Proposed Temporary Warehouse and Open Storage of Construction Materials, Machinery and Vehicles and Associated Filling of Land for a Period of 3 Years in "Agriculture" Zone, Various Lots in D.D. 112 and Adjoining Government Land, Shek Kong, Yuen Long, New Territories

Appendix II

Drainage Impact Assessment



Drainage Impact Assessment

March 2025 r1

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Drainage Impact Assessment

Table of Contents

1	Introduction	1
1.	1 Background	1
1	2 Application Site	1
2	Development Proposal	2
2.	1 The Proposed Development	2
3	Assessment Criteria	2
4	Proposed Drainage System	5
4.	1. Proposed Storage Tank	Error! Bookmark not defined.
4.	2. Proposed Channels	5
5	Conclusion	5
Tabl	st of Table le 1 - Key Development Parameters le 2– Design Return Periods under SDM	2 2
List	t of Figure	
Figu	re 1 – Site Location Plan	
Figu	re 2 – Existing Drainage Plan	
Figu	re 3-1 – Proposed Drainage System	
Figu	re 3-2 – Drainage Schedule	
Figu	re 4-1 – Existing Catchment Plan	
Figu	re 4-2 – Proposed Catchment Plan	
Figu	re 5 – Sections	
List	t of Appendix	
Appe	endix A – Design Calculation	
Арре	endix B - Development Layout Plan	

Appendix C – Reference Drawings

Drainage Impact Assessment

1 Introduction

1.1 Background

- 1.1.1 The applicant seeks planning permission from the Town Planning Board (the Board) under Section (S.) 16 of the Town Planning Ordinance (Cap. 131) (the Ordinance) to use Various Lots in D.D. 112 and Adjoining Government Land (GL), Shek Kong, Yuen Long (the Site) for 'Proposed Temporary Warehouse and open storage of construction materials, machinery and vehicles for a Period of 3 Years and Associated Filling of Land'.
- 1.1.2 This report aims to support the development in drainage aspect.

1.2 Application Site

- 1.2.1 The application site is situated beside Kam Sheung Road at Shek Kong. It has an area of approx. 18,031 m². The site location is shown in **Figure 1**.
- 1.2.2 The existing site is mainly cover with vegetation with level various from approx. +22 to + 23.9mPD. The proposed site intent to pave the whole site for formation of structures, parking, L/UL spaces and circulation.
- 1.2.3 The surrounding site levels are mainly higher at the east (approx. +24mPD) and lower (approx. +22mPD) at the west.
- 1.2.4 There are existing 12m with rectangular nullah at the north of the proposed site. Figure 2 indicate the existing drainage system of the area. The existing catchment plan is shown in Figure 4-1.

1

Drainage Impact Assessment

2 Development Proposal

2.1 The Proposed Development

- 2.1.1 The total site area is approximately 18,031 m². The existing site area is mainly coved by vegetation.
- 2.1.2 After the development the site would be fully paved. The catchment plan is shown in Figure 4-2.

Proposed Development	
Total Site Area (m²)	18,031
Paved Area after Development (m ²)	18,031

Table 1 - Site Development Area

3 Assessment Criteria

3.1.1 The Recommended Design Return Period based on Flood Level from SDM (Table 10) is adopted for this report. The recommendation is summarized in **Table 2** below.

Description	Design Return Periods
Intensively Used Agricultural Land	2 – 5 Years
Village Drainage Including Internal Drainage System under a polder Scheme	10 Years
Main Rural Catchment Drainage Channels	50 Years
Urban Drainage Trunk System	200 Years
Urban Drainage Branch System	50 Years

Table 2- Design Return Periods under SDM

3.1.2 The proposed drainage system intended to collect runoff from internal site and external catchment. 1 in 50 years return period is adopted for the drainage design.

Drainage Impact Assessment

- 3.1.3 Stormwater drainage design will be carried out in accordance with the criteria set out in the Stormwater Drainage Manual published by DSD. The proposed design criteria to be adopted for design of this stormwater drainage system and factors which have been considered are summarised below.
 - 1. Intensity-Duration-Frequency Relationship The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Zone. Therefore, for 1 in 50 years return period, the following values are adopted.

(Corrigendum_No.1_2024)

- 11.1% rainfall increase as per Table 28 of SDM Corrigendum No. 1/2022 to be included in the rainfall intensity estimation.
- 2. The peak runoff is calculated by the Rational Method i.e. $Q_p = 0.278CiA$

where
$$Q_p$$
 = peak runoff in m³/s
 C = runoff coefficient (dimensionless)
 i = rainfall intensity in mm/hr
 A = catchment area in km²

- 3. The run-off coefficient (C) of surface runoff are taken as follows:
 - Paved Area: C = 0.95
 Unpaved Area: C = 0.35

Mar-25 3

Drainage Impact Assessment

4. Manning's Equation is used for calculation of velocity of flow inside the channels:

Manning's Equation:
$$v = \frac{R^{\frac{1}{6}}}{n} R^{\frac{1}{2}} S_f^{\frac{1}{2}}$$

Where,

V = velocity of the pipe flow (m/s)

S_f = hydraulic gradient

n = manning's coefficient

R = hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes:

Colebrook-White Equation:
$$\underline{v} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}}\right)$$

where,

V = velocity of the pipe flow (m/s)

S_f = hydraulic gradient k_f = roughness value (m)

v = kinematics viscosity of fluid

D = pipe diameter (m) R = hydraulic radius (m)

