Planning Application for Proposed Comprehensive Development Scheme to include Wetland Restoration Proposal and Proposed Filling of Ponds/Land and Excavation of Land in "OU(CDWRA)" Zone at Various Lots in D.D. 104, North of Kam Pok Road East, Pok Wai, Yuen Long, New Territories

Prepared for

Fruit Design and Build Limited

Prepared by

Ramboll Hong Kong Limited

PLANNING APPLICATION FOR PROPOSED COMPREHENSIVE DEVELOPMENT SCHEME TO INCLUDE WETLAND RESTORATION PROPOSAL AND PROPOSED FILLING OF PONDS/LAND AND EXCAVATION OF LAND IN "OU(CDWRA)" ZONE AT VARIOUS LOTS IN D.D. 104, NORTH OF KAM POK ROAD EAST, POK WAI, YUEN LONG, NEW TERRITORIES

DRAINAGE IMPACT ASSESSMENT



Date **21 August 2024**

Checked by Edwin Lo

Registered Professional Engineer (Civil)

Binnies Hong Kong Limited

Signed

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Principal Consultant

Signed

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1. INTRODUCTION

- 1.1.1 The applicant proposes to develop the Application Site at various lots in DD104, north of Kam Pok Road East, Yuen Long, into a residential development cum wetland restoration area. The zoning of the Application Site is "Other Specified Uses Comprehensive Development to include Wetland Restoration Area" (OU(CDWRA)) on the draft Nam Sang Wai Outline Zoning Plan S/YL-NSW/9. A S16 application is required for the proposed development.
- 1.1.2 Ramboll Hong Kong Limited has been commissioned by the Project Proponent to conduct the said Drainage Impact Assessment (DIA) for the proposed development under this application. Architectural drawings and technical information of the Development Site were largely provided respectively by the project architect and other project team members.
- 1.1.3 The Application Site is also the subject of a previous planning application under the application no. A/YL-NSW/290 and a DIA report (R7192) was previously submitted in support of that planning application (Previous DIA). Since then, the layout plan of proposed development has been further reviewed taking into account the concerns of AFCD with respect to the layout of proposed wetland restoration area. Compared to the previous scheme in Previous DIA, the application boundary in current application remains the same. Thus, this DIA serves as an update to the above-mentioned previous submitted DIA report based on the current revised development scheme as requested by AFCD.

1.2 Application Site and its Environs

- 1.2.1 The Application Site is about 51,073 m², and it is immediate southeast of an existing low-rise residential development, Man Yuen Chuen and north of the Kam Pok Road East. The Application Site is partly occupied by abandoned ponds and partly by soil ground.
- 1.2.2 **Figure 1.1** shows the location of the Application Site and the environs.

1.3 Proposed Development

- 1.3.1 The original development scheme comprises a total of 114 units in 108 housing blocks of 3 to 5-storey high (i.e. 89 in the form of 2- to 4-storey on top of 1-level of communal basement carpark and 25 in 2-storey on top of 1-level of carport), two 2-storeys clubhouses, an underground sewage pumping station (SPS) and a proposed wetland restoration area (WRA). To respond to AFCD's comments, amendments to the original MLP are hence required. As a result, the proposed Wetland Restoration Area (WRA) has been increased and there is a net reduction in total no. of units have been reduced to 90 units in 84 housing blocks varying from 2-storeys to 4-storeys on top of 1-level of carport.
- 1.3.2 The indicative revised Master Layout Plan (MLP) and sections of the Proposed Development are included in planning statement.



2. DRAINAGE IMPACT ASSESSMENT ("DIA")

2.1 Scope of Work

2.1.1 The aim of this study is to assess the changes to runoff from the Application Site as a result of the proposed development and the potential impacts on the existing drainage system and surrounding areas. Drainage Record Plans from the Drainage Services Department (DSD) were obtained for the information of drainage impact assessment.

2.2 Assessment Criteria and Methodology

- 2.2.1 The assessment standard complies with the Stormwater Drainage Manual (2022 Edition), Stormwater Drainage Manual Corrigendum Nos. 1/2022 and 1/2024 published by DSD (DSD SDM). The Site is located within an urban drainage branch system and a 1 in 50 years return storm has therefore been adopted for the DIA.
- 2.2.2 The catchment runoff has been calculated using the "Rational Method", as outlined in the DSD SDM:

$$Q = 0.278 \ C \ i \ A$$

Where $Q = \text{peak runoff in m}^3/\text{s}$
 $C = \text{runoff coefficient (dimensionless)}$
 $i = \text{rainfall intensity in mm/hr}$
 $A = \text{catchment area in km}$

- 2.2.3 The existing Site consists of a group of abandoned ponds of around 4.9 ha in total. According to the latest development layout, the proposed development comprises houses. In the site inspection, it has been noted that parts of the Application Site have been paved concrete and pantry, unpaved soft landscape around intermixed among fishponds. Runoff coefficient of 0.95 has been adopted for the paved areas and 0.35 has been adopted for unpaved areas (i.e. soft landscape). A runoff coefficient of 0.35 has been adopted for fishponds in the existing scenario and a coefficient of 1.0 has been adopted for fishponds and the proposed WRA in the proposed scenario. A summary of catchment characteristics is provided in **Appendix 2.1**.
- 2.2.4 The design criteria for flood level depends on a combination of rainstorm event and tidal level, as well as the catchment characteristics. With reference to the Table 11 of DSD SDM, the determination of flood level is provided in **Table 2.1**.

Table 2.1 Determination of Flood Level

Flood Level Return Period Scenarios	Rainfall Return Period (year)	Sea Level Return Period (year)
50-years A (Case I)	50	10
50-years B (Case II)	10	50



2.2.5 With the reference to Table 28 of DSD SDM, as extracted in **Table 2.2** below, rainfall increase and sea level rise due to climate change has been considered in the calculations. As 1 in 50 years return storm is adopted for the assessment, the mid-21st century value has been considered.

Table 2.2 Rainfall Increase and Sea Level Rise due to Climate Change

	Rainfall Increase	Sea Level Rise (m)
Mid-21 st Century (2041 – 2060)	11.1%	0.20

2.3 Existing Drainage and Condition of the Application Site

- 2.3.1 According to the Drainage Record Plans obtained from DSD, there is no public drainage in the immediate vicinity of the Site. There is a drainage ditch to the west of the side, which runs northwards along the eastern perimeter of Ma Yuen Chuen. The drainage ditch turns westward before discharging into Ngau Tam Mei Drainage Channel.
- 2.3.2 Based on the ground levels shown on the basemaps, surface runoff within the Site runs westwards, entering the abovementioned drainage ditch. For the ponds outside Site to the north and northeast of the Site, and the construction site and open storage area to the east of the site, the existing ground level indicates that surface runoff would run westwards as well then enter the above-mentioned drainage ditch. For the Hong Kong and China Gas Company Limited Yuen Long Open Yard to the south of the Site, runoff runs towards the east and south into another drainage ditch, which discharges to Ngau Tam Mei Drainage Channel. For Ma Yuen Chuen and the open storage area to the west of the Site, based on site observation, there are gullies along Kam Pok Road collecting runoff from these areas. The runoff then discharges to Ngau Tam Mei Drainage Channel as well.
- 2.3.3 The existing drainage system in the vicinity of the Application Site and the direction of runoff flow are shown in **Figure 2.1**. Sections of the existing drainage ditch to the west of the Site are shown in **Appendix 5.1**. For the purpose of this study, this DIA has been prepared based on a proposed new drainage system for the proposed development as described in below paragraphs. During later detailed design stage, availability of the existing drainage system as shown in **Figure 2.1** and its maintenance responsibility to cater for discharge from the proposed development, will be further reviewed where appropriate.

2.4 Proposed Development and Drainage System

- 2.4.1 A set of 800mm peripheral drain is proposed along the east of the application site boundary to divert the surface runoff from Catchments C2, C3, C6 and C7 and to maintain the flow as its original flow direction.
- 2.4.2 A set of peripheral drains are proposed across the Site to collect stormwater runoff from the Proposed Development, Catchment C2, C3, C6 and C7. It runs to the proposed standard sized drains (at **Appendix 2.3**).
- 2.4.3 New drainage pipes are proposed along Kam Pok Road East towards west and then to Kam Pok Road. The storm water will be directly discharge to the Ngau Tam Mei Channel.
- 2.4.4 The designated location of the outlet has avoided the existing retaining walls and the cycle track underpass.



