From: Louis Tse

**Sent:** Wednesday, December 4, 2024 4:47 PM **To:** tpbpd/PLAND < tpbpd@pland.gov.hk >

Cc:

Subject: [FI] S.16 Application No. A/YL-SK/371 - FI to address departemntal comments

Dear Sir,

Attached herewith the revised FI to <u>supersede</u> the FI submitted on <u>04/12/2024</u> (*below email*), to address departmental comments of the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

Louis TSE | Town Planner R-riches Group (HK) Limited

R-riches Property Consultants Limited | R-riches Planning Limited | R-riches Construction Limited

**寄件者:** Louis Tse

**寄件日期:** 2024 年 12 月 4 日 下午 12:39

收件者: Town Planning Board <tpbpd@pland.gov.hk>

副本:

主旨: [FI] S.16 Application No. A/YL-SK/371 - FI to address departemntal comments

Dear Sir,

Attached herewith the further information to address departmental comments of the subject application.

Should you require more information, please do not hesitate to contact me. Thank you for your kind attention.

Kind Regards,

□Urgent □Return receipt □Expand Group □Restricted □Prevent Copy
Louis TSE   Town Planner R-riches Group (HK) Limited
R-riches Property Consultants Limited   R-riches Planning Limited   R-riches Construction Limited



Our Ref. : DD106 Lot 1012 & VL Your Ref. : TPB/A/YL-SK/371



The Secretary,
Town Planning Board,
15/F, North Point Government Offices,
333 Java Road,
North Point, Hong Kong

By Email

4 December 2024

Dear Sir,

#### 2<sup>nd</sup> Further Information

Proposed Temporary Open Storage of Construction Materials, Construction Machineries,
Auto Parts and Vehicles with Ancillary Facilities for a Period of 3 Years
and Associated Filling of Land and Pond in "Agriculture" Zone,
Various Lots in D.D. 106 and Adjoining Government land, Shek Kong, Yuen Long, New Territories

(S.16 Planning Application No. A/YL-SK/371)

We are writing to submit further information to address departmental comments of the subject application (**Appendix I**).

Should you	require more	information	regarding t	he application,	please	contact	our	Mr.
Danny NG at				or the undersign	ed at yo	our conv	enie	nce.
Thank you for your ki	nd attention.							

Yours faithfully,

For and on behalf of

**R-riches Property Consultants Limited** 

**Louis TSE**Town Planner

cc DPO/FSYLE, PlanD

(Attn.: (Attn.:





#### **Responses-to-Comments**

Proposed Temporary Open Storage of Construction Materials, Construction Machineries,

Auto Parts and Vehicles with Ancillary Facilities for a Period of 3 Years

and Associated Filling of Land and Pond in "Agriculture" Zone,

Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories

(Application No. A/YL-SK/371)

(i) The applicant would like to provide clarifications for the proposed development:

Existing Condition, Filling of Land and Pond of the Application Site (the Site)

- The Site occupied an area of 78,557m<sup>2</sup>, including 1,817m<sup>2</sup> of Government land (GL). Although the GL portion mostly encroached onto/in the vicinity of an existing natural stream located at the north of the Site flowing from east to west and connecting to Kam Tin River, according to the drainage impact assessment (DIA) submitted by the applicant (**Annex VI**), some of the runoff from the Site will be discharged into this existing stream. The GL portion is therefore deemed necessary to meet the operational needs for the proposed development.
- Portions of the Site (i.e. 22,190m²), including the dried ponds (i.e. 9,287m²) have been filled with soil at site level ranging from +12.7mPD to +15.1mPD (**Plan 1**). The current application seeks to regularize the existing land and pond filling areas. Under the current application, the Site is proposed to be filled with soil to a depth of no more than 2.3m for open storage area and circulation space, with approximately 1,320m² of the Site is proposed to be filled with no more than 0.2m of concrete at the top of the filled area to facilitate a flat surface for site formation of structures. The proposed site level varies from +15mPD to +16.6mPD after filling of land and pond (**Plan 1**). The applicant will strictly follow the proposed scheme, and no further filling of land or pond will be carried out during the planning approval period.
- A replacement page of application form is provided (Annex I).



#### Open Storage Activities within the Site

- The applicant intends to use the Site for open storage of the construction materials (i.e. tiles, pipes, socket, iron cover, water pump etc.), construction machineries (i.e. elevated platform, digging machine, generator, etc.), auto parts and vehicles (i.e. private car and light goods vehicle (LGV)) in order to support the open storage industry (**Annex II**). The open storage activities would only be carried out within the designated area as indicated in the layout plan. The stacking height of the open storage would not be more than 3m.
- 5.5 tonnes LGV will be deployed for transportation of construction materials, construction machineries, auto parts and goods to and out of the Site. As there is length restriction along the proposed routing to/out of the Site, only private car and LGV (i.e. gross weight not more than 5.5 tonnes) are allowed to enter/exit the Site at any time during the planning approval period. No medium or heavy goods vehicles exceeding 5.5 tonnes, including container tractors/trailers, as defined in the *Road Traffic Ordinance* are allowed to be parked/stored on or enter/exit the Site at any time during the planning approval period.

#### No Adverse Environmental Impact

- Solid metal walls (i.e. from 2.5m to 3.5m, subject to detail design during the construction stage) will be erected along the site boundary as sound and visual barriers, as well as to prevent dust and debris generated from the open storage activities, to minimize potential nuisance to the surrounding area. The boundary wall will be installed properly by licensed contractor to prevent misalignment of walls, to ensure that there is no gap or silt on boundary wall. In addition, maintenance will be conducted by the applicant on a regular basis.
- According to the landscape proposal provided by the applicant, preservation and direct plantation of a total number of 119 trees will also be adopted as a landscape buffer to minimize the landscape and visual impact to the nearby sensitive receivers (Annex V).
- During the operation of the proposed development, all dusty materials would be sprayed with water prior to any loading/unloading (L/UL) operation so as to maintain the dusty material wet, effective manual or automatic water spraying system would be provided and used at all L/UL areas. Wheel washing facilities would also be provided at the Site to thoroughly wash down muddy materials from the vehicle body and wheels before vehicles leaving the Site.
- During the construction stage, the applicant will follow the good practices stated in *Professional Persons Environmental Consultative Committee*



Practice Notes (ProPECC PNs) 2/23 to minimize the impact to the nearby watercourse water quality. Surface run-off from the construction phase will be discharged into storm drains through appropriately designed sand/silt removal facilities such as sand traps, silt traps, and sediment basins. Silt removal facilities, channels, and manholes will be maintained, and the deposited silt and grit will be removed on a regular basis, at the start and end of each rainstorm, to ensure that these facilities are always operational.

- During the operation of the proposed development, the major source of wastewater will be sewage from the portable toilet generated by staff. The applicant will implement good practices under *ProPECC PN 1/23* when designing on-site sewage system with the Site. Licensed collectors will be employed by the applicant to collect and dispose of sewage regularly.
- Sufficient drainage and fire installations facilities would be provided by the applicant after planning permission has been granted from the Town Planning Board to mitigate any adverse impact arising from the proposed development. Excavation work for the proposed drainage facilities will be carried out on top of the filled area upon the DIA is considered acceptable by the Drainage Services Department. As the excavation work is intended to facilitate the required drainage facilities and no excavation work would be conducted on the existing ground, adverse impact of Shui Lau Tin Site of Archaeological Interest should <u>not</u> be anticipated (**Annex III**). The applicant will reinstate the Site to a state that is suitable for agricultural activities after planning approval period.

#### (ii) A RtoC Table:

	Departmental Comments	Applicant's Responses
1. (	Comments of the Antiquities and Monuments Offic	e, Development Bureau (AMO, DEVB)
(	(Contact Person: Ms. LAU Sin Yung; Tel: 2208 4462)	
(a)	The application site is situated within Shui Lau Tin	Portions of the Site (i.e. 22,190m²), including the dried ponds (i.e. 9,287m²) have been filled with
	Site of Archaeological Interest (SAI). After	soil at site level ranging from +12.7mPD to +15.1mPD ( <b>Plan 1</b> ). Under the current application,
	reviewing the Further Information (FI01)	the Site is proposed to be filled with soil to a depth of no more than 2.3m for open storage area
	submitted by the applicant, the applicant is	and circulation space, with approximately 1,320m² of the Site is proposed to be filled with no
	required to conduct a desktop studies to	more than 0.2m of concrete at the top of the filled area to facilitate a flat surface for site
	provide information of the current condition of	formation of structures.
	the Subject Site (information including but not	



limited to the area that had been filled up, the thickness of the modern fill and area that had been disturbed) and details of the works, the disturbance to be caused by the proposed application to the archaeological deposits, the impact assessment on the archaeological potential of the Subject Site and so on in agreement with AMO. According to the result of the baseline review, if archaeological investigation is necessary, applicant shall engage an archaeologist to apply for a licence under the Antiquities and Monuments Ordinance, Cap. 53 to conduct it. An archaeological impact assessment (AIA) shall be submitted to AMO for agreement prior to applying for the licence. Subject to the findings of AIA, appropriate mitigation measures, if needed, shall be recommended for agreement by AMO and implemented by the Applicant to the satisfaction of AMO.

Regarding the drainage facilities, including peripheral u-channels and catchpits proposed in the revised DIA report, excavation work for the proposed drainage facilities (i.e. Approximately 478m² of the Site will be excavated of not more than 1m in depth for drainage facilities) will be carried out on top of the filled area upon the DIA is considered acceptable by the Drainage Services Department. As the small scale of excavation work is intended to facilitate the required drainage facilities and <u>no</u> excavation work would be conducted on the existing ground, adverse impact of Shui Lau Tin Site of Archaeological Interest should <u>not</u> be anticipated. Please refer to the photomontage and revised DIA at **Annexes III** and **VI** for details.

#### 2. Comments of the Commissioner for Transport (C for T)

(Contact Person: Mr. Phil CAI; Tel: 2399 2421)

(a) Given the size of the site is large, the applicant shall provide anticipated traffic generation during construction stage.

Mini dump trucks, flatbed trucks and mini excavators will be deployed for transportation of concrete to facilitate the land filling works during the construction stage. It is estimated that not more than <u>30</u> trips will be generated and attracted by the application site (the Site) per day, details are as follows:



		Time Period	Trip (	Generation	Trip	Attraction		Total	
		(Mon to Sat)							
		08:00 – 10:00		0		0		0	
		10:00 – 12:00		5		5	10		
		13:00 – 15:00		5		5		10	
		15:00 – 17:00		5		5		10	
		As number of vehicular trips generated and attracted by the proposed development is minimal, adverse traffic impact to nearby road network should not be anticipated.							
(b)	Please advise why 18 number of private car	The proposed trip ger	neration	at am and pr	n peak ar	e revised as f	follows:		
	parking space is provided for staff but the trip								
	generation at am and pm peak are 7 and 2 only.				Trip Gen	eration and A	Attraction		
		Time Period		PC		LGV		2-Way	
				In	Out	In	Out	Total	
		Trips at <u>AM peak</u> pe (09:00 – 10:00)	r hour	18	3	5	1	27	
		Trips at <u>PM peak</u> pe (17:00 – 18:00)	r hour	3	18	2	6	29	
		Traffic trip per hour (average)		3	3	5	5	16	
(c)	Please elaborate the reason of allocating the	As the nature of the	ʻopen st	torage' use	requires e	efficient vehi	cle circul	ation L/UL are	eas, and
	parking as well as loading / unloading space in the	the adequate parking spaces. The proposed layout with allocating the parking and L/UL space in the corners of the Site is considered necessary to accommodate the seamless movement of light							ce in the
	corners of the site.								of light
		goods vehicles, trucks traffic.	and oth	er equipmer	nt, as well	as the safe s	eparation	of staff and v	ehicular



# 3. Comments of the Chief Town Planner/Urban Design and Landscape, Planning Department (CTP/UD&L, PlanD) (Contact Person: Mr. HUI Yu San, Samuel; Tel: 3565 3957)

(a) It is noted that the application site boundary is revised and the site area is reduced. 60 nos. of existing trees are surveyed within site. 35 nos. of new tree are proposed within site to mitigate the loss of 33 nos. of trees proposed to be felled. Please find our comments from landscape planning perspective:

It is noted from the R to C table that no protected species was identified. However, *Dalbergia odorifera* (降香黃檀) (T12) which is the protected species under Protection of Endangered Species of Animals and Plants Ordinance (Cap.586) was identified in the Tree Survey Schedule. Please clarify.

According to the tree survey conducted on 16/08/2024, 31/10/2024, 29/11/2024 and 2/12/2024, a total of 119 nos. of trees were record within the Site, within which trees clusters were found among the northeastern, northwestern and western sides of the Site (Annex IV). 77 out of 119 nos. of identified trees are proposed to be felled, 42 are proposed to be retained and 77 of new trees are proposed to be planted. Please refer to tree treatment schedule at Annex IV for details of the tree information and treatment.

To facilitate the proposed development, a tree preservation and landscape proposal (TPLP) has been submitted by the applicant to provide landscape mitigation measures within the Site (Annex V). Direct plantation and preservation of a total of <u>119</u> trees will be adopted as a landscape buffer to minimize the landscape and visual impact to the nearby sensitive receivers.

Among the 119 nos. of identified trees, T12 and T92 are identified as *Pterocarpus indicus* (紫檀) and T61, T101 and T102 are identified as *Aquilaria sinensis* 土沉香 (牙香樹), which are the protected species under Protection of Endangered Species of Animals and Plants Ordinance (Cap. 586) (**Annex IV**). These protected species will be preserved and maintained by the applicant during the planning approval period.

As preservation and direct plantation will be adopted by the applicant to retain the tree resources within the Site, all these trees within the Site will be well maintained by the applicant during the planning approval period, adverse landscape impact generated from the proposed development should not be anticipated.

(b) Based on our site photos taken in May 2024, approximate 3 nos. of Aquilaria sinensis (土沉香) and which is rare and protected species were observed in the west of the site near the annotation "GV05" in Tree Survey Plan (Plan3). The applicant is advised to review the Tree Survey Plan and avoid removal of rare and protected species.

T61, T101 and T102 were newly records as *Aquilaria sinensis* 土沉香(牙香樹) at the Site near "GV05". These trees will be preserved and maintained by the applicant during the planning approval period.

(c) According to the aerial photo of 2023 and the site photos provided in the submission, dense woodland is observed in the west of the site. The applicant should review and provide the existing tree information for our consideration.

The existing tree information, including the west of the Site has been updated, please refer to the revised Tree Survey Report for details.

## 4. Comments of the Chief Engineer/Mainland North, Drainage Services Department (CE/MN, DSD) (Contact Person: Mr. Kenneth CHAN; Tel: 2300 1259)

(a) The application site is encroached onto/in the vicinity of an existing streamcourse. The applicant shall be required to place all the proposed works 3m away from the top of the bank of the streamcourse. All the proposed works in the vicinity of the streamcourse should not create any adverse drainage impacts, both during and after construction. Proposed flooding mitigation measures if necessary shall be provided at the resources of the applicant to my satisfaction.

Noted. All proposed works are 3m away from the top of the river bank. Fencing will also be erected along the existing streamcourse to separate the proposed development and streamcourse. All the proposed works in the vicinity of the streamcourse would not create any adverse drainage impacts. Please also refer to the revised drainage impact assessment report (Annex VI).

(b)	Please show the construction details as all discharge point and indicate all C.L., I.L and catchpit/streamcourse bottom level in the drawing.	Noted. The connection details at all discharge points are shown on Drawing No. V1053/003A~006A in Appendix A.
(c)	The details showing in the stormwater drainage design table are incompatible.	Noted. Please find the revised calculations in Appendix B.
(d)	Please show breakdown of catchment area (for each row) in the stormwater drainage table.	Noted. Please find the revised calculations in Appendix B.
(e)	Calculation to demonstrate the downstream drainage system receiving the discharge from the development has adequate spare capacity to accommodate the runoff is required.	Noted. The downstream drainage system for site discharges after development has sufficient spare capacity to accommodate the runoff. Please find the calculations in Appendix B.
(f)	The natural stream of the proposed discharge point near MH6 and Outlet C are not maintained by this Department, consent from the concerned department/maintenance parties/owners should be obtained for the proposed connection to their drainage systems.	Noted. Consent from the concerned departments/maintenance parties/owners would be obtained for the proposed connections to their drainage systems.
(g)	The applicant should submit form HBP1 to this Division for application of technical audit for any proposed connection to DSD's drainage facilities.	Noted. Form HBP1 shall be submitted for application of technical audit for any proposed connection to DSD's drainage facilities.



#### S.16 Planning Application No. A/YL-SK/371

(h)	The	applicant	t sha	all re	esolve		any
	conflict	disagreer	nent wit	h relevar	nt lot c	wne	r(s)
	and se	ek Lands[	o's perm	ission fo	or layi	ng r	new
	drains/	channels g ones ir	and/or	modify	/ing/up	grac	ling
	existing	g ones ir	n other	private	lots	or	on
	Govern	ment land	outside '	the appli	cation	site.	

Noted. We will liaise with relevant lot owner(s) to resolve any conflict/disagreement and seek LandsD's permission for laying new drains/channels and/or modifying/upgrading existing ones in other private lots or on Government land outside the Site.



#### **EXISTING CONDITION OF THE APPLICATION SITE**

APPLICATION SITE AREA (ABOUT) EXISTING SITE SURFACE : SOILED GROUND (ABOUT) EXISTING FILLED AREA : 22,190 m<sup>2</sup> (ABOUT) EXISTING SITE LEVELS : +12.7 mPD TO +15.1 mPD (ABOUT)

EXISTING FILLING OF POND AREA : 9,287 m<sup>2</sup> (ABOUT) EXISTING FILLING OF LAND AREA : 12,903 m<sup>2</sup> (ABOUT)

SITE LEVELS ARE FOR INDICATIVE PURPOSE ONLY.