Mar-25 4

Drainage Impact Assessment

4 Proposed Drainage System

4.1. Proposed Channels

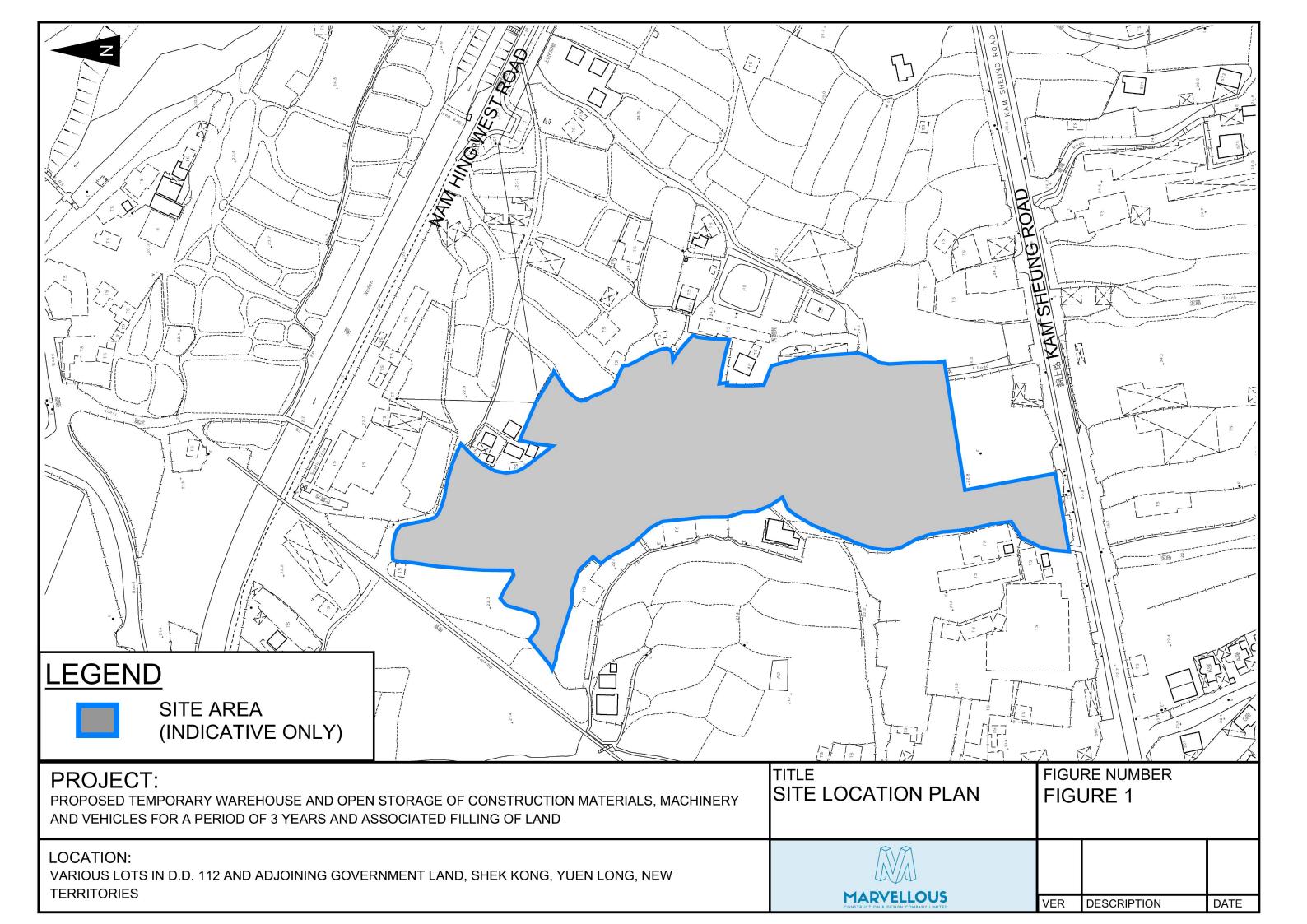
- 4.1.1 Proposed channels are designed for collection of runoff for internal and external catchment. They are proposed to carry the runoff to the north and eventually discharge to existing northern rectangular nullah. The discharge flow would only occupy 1.6% capacity of the existing nullah. The capacity checking is shown in **Appendix A**. No unacceptable drainage impact is anticipated.
- 4.1.2 The design calculations of proposed uchannel and pipe are shown in **Appendix A**.
- 4.1.3 The alignment, size, gradient and details of the proposed drains are shown in **Figure 3-1** and drainage schedule in **Figure 3-2**. The catchment plan is shown in **Figure 4-2**.
- 4.1.5 Reference Drawings are shown in **Appendix C** for reference.

