TEMPORARY DRAINAGE PROPOSAL



Temporary Drainage Proposal for Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Device for a Period of 3 Years at Lots 591(Part), 592(Part), 593(Part) and 618(Part) in D.D. 122, Ping Shan, Yuen Long, New Territories (A/YL-PS/734)

Issue 1 (October 2024)

Content Page

1. Int	roduction	3
1.1.	Objectives of the Report	3
1.2.	Report Structure	3
2. De	evelopment Proposal	4
2.1.	Existing Site Conditions	4
3. As	sessment Methodology	5
3.1.	Calculation Methodology for Runoff	5
3.2.	Calculation Methodology for Capacity Checking	5
3.3.	Summary of Assessment Assumptions	6
4. Po	tential Drainage Impact	7
4.1.	Change in Drainage Characteristics	7
4.2.	Potential Drainage Impact	7
5. Co	nclusion	9

Appendix

Appendix A Site Location

Appendix B Layout Plan

Appendix C Location and Photos of the Existing Drainage System

Appendix D Proposed Drainage Arrangement

Appendix E Design Calculation on proposed U channel and Pipe

Appendix F Typical designs of the U-channels and Catchpits

1. Introduction

This report presents the temporary drainage proposal for Proposed Temporary Public Vehicle Park with Electric Vehicle Charging Device for a Period of 3 Years at Lots 591(Part), 592(Part), 593(Part) and 618(Part) in D.D. 122, Ping Shan, Yuen Long, New Territories. For site location, please refer to **Appendix A**.

1.1. Objectives of the Report

This report shall be prepared to include the following:

- Identify the potential drainage impact assessment from the proposed application site
- Recommend and implement all necessary measures to mitigate adverse drainage impacts arising from the application site

1.2. Report Structure

The report contains the following sections:

- Section 1 on Introduction;
- Section 2 on Development Proposal;
- Section 3 on Assessment Methodology;
- Section 4 on Potential Drainage Impact; and /
- Section 5 on Conclusion

2. Development Proposal

2.1. Existing Site Conditions

The application site is located in Ping Shan, Yuen Long, New Territories, with a total area of around 2970 m^2 . The existing ground level varying between + 9.65 mPD and + 8.75 mPD. The site layout plan is provided in **Appendix B**.

The applied development is for temporary place of recreation, sport or culture (hobby farm). The type of application is the temporary use/development in rural areas for a period of 3 years. The application site is located at Lots 591(Part), 592(Part), 593(Part) and 618(Part) in D.D. 122, Ping Shan, Yuen Long, New Territories.

The application site is less than 1 ha in size and neither fall within flood prone areas such as lowlying areas and flooding blackspots nor involve pond filling and substantial earth filling, so it is regarded as simple site. There is an existing village drain in vicinity of the site that can be discharged to, the location and photos of the existing drain is shown in **Appendix C**.

3. Assessment Methodology

3.1. Calculation Methodology for Runoff

According to **Section 6.6.2 of the Storm Drainage Manual (SDM)**, an "Urban Drainage Branch System" refers to a network of interconnected drains that collect rainwater runoff from an urban area and transport it to a trunk drain, river, or sea. In simpler terms, the largest pipe size or the equivalent diameter in case of a box culvert in a branch system will normally be less than 1.8m.

Referring to SDM, since the proposed U-channels have dimensions smaller than 1.8m, the drainage system would be classified as an urban drainage branch. It is recommended to design the system with a return period of 50 years to ensure its adequacy in managing stormwater drainage.

To calculate the peak instantaneous runoff values before and after the development, the Rational Method with recommended physical parameters including runoff coefficient (C) and storm constants for different return periods are adopted referred to the SDM.

The Rational Method is adopted for hydraulic analysis and the peak runoff is calculated based on the following equation:

$$Q_p = 0.278 \, Ci \, A$$

where Qp = Peak Runoff, m^3 /s C = Runoff Coefficient i = Rainfall Intensity, mm/hr A = Catchment Area, km^2

The total area of the site will account for 2970 m^2 . The runoff coefficient of 1 is assumed.

Based on the storm constants for the 50-year return period recommended in the SDM, the appropriate rainfall intensities (i) are calculated as detailed in **Appendix E**.

3.2. Calculation Methodology for Capacity Checking

Since the catchment areas are less than 1ha, surface U-channels are recommended to be constructed to collect the stormwater runoff within the site. The collected stormwater should finally be diverted to the existing drain in vicinity of the site via the proposed U-channels and pipe.

For the worst-case scenario, bad condition of concrete u channel is assumed for the Manning's roughness coefficient i.e coefficient value is 0.016 for calculating capacities of concrete U-channel using Manning's Equation. The recommended roughness values $k_{\rm S}$ for concrete channels with float finish is 3.3 mm under normal condition.

Manning's Equation for calculating the channel and pipe capacities is adopted for this analysis:

$$\frac{V = R^{2/3} S^{1/2}}{n}$$

where V = mean velocity, m/s S = slope of the total energy line n = Manning's roughness coefficient R = hydraulic radius, m

3.3. Summary of Assessment Assumptions

The assumptions of the Drainage Proposal are summarized below for ease of reference:

- 50 years return period is adopted;
- Runoff coefficient of 1 for the paved area is assumed;
- Storm constants for 50 years return periods of North District Area
- Manning's roughness coefficient of 0.016 for the proposed concrete U-channels and concrete pipe are adopted; and
- Roughness values k_s of 3.3 mm for concrete channels with float finish is adopted.