- 2.4.5 Locations of existing drainage system for the proposed development (S1 to S27) are shown in **Appendix 5.1** and in **Figure 2.1**. Calculation of drainage discharge under existing condition is shown in **Appendix 2.2**. The boundary conditions of the application site provided by DSD is shown in **Appendix 4.1**.
- 2.4.6 Upon the completion of the Proposed Development, the Site would consist of residential blocks, clubhouse, driveways/EVAs, and a WRA. Except the WRA, the other areas will be paved. With the adoption of Rational Method, the WRA is considered as paved area under a conservative perspective in the hydraulic analysis/calculation. Consideration has been given to the rainfall intensity and sea level rise due to climate change effect when calculating the drainage discharge under backwater effect, which are demonstrated in **Appendix 2.3** and **Appendix 3.1**. The drainage discharge still works under backwater effect. Hydraulic checking of peripheral drain is provided in **Appendix 2.3**.
- 2.4.7 The indicative location of peripheral drains within the Application Site, and the indicative proposed drains are indicated in **Figure 3.1**. The exact alignment and locations of manholes will be subject to later detailed design stage. Should there be any special manhole types identified/ proposed during detailed design stage which will be handed over to DSD, checking of structural integrity by an Independent Checking Engineer (ICE) should be provided by the project proponent. This DIA is preliminary and prepared for the purpose of current planning application to illustrate the approach of proposed drainage system. During later detailed design stage, a DIA report with proposed drainage system should be prepared and submitted to DSD. The proposed drainage system should be reviewed and designed in such a way to comply with relevant requirements in the Stormwater Drainage Manual and to the satisfaction of DSD.

2.5 Maintenance Responsibility

- 2.5.1 Proposed drainage pipe from S1 to the site boundary within private lot will be maintained by the Project Proponent.
- 2.5.2 Proposed drainage pipes from site boundary within government land to S8 is proposed to be maintained by DSD. Details of maintenance responsibility will be further liaised/confirmed with relevant department in later detailed design stage.
- 2.5.3 Details of the maintenance responsibility of the drainage facilities are indicated in **Figure 3.6**.