# APPLICATION SITE 12.7 +14.4

#### **EXISTING SITE LEVEL OF** THE APPLICATION SITE

(INDICATIVE ONLY)



#### PROPOSED FILLING OF LAND AND POND AREA OF THE APPLICATION SITE

APPLICATION SITE AREA : 78.557 m<sup>2</sup> (ABOUT) PROPOSED FILLING OF LAND AREA : 78,557 m<sup>2</sup> (ABOUT) DEPTH OF LAND FILLING : NOT MORE THAN 2.3 m PROPOSED SITE LEVELS : +15.0 mPD TO +16.6 mPD (ABOUT)

PROPOSED FILLING OF POND AREA : 9,287 m<sup>2</sup> (ABOUT) DEPTH OF POND FILLING : NOT MORE THAN 0.5 m (ABOUT)

MATERIAL OF LAND AND POND FILLING : SOIL

PURPOSE OF LAND FILLING OPEN STORAGE AREA, AND CIRCULATION SPACE

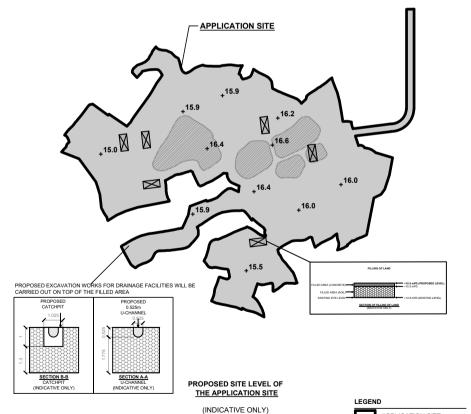
FILLING OF LAND AND POND FOR SITE

FORMATION OF STRUCTURES# : 1,320 m<sup>2</sup>

MATERIAL OF FILLING FOR SITE

FORMATION OF STRUCTURE : CONCRETE

#0.2m OF CONCRETE WILL BE FILLED ON TOP OF THE FILLED LAND/POND TO FACILITATE A FLAT SURFACE FOR SITE FORMATION OF STRUCTURES









PROPOSED TEMPORARY OPEN STORAGE OF CONSTRUCTION MATERIALS, CONSTRUCTION MACHINERIES, AUTO PARTS VEHICLES ANCILLARY FACILITIES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF LAND AND POND

VARIOUS LOTS IN D.D. 106 AND ADJOINING GOVERNMENT LAND SHEK KONG, PAT HEUNG, YUEN LONG, NEW TERRITORIES

SCALE							
1 : 5000 @ A4							
DRAWN BY	DATE						
MN	26.3.2024						
REVISED BY	DATE						
LT	28.11.2024						
APPROVED BY	DATE						
DWG TITLE							

APPLICATION SITE STRUCTURE

FILLING OF POND AREA

FILLING OF LAND AREA

+16.4 PROPOSED SITE LEVEL

FILLING OF LAND & POND AREA

PLAN 1 001

6. Type(s) of Applicat	ion 申請類	別						
(A) Temporary Use/Development of Land and/or Building Not Exceeding 3 Years in Rural Areas or Regulated Areas 位於鄉郊地區或受規管地區土地上及/或建築物內進行為期不超過三年的臨時用途/發展 (For Renewal of Permission for Temporary Use or Development in Rural Areas or Regulated Areas, please proceed to Part (B))								
(如屬位於鄉郊地區或受	規管地區臨時用	途/發展的規劃	<b>劉</b> 許 リ 續期 , 請填	[寫(B)部分)				
(a) Proposed use(s)/development 擬議用途/發展	Machineri Years and	es, Auto Part I Associated l	s and Vehicles w Filling of Land an	d Pond	es for a Period of 3			
	_		the proposal on a lay	out plan) (請用平面圖	說明擬議詳情)			
(b) Effective period of permission applied for 申請的許可有效期		year(s) 年 month(s) 個月						
(c) <u>Development Schedule 發</u>	<b>医細節表</b>							
Proposed uncovered land a					sq.m <b>d</b> About 約 sq.m <b>d</b> About 約			
Proposed covered land area				•	sq.m ⊯About ≲y			
Proposed number of buildi			染物數目		••••			
Proposed domestic floor ar	ea 擬議住用樓面	面積		IN/A	sq.m □About 約			
Proposed non-domestic flo	or area 擬議非住	用樓面面積			sq.m <b>忆</b> About 約			
Proposed gross floor area ‡	疑議總樓面面積		•••••	1,320	sq.m 🗹 About 約			
Proposed height and use(s) of 的擬議用途 (如適用) (Please STRUCTURE USE		_			上,請另頁說明)			
B1 SITE OFFICE AND WASHROOM B2 SITE OFFICE AND WASHROOM B3 SITE OFFICE AND WASHROOM B4 SITE OFFICE AND WASHROOM B5 SITE OFFICE AND WASHROOM B6 SITE OFFICE AND WASHROOM	220 m <sup>2</sup> (ABOUT) 220 m <sup>2</sup> (ABOUT)	220 m² (ABOUT) 220 m² (ABOUT) 220 m² (ABOUT) 220 m² (ABOUT) 220 m² (ABOUT) 220 m² (ABOUT)	4 m (ABOUT)(1-STOREY)					
Proposed number of car parking	or spaces by types	1,320 m <sup>2</sup> (ABOUT) 不同種粕使	古份的锻送數日	<u> </u>				
Private Car Parking Spaces 利	• • • • • •	11円(里規)字	中·1770 7.13% 时 	18				
Motorcycle Parking Spaces	軍車車位							
Light Goods Vehicle Parking								
Medium Goods Vehicle Parkin	- 1							
Heavy Goods Vehicle Parking	-	4 日 単 位						
Others (Please Specify) 其他	(高月グリッカ)							
Proposed number of loading/u	nloading spaces _	二落客貨車位的	内擬議數目					
Taxi Spaces 的士車位								
Coach Spaces 旅遊巴車位								
Light Goods Vehicle Spaces				18				
Medium Goods Vehicle Space								
Heavy Goods Vehicle Spaces								
Others (Please Specify) 其他	(萌列明)							

#### Annex II - Details of the Open Storage Uses

Open storage activities would only be taking place on the designated area within the application site (i.e. about 62,606m²). The examples of the construction materials/machineries, auto parts and vehicles are shown as follows:

#### Type of Open Storage of Construction Materials

(1) Tiles: from 0.3 m (L) x 0.3 m (W) and up to 0.6 m (L) x 0.6 m (W)



(2) Iron cover: 0.6 m (L) x 0.6 cm (W) x 0.1 m (H)



(3) Stainless Steel Socket: 0.05 m (L) x 0.05 m (W) x 0.3 m (H)



(4) Pressure Pipe: ranges from 0.3 m (W) to 0.9 m (W), with lengths typically around 1 m



(5) Water pump: 0.35 m (L) x 0.4 m (W) x 0.3 m (H)



#### Type of Open Storage of Construction Machineries

(1) Digging Machine: 0.4m (L) x 0.3m (W) x 0.4m (H)



(2) Generator: 1.2m (L) x 1.16 (W) x 1.2m (H)



(3) Elevated platform: 1.44m(L) x 0.76m (W) x 1.7m (H)



#### Type of Open Storage of Auto Parts and Vehicles

#### (1) Examples of Auto Parts





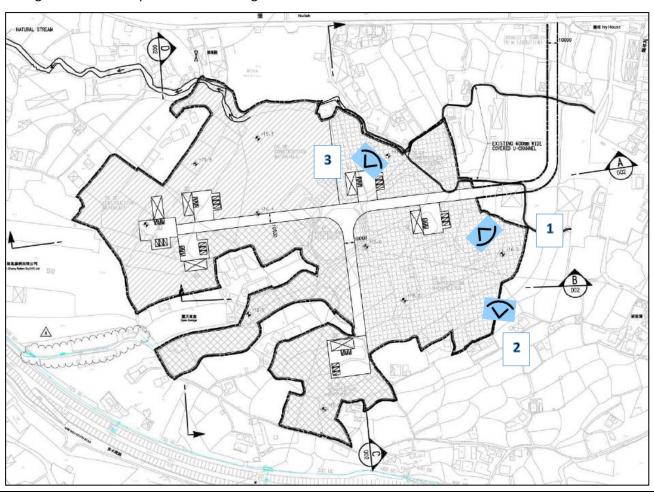


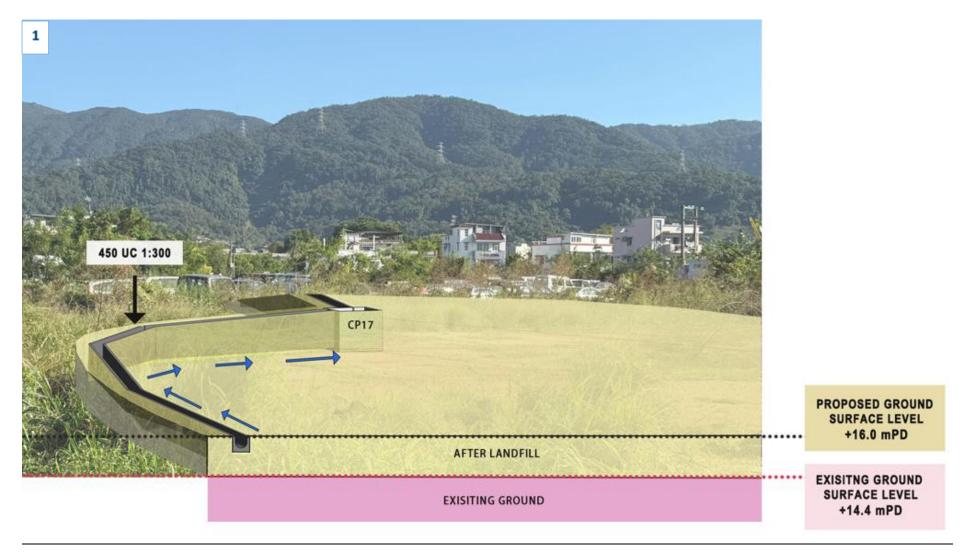
#### (2) Examples of Vehicle



#### Annex III – Excavation Works for the Proposed Drainage Facilities

The proposed excavation work for the proposed drainage facilities will be carried out on top of the filled area upon the drainage Impact assessment is considered acceptable by the Drainage Services Department. As the excavation work is intended to facilitate the required drainage facilities and no excavation works would take place on the existing ground, adverse impact of Shui Lau Tin Site of Archaeological Interest should <u>not</u> be anticipated. Photomontage to illustrate the proposed filling of land and the provision of drainage facilities are shown as follows:

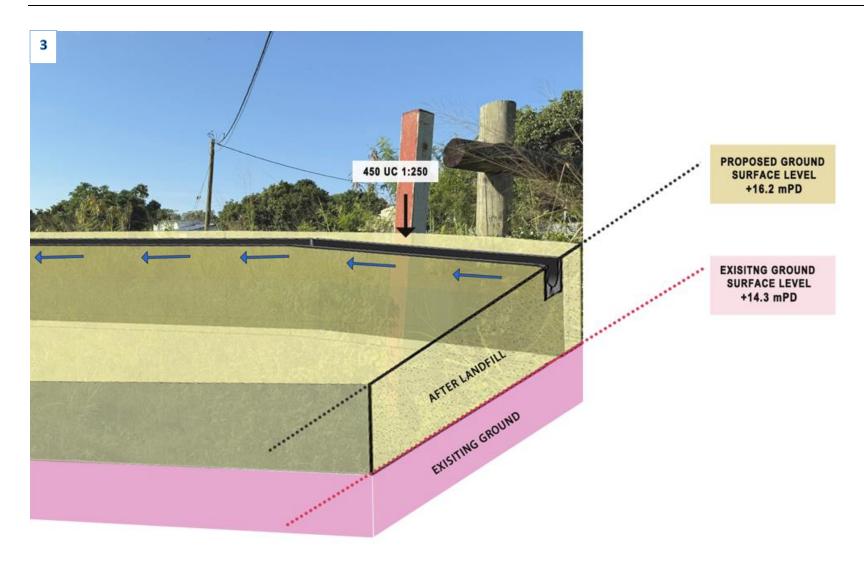




(Photo taken on 28.12.2024)



(Photo taken on 28.12.2024)



(Photo taken on 28.12.2024)



## **Tree Survey Report**

Date of Survey: 16<sup>th</sup> August, 31<sup>st</sup> October, 29<sup>th</sup> November, 2<sup>nd</sup> December 2024

#### **Location:**

Various Lots in D.D. 106 and Adjoining Government Land Shek Kong, Pat Heung, Yuen Long, New Territories

Prepared by:

Mak Ka Hei

Registered Arborist

Date: 2<sup>nd</sup> December 2024



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#### Appendix:

- I. Tree Survey Plan
- II. Tree Survey Schedule
- III. Photo Records

#### Disclaimer:

The tree survey conducted indicates the condition of the surveyed trees at the time of inspection only. The assessments of amenity value, form, health and structural condition of the trees surveyed are based on visual inspection from the ground only. No aerial inspection, root digging or mapping, or diagnostic testing has been conducted as part of this survey. Wing Ho Yuen Landscaping Company Limited cannot accept responsibility for future failure or defects detected after the time of inspection of the trees surveyed in this report.



#### 1. Introduction

The survey conducted is to record all the existing trees in the tree survey boundary. The survey include tree species identification, tree tagging with durable labels, the measurements of overall tree height, Diameter at Breast Height (DBH), average crown spread, the evaluation on amenity value, form, health and structural conditions.

The tree survey was conducted on 16<sup>th</sup> August, 31<sup>st</sup> October, 29<sup>th</sup> November, 2<sup>nd</sup> December 2024. Plants with DBH less than 95mm were not recorded in the survey.



#### 2. Summary of Existing Trees

The surveyed site is located at Various Lots in D.D. 106 and Adjoining Government Land Shek Kong, Pat Heung, Yuen Long, New Territories

At the time of inspection on 16<sup>th</sup> August, 31<sup>st</sup> October, 29<sup>th</sup> November, 2<sup>nd</sup> December 2024, **119 nos.** tree were found within the Site. **6 nos.** of dead trees (T30, T101, T102, T103, T104, T109) were recorded in the surveyed area.

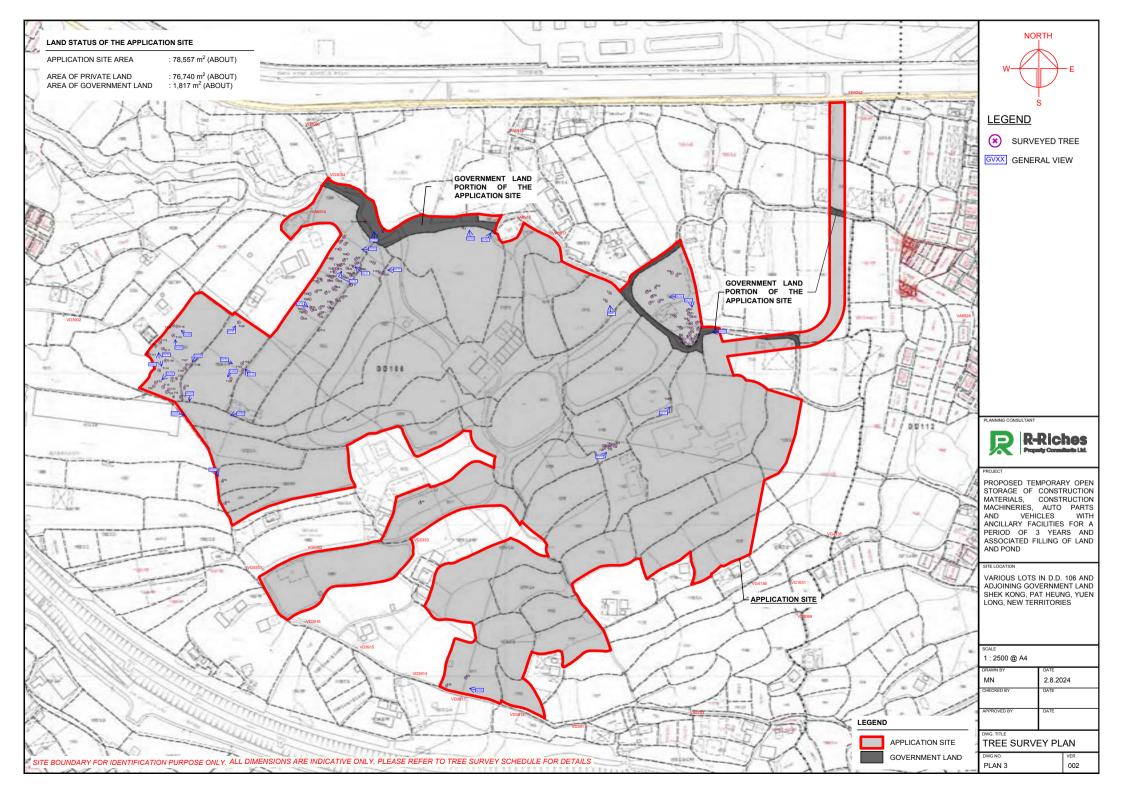
T12 and T92 were identified as *Pterocarpus indicus* 紫檀; T61, T101 and T102 were identified as *Aquilaria sinensis* 土沉香(牙香樹) which is rare and protected species. However, T101 and T102 were observed dead in the time of inspection.

Location of individual tree refers to Appendix I.

Details of tree conditions and photo records for individual tree are recorded in the Appendix II and Appendix III respectively.