4. Potential Drainage Impact

4.1. Change in Drainage Characteristics

There is no existing drainage provision inside the current site, the collected stormwater was discharged as surface runoff leading to the existing U channel near the site before the development.

The total area of the site will account for 2970 m^2 . There are 7 catchment areas in the design, catchment area 1 (CA1) is $190m^2$, catchment area 2 (CA2) is $180 m^2$, catchment area 3 (CA3) is $455 m^2$, catchment area 4 (CA4) is $530 m^2$, catchment area 5 (CA5) is $500 m^2$, catchment area 6 (CA6) is $180 m^2$ and catchment area 7 (CA7) is $895 m^2$.

The adjacent sites have adequate drainage facilities and no record of flooding for the application site has been found. To manage the stormwater flows after developing the site, this drainage proposal detailed the proposed drainage system consisting of a set of U-channels for diverting stormwater flows to avoid causing flooding to the site.

Since there are no changes in drainage characteristics, it is considered that the drainage discharge from the application site will not cause adverse impact to the entire downstream drainage system.

4.2. Potential Drainage Impact

The runoff from the application site is proposed to be collected by U-channels along the boundary of the low sides and discharged to the terminate catchpit with sand trap, and through 300mm pipe eventually lead to the existing village drain in vicinity of the site. The details of the proposed drainage works are illustrated in **Appendix D**.

For conservative approach, the critical scenario is considered for collecting all the flow in the catchment area leading to the 300 mm U-channel. The design calculations of the proposed U- channels are calculated as detailed in **Appendix E.** Typical designs of the U-channels and Catchpits are shown in **Appendix F**.

The design runoff arising from the proposed application site is to be discharged into the proposed 300 mm U-channel and 300mm pipe. The calculations is summarized in **Table 1**

U -channel	Catchment Area (m²)	Proposed U- channel Size (mm)	Estimated Peak Runoff (m³/s)	Capacity (%)	
UC1	190	300	0.012	15%	
UC2	370 ¹	300	0.024	30%	
UC3	825 ²	300	0.052	39%	
UC4	530	300	0.033	24%	
UC5	500	300	0.030	20%	
UC6	720 ³	300	0.043	45%	
UC7	895	300	0.055	36%	

Table 1 Summary of Estimated Runoffs and Proposed U-channel Size

¹Accumulated catchment area of CA1 and CA2

²Accumulated catchment area of CA1, CA2 and CA3

²Accumulated catchment area of CA5 and CA6

5. Conclusion

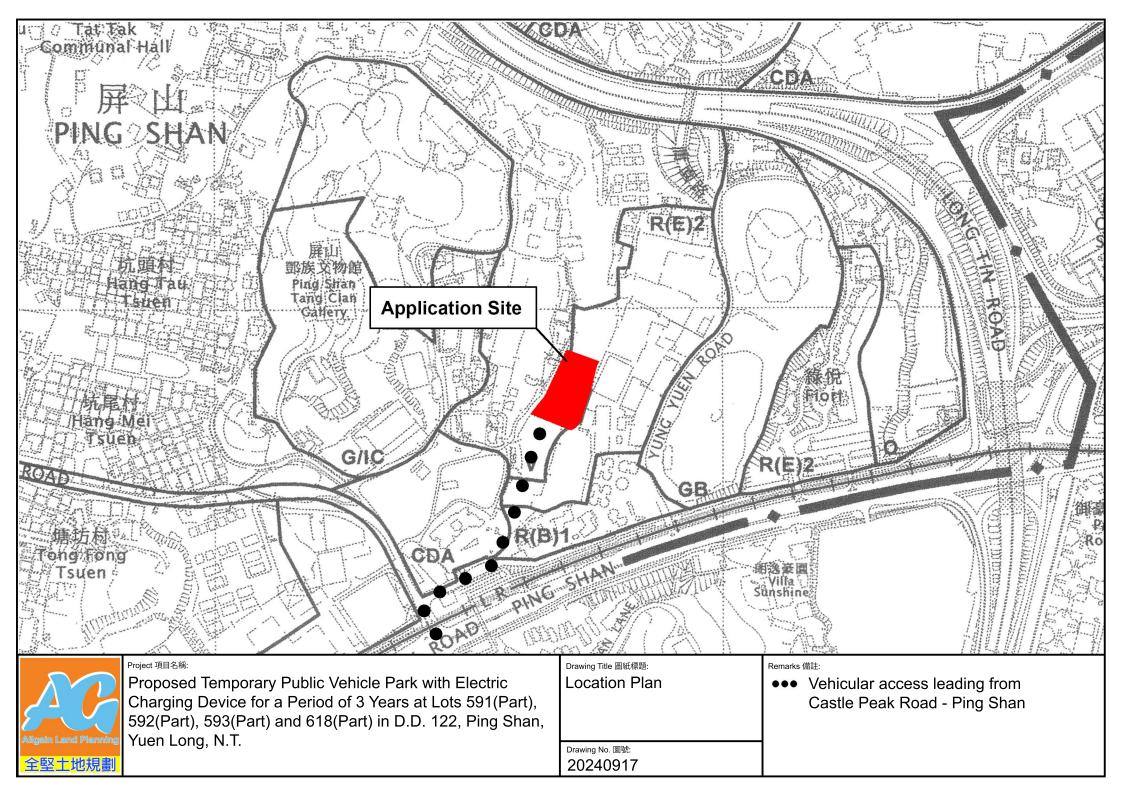
A temporary drainage proposal has been designed for proposed temporary public vehicle park with electric vehicle charging device for a period of 3 years at Lots 591(Part), 592(Part), 593(Part) and 618(Part) in D.D. 122, Ping Shan, Yuen Long, New Territories.