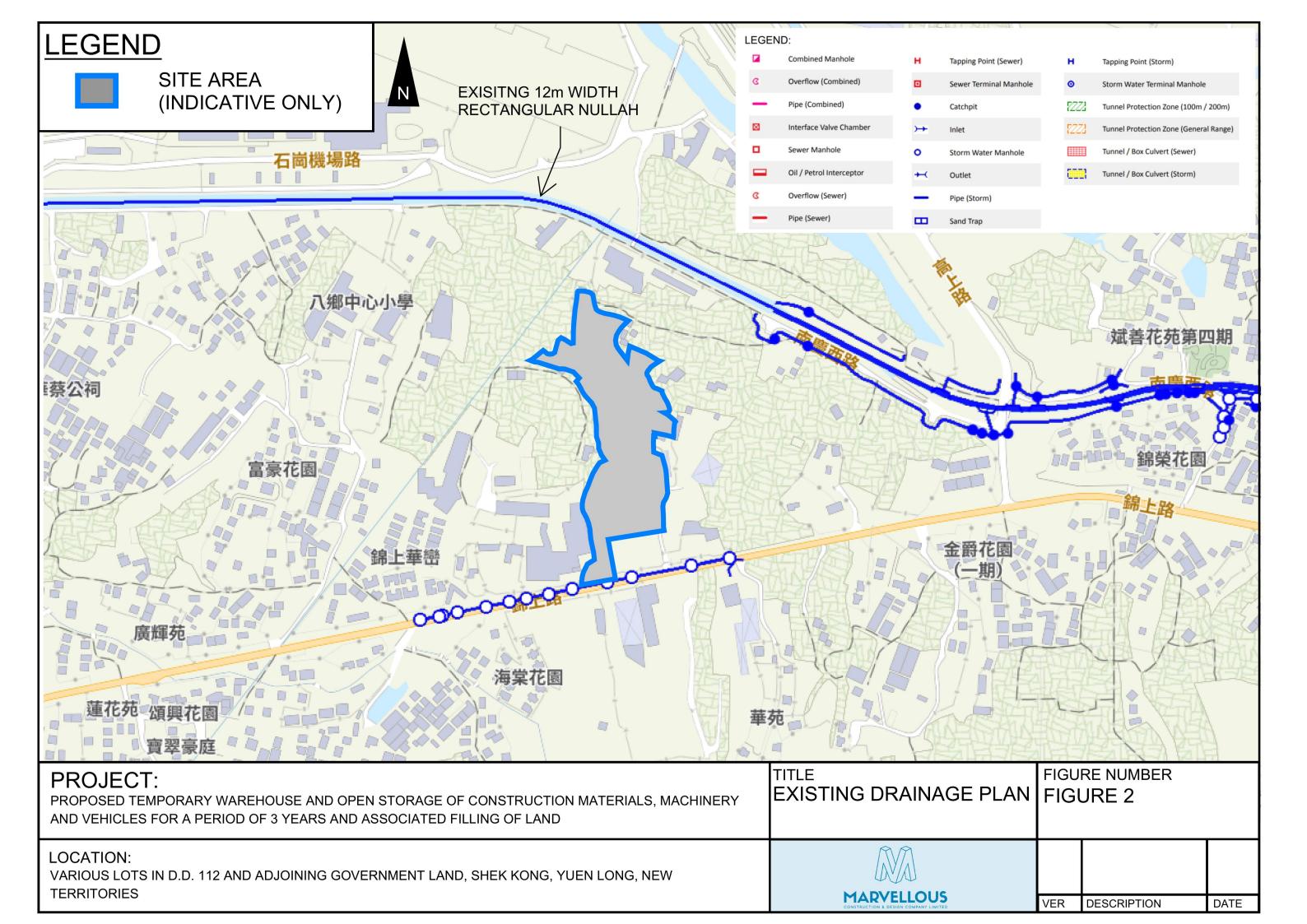
5 Conclusion

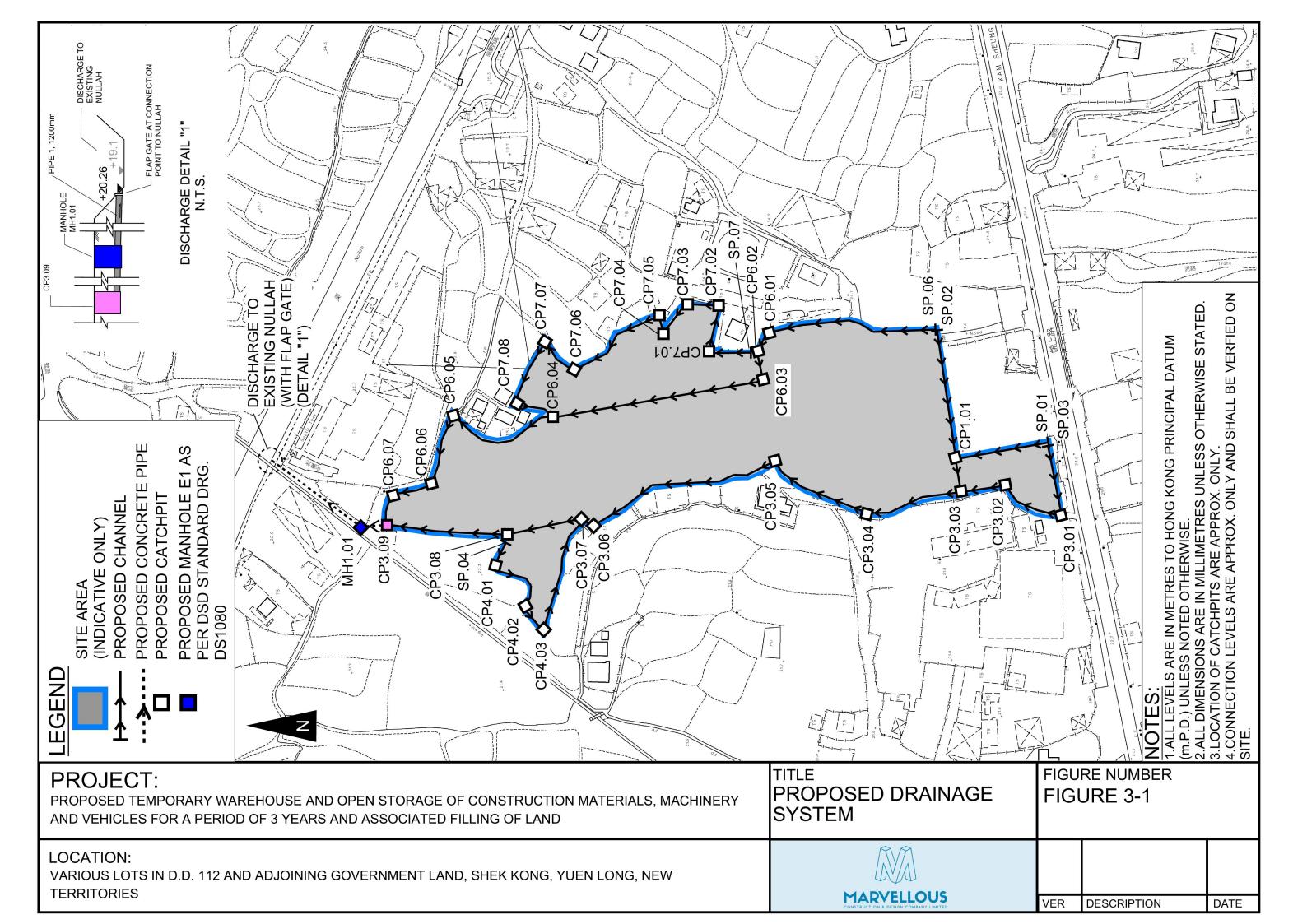
- 5.1.1 Drainage review has been conducted for the Proposed Development. U channels and pipe are proposed to mitigate the drainage impact to the nearby area.
- 5.1.2 With implementation of the above drainage system, no unacceptable drainage impact is anticipated.