# Appendix I – Tree Survey Plan





# Appendix II – Tree Treatment Schedule

		Species		Tree Size				Remarks
Tree No.	Photo No.	Scientific Name	Chinese Name	Height (m)	DBH (mm)	Crown Spread (m)	Proposed Treatment (Retain / Transplant / Fell)	(Old and Valuable Tree (OVT), potentially registrable OVT, rare species, protected species, ecological and historical significance, etc.)
T1	1	Dimocarpus longan	龍眼	6.0	180	7.5	Retain	
T2	2	Dimocarpus longan	龍眼	6.0	210	6.0	Fell	
Т3	3	Dimocarpus longan	龍眼	8.0	220	7.0	Retain	
T4	4	Citrus maxima	柚	8.0	190	7.0	Retain	
T5	5	Ficus religiosa	菩提樹	6.0	130	5.0	Retain	
Т6	6	Macaranga tanarius var. tomentosa	血桐	5.0	140	7.0	Retain	
T7	7	Leucaena leucocephala	銀合歡	7.0	120	4.0	Fell	
Т8	8	Leucaena leucocephala	銀合歡	8.0	140	2.0	Fell	
Т9	9	Leucaena leucocephala	銀合歡	8.0	150	3.0	Fell	
T10	10	Leucaena leucocephala	銀合歡	8.0	130	5.0	Fell	
T11	11	Leucaena leucocephala	銀合歡	8.0	160	5.0	Fell	
T12	12-15	Pterocarpus indicus	紫檀	16.0	410	10.0	Retain	rare species, protected species
T13	16	Macaranga tanarius var. tomentosa	血桐	5.0	100	3.0	Fell	
T14	17	Dimocarpus longan	龍眼	7.0	210	5.0	Fell	
T15	18	Macaranga tanarius var. tomentosa	血桐	7.0	180	6.0	Retain	
T16	19	Macaranga tanarius var. tomentosa	血桐	7.0	190	6.0	Retain	
T17	20	Macaranga tanarius var. tomentosa	血桐	7.0	300	8.0	Retain	
T18	21	Celtis sinensis	朴樹	10.0	180	6.0	Retain	
T19	22	Celtis sinensis	朴樹	11.0	170	5.0	Retain	
T20	23	Celtis sinensis	朴樹	10.0	160	5.0	Retain	
T21	24	Celtis sinensis	朴樹	9.0	170	6.0	Retain	
T22	25	Macaranga tanarius var. tomentosa	血桐	8.0	160	5.0	Retain	
T23	26	Macaranga tanarius var. tomentosa	血桐	8.0	150	4.0	Retain	
T24	27	Macaranga tanarius var. tomentosa	血桐	7.0	170	5.0	Retain	
T25	28	Macaranga tanarius var. tomentosa	血桐	8.0	160	7.0	Retain	
T26	29	Morus alba	桑	7.0	120	3.0	Fell	
T27	30	Macaranga tanarius var. tomentosa	血桐	6.0	130	4.0	Fell	
T28	31	Ficus hispida	對葉榕(牛乳樹)	7.0	160	5.0	Fell	
T29	32	Macaranga tanarius var. tomentosa	血桐	8.0	170	6.0	Fell	
T30	33	Dead tree	死樹	7.0	170	4.0	Fell	
T31	34	Ficus hispida	對葉榕(牛乳樹)	8.0	190	5.0	Fell	
T32	35	Celtis sinensis	朴樹	9.0	150	4.0	Fell	
T33	36	Celtis sinensis	朴樹	9.0	160	4.0	Fell	

		Species	ecies Tree Size			Remarks		
Tree No.	Photo No.	Scientific Name	Chinese Name	Height (m)	DBH (mm)	Crown Spread (m)	Proposed Treatment (Retain / Transplant / Fell)	(Old and Valuable Tree (OVT), potentially registrable OVT, rare species, protected species, ecological and historical significance, etc.)
T34	37	Celtis sinensis	朴樹	9.0	155	5.0	Fell	
T35	38	Litchi chinensis	荔枝	6.0	140	4.0	Retain	
T36	39	Litchi chinensis	荔枝	6.0	180	5.0	Fell	
T37	40	Litchi chinensis	荔枝	6.0	160	4.0	Fell	
T38	41	Litchi chinensis	荔枝	6.0	120	5.0	Fell	
T39	42	Litchi chinensis	荔枝	7.0	120	5.0	Fell	
T40	43	Dimocarpus longan	龍眼	7.0	130	4.0	Fell	
T41	44	Dimocarpus longan	龍眼	7.0	140	5.0	Fell	
T42	45	Dimocarpus longan	龍眼	7.0	170	4.0	Fell	
T43	46	Syzygium hancei	韓氏蒲桃(紅鱗蒲桃)	8.0	350	8.0	Fell	
T44	47	Averrhoa carambola	楊桃	5.0	170	6.0	Fell	
T45	48	Litchi chinensis	荔枝	6.0	180	6.0	Retain	
T46	49	Averrhoa carambola	楊桃	5.0	140	5.0	Fell	
T47	50	Psidium guajava	番石榴	6.0	120	2.0	Retain	
T48	51	Psidium guajava	番石榴	6.0	110	4.0	Retain	
T49	52	Psidium guajava	番石榴	6.0	120	3.0	Retain	
T50	53	Dimocarpus longan	龍眼	6.0	130	4.0	Retain	
T51	54	Dimocarpus longan	龍眼	6.0	140	5.0	Retain	
T52	55	Dimocarpus longan	龍眼	6.0	110	6.0	Retain	
T53	56	Mangifera indica	芒果	7.0	140	4.0	Fell	
T54	57	Dimocarpus longan	龍眼	8.0	230	5.0	Fell	
T55	58	Dimocarpus longan	龍眼	8.0	240	6.0	Fell	
T56	59	Dimocarpus longan	龍眼	8.0	220	5.0	Fell	
T57	60	Dimocarpus longan	龍眼	8.0	260	6.0	Fell	
T58	61	Dimocarpus longan	龍眼	8.0	250	5.5	Fell	
T59	62	Dimocarpus longan	龍眼	8.0	220	6.0	Fell	
T60	63	Litchi chinensis	荔枝	8.0	210	6.0	Fell	
T61	64-65	Aquilaria sinensis	土沉香(牙香樹)	5.5	210	3.0	Retain	rare species, protected species
T62	66	Macaranga tanarius var. tomentosa	血桐	6.0	240	4.0	Fell	
T63	67	Macaranga tanarius var. tomentosa	血桐	6.0	180	4.0	Fell	
T64	68	Ficus hispida	對葉榕(牛乳樹)	7.0	130	5.0	Fell	
T65	69	Leucaena leucocephala	銀合歡	7.0	150	4.0	Fell	

		Species			Tree Size			Remarks
Tree No.	Photo No.	Scientific Name	Chinese Name	Height (m)	DBH (mm)	Crown Spread (m)	Proposed Treatment (Retain / Transplant / Fell)	(Old and Valuable Tree (OVT), potentially registrable OVT, rare species, protected species, ecological and historical significance, etc.)
T66	70	Pongamia pinnata	水黃皮	7.0	130	6.0	Fell	
T67	71	Macaranga tanarius var. tomentosa	血桐	7.0	140	5.0	Fell	
T68	72	Pongamia pinnata	水黃皮	7.0	140	6.0	Fell	
T69	73	Macaranga tanarius var. tomentosa	血桐	6.0	120	4.0	Fell	
T70	74	Macaranga tanarius var. tomentosa	血桐	5.0	100	5.0	Fell	
T71	75	Macaranga tanarius var. tomentosa	血桐	6.5	110	6.0	Fell	
T72	76	Macaranga tanarius var. tomentosa	血桐	8.0	95	4.0	Fell	
T73	77	Macaranga tanarius var. tomentosa	血桐	6.0	95	5.0	Fell	
T74	78	Macaranga tanarius var. tomentosa	血桐	6.0	220	5.5	Fell	
T75	79	Leucaena leucocephala	銀合歡	6.0	100	5.0	Fell	
T76	80	Macaranga tanarius var. tomentosa	血桐	6.0	120	3.0	Fell	
T77	81	Leucaena leucocephala	銀合歡	7.0	130	4.0	Fell	
T78	82	Macaranga tanarius var. tomentosa	血桐	5.0	100	3.0	Fell	
T79	83	Macaranga tanarius var. tomentosa	血桐	6.0	95	3.0	Fell	
T80	84	Leucaena leucocephala	銀合歡	7.0	130	4.0	Fell	
T81	85	Leucaena leucocephala	銀合歡	6.0	140	3.0	Fell	
T82	86	Leucaena leucocephala	銀合歡	6.0	150	4.0	Fell	
T83	87	Leucaena leucocephala	銀合歡	7.0	120	5.0	Fell	
T84	88	Leucaena leucocephala	銀合歡	7.0	120	3.5	Fell	
T85	89	Macaranga tanarius var. tomentosa	血桐	6.0	130	4.0	Fell	
T86	90	Macaranga tanarius var. tomentosa	血桐	6.0	140	4.5	Fell	
T87	91	Leucaena leucocephala	銀合歡	7.0	100	4.0	Fell	
T88	92	Macaranga tanarius var. tomentosa	血桐	6.0	120	3.0	Fell	
T89	93	Macaranga tanarius var. tomentosa	血桐	7.0	110	4.0	Fell	
T90	94	Macaranga tanarius var. tomentosa	血桐	6.0	120	4.5	Fell	
T91	95-96	Ficus hispida	對葉榕(牛乳樹)	7.0	160	6.0	Fell	
T92	98	Pterocarpus indicus	紫檀	10.0	400	8.0	Retain	rare species, protected species
T93	99	Macaranga tanarius var. tomentosa	血桐	5.0	95	4.0	Fell	
T94	100	Macaranga tanarius var. tomentosa	血桐	6.0	120	3.0	Fell	
T95	101	Macaranga tanarius var. tomentosa	血桐	7.0	110	3.0	Fell	
T96	102	Macaranga tanarius var. tomentosa	血桐	7.0	100	3.0	Fell	
T97	103	Macaranga tanarius var. tomentosa	血桐	6.0	95	4.0	Fell	
T98	104	Macaranga tanarius var. tomentosa	血桐	6.0	100	3.0	Fell	

		Species			Tree Size			Remarks
Tree No.	Photo No.	Scientific Name	Chinese Name	Height (m)	DBH (mm)	Crown Spread (m)	Proposed Treatment (Retain / Transplant / Fell)	(Old and Valuable Tree (OVT), potentially registrable OVT, rare species, protected species, ecological and historical significance, etc.)
T99	105	Macaranga tanarius var. tomentosa	血桐	7.0	100	4.0	Fell	
T100	106	Macaranga tanarius var. tomentosa	血桐	6.0	95	2.0	Fell	
T101	107	Aquilaria sinensis	土沉香(牙香樹)	5.0	110	2.0	Retain	rare species, protected species
T102	108	Aquilaria sinensis	土沉香(牙香樹)	5.0	100	2.0	Retain	rare species, protected species
T103	109	Dead tree	死樹	6.0	130	2.0	Fell	
T104	110	Dead tree	死樹	6.0	210	4.0	Fell	
T105	111	Ficus hispida	對葉榕(牛乳樹)	5.0	120	4.0	Fell	
T106	112	Macaranga tanarius var. tomentosa	血桐	4.0	110	4.0	Retain	
T107	113	Ficus hispida	對葉榕(牛乳樹)	5.0	120	4.0	Retain	
T108	114	Bischofia javanica	秋楓	8.0	210	5.0	Retain	
T109	115	Dead tree	死樹	7.0	140	4.0	Fell	
T110	116	Cinnamomum camphora	樟	9.0	230	5.0	Retain	
T111	117	Macaranga tanarius var. tomentosa	血桐	6.0	150	4.0	Retain	
T112	118	Melia azedarach	楝(苦楝)	8.0	210	4.0	Retain	
T113	119	Macaranga tanarius var. tomentosa	血桐	7.0	110	3.0	Retain	
T114	120	Melia azedarach	楝(苦楝)	8.0	130	5.0	Retain	
T115	121	Celtis sinensis	朴樹	12.0	680	13.0	Retain	
T116	122	Macaranga tanarius var. tomentosa	血桐	7.0	140	4.0	Retain	
T117	123	Macaranga tanarius var. tomentosa	血桐	8.0	130	4.0	Retain	
T118	124-125	Celtis sinensis	朴樹	12.0	450	11.0	Retain	
T119	126	Livistona chinensis	蒲葵	9.0	240	4.0	Retain	

**Summary Table** 

	Number of Tree(s)
Tree to be Retained	42
Tree to be Transplanted	0
Tree to be Felled	77
Total Number of Existing Tree(s)	119



# Appendix III – Photo Records

# General View



General view 01





General view 03





General view 05





General view 07





General view 09





General view 11





General view 13





General view 15





General view 17





General view 19





General view 21





General view 23





General view 25





General view 27





General view 29



1 - T1 (Overview)



2 - T2 (Overview)



3 - T3 (Overview)



4 - T4 (Overview)





7 - T7 (Overview)



6 - T6 (Overview)



8 - T8 (Overview)



9 - T9 (Overview)

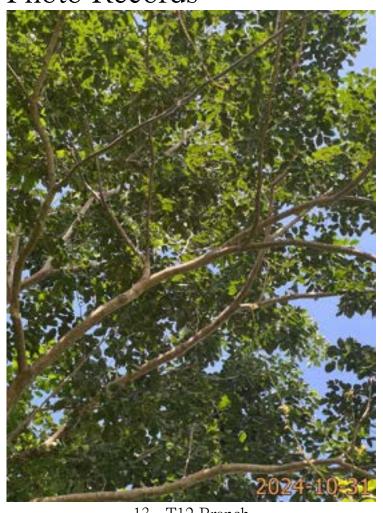


10 - T10 (Overview)



11 - T11 (Overview)





13 - T12 Branch



14 - T12 Crown



15 - T12 Root



16 - T13 (Overview)



17 - T14 (Overview)



18 - T15 (Overview)



19 - T16 (Overview)



20 - T17 (Overview)





22 - T19 (Overview)



23 - T20 (Overview)



24 - T21 (Overview)





27 - T24 (Overview)

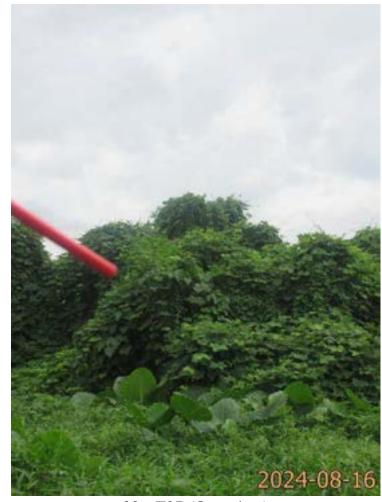


26 - T23 (Overview)





29 - T26 (Overview)



30 - T27 (Overview)



31 - T28 (Overview)



32 - T29 (Overview)



33 - T30 (Overview) (Dead tree)



34 - T31 (Overview)



35 - T32 (Overview)

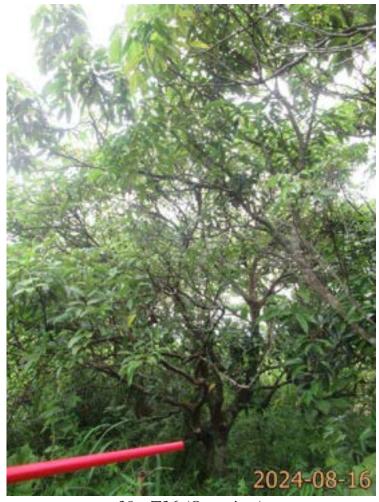




37 - T34 (Overview)



38 - T35 (Overview)



39 - T36 (Overview)



40 - T37 (Overview)



41 - T38 (Overview)





43 - T40 (Overview)



44 - T41 (Overview)



45 - T42 (Overview)



46 - T43 (Overview)



47 - T44 (Overview)







51 - T48 (Overview)



50 - T47 (Overview)





53 - T50 (Overview)



54 - T51 (Overview)

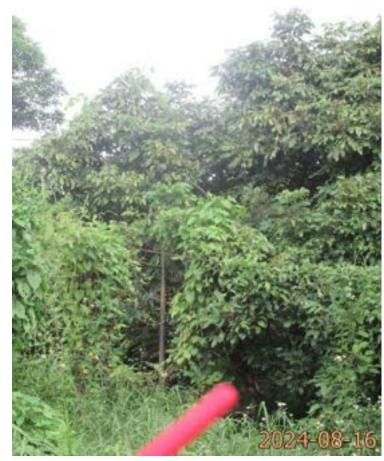


55 - T52 (Overview)





57 - T54 (Overview)



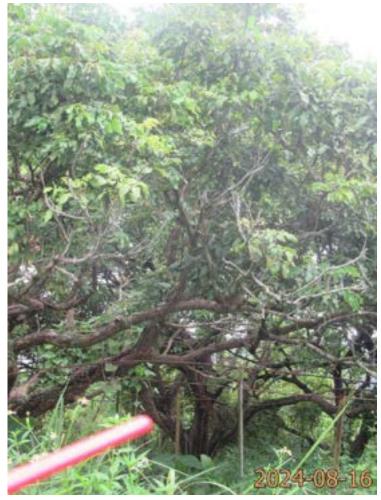
58 - T55 (Overview)



59 - T56 (Overview)



60 - T57 (Overview)



61 - T58 (Overview)



62 - T59 (Overview)

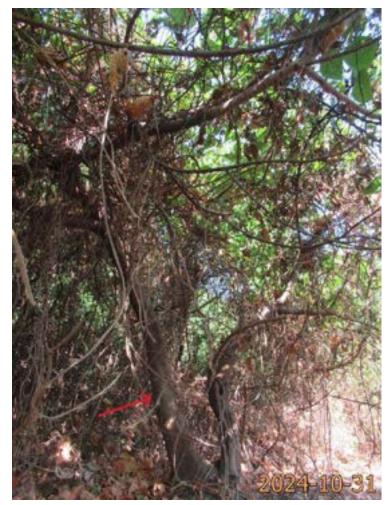


63 - T60 (Overview)





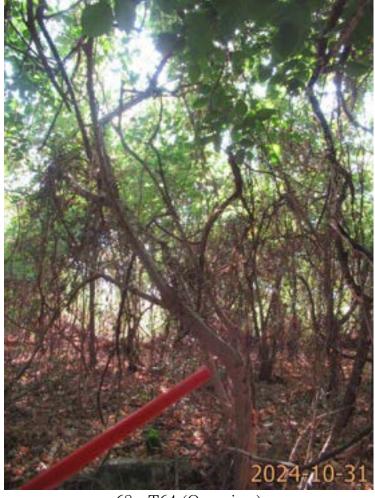
65 - T61 Co-dominant trunks



66 - T62 (Overview)



67 - T63 (Overview)



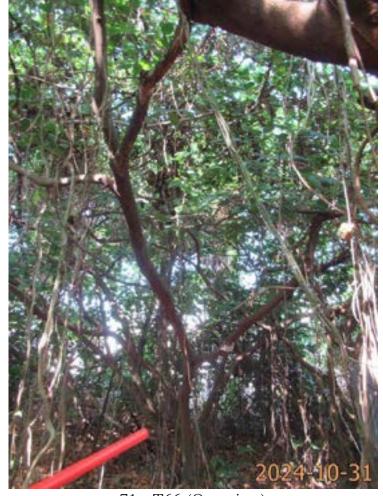
68 - T64 (Overview)



69 - T65 (Overview)