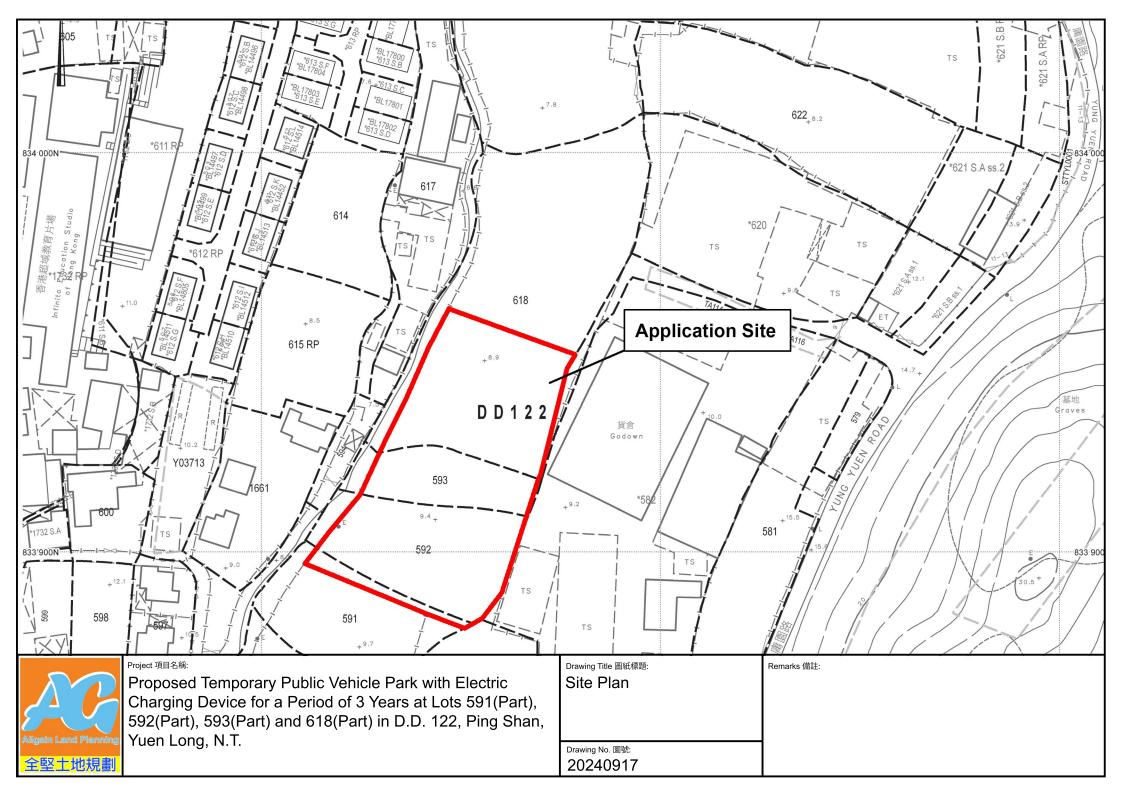
In the design calculation, 300 mm U-channel and 300mm pipe found adequate and proposed to convey at the peak runoff under the 50 years return period from the application site.

The adjacent sites have adequate drainage facilities, and no record of flooding has been found. The stormwater collected from this application site is proposed to be discharged to the existing village drain in vicinity of the site without overloading the existing drainage system.

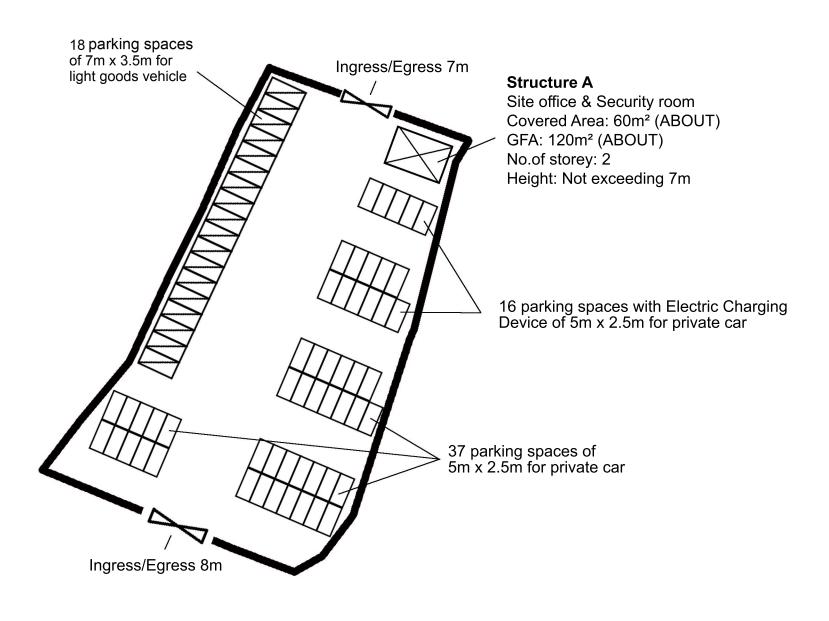
To manage the stormwater flows after developing the site, this drainage proposal detailed the proposed drainage system consisting of a set of U-channels, pipe and catchpits diverting stormwater flows to avoid causing flooding to the site.