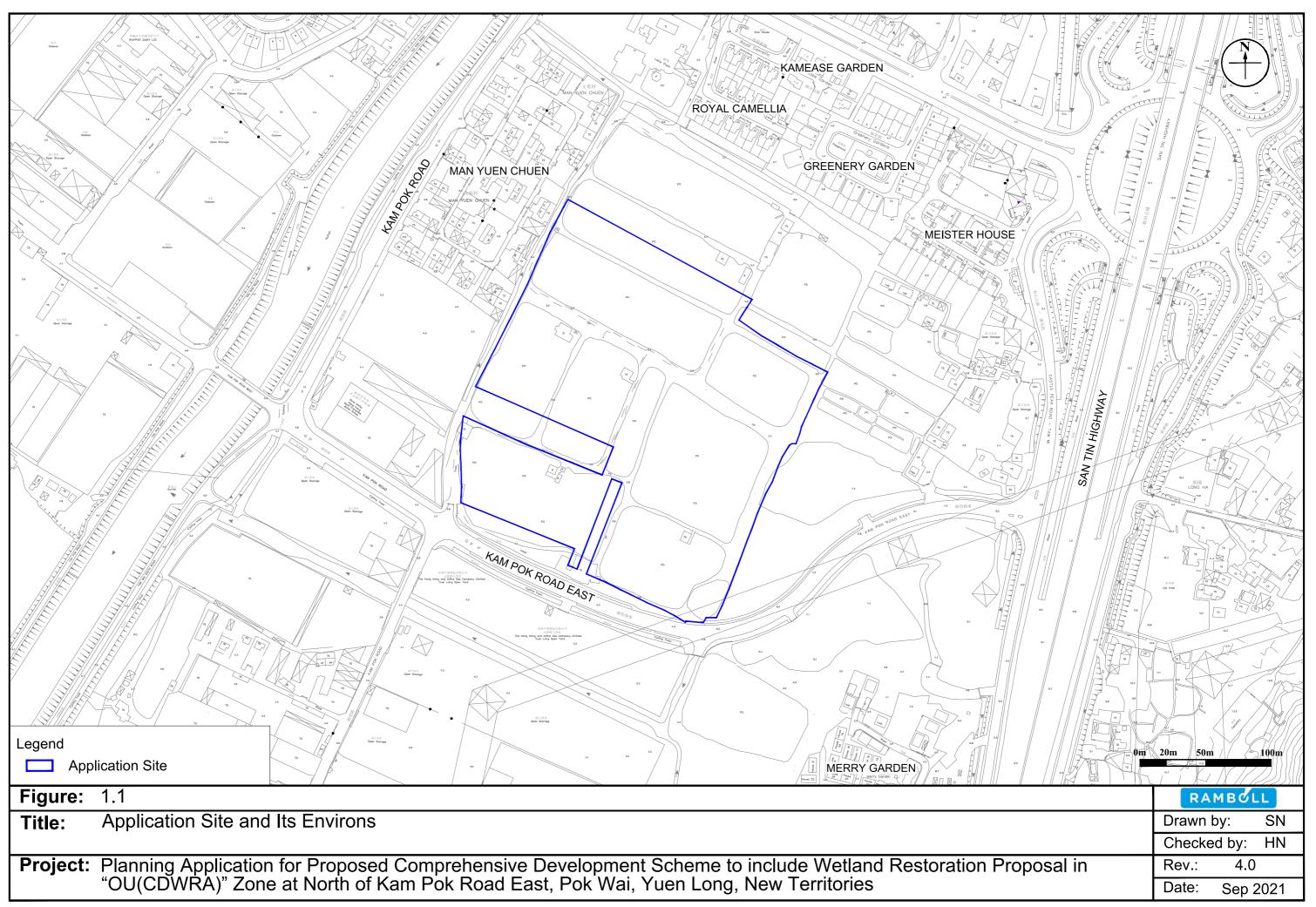
3. OVERALL CONCLUSION

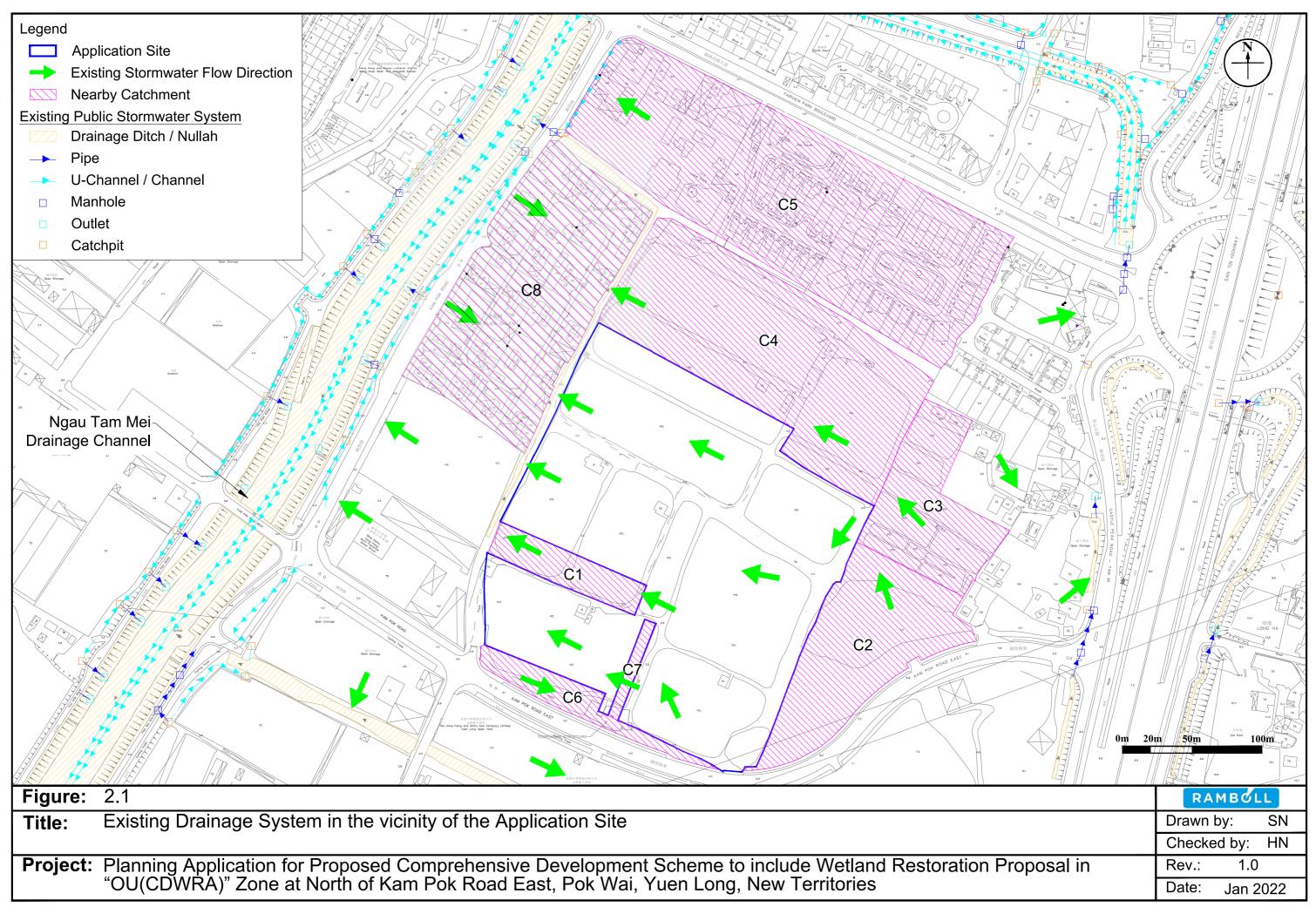
- 3.1.1 A residential development cum wetland restoration area is proposed for the Application Site at various lots in DD104, north of Kam Pok Road East, Yuen Long. The potential drainage impact has been quantitatively addressed.
- 3.1.2 The Proposed Development will increase the runoff generated from the Application Site. Runoff from the Proposed Development and nearby Catchments to the east of the Site will be conveyed to the proposed drainage system, and then discharging into Ngau Tam Mei Drainage Channel. The project proponent will be responsible for implementation of the required drainage works while the maintenance responsibility has been proposed in this DIA, which will be further liaised with relevant department during detailed design stage.
- 3.1.3 Temporary drainage measures shall be implemented to ensure that the flooding conditions will not be worsened during construction. Periodic inspection by the Authorized Person or his representative will be carried out during construction. During later detailed design stage, a DIA report with proposed drainage system should be prepared and submitted to the satisfaction of DSD.
- 3.1.4 Should there be any special manhole types identified/ proposed during detailed design stage which will be handed over to DSD, checking of structural integrity by an Independent Checking Engineer (ICE) should be provided by the project proponent.
- 3.1.5 With the implementation of the above proposed drainage measures and temporary drainage works during construction, if any, the Proposed Development at the Application Site is technically feasible, having no insurmountable impact from drainage point of view.