70 - T65 Uprooted



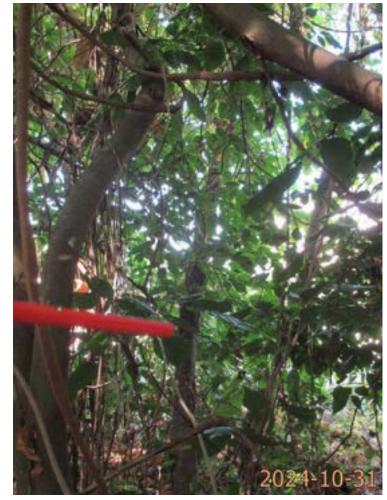
71 - T66 (Overview)



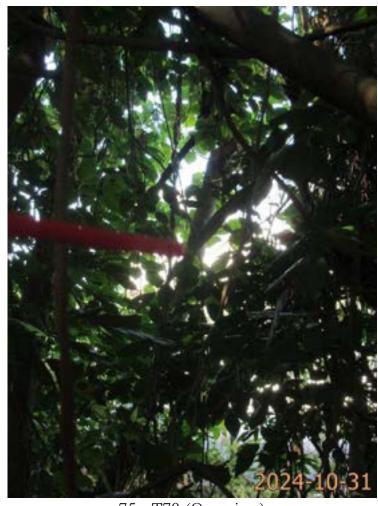
72 - T67 (Overview)



73 - T68 (Overview)



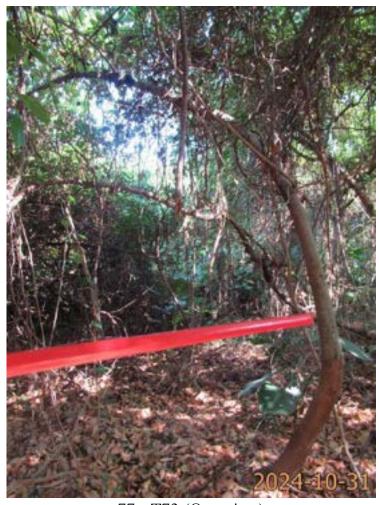
74 - T69 (Overview)



75 - T70 (Overview)



76 - T71 (Overview)





79 - T74 (Overview)



78 - T73 (Overview)





81 - T76 (Overview)



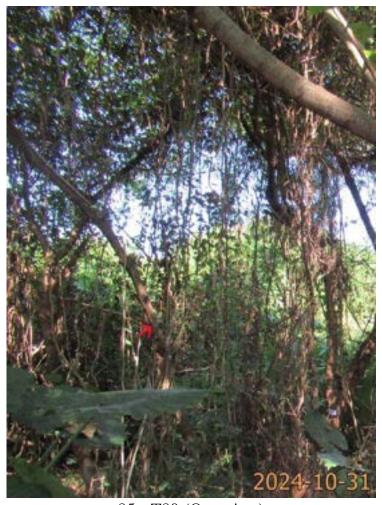
82 - T77 (Overview)



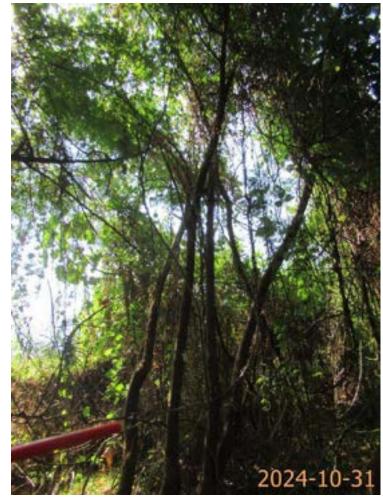
83 - T78 (Overview)



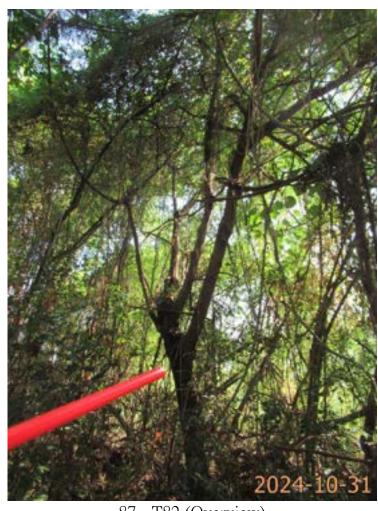
84 - T79 (Overview)



85 - T80 (Overview)

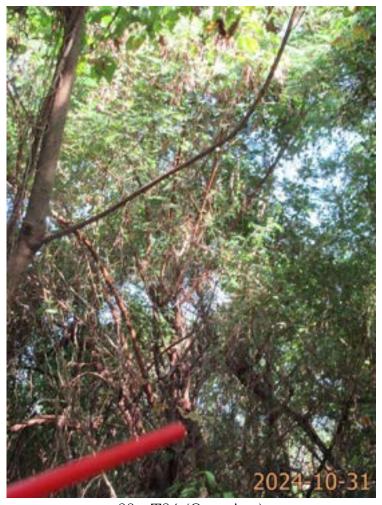


86 - T81 (Overview)



87 - T82 (Overview)





89 - T84 (Overview)



90 - T85 (Overview)



91 - T86 (Overview)



92 - T87 (Overview)



93 - T88 (Overview)



94 - T89 (Overview)



95 - T90 (Overview)



96 - T91 (Overview)



97 - T91 Co-dominant trunks



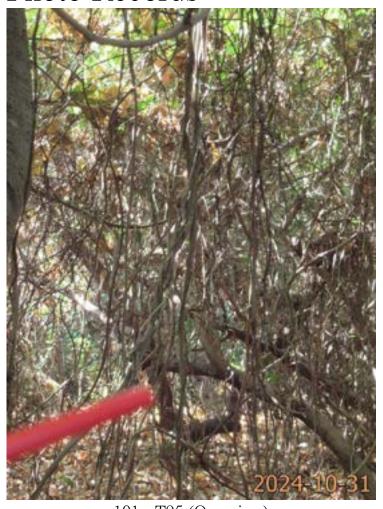
98 - T92 (Overview)



99 - T93 (Overview)



100 - T94 (Overview)



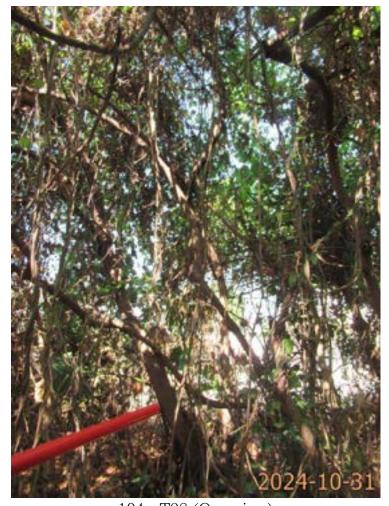
101 - T95 (Overview)



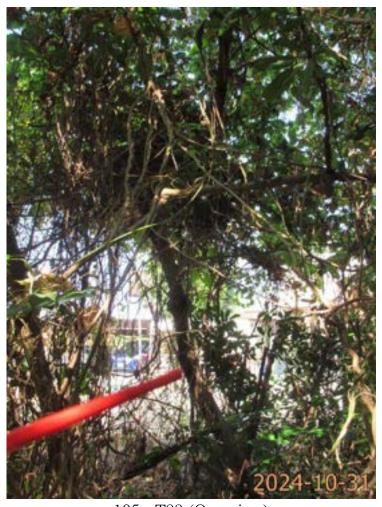
102 - T96 (Overview)



103 - T97 (Overview)



104 - T98 (Overview)



105 - T99 (Overview)





107 - T101 (Overview) (Dead tree)



108 - T102 (Overview) (Dead tree)



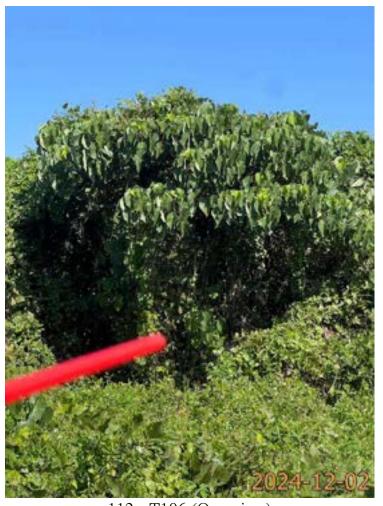
109 - T103 (Overview) (Dead tree)



110 - T104 (Overview) (Dead tree)



111 - T105 (Overview)



112 - T106 (Overview)



113 - T107 (Overview)



114 - T108 (Overview)

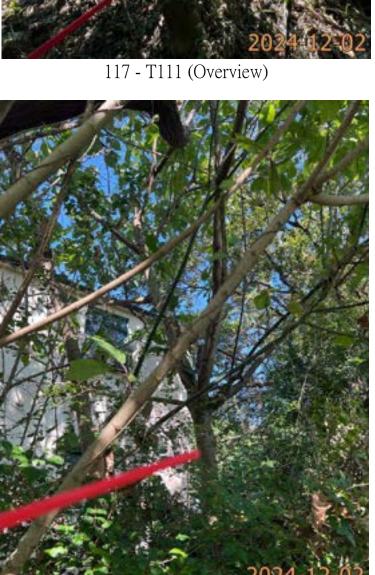


115 - T109 (Overview) (Dead tree)



116 - T110 (Overview)





119 - T113 (Overview)



118 - T112 (Overview)



120 - T114 (Overview)



121 - T115 (Overview)



122 - T116 (Overview)



123 - T117 (Overview)



124 - T118 (Overview)



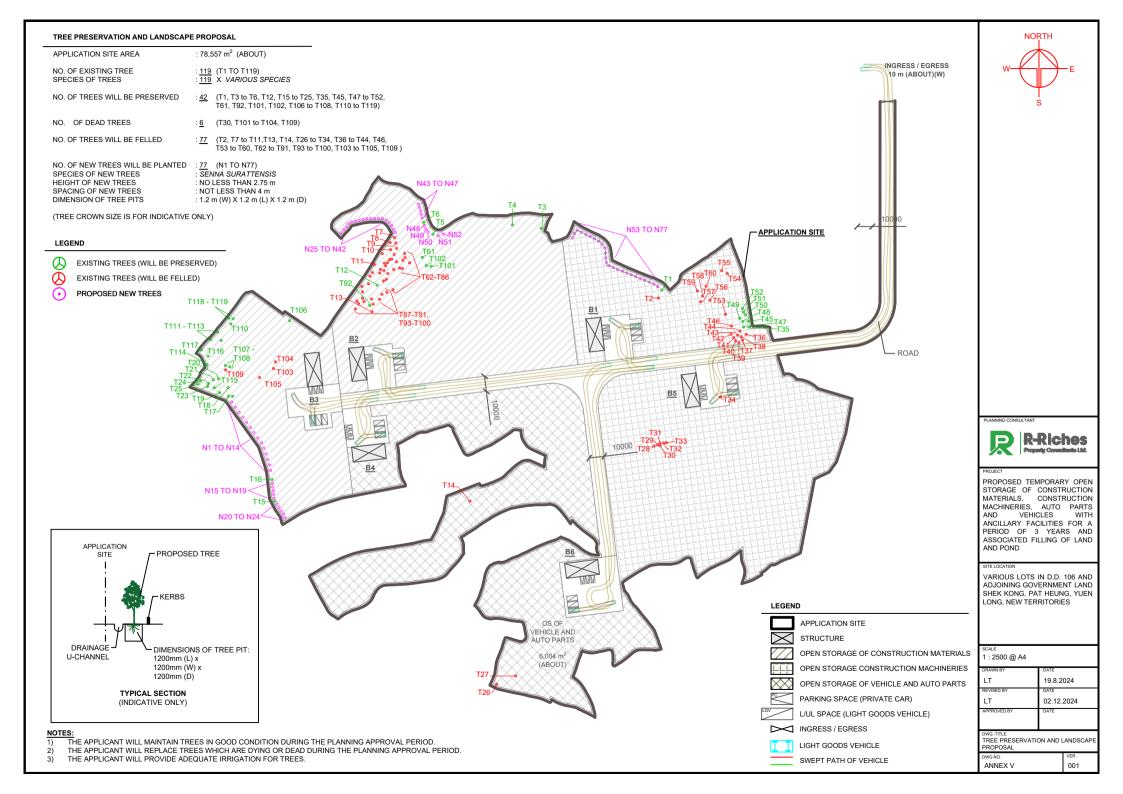




126 - T119 (Overview)

### Annex V

Tree Preservation and Landscape Proposal



### **United Crown Holdings Limited**

Proposed Temporary Open Storage of Construction Materials, Construction Machineries, Auto Parts and Vehicles with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories

#### **Drainage Impact Assessment**



Document No. V1053/01 Issue 2

December 2024



V1053/01 Issue 2 December 2024

Proposed Temporary Open Storage of Construction Materials Construction Machineries, Auto Parts and Vehicles with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories

**Drainage Impact Assessment** 

Approved for Issue by:

Kenny W K Lam RPE (Civil)

FW0275905

Position:

Deputy Managing Director

Date:

3 December 2024

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V1053/01 Issue 2 December 2024

Proposed Temporary Open Storage of Construction Materials Construction Machineries, Auto Parts and Vehicles with Ancillary Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories

#### **Drainage Impact Assessment**

Issue	Prepared by	Reviewed by	Date
1	EM	BLE	20 Sep 2024
2	EM	BLE	3 Dec 2024

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#### **Abbreviations**

D.D. Demarcation District

DSD Drainage Services Department SDM Stormwater Drainage Manual

V1053/001 Issue 2



#### 1.0 Introduction

- 1.1 This submission presents the drainage impact assessment of the proposed temporary open storage for construction materials construction machineries, auto parts and vehicles with ancillary facilities for a period of 3 years, the associated filling of land and pond at various lots in D.D. 106 and the adjoining government land at Shek Kong, Pat Heung, Yuen Long, New Territories ("Site").
- 1.2 The Site has an area of about 78,557m<sup>2</sup> and it is currently occupied by the open space uses. 6 nos. of a 1- storey structure are proposed at the Site for offices and washrooms with total GFA of about 1,320 m<sup>2</sup>. The general layout plan of the Site is shown on the **Drawing No. V1053/001A.**
- 1.3 Due to the concerns of possible drainage impact arising from the change of uses, Mannings (Asia) Consultants Limited (MACL) was appointed by the United Crown Holdings Limited to undertake a Drainage Impact Assessment (DIA) to demonstrate the acceptability of drainage impact upon the surrounding environment.

V1053/01 Issue 2 Drainage Impact Assessment



#### 2.0 Design Methodology and Assumptions

#### Design Code

- 2.1 The below design codes are to be followed for this design assessment:
  - Stormwater Drainage Manual (DSD) Fifth Edition, January 2018;
  - Stormwater Drainage Manual (DSD) Corrigendum No. 1/2022;
  - Stormwater Drainage Manual (DSD) Corrigendum No. 1/2024;
  - Stormwater Drainage Manual (DSD) Corrigendum No. 2/2024;
  - BS 5911 Code of Practice for Precast Concrete Pipe Design
  - DSD Standard Drawings

#### **Design Parameters**

#### 2.2 Design Parameters

#### a) Runoff Coefficient

Table 2-1 Runoff Coefficients

Surface Characteristic	Runoff Coefficient, C	
Roof of Structure	1.00	
Existing Concrete	0.95	
Grassland (heavy soil**) Flat	0.25	

Roughness Coefficient for pipe flow k<sub>s</sub>= 3

#### b) Minimum Pipeline Cover and Manhole Spacing Requirements

Table 2-2 Minimum Pipeline Cover and Manhole Spacing Requirements

Minimum pipeline cover			
In Roads	0.9 m		
In footways and verges	0.45 m		
Manhole spacing requirements			
D<675 mm	80 m		
675 < D < 1050	100 m		
D > 1050	120 m		

#### c) Bedding factors

-	Granular bedding	: 1.9
-	Plain concrete bedding	: 2.6
-	Reinforced concrete bedding with allowance	: 3.4
	for minimum steel area	
-	Concrete Surround	: 4.5

#### d) Design Flow Velocity



- Minimum : 1 m/s

- Maximum : 3 m/s (desirable) : 6 m/s (absolute)

2.3 The return period of 1 in 10 years is to be adopted for the drainage impact assessment.

2.4 According to Section E of the SDM Corrigendum No. 1/2022, Table 28 representing the rainfall increase due to climate change in mid of 21st Century in Table 2.3 below.

Table 2.3 Rainfall Increase due Climate Change

Time	Rainfall Increase
Mid of 21st Century (2041-2060)	11.1%

- 2.5 Description of Analysis Method
  - a) Rational method is to be adopted for calculation of the peak runoff. The formula is extracted from Section 7.5.2(a) of Stormwater Drainage Manual (SDM) which is to estimate the stormwater runoff as shown below:

$$Q_p = 0.278 \text{ CiA}$$

Where 
$$Q_p = \text{peak runoff in m}^3/\text{s}$$

C = runoff coefficient (dimensionless)

i = rainfall intensity in mm/hr A = catchment area in km<sup>2</sup>

- b) 10% reduction of the flow area is allowed taken into account of the decomposition of siltation as per DSD's SDM 2018.
- c) The time of concentration used for determining the duration of the design storm is considered by the time of entry and the time of flow,

$$t_c = t_e + t_f$$
  $t_f = L/V$ 

d) where to = inlet time (time taken for flow from the remotest point to reach the most upstream point of the urban drainage system)

$$\begin{array}{ccc} Where & & t_f & = flow\ time \\ & L & = Length\ of\ drain \\ V & = flow\ velocity \\ \end{array}$$

e) The time of entry or time of flow in the hinterland is calculated using the Bransby William's Equation.

$$t_e = \frac{0.14465 \, L}{A^{0.1} H^{0.2}}$$

Where  $t_e = time of concentration (min)$ 



L = catchment length (m) A = catchment area (m2)

H = average catchment slope (m/100m)

f) The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship.

$$i = \frac{a}{(t_d + b)}$$

Where

i = extreme mean intensity in mm/hr

 $t_d$  = duration in minutes (td<240), and

a, b, c =storm constants given in table 3 of SDM as below

g) According to Section A of the SDM Corrigendum No. 1/2024, Table 3a representing the Storm Constants for 10 years Return Periods of HKO Headquarters in Table 2.4 below.