APPENDIX A





APPENDIX B



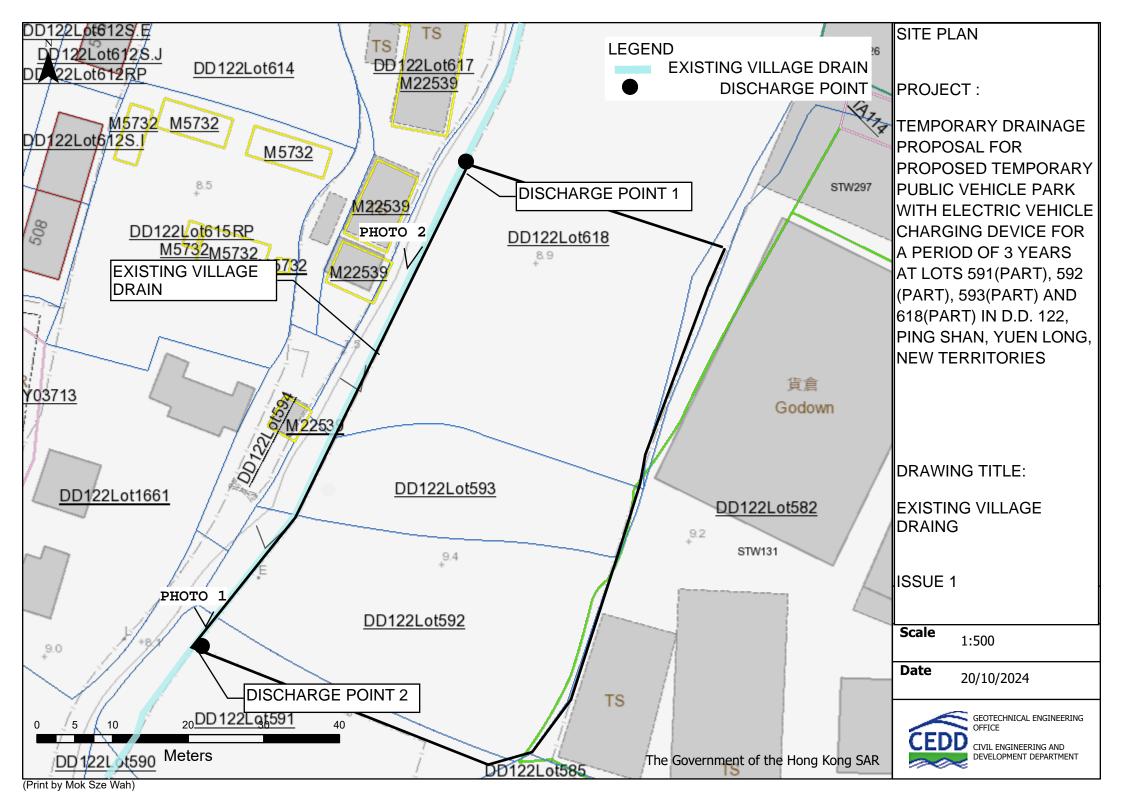


Project 項目名稱:

Proposed Temporary Public Vehicle Park with Electric Charging Device for a Period of 3 Years at Lots 591(Part), 592(Part), 593(Part) and 618(Part) in D.D. 122, Ping Shan, Yuen Long, N.T.

Drawing Title 圖紙標題: Site Plan	Remarks 備註: Structure
	Light goods vehicle
Drawing No. 圖號·	Private car

