOARDING | TP • TELEPHONE POLE | AFC MANHOLE, ATC | MANHOLE, STORM WATER | FB FLOWER BED FP FOOTPATH | | | | New Territo | ries |
| | CATE | — KL — KERB LINE (BOTTOM) | ● ^{IL} TRAFFIC LIGHT H | CATV MANHOLE, CABLE TV | PCEW MANHOLE, PACIFIC CENTURY CABLE & WIRELESS | NU NULLAH OH OVERHANG PL PLATFORM | HONG KONG METRIC GRIE | | | | |
| PRINCIPAL DATUM HONG KONG PARKING METER DEPARTMENT | PERMANENT BUILDING AND CANOPY | - ST - SLOPE TOP | FIRE HYDRANT PM PARKING METER | ECTI SERVICES DEPARTMENT HKTC MANHOLE, PCCW-HKT | N₩I MANHOLE, NEW WORLD TELEPHONE | PO POND PZ PIEZOMETER TUBE RP RAMP | PRINCIPAL DATUM HONG | KONG | | SAM MAK & ASSOCIATES SURVEYORS (HK) LTD | PRELIMINARY |
| With TREE WITH TREE NO. SB SLOPE BUILTON NUMBER ARIAL SURVEYS ALL UNITS IN LAND, ENGINEERING, MARINE & AERIAL SURVEYS Virtual Total | ₩ _{T1} TREE WITH TREE NO. | - FX - FENCE | BOLLARD AT ROAD | DSD MANHOLE, DRAINAGE | WF&I MANHOLE, CLP POWER | RU RUIN SS SHRINE TS TEMPORARY STRUCTURE | ALL UNITS IN METRES | | | ROOM 901, MARINA HOUSE, NO.68 HING MAN STREET, SHAUKEIWAN, HONG KONG | |
| INITIAL 10/01/2023 TEL: 28951918 FAX: 28901759 E-moil: hkoffice@sammak.com G GRAVE FR BARRIERS O GULLY INITIAL 10/01/2023 TEL: 28951918 FAX: 28901759 E-moil: hkoffice@sammak.com (10/01/2023) G GRAVE FR BARRIERS O GULLY INITIAL 10/01/2023 TEL: 28951918 FAX: 28901759 E-moil: hkoffice@sammak.com (10/01/2023) G GRAVE FR BARRIERS O GULLY INITIAL 10/01/2023 INITIAL 10/01/2023 INITIAL Cheung Chee Pak MANHOLE, PUBLIC LIGHTING FAX GRAVE DATE OF SURVEY: DECEMBER 2022 REVISION DATE DESCRIPTION PL AN NO 9215/01/01 (Sheet 1 of 3) Authorized Land Surveyor | (BOTTOM) INCLUED WALL G GRAVE | | GULLY | Image: Services department PL Manhole, public lighting | TET MANHOLE, TOWNGAS TELECOM | VB VEHICLE BARRIERS WE WELL WIP WORK IN PROGRESS WT WATER TANK | NOTE ALL SPOT LEVEL POSITIONS ARE THE DECIMAL POINT OR A CROSS DATE | E OF SURVEY: DECEMBER 2022 REVISION DATE | 23 DESCRIPTION | ILL: 28951918 FAX: 28901759 E-mail: hkoffice@sammak.cor PLAN NO 9215/01/01 (Sheet 1 of 3) | Cheung Chee Pak |

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SERVICES DEPARTMENT

MANHOLE, PUBLIC LIGHTING

T€T

| MANHOLE, HUTCHISON | ABBREVIATION: BD BOULDER BE BERM | | | | | | Fruit Design & Build | Ltd. |
|---|--|--|--|----------|------------|-------------|---|---|
| MANHOLE, HUTCHISON GLOBAL CROSSING LIMITED BROADBAND CATCH PIT MANHOLE, FOUL WATER/SEWER MANHOLE, STORM WATER MANHOLE, PACIFIC CENTURY CARLE & WIRELESS | BS BOUNDARY STONE CA CANOPY CUL CULVERT CO COLUMN CONC CONCRETE EB ELECTRIC BOX END END OF OBJECT FB FLOWER BED FP FOOTPATH IL INVERT LEVEL NU NULLAH OH OVERHANG | SCALE 1 : 1 | 100 (SIZE A1) 6 8 ¹⁰ metres METRIC GRID | | | | Topographical Surv Nam Sang Wai New Territories | ∕еу |
| MANHOLE, NEW WORLD TELEPHONE MANHOLE, CLP POWER MANHOLE, WHAFT T & T | PL PLATFORM PO POND PZ PIEZOMETER TUBE RP RAMP RU RUIN SS SHRINE TS TEMPORARY STRUCTURE VB VEHICLE BARRIERS WE WELL | PRINCIPAL DAT ALL UNITS IN | UM HONG KONG METRES | INITIAL | 10/01/2023 | | SAM MAK & ASSOCIATES SURVEYORS (HK) LTD. consultants in land, engineering, marine & aerial surveys room 901, marina house, no.68 hing man street, shaukeiwan, hong kong. tel : 28951918 FAX : 28901759 E-mail: hkoffice@sammak.com | PRELIMINARY |
| MANHOLE, TOWNGAS TELECOM | WIP WORK IN PROGRESS WT WATER TANK | ALL SPOT LEVEL POSITIONS ARE THE DECIMAL POINT OR A CROSS | DATE OF SURVEY: DECEMBER 2022 | REVISION | DATE | DESCRIPTION | PLAN NO. 9215/01/01 (Sheet 3 of 3) | Cheung Chee Pak Authorized Land Surveyor |