- End of Text -

FIGURES







			DF	RINAC	E SCI	HEDU	LE			
									U/S	
				Size	Gradient				MH/PIT	
US MH/PIT	DS MH/PIT	US GL	DS GL	mm	1 in	Type	USIL	DSIL	TYPE#	Remark
SP01	CP1.01	23.9	22.8	525	200	UC	23.38	22.275	SP	#SP: Start Poin
CP1.01	CP3.03	22.8	22.4	825	200	UC	21.98	21.58	CP	
SP02	CP1.01	23.8	22.8	825	200	UC	22.98	21.98	SP	
SP03	CP3.01	23.9	23.4	375	200	UC	23.53	23.03	SP	
CP3.01	CP3.02	23.4	22.4	375	200	UC	23.03	22.03	CP	
CP3.02	CP3.03	22.4	22.4	375	200	UC	22.03	21.93	CP	
CP3.03	CP3.04	22.4	22.4	825	250	UC	21.58	21.40	CP	
CP3.04	CP3.05	22.4	22.5	825	250	UC	21.40	21.21	CP	
CP3.05	CP3.06	22.5	22.1	900	250	UC	21.40	20.86	CP	
CP3.05	CP3.00	22.1	22.1	900	250	UC	20.86	20.84	CP	
CP3.06	CP3.07	22.1	22.1	1050	250	UC	20.84	20.84	CP	<u> </u>
CP3.07	CP3.08	22.1	22.4	1050	250	UC	20.84	20.71	CP	+
CP3.08	MH1.01	22.4	22.6	1200	250	PIPE	20.71	20.49	CP	+
MH1.01	Existing Nullah	22.6	22.3	1200	250	PIPE	20.45	20.45	E1	
SP04	CP4.01	22.4	22.3	375	200	UC	22.03	21.93	SP	
CP4.01	CP4.02	22.3	22.2	375	200	UC	21.93	21.78	CP	
CP4.02	CP4.03	22.2	21.8	375	200	UC	21.78	21.43	CP	
CP4.03	CP3.07	21.8	22.1	375	200	UC	21.43	21.19	CP	
SP06	CP6.01	23.8	23.5	825	300	UC	22.98	22.68	SP	
CP6.01	CP6.02	23.5	23.5	825	300	UC	22.68	22.64	CP	
CP6.02	CP6.03	23.5	23.3	825	300	UC	22.64	22.48	CP	
CP6.03	CP6.04	23.3	23.2	900	200	UC	22.40	21.93	CP	
CP6.04	CP6.05	23.2	23.1	1050	300	UC	21.93	21.77	CP	
CP6.05	CP6.06	23.1	22.9	1050	300	UC	21.77	21.66	CP	
CP6.06	CP6.07	22.9	22.7	1050	300	UC	21.66	21.60	CP	
CP6.07	CP3.09	22.7	22.6	1050	300	UC	21.60	21.55	CP	
SP07	CP7.01	23.5	23.5	525	200	UC	22.98	22.88	SP	-
CP7.01	CP7.02	23.5	24.0	525	200	UC	22.88	22.77	CP	-
CP7.02	CP7.03	24.0	23.5	525	200	UC	22.77	22.70	CP	
CP7.03	CP7.04	23.5	23.9	600	300	UC	22.70	22.64	CP	
CP7.04	CP7.05	23.9	23.9	600	300	UC	22.64	22.62	CP	
CP7.05	CP7.06	23.9	23.5	600	300	UC	22.62	22.47	CP	
CP7.06	CP7.07	23.5	23.5	600	300	UC	22.47	22.41	CP	
CP7.07	CP7.08	23.5	23.5	600	300	UC	22.41	22.30	CP	
CP7.08	CP6.04	23.5	23.2	600	300	UC	22.30	22.24	СР	