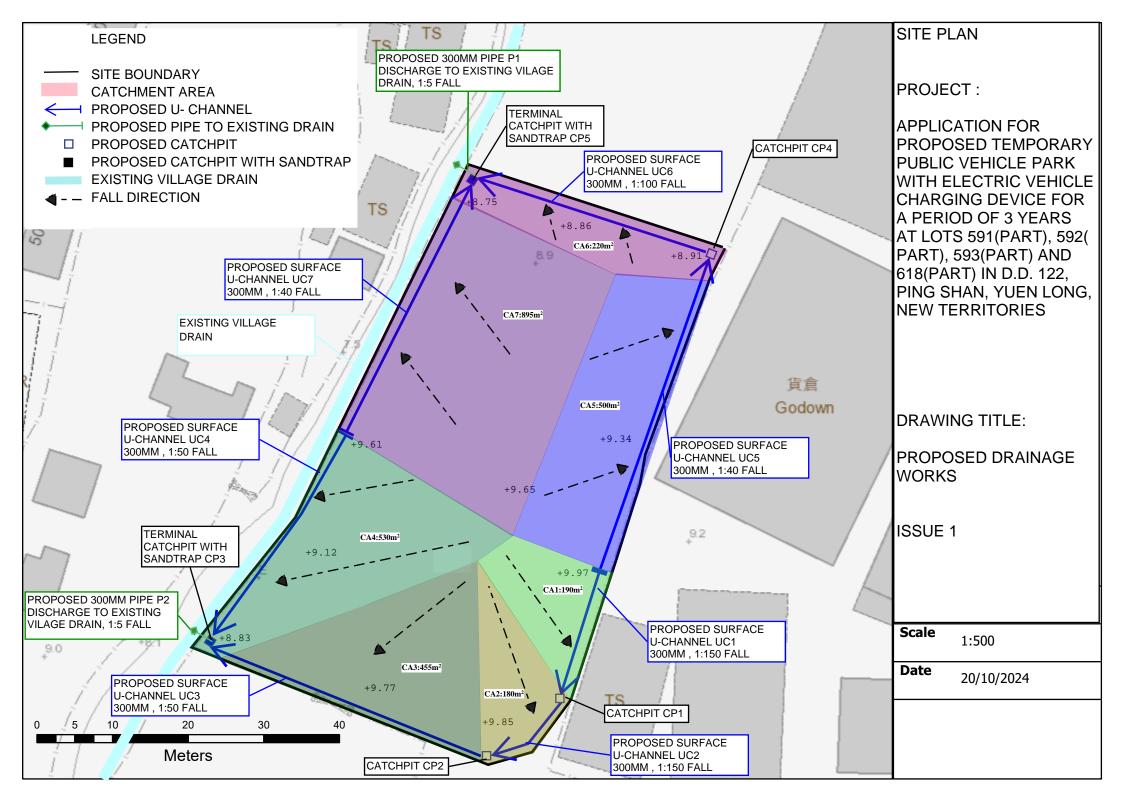
APPENDIX C







APPENDIX D



APPENDIX E

Calculation Sheet	Date:	2024-10-24
Project Title:	Project No.:	YL-PS-734
	Designed by:	RF
APPLICATION FOR PROPOSED TEMPORARY PUBLIC VEHICLE PARK WITH ELECTRIC VEHICLE CHARGING DEVICE FOR A PERIOD OF 3 YEARS AT LOTS 591(PART), 592(PART), 593(PART) AND 618(PART) IN D.D. 122, PING SHAN, YUEN LONG, NEW TERRITORIES	Appendix :	E
DOTO SALTANIA SALTANIA SALVANIA ANTANIA MARIANI NI MARIANI NI MANTANIA MARIANIA MARI	Sheet No.:	1

Design for Proposed U Channel for Development Area

Catch	pit No.	it No. Catchment Level U Channel							Manning's Equation																	
From (U/S)	To (D/S)	Incre. Area (m²)	Accum. Area (m²)	Runoff Coef. C	Ave. Slope / 100m	U/S G.L. (mPD)	D/S G.L. (mPD)	U/S I.L. (mPD)	D/S I.L. (mPD)	Mat- erial	Width (mm)	Depth (mm)	Lgth (m)	Grad. (1 in)	U Channel Area A (m2)	Wetted Peri.P (mm)	Hyd. Radius R (mm)		Vel. V at Full Bore (m/s)	Cap. Q _o (m ³ /s)	Velocity Check	Time of Conc. t _c (min)	Rainfall Intensity i (mm/hr)	Runott	Capacity % (Q/Q _o)	Capacity Check
-	CP1	190	190	1	1	9.97	9.91	9.77	9.65	CO	300	200	17.3	150	0.07	0.57	0.11	0.016	1.203	0.079	OK	1.481	229.01	0.012	15%	OK
CP1	CP2	180	370	1	1	9.91	9.85	9.60	9.52	CO	300	200	13.3	150	0.07	0.57	0.11	0.016	1.203	0.079	OK	1.497	228.89	0.024	30%	OK
CP2	CP3	455	825	1	1	9.85	8.83	9.41	8.60	CO	300	200	40.4	50	0.07	0.57	0.11	0.016	2.083	0.136	OK	1.507	228.82	0.052	39%	OK
-	CP3	530	530	1	- 1	9.61	8.83	9.41	8.76	CO	300	200	32.5	50	0.07	0.57	0.11	0.016	2.083	0.136	OK	2.511	222.06	0.033	24%	OK
-	CP4	500	500	1	- 1	9.97	8.91	9.77	8.68	CO	300	200	43.7	40	0.07	0.57	0.11	0.016	2.329	0.152	OK	3.395	216.54	0.030	20%	OK
CP4	CP5	220	720	1	1	8.91	8.75	8.68	8.31	CO	300	200	36.6	100	0.07	0.57	0.11	0.016	1.473	0.096	OK	3.422	216.38	0.043	45%	OK
-	CP5	895	895	1	1	9.61	8.75	9.41	8.48	CO	300	200	37.2	40	0.07	0.57	0.11	0.016	2.329	0.152	OK	2.727	220.67	0.055	36%	OK