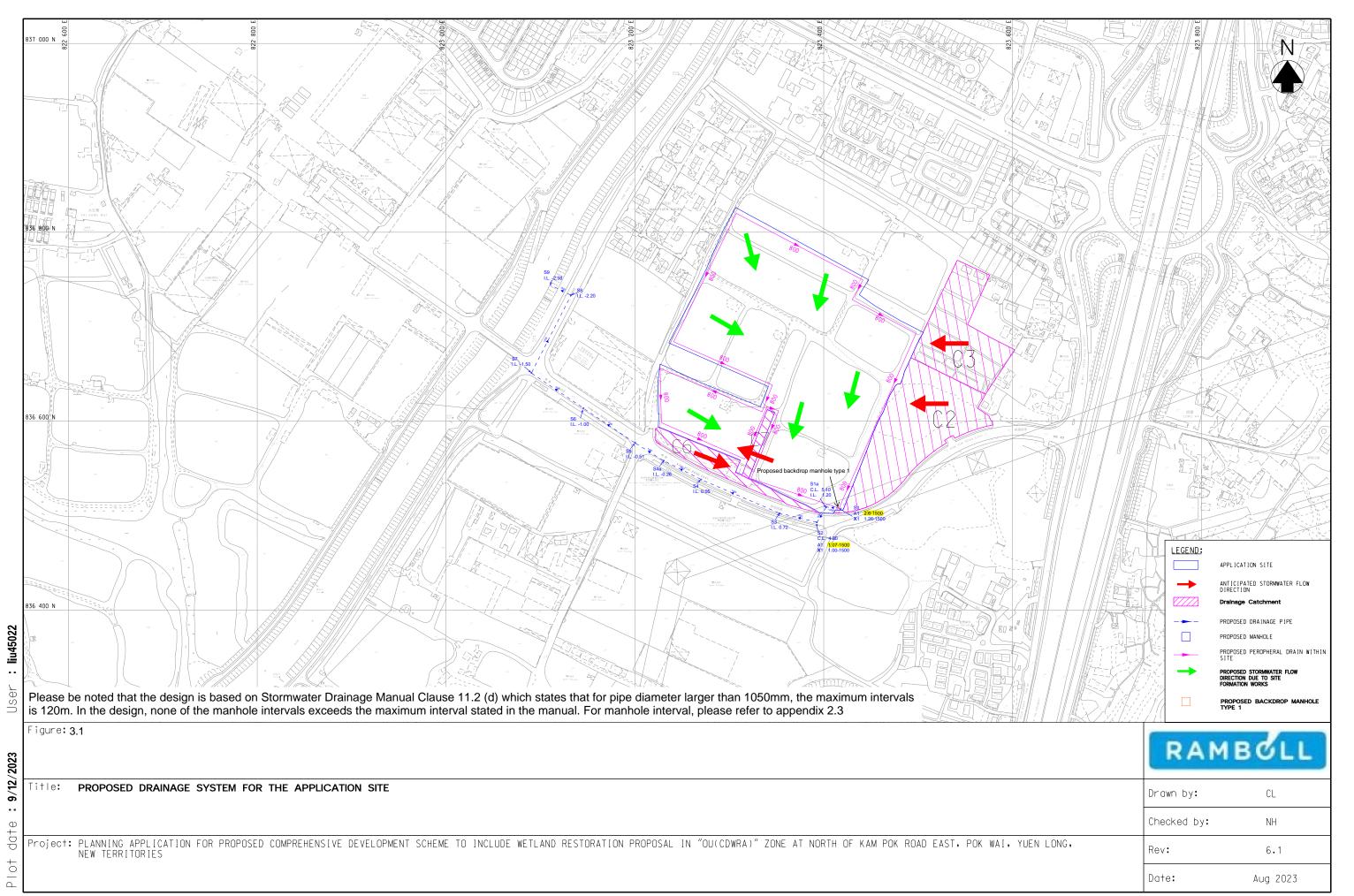


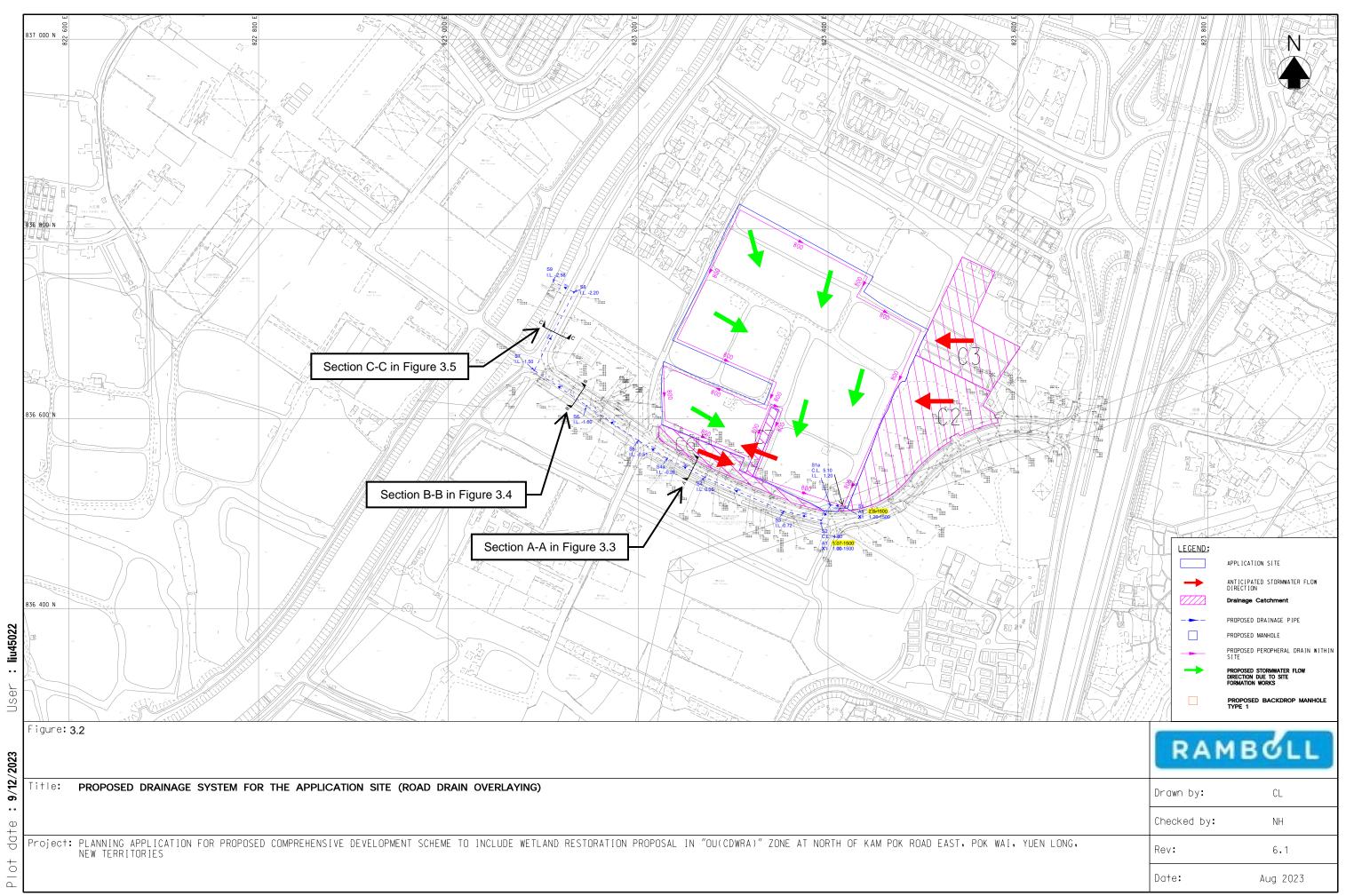
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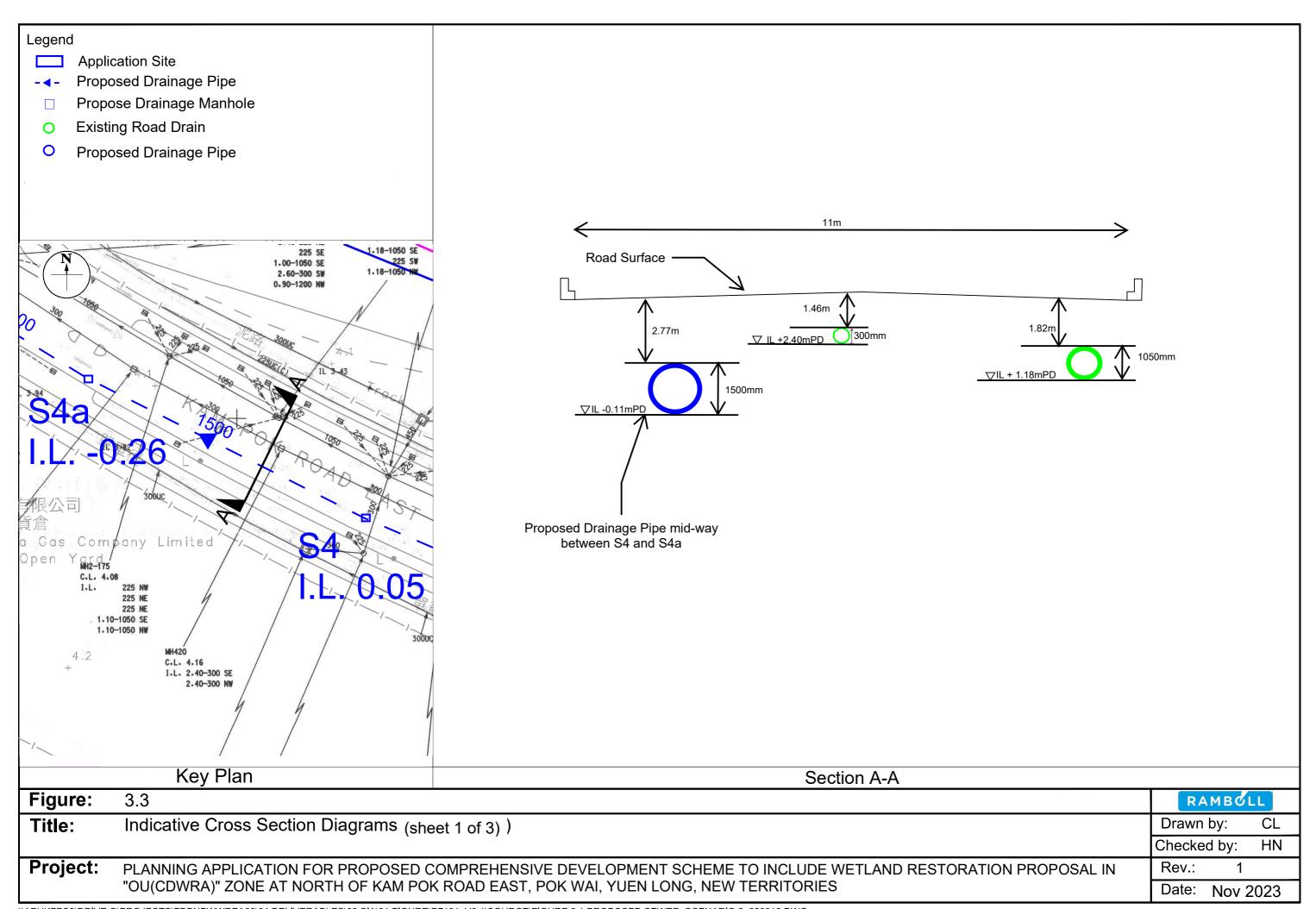