Table 2-4 Storm Constant of SDM

Return Period T (years)	10
a	485
ь	3.11
С	0.397

h) Colebrook-White Equation is used in hydraulic design for pipe flow.

$$V = -\sqrt{(32gRs)}\log\left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{(32gRs)}}\right)$$

Where:

V = mean velocity (m/s)

g = gravitational acceleration  $(m/s^2)$ 

R = hydraulic radius (m) D = pipe diameter (m)

 $k_s$  = equivalent sand roughness (m)

v = kinematic viscosity of fluid  $(m^2/s)$ 

s = frictional slope (energy gradient due to frictional loss)



#### 3.0 Current Flooding Susceptibility and Proposed Drainage

#### Current Site Condition and Flooding Susceptibility

3.1 The topography of the Site is generally flat and currently situated with levels ranging from +12.7 mPD to +15.10 mPD. In general, the direction of existing surface runoff flows from east to west. Since the ground levels of the Site are generally higher than those of the existing surrounding area, flooding susceptibility of the Site is considered as low.

#### **Proposed Development**

3.2 6 nos. of a 1- storey structure are proposed at the Site for offices and washrooms as stated in Para. 1.2. After completion of the project, the finished ground level of the Site will be raised to approximately +15.0 mPD to +16.6 mPD. Part of the unpaved areas is proposed to be occupied by 6 new covered structures whilst the remaining unpaved area would be unchanged in regards of the finished surface and continued to be an opened space area. In addition, some of these unpaved opened areas are proposed to be served as access road and parking spaces. A layout plan of the proposed development with **Drawing No. V1053/001A** is enclosed in **Appendix A**.

#### Proposed Drainage

- 3.3 According to the site survey and observation, there is a natural stream located at the north of the Site flowing from east to west and connecting to the Kam Tin River. Site photos are shown in **Appendix C**. Some of the runoff from the Site will be discharged into this natural stream which the assessment of the flow would be shown in the following of this report.
- 3.4 In addition, there are existing u-channels and underground pipes located at the south of the Site. From our observations, these existing facilities discharge part of the storm drain of the Site's surface runoff into the stream at the south. Site photos are shown in **Appendix** C.
- 3.5 The catchment plan after upon completion of the proposed development is demonstrated on the **Drawing No. V1053/010A** in **Appendix A.** The surface runoff within the Site's area will be collected by the proposed drainage systems and to be discharged into the existing drains. The drainage layout plans are shown on the **Drawing Nos. V1053/003A** to 006A in **Appendix A.**
- 3.6 The proposed U-channels and drainage pipes are designed to have sufficient capacities for the estimated runoff from the unpaved area and structure roofing in the Site. Details of the calculation are enclosed in **Appendix B**.



#### 4.0 Changes to the Drainage Characteristics and Potential Drainage Impact

#### Changes in Land use and Surface Runoff Characteristics

4.1 The Site is currently covered in grassland with few temporary structures. After completion of the project, the Site will remain as an unpaved area except the proposed structure. Runoff coefficient are shown in Table 2-1 under Para. 2.2.

#### Changes to Surface Runoff Hydrographs

4.2 Changes in land use from unpaved area to paged area would lead to higher and faster surface runoff. However, considering the paved area of the proposed development is relatively small, the changes to surface runoff hydrographs is considered as negligible.

Changes in Flood Storage

4.3 No flood storage was found near the Site.

#### Changes of Timing of Peak Runoff

4.4 Changes of time of concentration of Outlet A, B, C and D before and after development are summarized in below table. The calculation is attached in **Appendix B**.

Outlet	Time of concentration (min)		
	Before Development After Development		
Outlet A	7.87	7.94	
Outlet B	12.38	14.84	
Outlet C	5.77	5.79	
Outlet D	9.42	9.27	

#### Hydraulic Capacity of the Proposed Drainage System

- 4.5 The proposed drainage system mentioned in Para. 3.3 to Para 3.5 are designed to have sufficient capacity to cater the flow from the Site. Detailed calculation is attached in **Appendix B**.
- 4.6 The design runoff, capacity and utilization of the U-channels are summarized in below table.



Proposed U-Channel	Design Runoff (m <sup>3</sup> /s)	Capacity (m <sup>3</sup> /s)	Utilization		
To Outlet A					
Upstream U-Channel to CP22	0.05	0.08	0.63		
CP22 to CP23	0.14	0.24	0.58		
CP25 to CP24	0.16	0.24	0.67		
Upstream U-Channel to CP25	0.08	0.24	0.33		
Upstream U-Channel to CP18	0.05	0.24	0.21		
Upstream U-Channel to CP18	0.06	0.24	0.25		
CP20 to CP19	0.04	0.24	0.17		
Upstream U-Channel to CP20	0.03	0.24	0.13		
To Outlet B					
Upstream U-Channel to CP3	0.07	0.24	0.29		
CP3 to CP1	0.11	0.24	0.46		
CP1 to CP28	0.45	0.51	0.88		
To Outlet C					
Upstream U-Channel to CP17	0.07	0.24	0.29		
Upstream U-Channel to CP17	0.17	0.24	0.71		
To Outlet D	To Outlet D				
Upstream U-Channel to CP13	0.10	0.24	0.42		
Upstream U-Channel to CP13	0.09	0.24	0.38		
Upstream U-Channel to CP7	0.06	0.24	0.25		
Upstream U-Channel to CP7	0.07	0.24	0.29		
Upstream U-Channel to CP10	0.03	0.08	0.38		

4.7 The design runoff, capacity and utilization of the proposed pipes are summarized in below table

Proposed Pipe	Design Runoff (m <sup>3</sup> /s)	Capacity (m <sup>3</sup> /s)	Utilization
To Outlet A			
CP18 to MH3	0.10	0.15	0.67
CP19 to MH3	0.04	0.15	0.27
MH3 to MH4	0.13	0.24	0.54
MH4 to MH5	0.13	0.27	0.48
MH5 to MH6	0.13	0.31	0.42
CP23 to MH6	0.14	0.30	0.47
CP24 to MH6	0.16	0.27	0.59
MH6 to Natural Stream	0.41	1.63	0.25
To Outlet C			
CP17 to MH7	0.24	0.37	0.65
MH7 to MH8	0.24	0.37	0.65
MH8 to Open Channel	0.24	0.33	0.73
To Outlet D			
CP13 to MH2	0.18	0.36	0.50
CP10 to MH2	0.03	0.11	0.27
MH2 to MH9	0.20	0.35	0.57
CP7 to MH9	0.13	0.35	0.37
MH9 to MH1	0.33	0.71	0.46
MH1 to Natural Stream	0.33	0.56	0.59



#### Changes in peak runoff and peak velocity at Outfalls

4.8 Below table shows the comparison of the peak runoff and peak velocity of the Outlet A, B, C and D before and after the development. Detailed calculation is attached in **Appendix B**.

	Before Development		After Development	
	Peak Runoff (m <sup>3</sup> /s)	Peak Velocity (m/s)	Peak Runoff (m <sup>3</sup> /s)	Peak Velocity (m/s)
Outlet A	0.70	1.545	0.68	1.545
Outlet B	0.48	1.588	0.45	1.588
Outlet C	0.99	1.889	0.98	1.889
Outlet D	0.51	2.162	0.60	2.162

#### Potential Drainage Impact to Existing Drainage System

4.9 The surface characteristics of the Site (i.e. Catchment Area Nos. A to D) under the existing conditions and the proposed works are summarized in Table 4-1.

Table 4-1 Existing and Proposed Catchment

Catahmant	Existing Ca	tchment (m <sup>2</sup> )	Proposed Catchment (m <sup>2</sup> )	
Catchment	unpaved	paved	unpaved	paved
A	36,027	5,227	31,307	6,107
В	12,325	7025	12,208	7025
С	50,423	5,110	48,500	5,330
D	32,941	1,173	38,542	1,393

4.10 The estimated runoff from the existing land use and the proposed land use is summarized in below Table 4-2.

Table 4-2 Estimated Runoff of the Existing Land Use and Proposed Land Use

Drainage System	Existing runoff (m <sup>3</sup> /s)	Future runoff (m <sup>3</sup> /s)
A	0.70	0.68
В	0.48	0.45
С	0.99	0.98
D	0.51	0.60

- 4.11 From the above results, there is no increase in surface runoff arising from the land use changes for the drainage systems A, B and C. As such, it is anticipated that there is no adverse drainage impact to the existing drainage after implementation of the land use changes.
- 4.12 For drainage system B at downstream, the existing 400mm wide U-channel will be reconstructed to 600mm wide U-channel. The existing 600mm wide U-channel has been checked and it shall provide sufficient capacity to cater for this additional flow upon completion of the proposed development.



- 4.13 For the drainage system D, there is slightly increase in surface runoff arising from the land use changes. The existing 750mm dia. pipe has been checked and it shall provide sufficient capacity to cater for this additional flow upon completion of the proposed development.
- 4.14 The downstream existing drainage systems are checked and they have adequate spare capacities to accommodate the runoff discharged from the development. Details of the calculation are enclosed in **Appendix B.**

#### Temporary Drainage during Construction

4.15 According to the site survey and observation, there is no existing drainage system in the Site. Therefore, no existing drainage system would be affected during the construction. Temporary drainage is considered not necessary.

#### Details of Works to Existing Drainage System

4.16 Proposed drainage systems are connecting to existing drainage systems as shown in **Drawing No. V1053/003A** to **006A** in **Appendix A**.

#### Potential Drainage Impacts to Other Land Users

4.17 All runoff in the Site will be collected and drain to existing drainage system, no drainage impact to other land users is anticipated.



#### 5.0 Drainage Impact Mitigation Measures

- 5.1 As discussed in Para. 4.12, the existing 400mm wide U-channel will be reconstructed to 600mm wide U-channel for downstream drainage system B. No adverse drainage impact should be aroused after the mitigation measure.
- 5.2 The Contractor should monitor during the construction to ensure that there is no adverse drainage impact to the nearby drainage systems and adjacent land users.



#### 6.0 Monitoring Requirements

#### Monitoring Requirements of During Construction

Monitoring of the drainage system is required during construction to ensure that there are no adverse impacts which may result in flooding or deterioration in the water quality.

- 6.1 Monitoring shall include:
  - Any siltation or blockages in channels, slit traps or sediment basins;
  - Checking the drainage is performing in accordance with the design;
  - Checking for damage; and
  - Visual inspection of any high sediment levels
- 6.2 The detailed requirements of drainage monitoring should be as shown in the following table:

Table 6.1 – Detailed Requirements for Drainage Monitoring

Type / location of monitoring	Minium Frequency	Action by
Prepare method statements	Before the start of any works that could impact on drainage	Contractor
	that could impact on dramage	
Inspect existing drainage systems and all	Daily, Weekly, Before every	Contractor
Construction drainage systems	rainstorm warning	
for blockages or breakages	After every rainstorm	Contractor
Inspect sedimentation basins and silt traps	Daily, Weekly, Before every rainstorm warning	Contractor
	After every rainstorm	Contractor



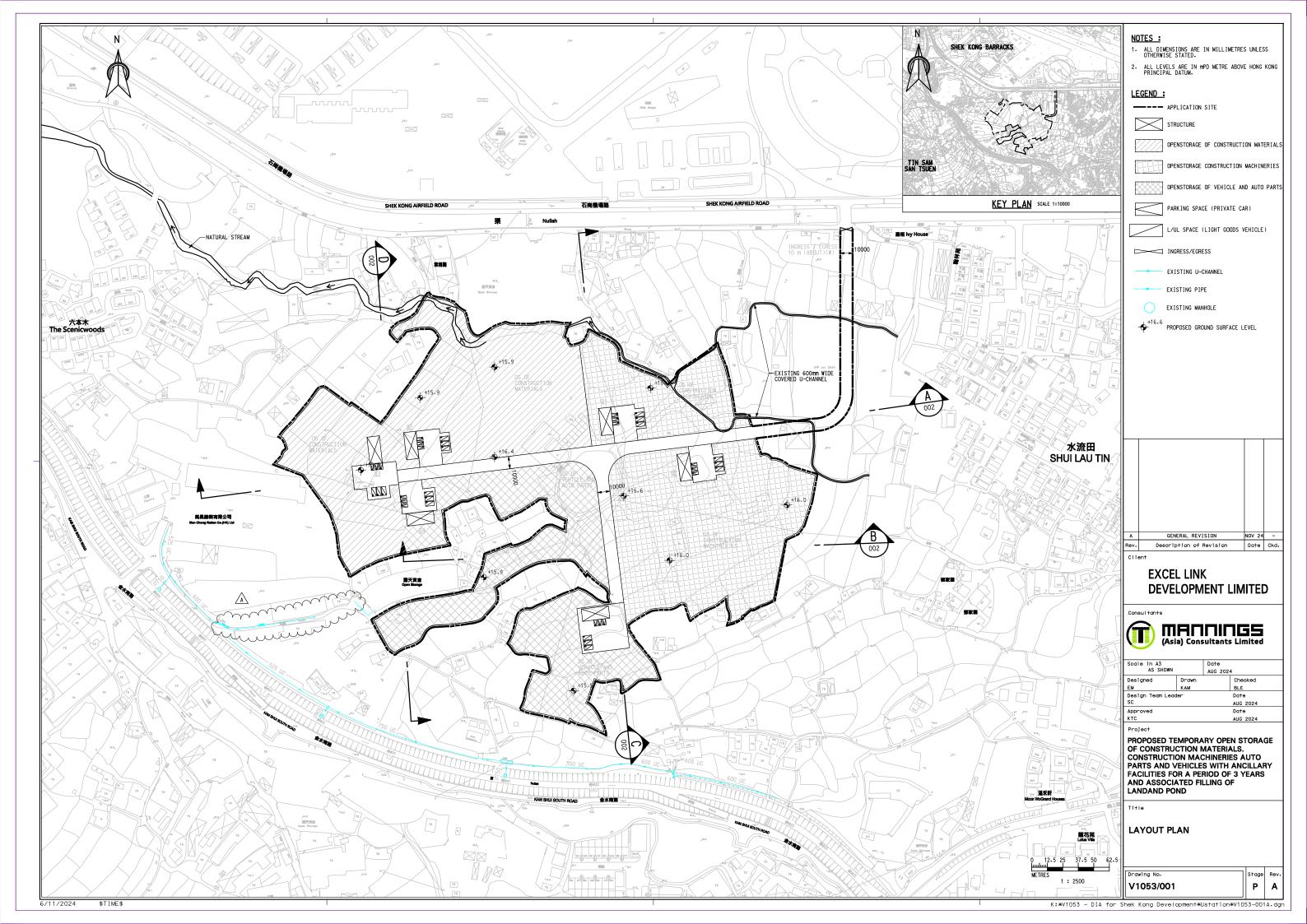
#### 7.0 Conclusion

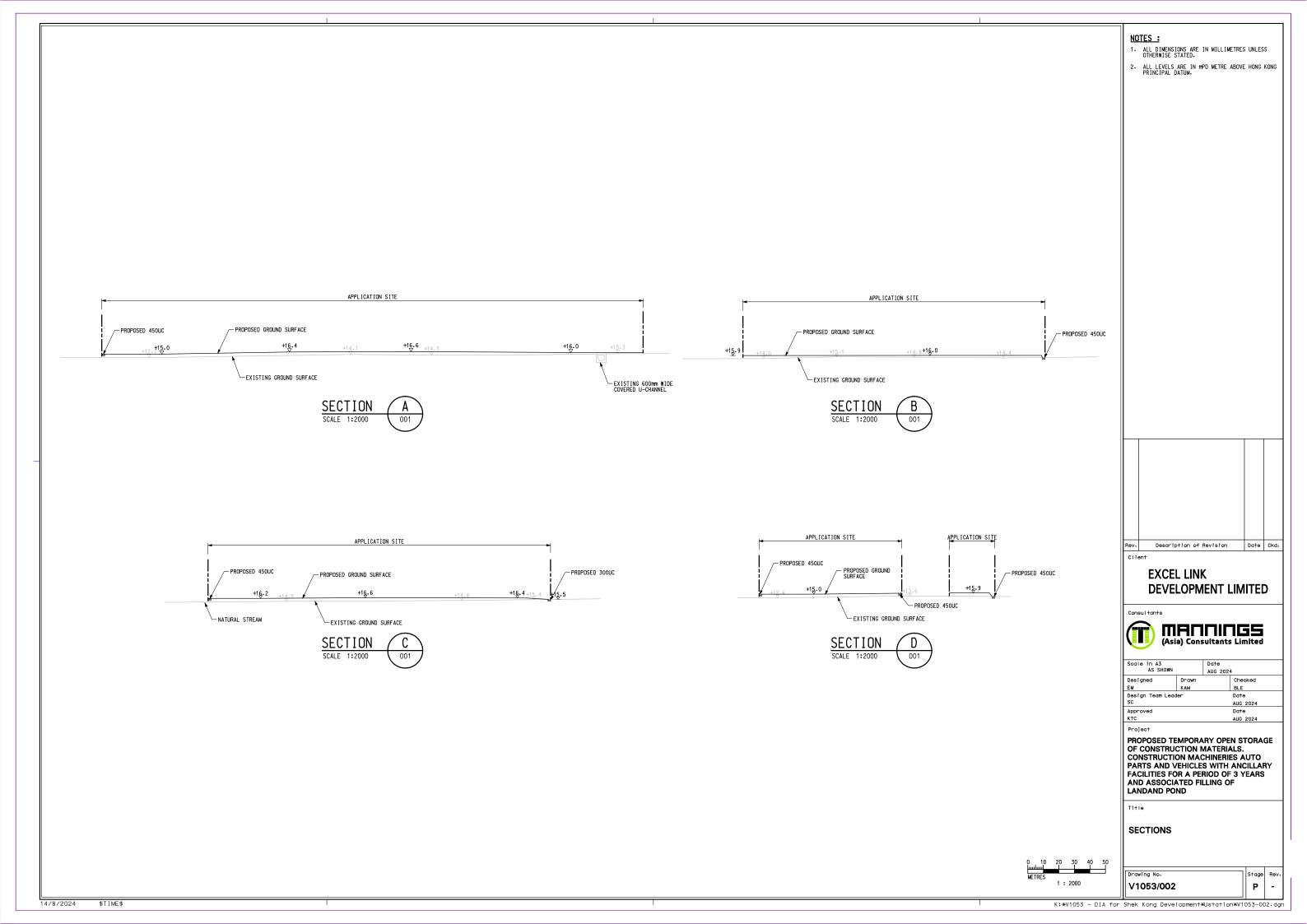
- 7.1 A Drainage Impact Assessment has been conducted for the proposed land use changes in Shek Kong. There is no increase in surface runoff for the catchment area nos. A, B and C.
- 7.2 The downstream existing drainage systems are checked for the updated runoff from the catchment and they have adequate spare capacities to accommodate the runoff discharged from the development.