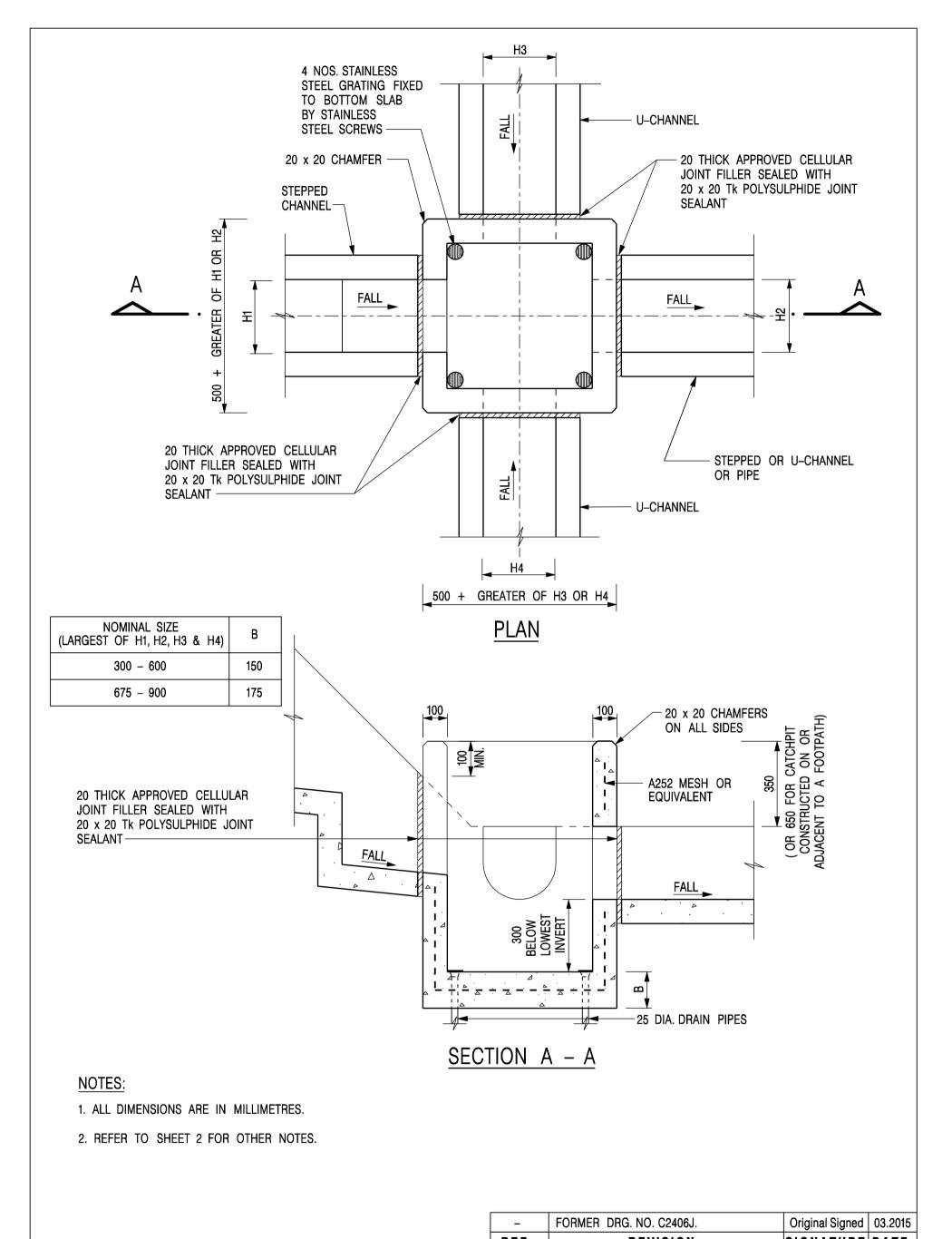
Formulae: Time of Concentration $t_o + t_f$ = 1.481 min where Inlet Time Flow Time å (Pipe Length / Flow Velocity) Pipe Flow Velocity $(R)^{2/3}s^{1/2}/n$ for Manning's Equation Gravitational Acceleartion = 9.81 m/s² where Hydraulic Radius Frictional Slope Ref. DSD SDM Table 14 Surface Roughness 3.3 mm for concrete CO cast iron 0.06 mm for CI ductile iron 0.6 mm for DI Kine. Viscosity 1.141E-06 Ref. DSD SDM Table 13 Manning's Coef. 0.016 for concrete CO cast iron 0.015 CI 0.015 for ductile iron DI Pipe Flow Capacity $(pD^{2}/4)V$ Rainfall Intensity District Area Northern 1167.6 for a return period of 50 years Ref. DSD SDM Table 3d $a/(t_c+b)^c$ 16.76 0.561 Runoff = 0.278*CiA Runoff Coefficient where C Catchment Area

Calculation Sheet	Date:	2024-10-24
Project Title:	Project No.:	YL-PS-734
APPLICATION FOR PROPOSED TEMPORARY PUBLIC VEHICLE PARK WITH ELECTRIC VEHICLE CHARGING DEVICE FOR A PERIOD OF 3	Designed by:	RF
YEARS	Appendix :	E
AT LOTS 591(PART), 592(PART), 593(PART) AND 618(PART) IN D.D. 122, PING SHAN, YUEN LONG, NEW TERRITORIES	Sheet No.:	2

Checking for Pipe Connecting to Existing Village Drain

	Cato	hpit	(Catchmen	t		Pipe Manning's Equation												
	From (U/S)	To (D/S)	Incre. Area (m²)	Accum. Area (m²)	Runoff Coef. C		D/S I.L. (mPD)	Mat- erial	Dia. (mm)	Lgth (m)	Grad. (1 in)	Mng's Coef. n	Vel V	Cap. Q _o (m ³ /s)	Time of Conc. t _c (min)	Rainfall Inten- sity i (mm/hr)	Runoff Q (m ³ /s)	Runoff / Cap. (Q/Q _o)	Capacity Check
	CP3	Existing	1355	1355	1	8.830	8.230	CO	300	3	5	0.016	4.971	0.351	5.010	207	0.078	22%	OK
- [CP5	Existing	1615	1615	1	8.750	8.150	CO	300	3	5	0.016	4.971	0.351	5.010	207	0.093	26%	OK

APPENDIX F



CATCHPIT WITH TRAP (SHEET 1 OF 2)

卓越工程 建設香港

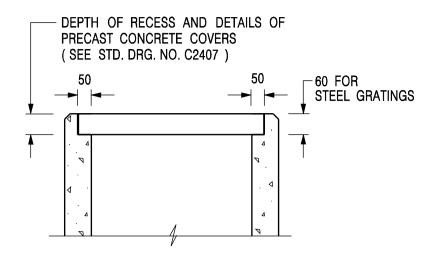
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 DRAWING NO.