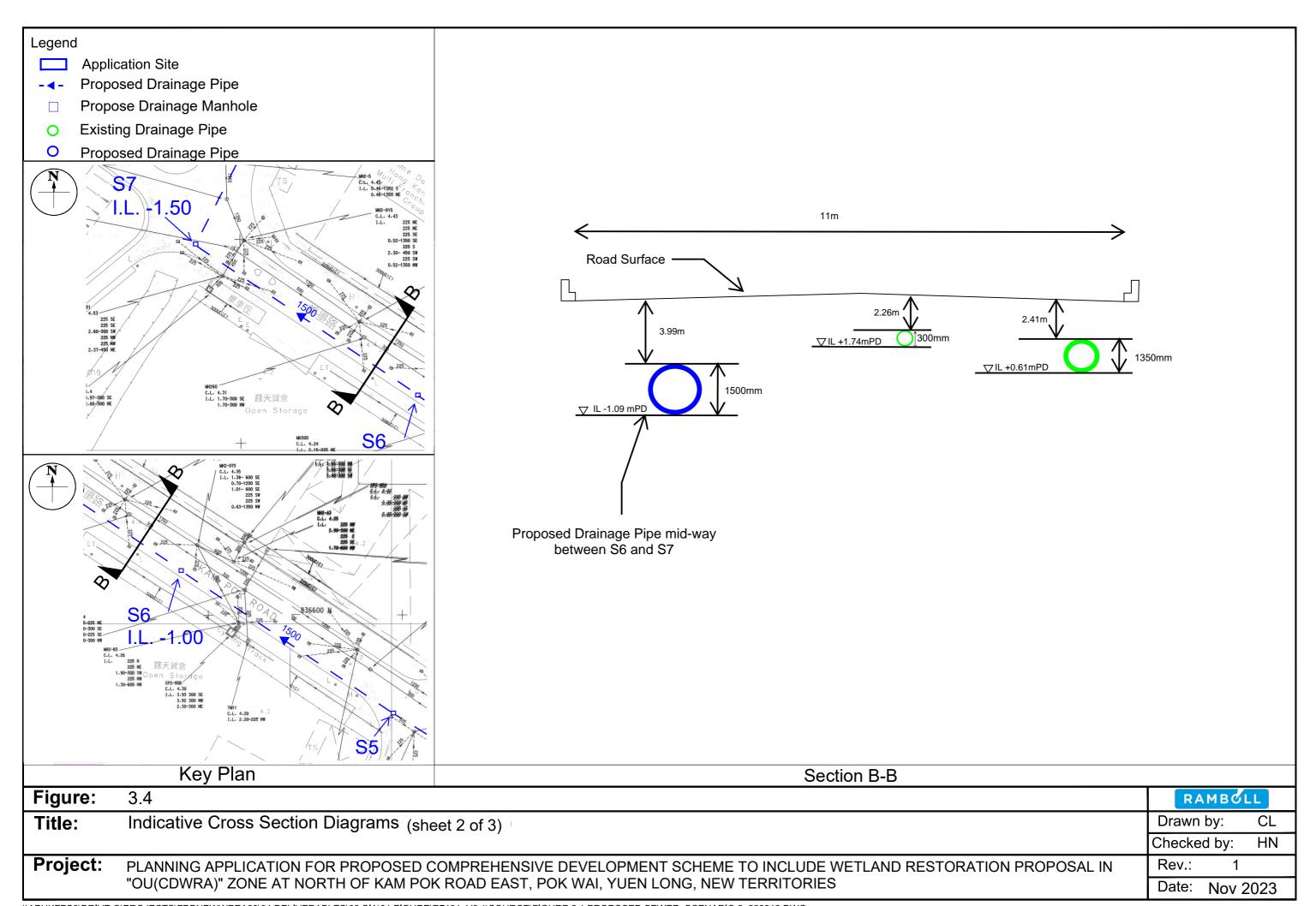


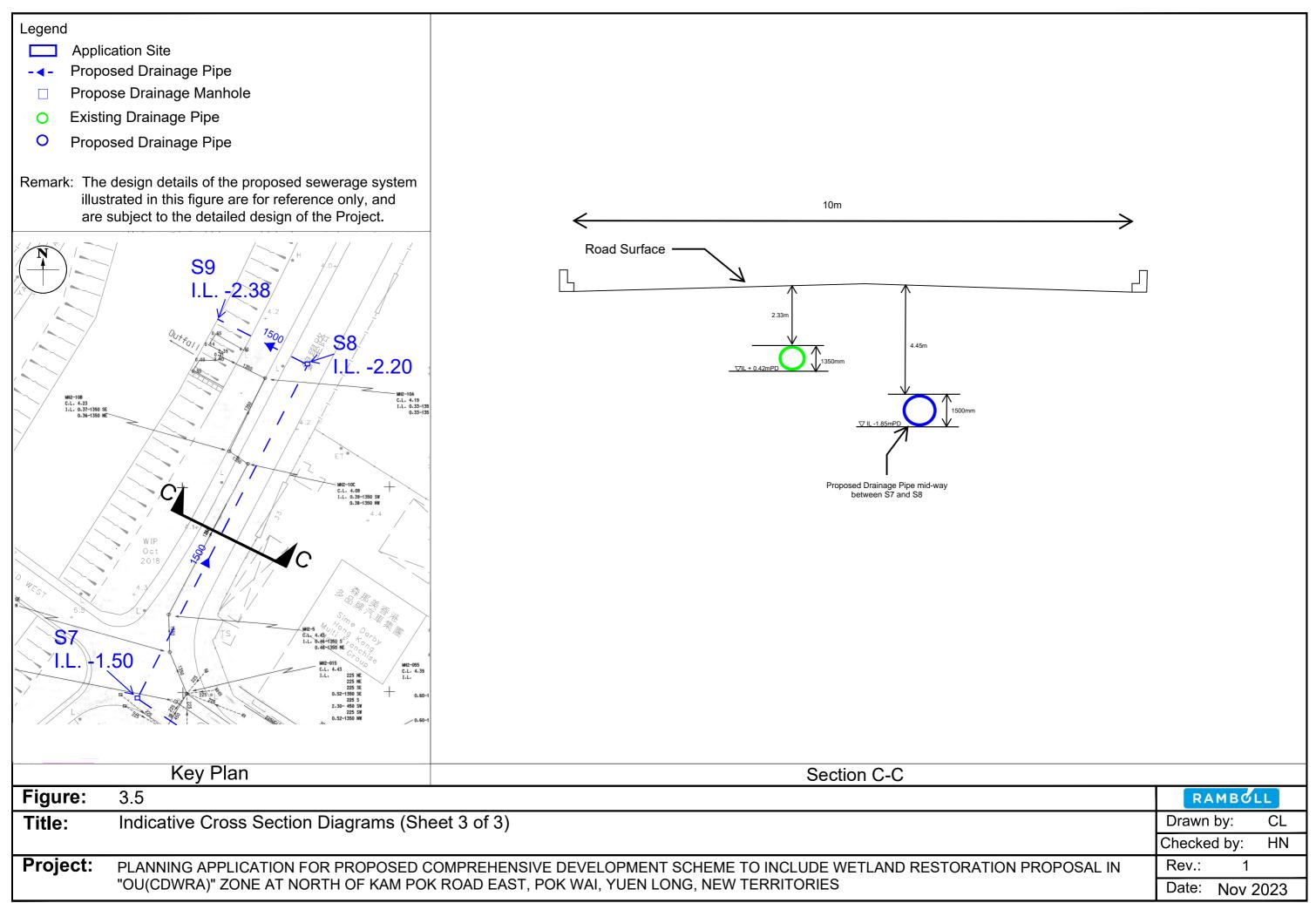


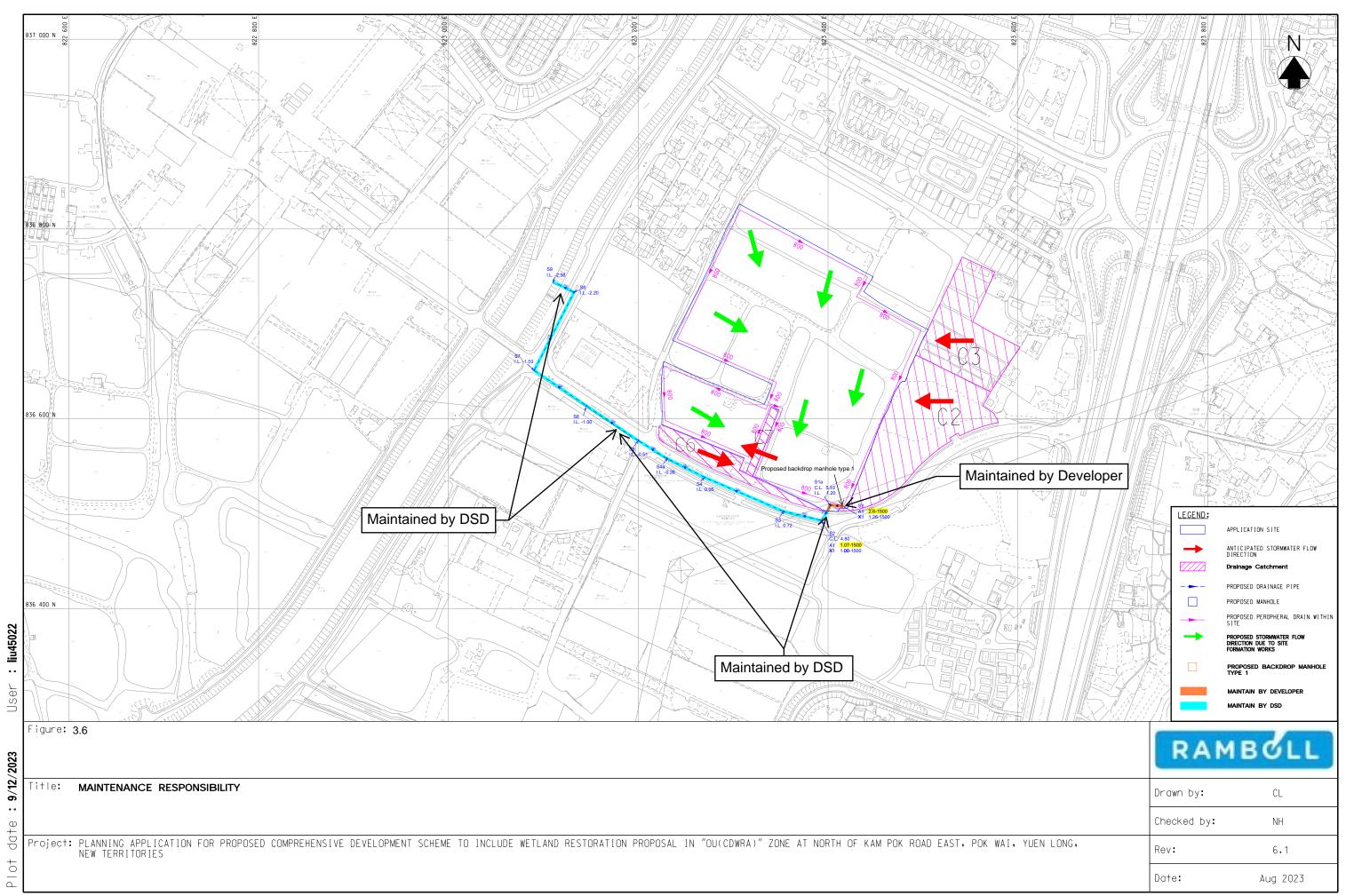






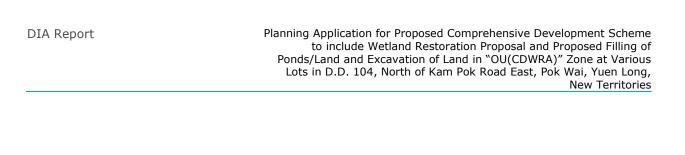






Appendix 1.1 Indicative Master Layout Plan of the Proposed Development





Please refer to the Planning Statement



Appendix 2.1 Summary of Catchment Characteristic



Summary of Catchment Characteristic

Summary of	Catchment Characteristic			
	Catchment	Area	Coverage	Runoff Coefficient
		(m ²)		
Existing	Pond	49,000.0	96%	0.35
(Site)	Unpaved pond bund / footpath	2,073.3	4%	0.35
Fustana	Total	51,073.3	40.00/	
Future	Paved area	20,582.0	40.3%	0.95
(Site)	Wetland Restoration Area (WRA) Uncovered horizontal planting area,	24,702.0	48.4%	1.00
	grass paver	5,789.0	11.3%	0.35
	Total	51,073.3		
C1	Pond	2191.3	79%	1.00
02	Unpaved pond bund / footpath	593.5	21%	0.35
	Total	2,784.8	21/0	0.55
C2	Construction site for village houses	4365.1	40%	0.95
	Open storage area	6,547.6	60%	0.35
	Total	10,912.7	00 70	0.55
С3	Pond	5,954.1	81%	1.00
	Unpaved pond bund / footpath	942.4	13%	0.35
	Village houses	497.7	7%	0.95
	Total	7,394.2	7 70	0.55
C4	Pond	15,359.8	70%	1.00
	Unpaved pond bund / footpath	6,657.0	30%	0.35
	Total	22,016.8	3373	0.00
C5	Pond	626.5	2%	1.00
	Village houses / paved area	30,333.1	96%	0.95
	Landscaping area	660.0	2%	0.35
	Total	31,619.6	-	
C6	Pond	734.0	29%	1.00
	Temporary structure / paved area	1,824.5	71%	0.95
	Total	2,558.5		
С7	Pond	537.2	86%	1.00
	Temporary structure / paved area	86.2	14%	0.95
	Total	623.4		
C8	Unpaved area	340.3	20%	0.35
	Paved area	1,355.0	80%	0.95
	Total	1,695.3		
I.		,		

Appendix 2.2 Drainage Impact Assessment Calculations under Existing Condition



Appendix 2.2 Drainage Impact Assessment Calculations under Existing Condition

Note

 $1) \ Colebrook-White's \ equation \ is \ adopted \ for \ full-bore \ pipe \ velocity \ calculation; \ Manning's \ equation \ is \ adopted \ for \ non-circular \ features.$

											Runoff from	Full Flow	Full Flow	Cumulative	
		US I.L.	DS I.L.			Bottom Width		Top Width	Side Slope		Catchment	Velocity	Capacity	Runoff	
From	To	(mPD)	(mPD)	No. of pipe	Shape	(m)	Height (m)	(m)	(m/m)	Inflow Catchment	(m ³ /s)	(m/s)	(m^3/s)	(m ³ /s)	Utilization