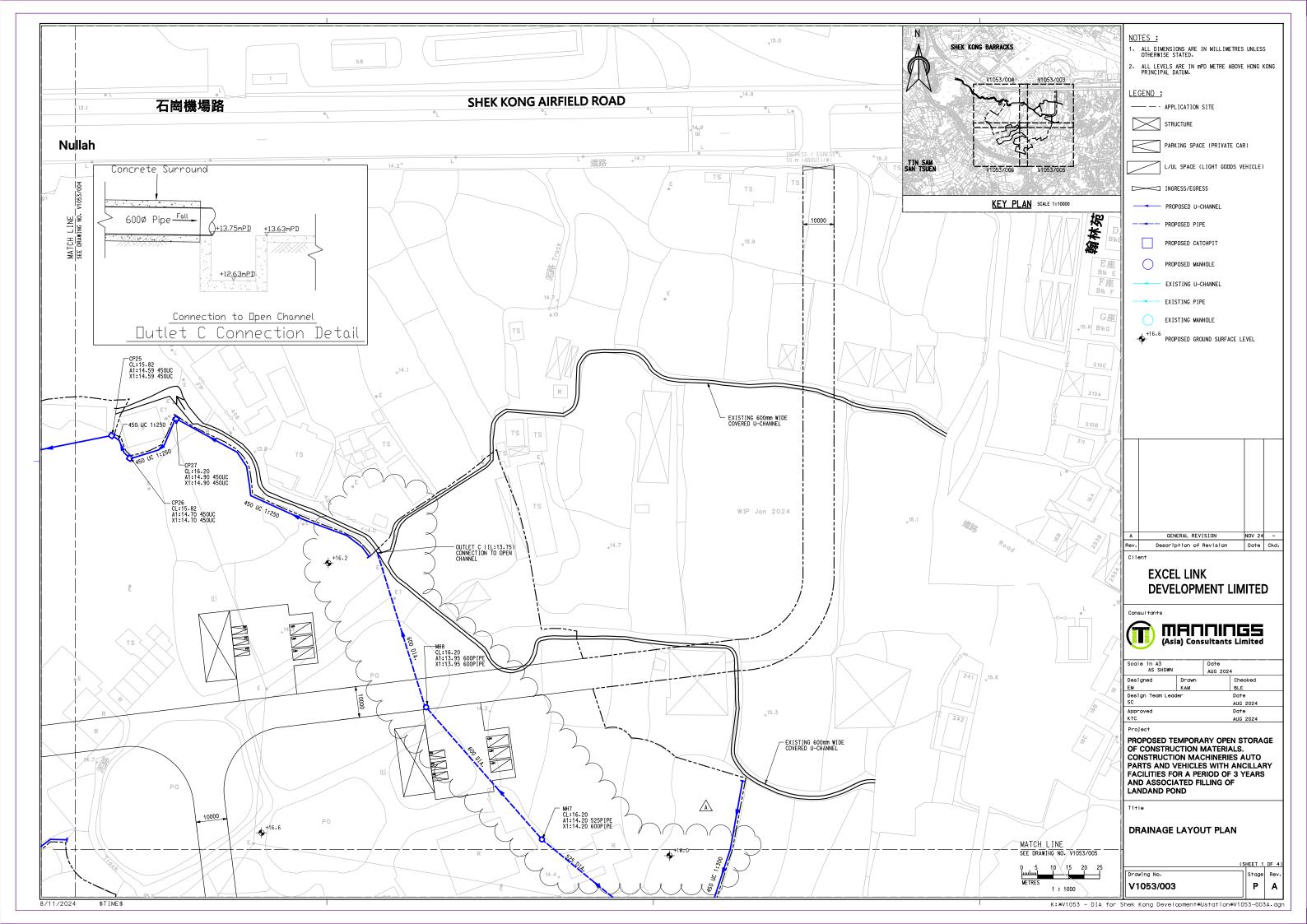


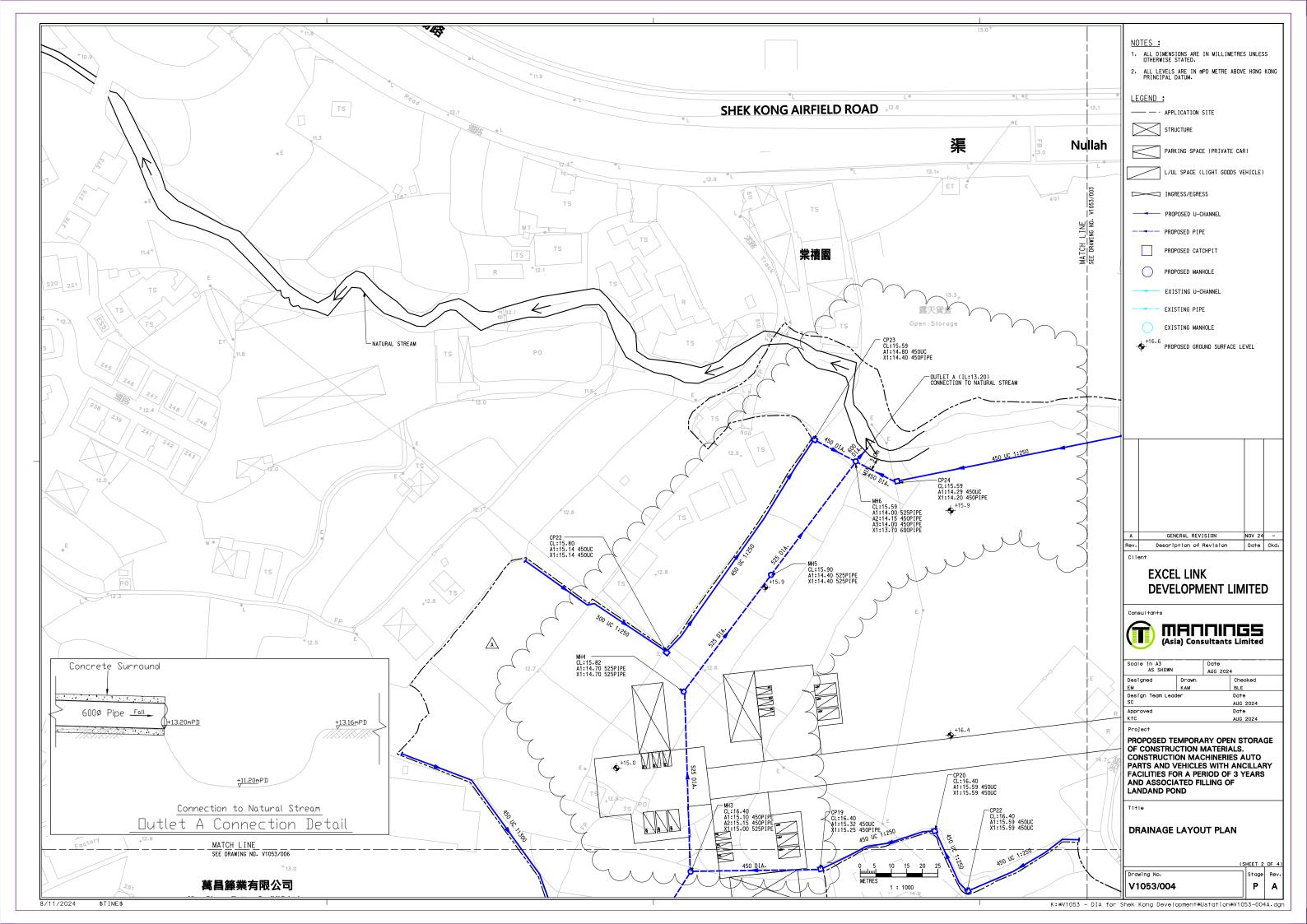
Appendix A

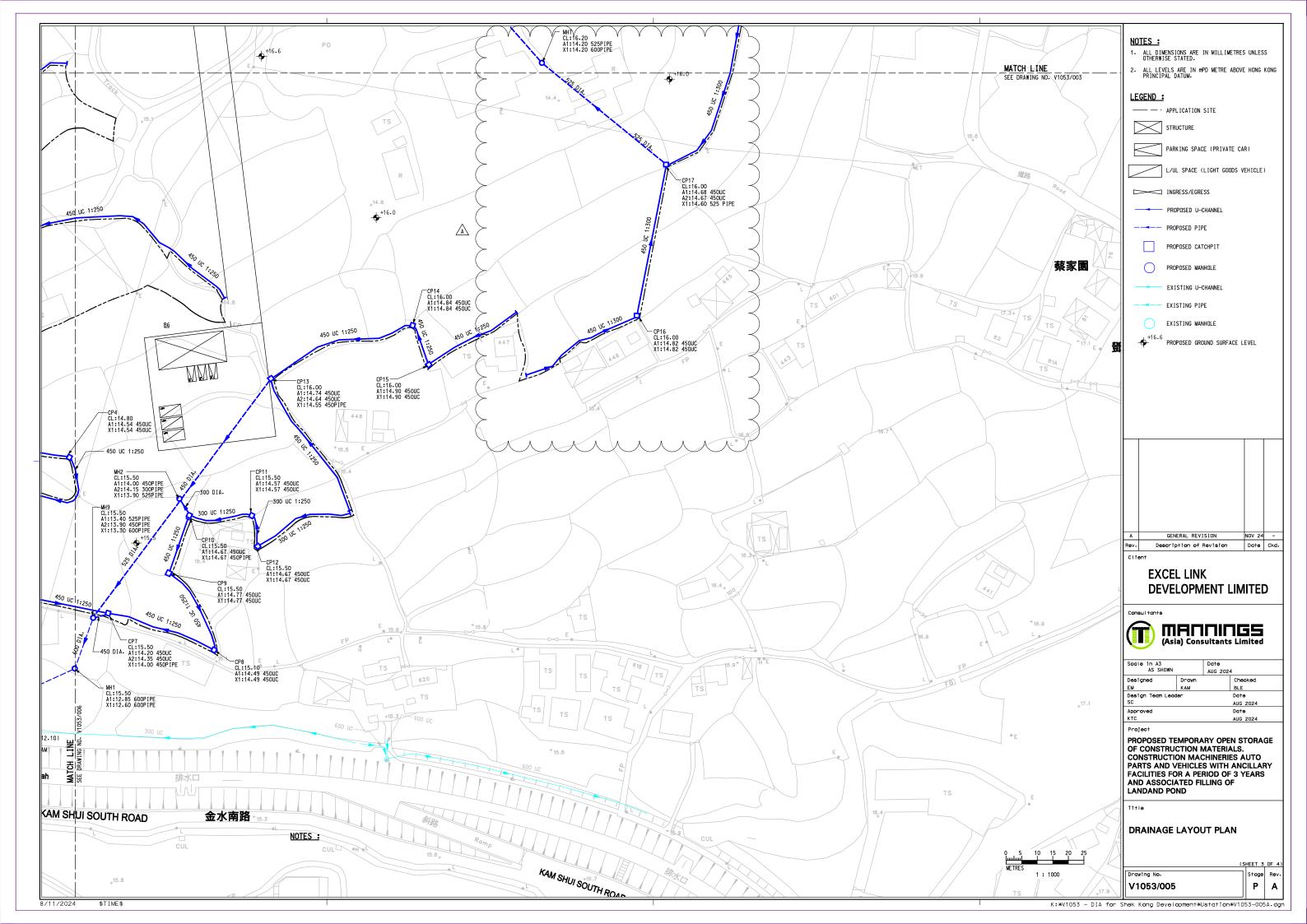
Drawings

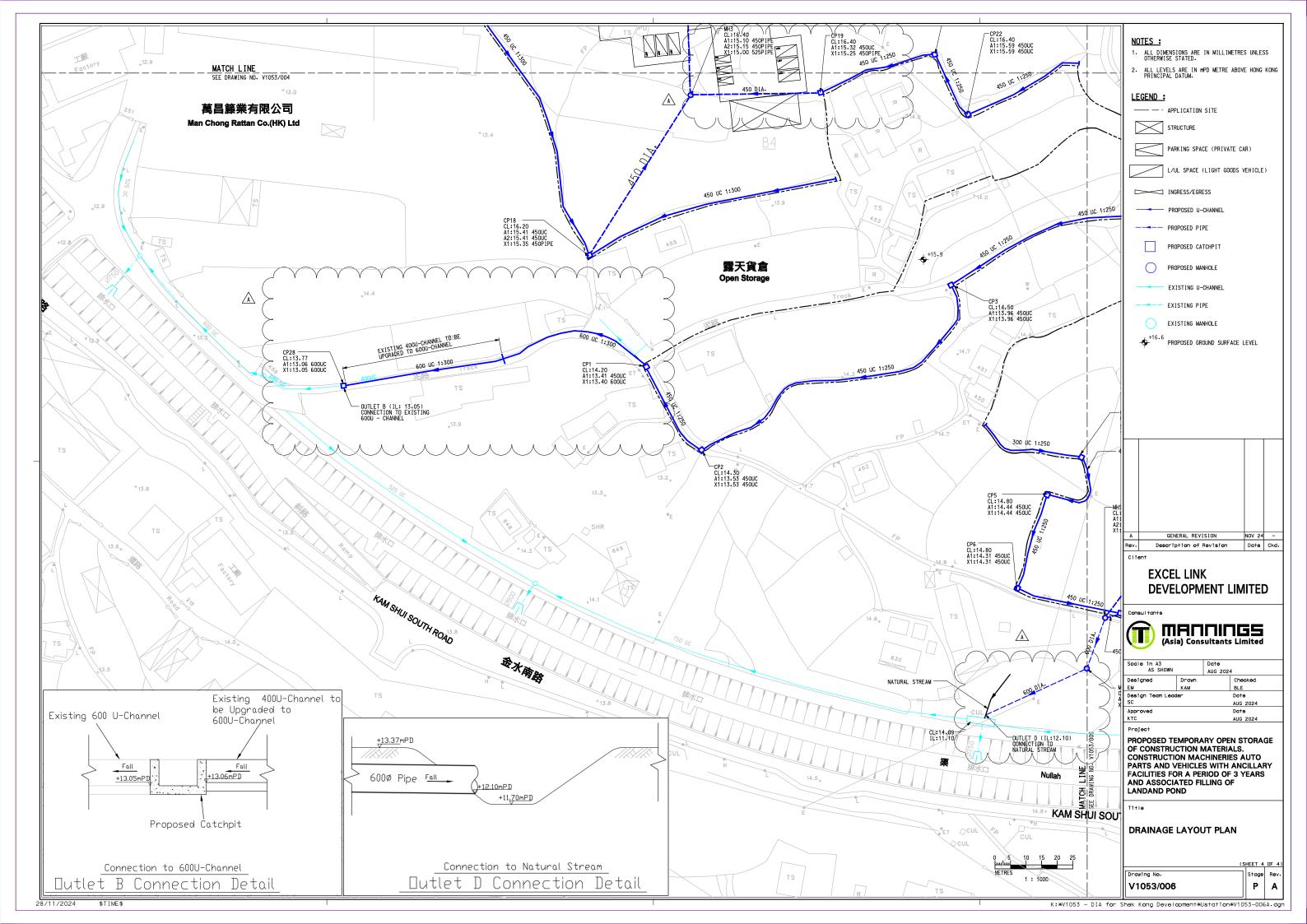


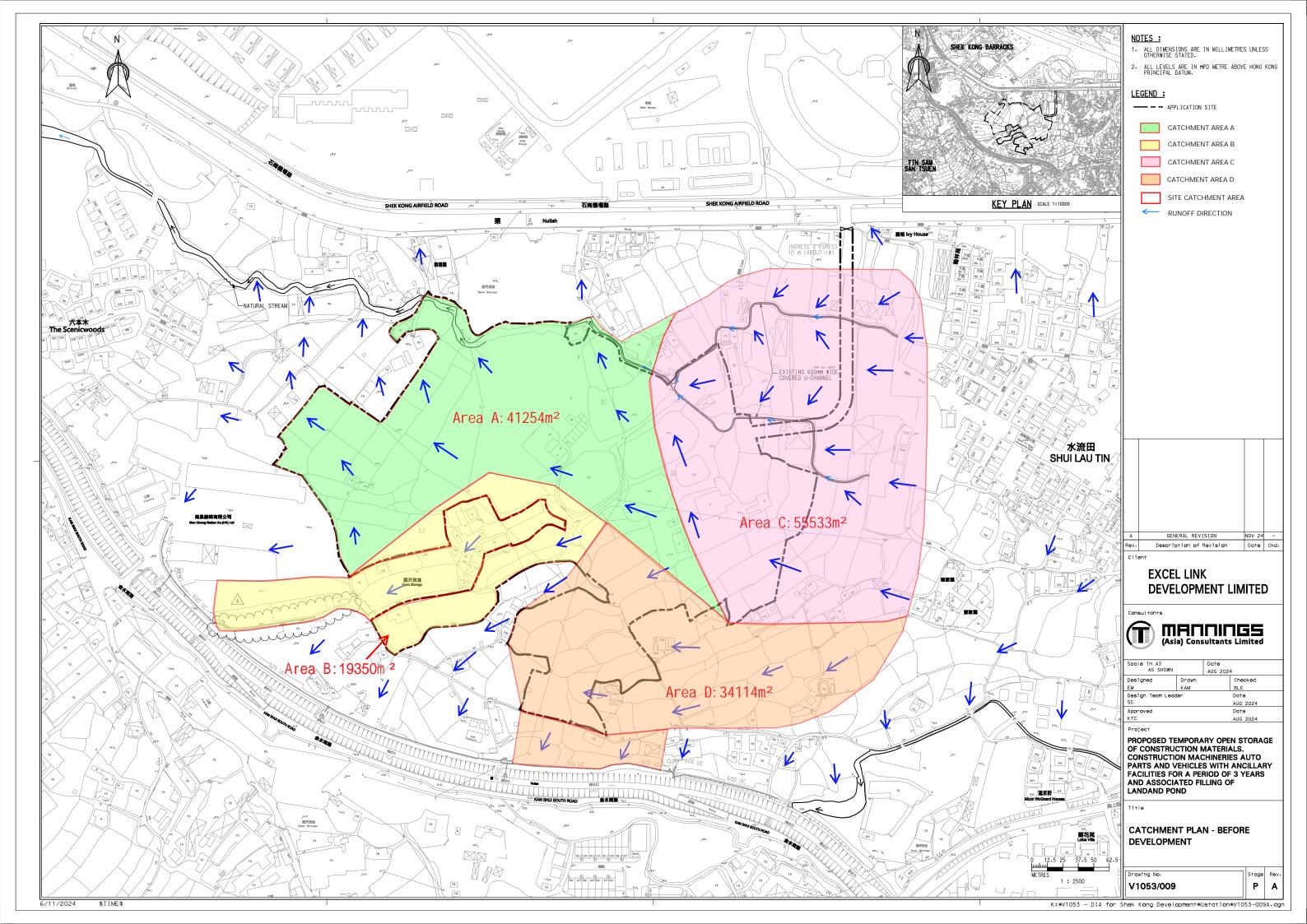


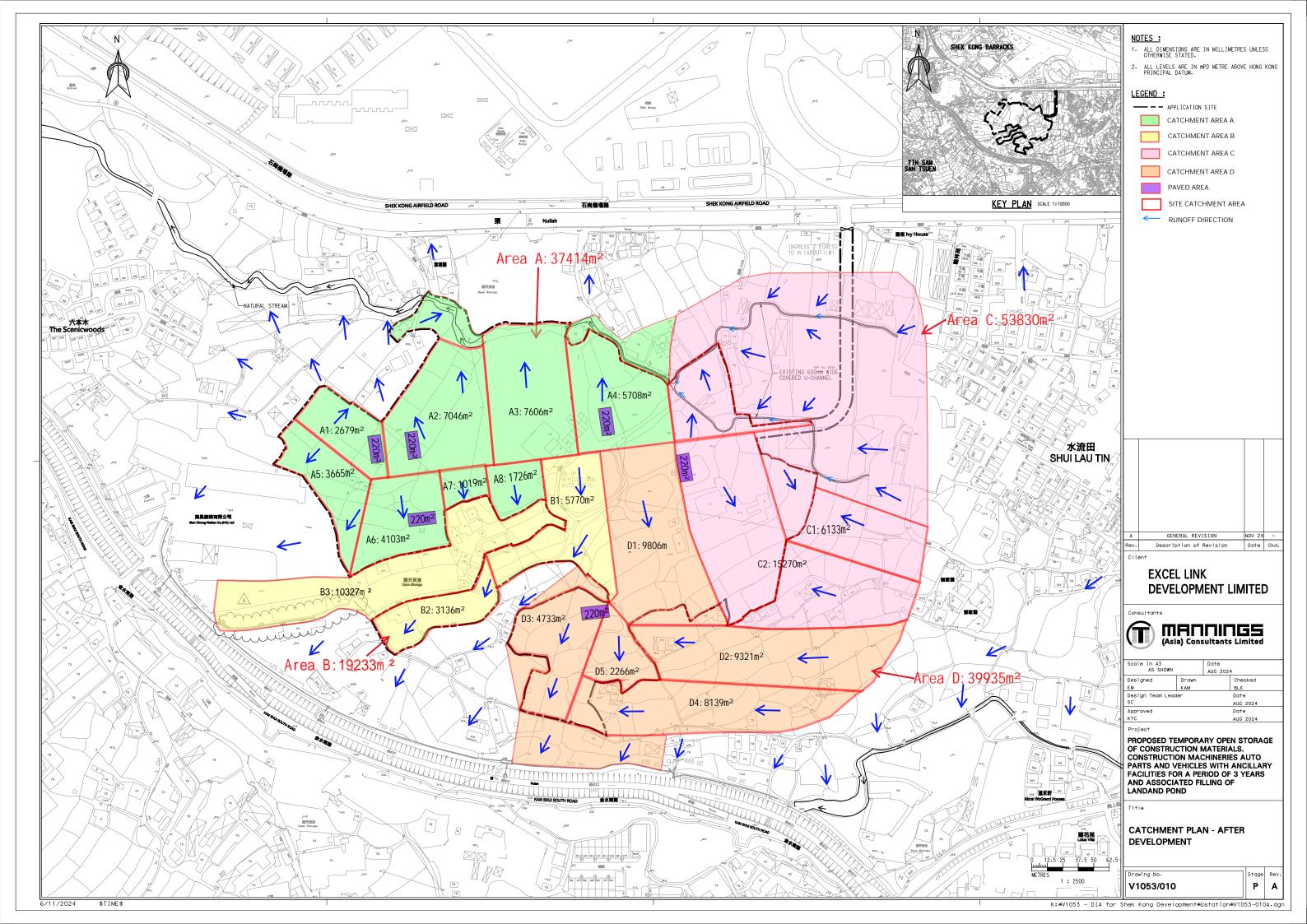


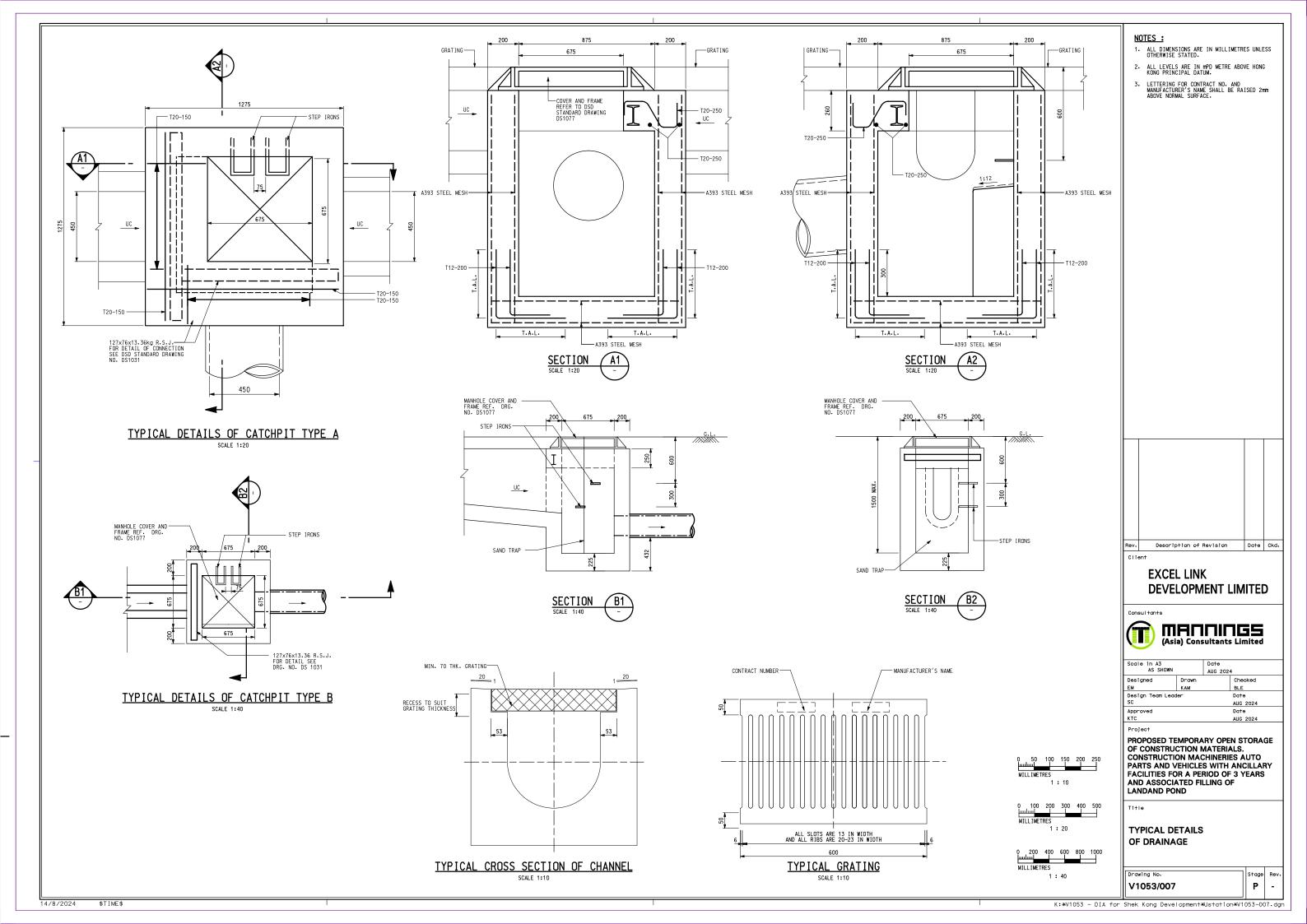














Appendix B

**Design Calculations** 

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM.

Qp = 0.278 C i A

Where Qp = peak runoff in m3/s I = rainfall intensity in mm/hr

A = catchment area in km2

The parameters and assumptions refer to section 3.

The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship. Use of Storm Constants for 10 years Return Periods of HKO Headquarters

 $i = a / (t_d + b)^c$ 

i =extreme mean intensity in mm/hr

t<sub>d</sub> =duration in minutes (td<240), and

a, b, c = storm constants given (note:a=485.0, b=3.11, c=0.397)

### **Before Development**

### **Runoff Estimation at Cacthment A**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)		to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design	riinott =	10 year Total runoff (m³/s)
41254	188	14.55	9.98	2.43	7.87	456	1.545	4.92	12.79	0.25	36027	161.75	0.40	0.45	0.70
41234	100	14.55	9.90	2.43	1.01	430	1.545	4.92	12.79	0.95	5227	101.73	0.22	0.25	0.70

### **Runoff Estimation at Cacthment B**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	I runott = I	10 year Total runoff (m³/s)
19350	200	15.1	14.1	0.50	12.38	80	1.588	0.84	13.22	0.25	12325	160.01	0.14	0.15	0.48
19550	200	13.1	14.1	0.50	12.30	00	1.500	0.04	13.22	0.95	7025	100.01	0.30	0.33	0.40

#### **Runoff Estimation at Cacthment C**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
55533	126	14.55	12.87	1.33	5.77	289	1.889	2.55	8.32	0.25	50423	184.36	0.65	0.72	0.99
55555	120	14.55	12.07	1.55	5.77	209	1.009	2.55	0.32	0.95	5110	104.30	0.25	0.28	0.99

#### **Runoff Estimation at Cacthment D**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100		Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
34114	172	16.3	15.1	0.70	9.42	70	2.620	0.52	9.94	0.25	32941	174.93	0.40	0.44	0.51
34114	172	10.5	13.1	0.70	9.42	, 0	2.020	0.52	3.34	0.95	1173	174.93	0.05	0.06	0.51

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM.

Qp = 0.278 C i A

Where Qp = peak runoff in m3/s

I = rainfall intensity in mm/hr

A = catchment area in km2

The parameters and assumptions refer to section 3.

The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship. Use of Storm Constants for 10 years Return Periods of HKO Headquarters

 $i = a / (t_d + b)^c$ 

i =extreme mean intensity in mm/hr

t<sub>d</sub> =duration in minutes (td<240), and

a, b, c = storm constants given (note:a=485.0, b=3.11, c=0.397)

#### After Development

### **Runoff Estimation at Cacthment A**

	Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)		to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
ſ											0.25	31307		0.35	0.39	
	37414	188	14.55	9.98	2.43	7.94	456	1.545	4.9	12.86	0.95	5227	161.44	0.22	0.25	0.68
											1.00	880	1	0.04	0.04	

### **Runoff Estimation at Cacthment B**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)		to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
19233	200	15.1	1/1	0.50	14.84	80	1.588	0.8	15.68	0.25	12208	151.35	0.13	0.14	0.45
19233	200	13.1	14.1	0.50	14.04	00	1.500	0.0	13.00	0.95	7025	101.00	0.28	0.31	0.45