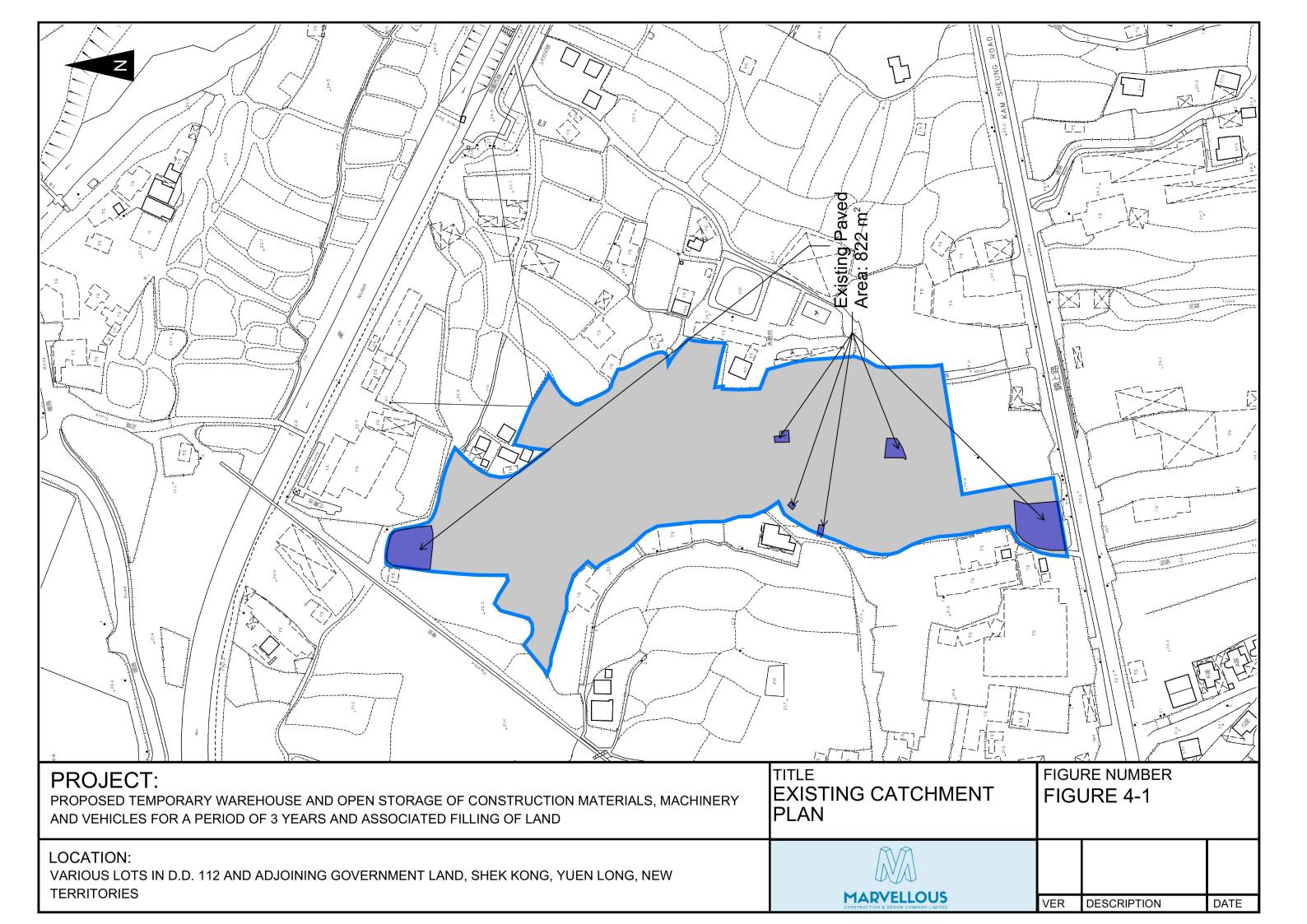
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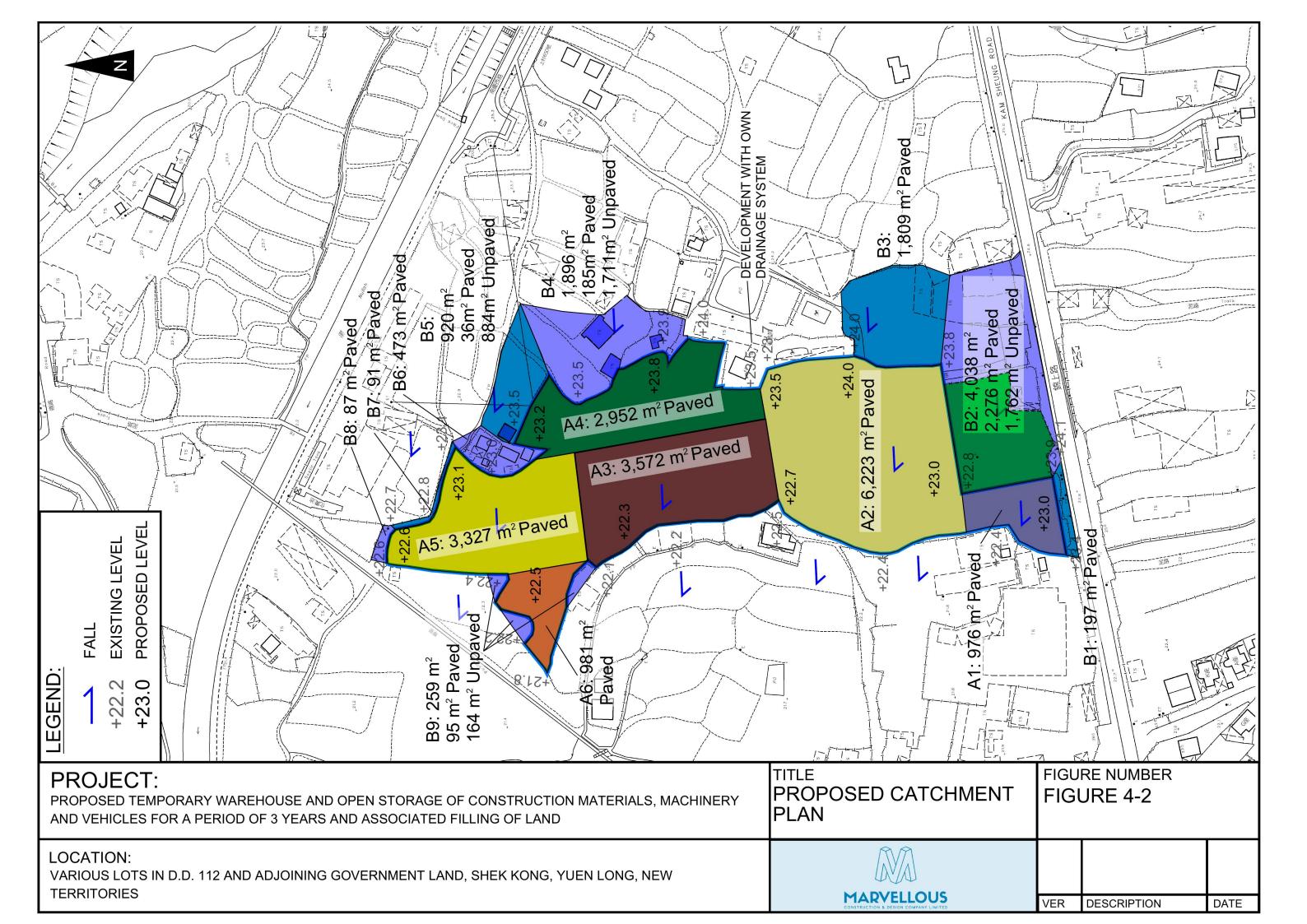
1.ALL LEVELS ARE IN METRES TO HONG KONG PRINCIPAL DATUM (m.P.D.) UNLESS NOTED OTHERWISE.