S1	S2	2.96	2.55	1	Trapezoidal	1.47	0.70	2.54	1.53	C1, C6, C7	0.293	7.221	8.771	8.771	3.3%
S2	S3	2.55	2.48	1	Trapezoidal	2.38	1.27	4.64	1.78	-	0.000	4.207	14.878	14.878	2.0%
S3	S4	2.48	2.54	1	Trapezoidal	2.40	1.17	3.89	1.27	-	0.000	4.106	14.349	14.349	2.0%
S4	S5	2.54	2.41	1	Trapezoidal	0.61	0.91	5.15	2.49	C8	0.104	3.405	2.718	2.718	14.6%
S5	S6	2.41	2.49	1	Trapezoidal	1.44	1.16	5.08	1.57	-	0.000	3.971	9.035	9.035	4.4%
S6	S7	2.49	2.44	1	Trapezoidal	1.06	1.13	4.51	1.53	-	0.000	2.974	5.445	5.445	7.3%
S7	S8	2.44	2.41	1	Trapezoidal	1.02	1.01	3.87	1.41	-	0.000	2.022	3.191	3.191	12.4%
S8	S9	2.41	2.40	1	Trapezoidal	1.03	0.53	2.92	1.78	-	0.000	0.929	0.588	0.588	67.4%
S9	S10	2.40	2.39	1	Trapezoidal	1.44	0.68	4.63	2.35	-	0.000	1.055	1.117	1.117	35.5%
S10	S11	2.39	2.37	1	Trapezoidal	1.49	0.72	4.63	2.18	C2, C3, Site	1.800	1.599	1.886	1.886	116.5%
S11	S12	2.37	2.27	1	Trapezoidal	1.60	0.76	3.90	1.51	-	0.000	3.715	5.343	5.343	41.1%
S12	S13	2.27	2.28	1	Trapezoidal	0.98	1.10	5.26	1.95	-	0.000	1.227	1.878	1.878	117.0%
S13	S14	2.28	2.26	1	Trapezoidal	1.44	1.67	7.06	1.68	-	0.000	2.332	8.527	8.527	25.8%
S14	S15	2.26	2.25	1	Trapezoidal	0.44	0.32	0.76	0.50	C4	0.178	0.646	0.201	0.201	1183.0%
S15	S16	2.25	2.20	1	Trapezoidal	0.40	0.36	0.70	0.42	-	0.000	1.502	0.615	0.615	386.3%
S16	S17	2.20	2.00	1	Trapezoidal	0.50	0.48	2.44	2.02	-	0.000	3.188	1.016	1.016	233.9%
S17	S18	2.00	1.73	1	Trapezoidal	0.61	0.54	3.03	2.24	-	0.000	4.075	1.686	1.686	140.9%
S18	S19	1.73	1.61	1	Trapezoidal	0.62	0.94	5.32	2.50	-	0.000	2.855	2.406	2.406	98.7%
S19	S20	1.61	1.59	1	Trapezoidal	2.24	1.09	6.94	2.16	-	0.000	1.972	5.313	5.313	44.7%
S20	S21	1.59	1.60	1	Trapezoidal	1.84	0.99	6.08	2.14	-	0.000	1.328	2.725	2.725	87.2%
S21	S22	1.60	1.54	1	Trapezoidal	1.13	0.63	6.55	4.30	-	0.000	2.345	1.697	1.697	140.0%
S22	S23	1.54	1.46	1	Trapezoidal	2.05	0.98	7.16	2.61	-	0.000	3.781	8.091	8.091	29.4%
S23	S24	1.46	1.23	1	Trapezoidal	0.60	1.11	6.23	2.54	-	0.000	5.153	5.342	5.342	44.5%
S24	S25	1.23	1.87	1	Trapezoidal	1.64	1.22	6.82	2.12	-	0.000	11.207	27.253	27.253	8.7%
S25	S26	1.87	1.98	1	Trapezoidal	2.61	1.17	5.01	1.03	-	0.000	5.357	21.158	21.158	11.2%
S26	S27	1.98	1.32	1	Trapezoidal	1.89	1.57	9.32	2.37		0.000	14.301	51.600	51.600	4.6%
S27	Outlet	1.32	1.58	1	Trapezoidal	3.23	2.96	8.49	0.89	C5	2.087	19.690	344.174	344.174	1.3%