### **Runoff Estimation at Cacthment C**

	Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)		to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
Ī											0.25	48500		0.62	0.69	
	53830	126	14.55	12.87	1.33	5.79	289	1.889	2.6	8.34	0.95	5110	184.25	0.25	0.28	0.98
											1.00	220		0.01	0.01	

#### **Runoff Estimation at Cacthment D**

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)		to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
										0.25	38542		0.47	0.52	
39935	172	16.3	15.1	0.70	9.27	70	2.620	0.5	9.79	0.95	1173	175.72	0.05	0.06	0.60
										1.00	220		0.01	0.01	

Mannings (A	Asia) Consultants Ltd.	Job No.		Sheet No.	Rev.
Calculation She	eet	Member / Loc	cation		
Job Tilte:	Proposed Temporary Open Storage of Construction Materials,	Drg. Ref.			
	Construction Machineries, Auto Parts and Vehicles with Ancillary				
	Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New				
	Territories	Made By		Date	

The drainage design is referring to DSD's SDM 2018 & Corrigendum No. 1/2022 and Corrigendum No. 1/2024

1 in 10 year design return period is taken.

The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship. Use of Storm Constants for 10 years Return Periods of HKO Headquarters

 $i = a / (t_d + b)^c$ 

i =extreme mean intensity in mm/hr

t<sub>d</sub> =duration in minutes (td<240), and a, b, c = storm constants given (note:a=485.0, b=3.11, c=0.397)

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM.

Qp = 0.278 C i A

Where Qp = peak runoff in m<sup>3</sup>/s I = rainfall intensity in mm/hr

A = catchment area in km²

#### Runoff Estimation

Location	Natural Catch. (m²)	Longest flow path (m)	Gradient (m per 100m)	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA (m³/s)	11.1%×10 year design runoff = 0.278CiA (m³/s)	Total Flow(m³/s)
A1	2679 220	30	0.007	5.37	0.90	6.27	0.25 1.00	2679 220	199.44	0.04 0.01	0.04 0.01	0.05
A2	9725 440	98	0.010	14.16	1.02	15.18	0.25 1.00	9725 440	153.00	0.10 0.02	0.11 0.02	0.14
А3	13314 220	93	0.010	15.73	0.81	16.55	0.25 1.00	13314 220	148.67	0.14 0.01	0.15 0.01	0.16
A4	5708 220	94	0.006	15.73	0.34	16.08	0.25 1.00	5708 220	150.11	0.06 0.01	0.07 0.01	0.08
A5	3665	37	0.005	6.69	0.70	7.39	0.25	3665	190.68	0.05	0.05	0.05
A6	4103 220	72	0.003	14.71	0.06	14.77	0.25 1.00	4103 220	154.36	0.04	0.05 0.01	0.06
A7	2745	30	0.030	7.16	0.70	7.86	0.25	2745	187.40	0.04	0.04	0.04
A8	1726	41	0.024	5.92	1.25	7.16	0.25	1726	192.37	0.02	0.03	0.03
B1	5770	80	0.011	11.94	1.11	13.05	0.25	5770	160.70	0.06	0.07	0.07
B2	8906	158	0.004	13.05	1.79	14.84	0.25	8906	154.12	0.10	0.11	0.11
В3	12208 7025	200	0.005	14.84	0.81	15.65	0.25 0.95	12208 7025	151.45	0.13 0.28	0.14 0.31	0.45
C1	6133	105	0.009	16.45	0.10	16.55	0.25	6133	148.66	0.06	0.07	0.07
C2	15270 220	134	0.010	18.69	0.98	19.67	0.25 1.00	15270 220	140.21	0.15 0.01	0.17 0.01	0.17
D1	9806	138	0.007	21.79	0.74	22.52	0.25	9806	133.80	0.09	0.10	0.10
D2	9321	190	0.008	29.02	0.06	29.08	0.25	9321	122.23	0.08	0.09	0.09
D3	4733 220	100	0.007	16.74	0.19	16.93	0.25 1.00	4733 220	147.53	0.05 0.01	0.05 0.01	0.06
D4	8139	193	0.005	32.50	0.74	33.24	0.25	8139	116.47	0.07	0.07	0.07
D5	2266	61	0.007	11.14	0.10	11.24	0.25	2266	168.46	0.03	0.03	0.03
Check Existing 600U-C	hannel (Οι	utlet B)										
Proposed Catchment Area B	12208 7025	200	0.005	15.65	0.87	16.52	0.25 0.95	12208 7025	148.75	0.13 0.28	0.14 0.31	0.45