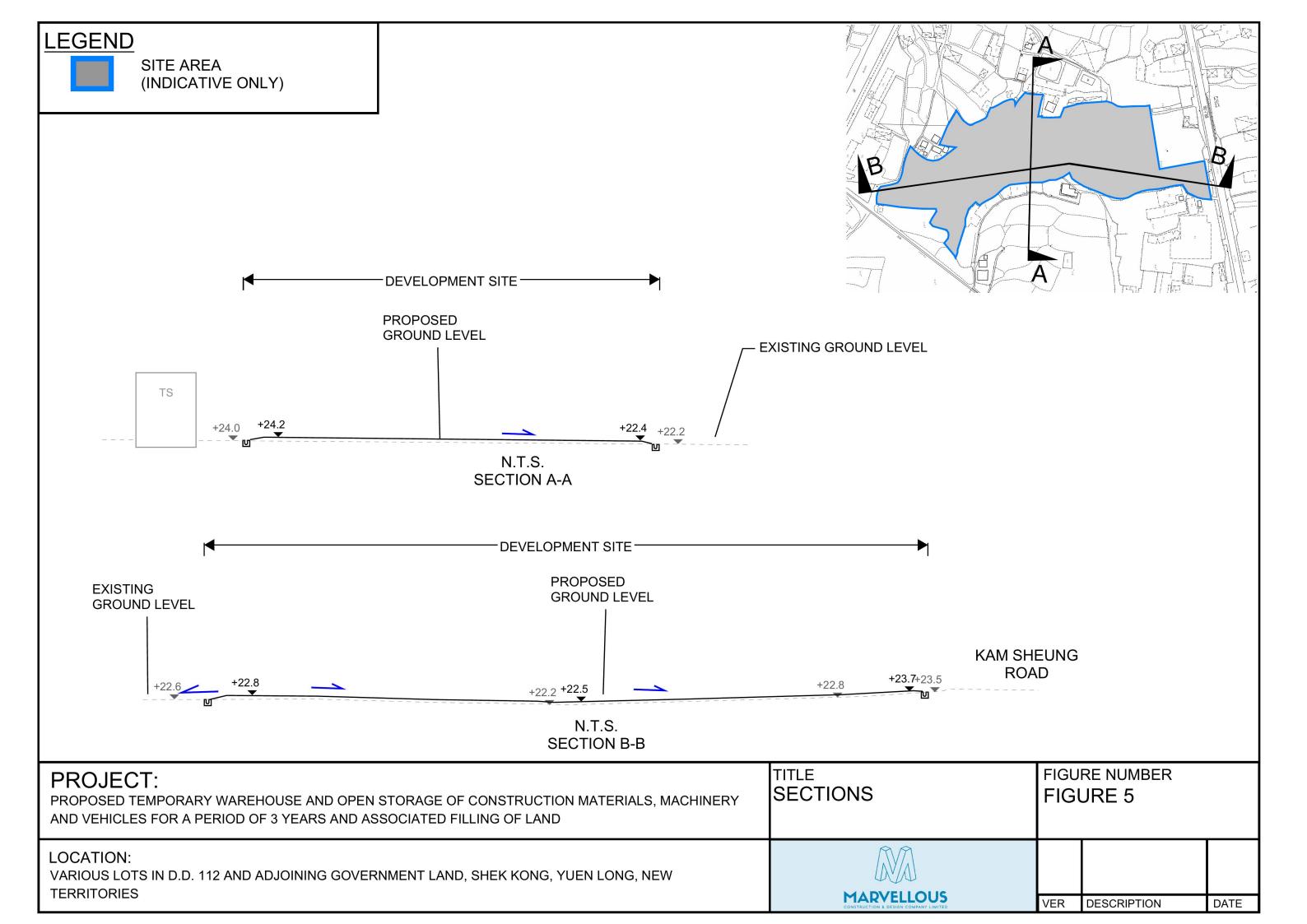
2.ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE STATED.
3.LOCATION OF MANHOLE AND CATCHPITS ARE APPROX. ONLY.
4.COVER LEVELS AND CONNECTION LEVELS ARE APPROXMIATE ONLY