Remark: Please refer to Figure 2.1 for the location of existing drainage system, and Appendix 5.1 for the locations of S1 to S27.

Appendix 2.3 Drainage Impact Assessment Calculations under Proposed Condition



Appendix 2.3 Drainage Impact Assessment Calculations under Proposed Condition

Note

1) Colebrook-White's equation is adopted for full-bore pipe velocity calculation; Manning's equation is adopted for non-circular features.

	Capacity Check for Proposed Drainage Pipe																
		Manhole	US I.L.	DS I.L.			Bottom Width		Top Width	Side Slope			Runoff from Catchment	Full Flow Velocity	Full Flow Capacity	Cumulative Runoff	
From	To	Interval (m)	(mPD)	(mPD)	No. of pipe	Shape	(m)	Height (m)	(m)	(m/m)	Pipe Size (m)	Inflow Catchment	(m^3/s)	(m/s)	(m^3/s)	(m^3/s)	Utilization
S1	S1a	10	1.26	1.20	1	Circular	-	-		-	1.50	C2, C3, Site, C6, C7	3.591	2.743	4.363	3.591	82.3%
S1a	S2	19	1.20	1.07	1	Circular	-	-	•	-	1.50	-	0.000	2.904	4.619	3.591	77.7%
S2	S3	42	1.00	0.72	1	Circular	-	-	-	-	1.50	-	0.000	2.878	4.577	3.591	78.5%
S3	S4	96	0.72	0.05	1	Circular	-	-	-	-	1.50		0.000	2.963	4.713	3.591	76.2%
S4	S4a	46	0.05	-0.26	1	Circular	-	-	-	-	1.50	-	0.000	2.923	4.648	3.591	77.3%
S4a	S5	37	-0.26	-0.51	1	Circular	-	-	-	-	1.50	-	0.000	2.892	4.600	3.591	78.1%
S5	S6	71	-0.51	-1.00	1	Circular	-	-		-	1.50	-	0.000	2.942	4.679	3.591	76.7%
S6	S7	71	-1.00	-1.50	1	Circular	-	=	-	-	1.50	-	0.000	2.972	4.727	3.591	76.0%
S7	S8	98	-1.50	-2.20	1	Circular	-	=	-	-	1.50	-	0.000	2.991	4.757	3.591	75.5%
S8	Outlet	26	-2.20	-2.38	1	Circular	-	-	-	-	1.50	-	0.000	2.955	4.700	3.591	76.4%

Remarks: 1. Please refer to Figure 3.1 for the location of proposed drainage system.

2. Sedimentation reduction has been taken into account

 $3.\,The\,calculation\,has\,complied\,with\,the\,latest\,Stormwater\,Draiange\,Manual\,Corrigendum\,No.\,1/2024$

4. The design of proposed draiange system and manholes should be reviewed in detailed design stage, submitted and agreed with DSD.

	Capacity Check for Proposed Peripheral Drain														
	US I.L. (mPD)	DS I.L. (mPD)	No. of pipe	Shape	Bottom Width (m)	Height (m)	Top Width (m)	Side Slope (m/m)	Pipe Size (m)	Inflow Catchment	Runoff from Catchment (m³/s)	Full Flow Velocity (m/s)	Full Flow Capacity (m ³ /s)	Cumulative Runoff (m³/s)	Utilization
Pheripherial Drain	3.50	2.60	1	U-shape	-	-	-	-	0.80	C2, C3	0.594	1.434	0.737	0.594	80.5%
Pheripherial Drain	3.50	2.60	1	U-shape	-	-	-		0.80	C6, C7	0.225	1.434	0.737	0.225	30.5%

Appendix 3.1 Calculation of Backwater Effect



	Size		Hydraulics										
Type	Width	Height	Length	Radius	Flow	Velocity	Friction slope	Friction	Inlet	/Outlet	Water Level (1)	Ground	Check ⁽²⁾
	(m)	(m)	(m)	(m)	(m³/s)	(m/s)	(S _f)	Loss	К	Loss	(mPD)	Level (mPD)	
											4.563		
									1	0.0014	4.564		
Proposed Circular Drain	1.50	1.50	25.85	0.38	0.296666667	0.167878992	0.000023449	0.001			4.565	4.2	
Proposed Circular Drain	1.50	1.50	98.16	0.38	0.296666667	0.167878992	0.000023449	0.002			4.567	4.2	
Proposed Circular Drain	1.50	1.50	70.1	0.38	0.296666667	0.167878992	0.000023449	0.002			4.569	4.8	
Proposed Circular Drain	1.50	1.50	69.3	0.38	0.296666667	0.167878992	0.000023449	0.002			4.571	4.4	
Proposed Circular Drain	1.50	1.50	37.49	0.38	0.296666667	0.167878992	0.000023449	0.001			4.574	4.0	
Proposed Circular Drain	1.50	1.50	45.52	0.38	0.296666667	0.167878992	0.000023449	0.001			4.575	4.1	
Proposed Circular Drain	1.50	1.50	95.71	0.38	0.890000000	0.503636975	0.000211045	0.020			4.595	4.0	
Proposed Circular Drain	1.50	1.50	42.4	0.38	0.840000000	0.475342763	0.000187999	0.008			4.622	4.2	
Proposed Circular Drain	1.50	1.50	19.33	0.38	0.840000000	0.475342763	0.000187999	0.004			4.626	4.8	
									0.5	0.0007	4.627	5.1	ОК

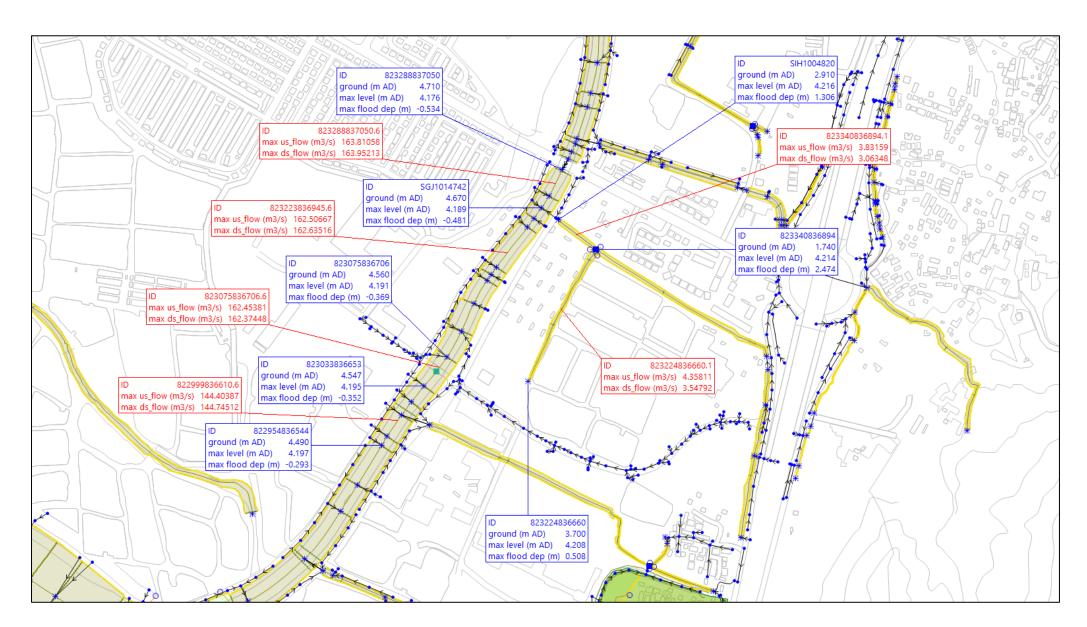
Remark:

⁽¹⁾ With reference to the Appendix 4, flood level of 50B (PP with climate change) provided by DSD, the maximum water level of the Ngau Tam Mei Drainage Channel at the oulet is +4.563 mPD.