#### Stormwater Drainage Design

M	anhole	Catchmo	ent Area			Nambari	Grad	lient, S <sub>f</sub>	B		Ti	Ti		40		10 year	11.1%×10	Total		Adjusted	Cove	r Level	Inver	t Level	utilization
From	То	Increment (m²)	Accu. (m²)	Catchment Served	Length (m)	Nominal Diameter (mm)	(%)	1 in	Roughness Coefficient (m)	Velocity (m/s)	Time of Flow (min)	Time of Conc. (min)	Rainfall Duration (min)	10 year Intensity (mm/hr)	Runoff Coeff.	Runoff (m³/s)	year Runoff (m³/s)	Total Flow (m <sup>3</sup> /s)	Capacity (m³/s)	Capacity > Total Flow ?	From (mPD)	To (mPD)	From (mPD)	To (mPD)	-
CP18	МНЗ	0	7768 220	A5 and A6	58	450	0.4	232.0	3.0	1.015	0.95	15.73	15.73	151.21	0.25 1.00	0.08	0.09	0.10	0.15	Yes	16.20	16.40	15.35	15.10	0.70
CP19	MH3	0	2745	A7 and A8	43	450	0.5	215.0	3.0	1.054	0.68	8.54	8.54	182.98	0.25	0.03	0.04	0.04	0.15	Yes	16.40	16.40	15.25	15.05	0.26
MH3	MH4	2745 0	10513 220	A5、A6、A7 and A8	58	525	0.5	193.3	3.0	1.231	0.79	16.51	16.51	148.77	0.25 1.00	0.11 0.01	0.12 0.01	0.13	0.24	Yes	16.40	16.00	15.00	14.70	0.55
MH4	MH5	0	10513 220	A5、A6、A7 and A8	45	525	0.7	150.0	3.0	1.398	0.54	17.05	17.05	147.19	0.25 1.00	0.11	0.12 0.01	0.13	0.27	Yes	16.00	15.90	14.70	14.40	0.48
MH5	MH6	0	10513 220	A5、A6、A7 and A8	47	525	0.9	117.5	3.0	1.580	0.50	17.54	17.54	145.78	0.25	0.11	0.12	0.13	0.31	Yes	15.90	15.59	14.40	14.00	0.42
CP23	MH6	0	9725	A1 and A2	14	450	1.8	56.0	3.0	2.068	0.11	15.29	15.29	152.63	0.25	0.10	0.11	0.14	0.30	Yes	15.59	15.90	14.40	14.15	0.46
	-	0	440 13314												1.00 0.25	0.02	0.02								
CP24	MH6	0	220	A3 and A4	13	450	1.5	65.0	3.0	1.919	0.11	16.19	16.19	149.76	1.00	0.01	0.01	0.16	0.27	Yes	15.59	15.90	14.20	14.00	0.60
MH6	Natural Stream	23039 660	33552 880	A1、A2、A3、 A4、A5、A6、 A7 and A8	5	600	10.0	10.0	3.0	6.072	0.82	18.37	18.37	143.53	0.25 1.00	0.33	0.37	0.41	1.63	Yes	15.90	15.80	13.70	13.20	0.25
		1		Ar and Ao	!			1					1			1	1					!			
CP17	MH7	0	21403 220	C1 and C2	49	600	0.6	163.3	3.0	1.462	0.56	19.67	19.67	140.21	0.25 1.00	0.21 0.01	0.23 0.01	0.24	0.37	Yes	16.00	16.20	14.65	14.35	0.65
MH7	MH8	0	21403 220	C1 and C2	58	600	0.6	165.7	3.0	1.451	0.67	20.34	20.34	138.62	0.25 1.00	0.21	0.23	0.24	0.37	Yes	16.20	16.20	14.35	14.00	0.65
MH8	Open Channel	0	21403 220	C1 and C2	52	600	0.5	208.0	3.0	1.295	0.67	21.01	21.01	137.08	0.25	0.20	0.23	0.24	0.33	Yes	16.20	16.20	14.00	13.75	0.72
CP13	MH2	0	19127	D1 and D2	47	525	1.2	85.5	3.0	1.853	0.42	29.50	29.50	121.60	0.25	0.16	0.18	0.18	0.36	Yes	16.00	15.50	14.55	14.00	0.50
CP10	MH2	0	2266	D5	5	300	2.0	50.0	3.0	1.670	0.05	11.29	11.29	168.22	0.25	0.03	0.03	0.03	0.11	Yes	15.50	15.50	14.25	14.15	
MH2	MH9	2266 0	21393 12872	D1、D2 and D5	46	525	1.1	92.0	3.0	1.785	0.43	30.06	30.06	120.78	0.25	0.18	0.20 0.12	0.20	0.35	Yes	15.50	15.50	13.90	13.40	0.57
CP7	MH9	0 12872	220 34265	D3 and D4	4	450	2.5	40.0	3.0	2.447	0.03	30.09	30.09	120.74	1.00	0.01	0.01	0.13	0.35	Yes	15.50	15.50	14.00	13.90	0.37
MH9	MH1	0	220	D4 and D5	20	600	2.3	44.4	3.0	2.805	0.12	30.21	30.21	120.57	0.25 1.00	0.01	0.32 0.01	0.33	0.71	Yes	15.50	15.90	13.30	12.85	0.46
MH1	Natural Stream	0	34265 220	D1、D2、D3、 D4 and D5	36	600	1.4	72.0	3.0	2.203	0.27	30.48	30.48	120.18	0.25 1.00	0.29	0.32	0.33	0.56	Yes	15.50	13.87	12.60	12.10	0.58
Check Existing Pi	ре																								
Proposed C	atchment Area D	4277 0 0	38542 1173 220	D	10	750	1.0	100.0	3.0	2.162	0.08	9.35	9.35	178.18	0.25 0.95 1.00	0.48 0.06 0.01	0.53 0.06 0.01	0.60	0.86	Yes	14.09	12.17	11.10	11.00	0.70

Mean Velocity is calculated by Colebrook- White equation

Where:

y = Mean Velocity (m/s)
R = Hydraulic Diameter (m)
Ks = Surface Roughness (m)
V = Kinematic viscosity (kg/ms)
Sf = Slope of Hydraulic Gradient
g = Gravity (m/s2)

The Roughness Coefficient Ks is assumed to be 3 for concrete. Peak Runoff is estimated using rational method according to SDM.

The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration –Frequency (IDF) Relationship. Use of Storm Constants for 10 years Return Periods of HKO Headquarters i = a / (t<sub>x</sub>+b)<sup>c</sup>

 $\overline{V} = -\sqrt{32gRS_f} \log \left[ \frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right]$ 

i =extreme mean intensity in mm/hr t<sub>d</sub> =duration in minutes (td<240), and

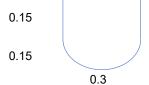
a, b, c = storm constants given (note:a=485.0, b=3.11, c=0.397)

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Calculation	on Sheet	Member / Location		
Job Tilte:		Drg. Ref.		
	Machineries, Auto Parts and Vehicles with Ancillary Facilities for a Period of 3			
	Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New			
	Territories	Made By	Date	

### Checking of Capacity (300UC)

# Input Data





#### Flow capacity, Q

$$Q = \frac{A x r^{2/3} x s^{1/2}}{n}$$

where A = cross sectional area of flow ( $m^2$ ) = 0.080343  $m^2$ r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

n = Manning coefficient of roughness

#### Hydraulic radius

$$r$$
 =  $\frac{A}{P}$   
 $p$  = wetted perimeter (m) = 0.77 m  
 $r$  = 0.10  $m$ 

### Slope

$$s = 0.004 \text{ m/m}$$

### Manning coefficient of roughness

Q = 
$$0.08 \text{ m}^3/\text{s}$$
 > Design runoff, OK!  
V = Q/A =  $1.00 \text{ m/s}$ 

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## Checking of Capacity (450UC)

#### Input Data



0.225

0.45

### Flow capacity, Q

$$Q = \frac{A \times r^{2/3} \times s^{1/2}}{r}$$

where A = cross sectional area of flow (m<sup>2</sup>) = 0.180772 m<sup>2</sup> r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

*n* = Manning coefficient of roughness

### Hydraulic radius

p = wetted perimeter (m) = 1.16 m

r = 0.16 m

#### Slope

$$s = 0.004 \text{ m/m}$$

#### Manning coefficient of roughness

$$n = 0.014$$

Q = 
$$0.24 \text{ m}^3/\text{s}$$
 > Design runoff, OK!

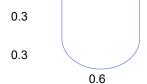
$$V = Q/A = 1.31 \text{ m/s}$$

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Job Tilte:		Drg. Ref.					
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	in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New						
	Territories	Made Bv	Date				

# Checking of Capacity (600UC)

#### Input Data





### Flow capacity, Q

$$Q = \frac{A x r^{2/3} x s^{1/2}}{n}$$

where A = cross sectional area of flow (m²) = 0.321372 m²
r = hydraulic radius (m)
s = slope of the water surface or the linear hydraulic head loss (m/m)

n = Manning coefficient of roughness

### Hydraulic radius

$$r = \frac{A}{P}$$
 $p = \text{wetted perimeter (m)} = 1.54 \text{ m}$ 
 $r = 0.21 \text{ m}$ 

#### Slope

$$s = 0.004 \text{ m/m}$$

#### Manning coefficient of roughness

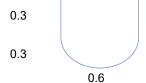
Q = 
$$0.51 \text{ m}^3/\text{s}$$
 > Design runoff, OK!  
V = Q/A =  $1.59 \text{ m/s}$ 

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Calculation	on Sheet	Member / Location				
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	Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories	Made Bv	Date			

### Checking of Capacity of Existing 600U-Channel of Outlet B

#### Input Data





### Flow capacity, Q

$$Q = \frac{A \times r^{2/3} \times s^{1/2}}{n}$$

where A = cross sectional area of flow (m<sup>2</sup>) = 0.321372 m<sup>2</sup> r = hydraulic radius (m) s = slope of the water surface or the linear hydraulic head loss (m/m)

s = slope of the water surface or the linear hydraulic head loss (m/m)
 n = Manning coefficient of roughness

### Hydraulic radius

p = wetted perimeter (m) = 1.54 m

r = 0.21 m

# Slope

$$s = 0.004 \text{ m/m}$$

#### Manning coefficient of roughness

Q = 
$$0.51 \text{ m}^3/\text{s}$$
 > Design runoff, OK!

$$V = Q/A = 1.59 \text{ m/s}$$

Rational method is used for calculation of the peak runoff. The formula is extracted from Section 7.5.2 (a) of SDM.

Qp = 0.278 C i A

Where Qp = peak runoff in m3/s

I = rainfall intensity in mm/hr

A = catchment area in km2

The parameters and assumptions refer to section 3.

The rainfall intensity is extracted from the Section 4.3.2 of SDM which is to estimate the Intensity-Duration – Frequency (IDF) Relationship.

Use of Storm Constants for 10 years Return Periods of HKO Headquarters

 $i = a / (t_d + b)^c$ 

i =extreme mean intensity in mm/hr

t<sub>d</sub> =duration in minutes (td<240), and

a, b, c = storm constants given (note:a=485.0, b=3.11, c=0.397)

Runoff Estimation at Downstream Natural Stream of Outlet A Before Development

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	(per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
96787	188	14.55	9.98	2.43	7.22	509	1.545	5	12.7	0.25	86450	162.04	0.97	1.08	1.57
30101	100	14.55	9.90	2.40	1.22	309	1.040	3	12.1	0.95	10337	102.04	0.44	0.49	1.57

Runoff Estimation at Downstream Open Channel of Outlet C Before Development

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)					
55533	126	14.55	12.87	1.33	5.77	340	1.889	3	8.8	0.25	50423	181.56	0.64	0.71	0.98					
55555	120	17.55	12.01	1.55	5.77	540	1.509	3	0.0	0.0	0.0	0.0	0.0	0.0	0.95	5110	101.00	0.25	0.27	0.30

Runoff Estimation at Downstream Natural Stream of Outlet A After Development

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	Gradient (per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
										0.25	79807		0.90	1.00	
91244	188	14.55	9.98	2.43	7.27	509	1.545	5	12.8	0.95	10337	161.86	0.44	0.49	1.54
										1.00	1100		0.05	0.05	

Runoff Estimation at Downstream Open Channel of Outlet C After Development

Natural Catch. (m²)	Longest flow path (m)	Highest (mPD)	Lowest (mPD)	(per 100m) = (h <sub>1</sub> - h <sub>2</sub> )/L x 100	to (min) = 0.14465L/ (H <sup>0.2</sup> A <sup>0.1</sup> )	Length of Nullah L (m)	flow vel. (m³/s)	t <sub>f</sub> = L/v (min)	tc = to + t <sub>f</sub> (min)	Runoff coeff.	Total Catch. Area (m²)	10 year Intensity (mm/hr)	10 year design runoff = 0.278CiA	11.1%×10 year design runoff = 0.278CiA	10 year Total runoff (m³/s)
53830	126	14.55	12.87	1.33	5.79	340	1.889	3	8.8	0.25 0.95	48500 5110	181.45	0.61 0.24	0.68 0.27	0.96
		,,,,					,,,,	,		1.00	220		0.01	0.01	

Mannin	gs (Asia) Consultants Ltd.	Job No.	Sheet NoR	₹ev.			
Calculatio	n Sheet	Member / Location	Member / Location				
Job Tilte:	Proposed Temporary Open Storage of Construction Materials, Construction Machineries, Auto Parts and Vehicles with Ancillary	Drg. Ref.					
	Facilities for a Period of 3 Years and Associated Filling of Land and						
	Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining	Made By	Date				

1.2

### Checking of Capacity of Natural Stream of Outlet A

#### Input Data

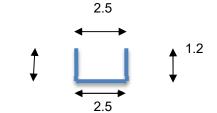
Ditch width (Bottom) = 2.5 m

Ditch width (top) = 2.5 m

Ditch height (Right) = 1.2 m

Ditch height (Left) = 1.2 m

Max. Design flow = 1.54 m³/s



 $\,m^2\,$ 

# Flow capacity, Q

$$Q = \frac{A x r^{2/3} x s^{1/2}}{n}$$

where A = cross sectional area of flow (m<sup>2</sup>) =

r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

*n* = Manning coefficient of roughness

### Hydraulic radius

p = wetted perimeter (m) = 4.90 m

r = 0.61 m

#### Slope

$$s = 0.01 \text{ m/m}$$

### Manning coefficient of roughness

$$n = 0.033$$

#### Therefore,

Q =  $4.6349 \text{ m}^3/\text{s}$  > Design flow, OK!

V = 1.545 m/s

Mannin	gs (Asia) Consultants Ltd.	Job No. Sheet No.				
Calculatio	n Sheet	Member / Location				
Job Tilte:	Proposed Temporary Open Storage of Construction Materials, Construction Machineries, Auto Parts and Vehicles with Ancillary	Drg. Ref.				
	Facilities for a Period of 3 Years and Associated Filling of Land and Pond in "Agriculture" Zone, Various Lots in D.D. 106 and Adjoining	Made By	Date			

## Checking of Capacity of Open Channel of Outlet C

## Input Data

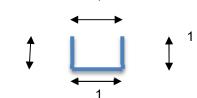
Ditch width (Bottom) = 1 m

Ditch width (top) = 1 m

Ditch height (Right) = 1 m

Ditch height (Left) = 1 m

Max. Design flow = 0.96 m<sup>3</sup>/s



# Flow capacity, Q

$$Q = \frac{A x r^{2/3} x s^{1/2}}{n}$$

where A = cross sectional area of flow  $(m^2)$  = 1  $m^2$ 

r = hydraulic radius (m)

s = slope of the water surface or the linear hydraulic head loss (m/m)

*n* = Manning coefficient of roughness

## Hydraulic radius

p = wetted perimeter (m) = 3.00 m

r = 0.33 m

### Slope

$$s = 0.01 \text{ m/m}$$

### Manning coefficient of roughness

$$n = 0.018$$

#### Therefore,

Q =  $1.8886 \text{ m}^3/\text{s}$  > Design flow, OK!

V = 1.8886 m/s



Appendix C

Site Photos

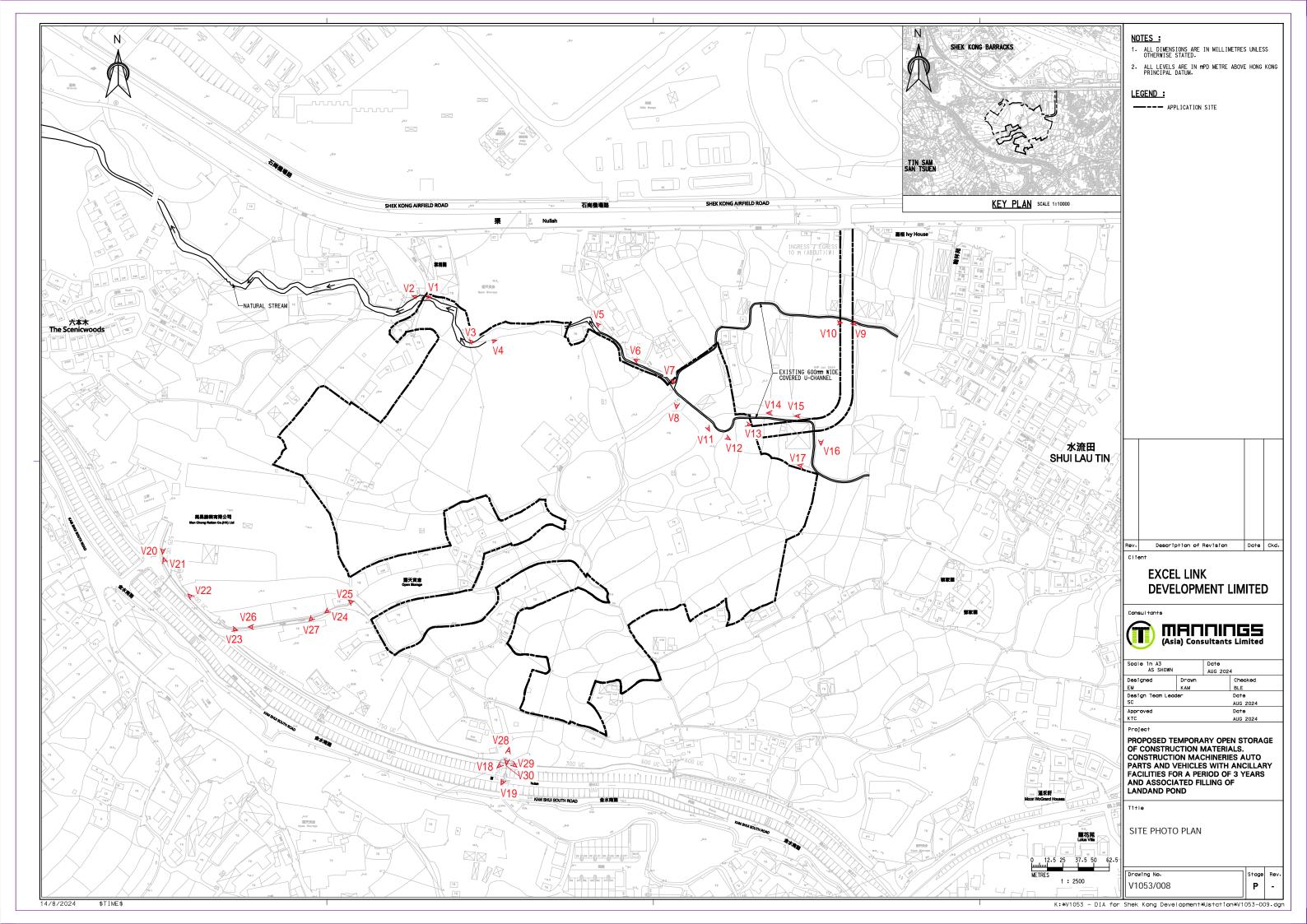




Photo V1



Photo V2





Photo V3



Photo V4



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Photo V5



Photo V6





Photo V7



Photo V8







Photo V10









Photo V12





Photo V13



Photo V14





Photo V15



Photo V16





Photo V17



Photo V18





Photo V19



Photo V20







Photo V22





Photo V23



Photo V24





Photo V25



Photo V26





Photo V27



Photo V28





Photo V29



Photo V30