DATE JAN 1991

C2406 /1

We Engineer Hong Kong's Development



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- 8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

Α	MINOR AMENDMENT.	Original Signed	04.2016
_	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

CATCHPIT WITH TRAP (SHEET 2 OF 2)

卓越工程 建設香港

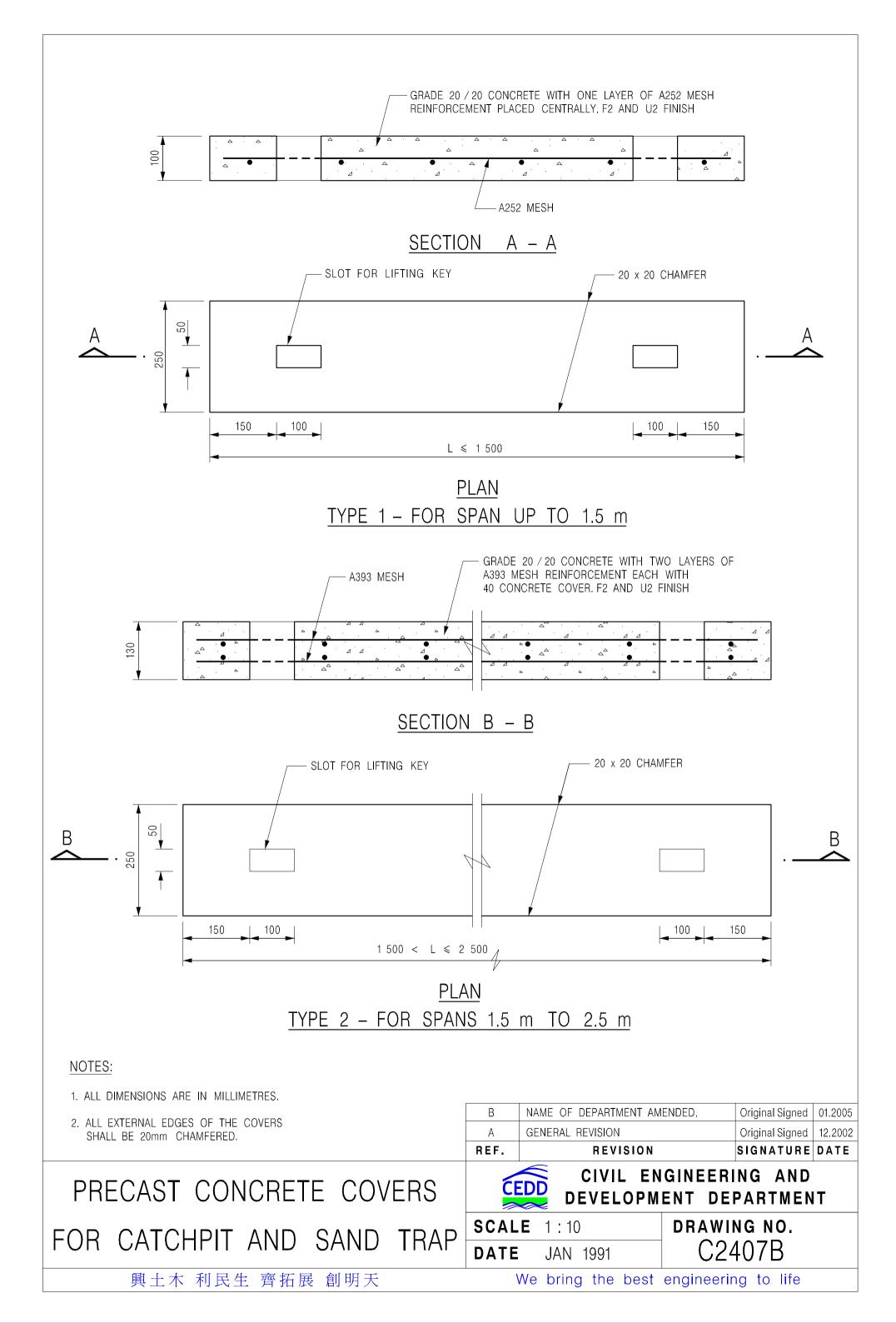


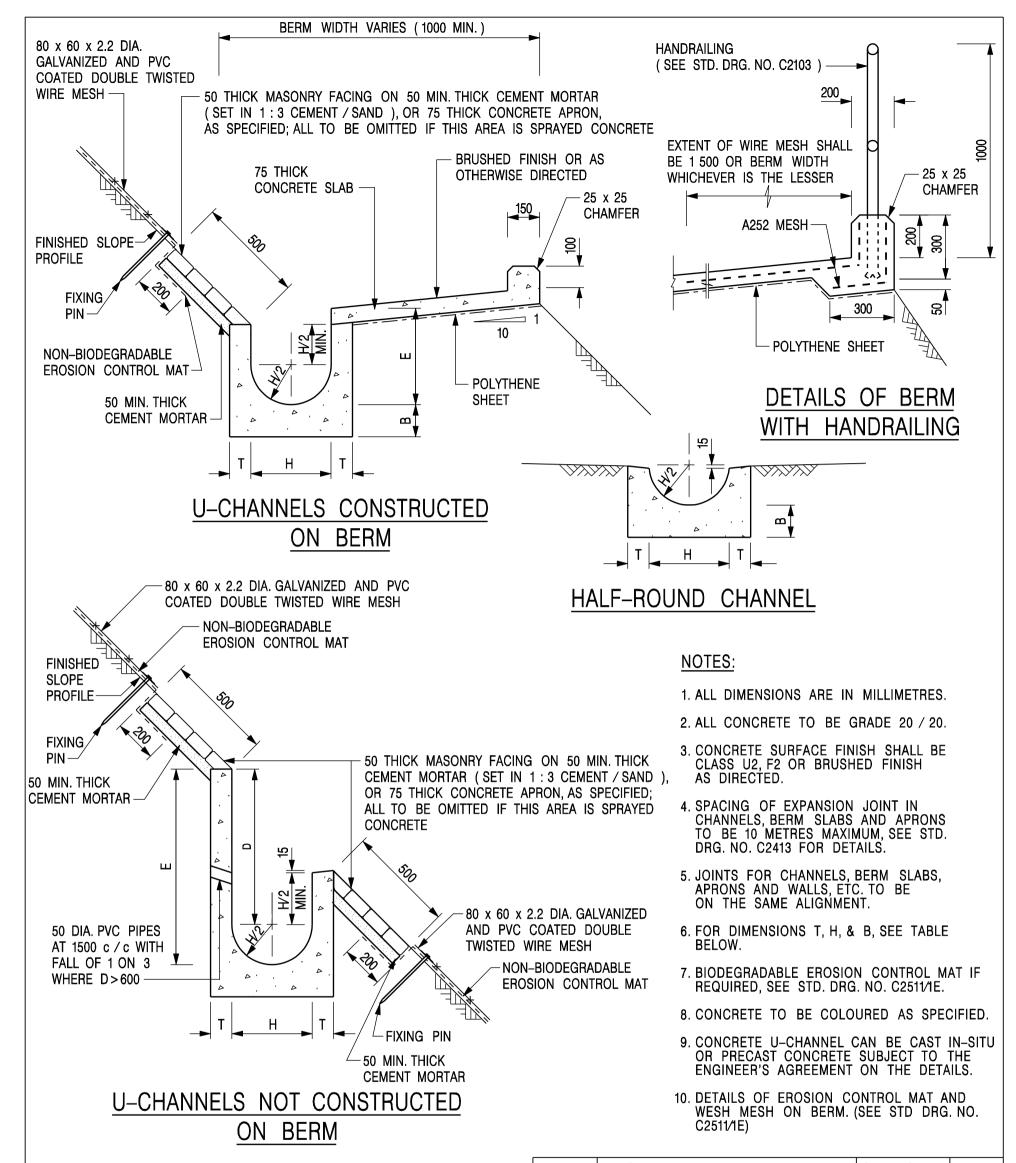
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:20
 DRAWING NO.

 DATE JAN 1991
 C2406 /2A

We Engineer Hong Kong's Development





NOMINAL SIZE H	Т	В	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100
375 - 600	100	150	WHEN E>650
675 – 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
В	MINOR AMENDMENTS.	Original Signed	3.94
С	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
D	MINOR AMENDMENT.	Original Signed	08.2001
E	DRAWING TITLE AMENDED.	Original Signed	11.2001
F	GENERAL REVISION.	Original Signed	12.2002
G	MINOR AMENDMENT.	Original Signed	01.2004