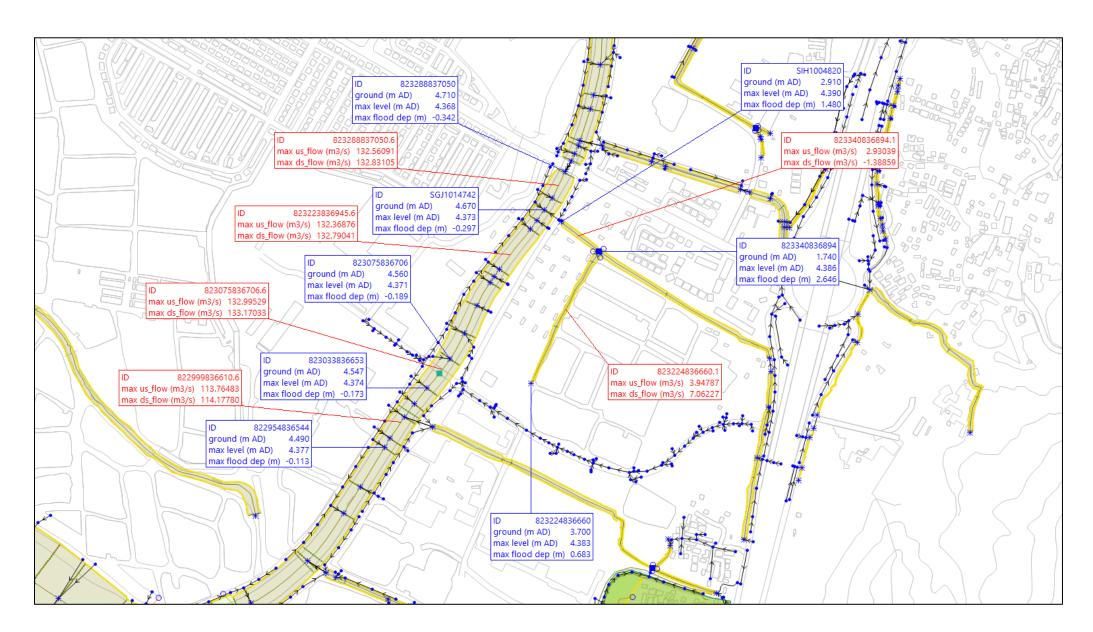
⁽²⁾ According to Appendix 4, the boundary water level of Ngau Tam Mei River is already higher than the ground level, hence, the unfavourable condition is not a result of the Proposed Development. Thus, no mitigation works is proposed under this project.

Appendix 4.1 Boundary Condition provided by DSD

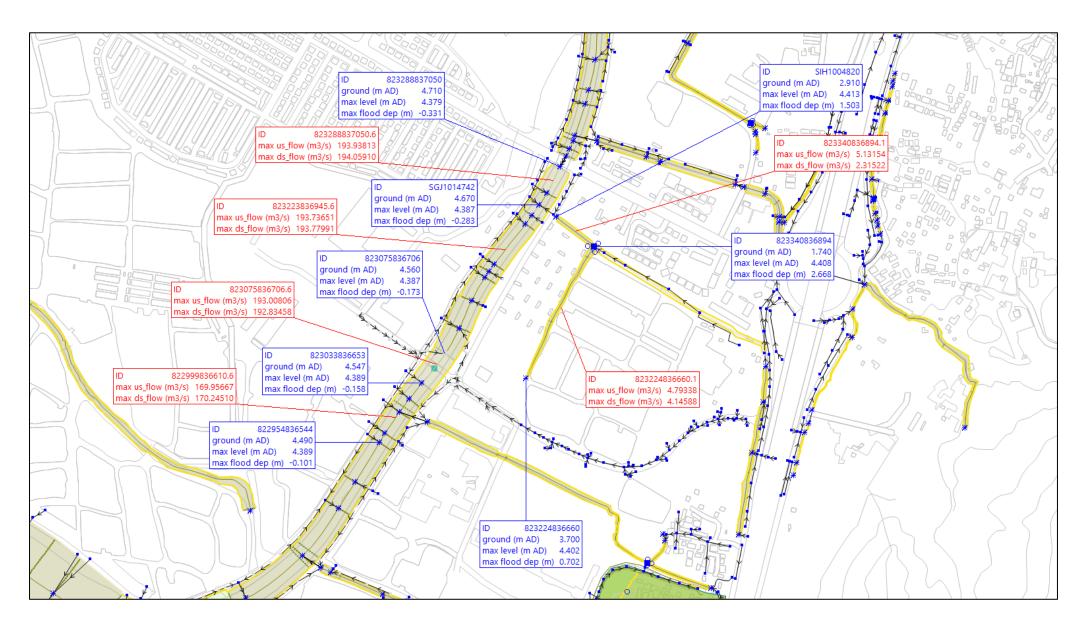




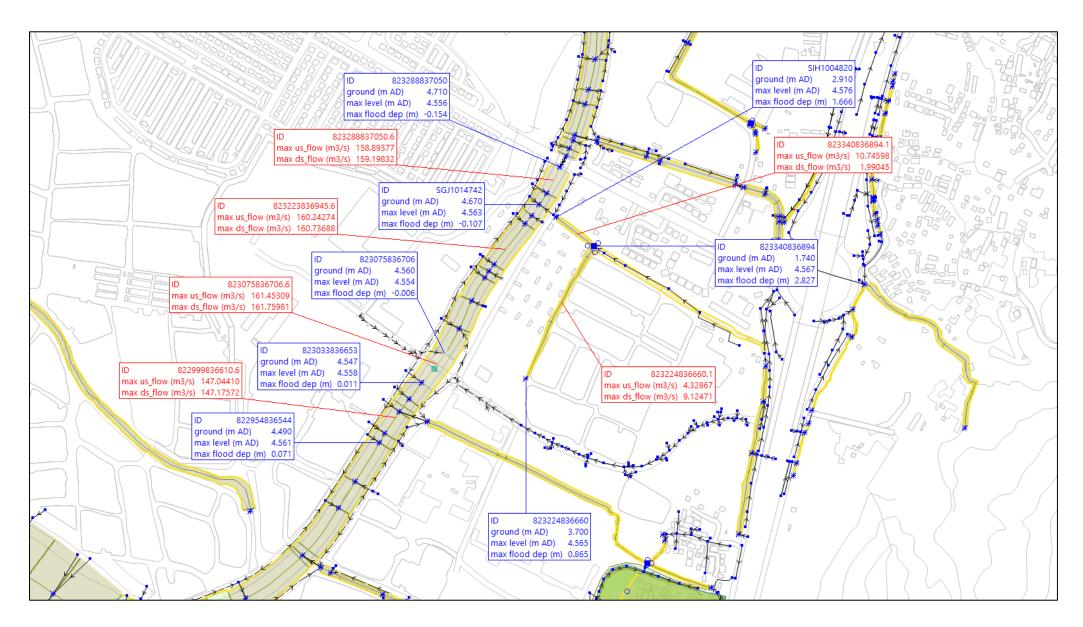
50A (EE)



50B (EE)



50A (PP with climate change)



50B (PP with climate change)

Appendix 5.1 Site Surveying of Existing Drainage Ditch