AND SHOULD BE CHECKED ON SITE.

PROJECT: PROPOSED TEMPORARY WAREHOUSE AND OPEN STORAGE OF CONSTRUCTION MATERIALS, MACHINERY AND VEHICLES FOR A PERIOD OF 3 YEARS AND ASSOCIATED FILLING OF LAND	TITLE DRAINAGE SCHEDULE	FIGURE NUMBER FIGURE 3-2	
LOCATION: VARIOUS LOTS IN D.D. 112 AND ADJOINING GOVERNMENT LAND, SHEK KONG, YUEN LONG, NEW TERRITORIES	MARVELLOUS CONSTRUCTION & DESIGN COMPANY LIMITED	VER DESCRIPTION DA)ATE







APPENDIX

Appendix A: Design Calculation

НКО

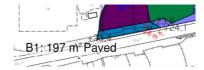
1 in 50

n	0.014
Ks	0.15
Viscosity	0.000001

	HKO a	505.5
Storm Constant	НКО Ь	3.29
	НКО с	0.355

Time of Concentration Checking

Catchment	Flow Distance	Highest Level	Laurant Laura	Gradient (per 100m)	to (min) =	tc =
Catchinent	Flow Distance	nighest Level	Lowest Level	= (H1-H2)/L x 100	0.14465L/ (H ^{0.2} A ^{0.1})	to+tf
A	L	H1	H2			
(m2)	(m)	(mPD)	(mPD)		(min)	(min)
197	8.6	24.1	23.9	2.326	0.6	0.6



Outcomment Area re	ibic (Alca III III)																
Catchment	A1	A2	A3	A4	A5	A6	B1	B2	В3	B4	B5	B6	B7	B8	B9	Total Site Area (After Development)	Outer Catchment (B1 - B9)
Total Area	976	6223	3572	2952	3327	981	197	4038	1809	1896	920	473	91	87	259	18031	9770
Hard Paved Area	976	6223	3572	2952	3327	981	197	2276	1809	185	36	473	91	87	95	18031	5249
Unpaved Area	0	0	0	0	0	0	0	1762	0	1711	884	0	0	0	164	0	4521
Equival. Area	927.2	5911.85	3393.4	2804.4	3160.65	931.95	187.15	2778.9	1718.55	774.60	343.60	449.35	86.45	82.65	147.65	17129.45	6568.9