Н	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
l	MINOR AMENDMENT.	Original Signed	07.2018

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

卓越工程 建設香港

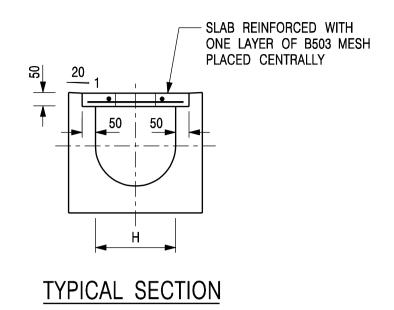


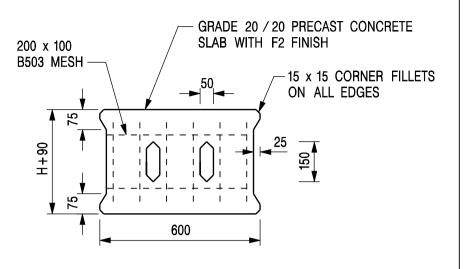
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE 1:25
 DRAWING NO.

 DATE JAN 1991
 C2409I

We Engineer Hong Kong's Development

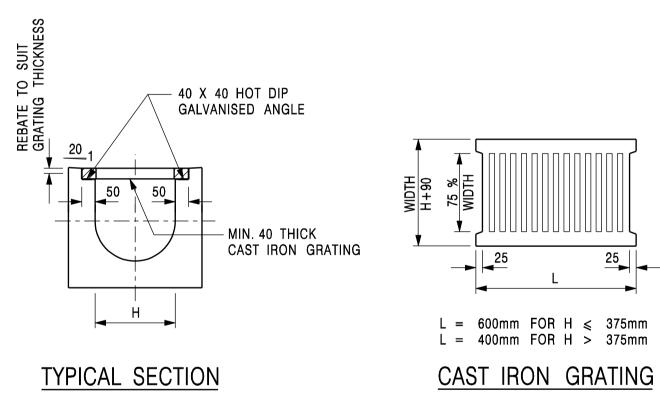




PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)



(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES
- 2. H=NOMINAL CHANNEL SIZE.
- 3. ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

Е	NOTES 3 & 4 AMENDED.	Original Signed	12.2014
D	NOTE 4 ADDED.	Original Signed	06.2008
С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
Α	CAST IRON GRATING AMENDED.	Original Signed	12.2002
REF.	REVISION	SIGNATURE	DATE

COVER SLAB AND CAST IRON GRATING FOR CHANNELS



CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 DRAWING NO. C2412E

卓越工程 建設香港 We Engineer Hong Kong's Development