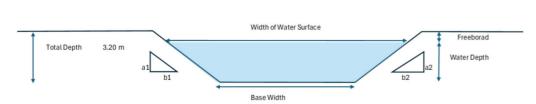
0.35 0.95

Calculation	Table of	Drainage	Systen

	of Drainage System			100 100		12000				12-10-012		120000000000000000000000000000000000000						20000200700					Water Conductors				
US MH/PIT	DS MH/PIT	US GL	DS GL	Size	Gradient	Type	US IL	DS IL	U/S MH/PIT	Length	V	Capacity	Catchment ID1	Catchment ID2									Total Equivalent Area	ToC	Intensity		Utilitizatio
				mm	1 in				TYPE*	m	m/s**	m³/s			ID3	ID4	ID5	ID6	ID7	ID8	ID9	ID10	m²	min	mm/hr ***	Discharge m ³ /s	
SP01	CP1.01	23.90	22.80	525	200	UC	23.38	22.28	SP	41.4	1.51	0.34	B2										2778.90	0.60	347	0.27	79.9%
CP1.01	CP3.03	22.80	22.40	825	200	UC	21.98	21.58	CP	14	2.05	1.12	A2	B2									8690.75	1.06	333	0.80	71.9%
SP02	CP1.01	23.80	22.80	825	200	UC	22.98	21.98	SP	57	2.05	1.12	A2	B2									8690.75	0.60	347	0.84	74.9%
SP03	CP3.01	23.90	23.40	375	200	UC	23.53	23.03	SP	31.6	1.21	0.14	A1	B1									1114.35	0.60	347	0.11	78.6%
CP3.01	CP3.02	23.40	22.40	375	200	UC	23.03	22.03	CP	29.6	1.21	0.14	A1	B1									1114.35	1.04	334	0.11	75.7%
CP3.01	CP3.03	22.40	22.40	375	200	UC	22.03	21.93	CP	19.9	1.21	0.14	A1	B1									1114.35	1.44	323	0.10	73.7%
CP3.02	CP3.03	22.40	22.40	825	250	UC	21.58	21.40	CP	42.7	1.83	1.00	A1	A2	B1	B2							9805.10	1.72	323	0.10	86.3%
													2000			B2 B2											
CP3.04	CP3.05	22.40	22.50	825	250	UC	21.40	21.21	CP	47.8	1.83	1.00	A1	A2 A2	B1		R2						9805.10	2.11	309	0.84	84.1%
CP3.05	CP3.06	22.50	22.10	900	250	UC	21.21	20.86	CP	88.2	1.94	1.26	A1		A3	B1	O.L.						13198.50	2.54	300	1.10	87.3%
CP3.06	CP3.07	22.10	22.10	900	250	UC	20.86	20.84	CP	4.5	1.94	1.26	A1	A2	A3	B1	B2	24	20	D.C.			13198.50	3.30	288	1.06	83.6%
CP3.07	CP3.08	22.10	22.40	1050	250	UC	20.84	20.71	CP	34.1	2.15	1.90	A1	A2	A3	A5	A6	B1	B2	B9			17438.75	3.34	287	1.39	73.1%
CP3.08	CP3.09	22.40	22.60	1050	250	UC	20.71	20.49	CP	54.1	2.15	1.90	A1	A2	A3	A5	A6	B1	B2	B9			17438.75	3.60	283	1.37	72.1%
CP3.09	MH1.01	22.60	22.60	1200	250	PIPE	20.49	20.45	СР	9.5	2.53	2.57	Total Site Area (After Development)										23698.35	4.02	277	1.83	71.0%
MH1.01	Existing Nullah	22.60	22.30	1200	250	PIPE	20.45	20.26	E1	48.8	2.53	2.57	Total Site Area (After Development)	Outer Catchment (B1 - B9)									23698.35	4.08	276	1.82	70.7%
SP04	CP4.01	22.40	22.30	375	200	UC	22.03	21.93	SP	11.1	1.21	0.14	A6	B9									1079.60	0.60	347	0.10	76.1%
CP4.01	CP4.02	22.30	22.20	375	200	UC	21.93	21.78	CP	29.9	1.21	0.14	A6	B9									1079.60	0.75	342	0.10	75.1%
CP4.02	CP4.03	22.20	21.80	375	200	UC	21.78	21.43	CP	14.1	1.21	0.14	A6	B9									1079.60	1.16	330	0.10	72.6%
CP4.03	CP3.07	21.80	22.10	375	200	UC	21.43	21.19	CP	46.6	1.21	0.14	A6	B9									1079.60	1.36	325	0.10	71.5%
SP06	CP6.01	23.80	23.50	825	300	UC	22.98	22.68	SP	85.4	1.67	0.91	A2	B3									7630.40	0.60	347	0.74	80.5%
CP6.01	CP6.02	23.50	23.50	825	300	UC	22.68	22.64	CP	9.4	1.67	0.91	A2	B3									7630.40	1.45	323	0.69	75.0%
CP6.02	CP6.03	23.50	23.30	825	300	UC	22.64	22.48	CP	12.8	1.67	0.91	A2	B3									7630.40	1.55	321	0.68	74.5%
CP6.03	CP6.04	23.30	23.20	900	200	UC	22.40	21.93	CP	93.8	2.17	1.41	A2	A3	A4	B3							13828.20	1.67	318	1.22	86.6%
CP6.04	CP6.05	23.20	23.10	1050	300	UC	21.93	21.77	CP	49.4	1.96	1.74	A2	A3	A4	A5	B3	B4	B5	B6			18556.40	2.90	294	1.52	87.3%
CP6.05	CP6.06	23.10	22.90	1050	300	UC	21.77	21.66	CP	31.1	1.96	1.74	A2	A3	A4	A5	В3	B4	B5	B6	B7		18642.85	3.32	287	1.49	85.7%
CP6.06	CP6.07	22.90	22.70	1050	300	UC	21.66	21.60	CP	17.7	1.96	1.74	A2	A3	A4	A5	B3	B4	B5	B6	B7		18642.85	3.58	283	1.47	84.5%
CP6.07	CP3.09	22.70	22.60	1050	300	UC	21.60	21.55	CP	12.9	1.96	1.74	A2	A3	A4	A5	В3	B4	B5	B6	В7	B8	18725.50	3.73	281	1.46	84.2%
SP07	CP7.01	23.50	23.50	525	200	UC	22.98	22.88	SP	18.8	1.51	0.34	A4										2804.40	0.60	347	0.27	80.6%
CP7.01	CP7.01	23.50	24.00	525	200	UC	22.88	22.77	CP	21.6	1.51	0.34	A4										2804.40	0.81	340	0.27	79.2%
CP7.01	CP7.02	24.00			200	UC	22.77	22.70	CP	15.1	1.51	0.34	A4										2804.40	1.04	334		77.6%
CP7.02 CP7.03	CP7.03 CP7.04	24.00	23.50 23.90	525 600	300	UC	22.77	22.70	CP	15.1	1.51	0.34	A4 A4	B4									2804.40 3579.00	1.04	334	0.26	83.8%
									CP				A4 A4	84 84													
CP7.04	CP7.05	23.90	23.90	600	300	UC	22.64	22.62		7.5	1.35	0.39	200										3579.00	1.41	324	0.32	82.6%
CP7.05	CP7.06	23.90	23.50	600	300	UC	22.62	22.47	CP	46.3	1.35	0.39	A4	B4									3579.00	1.50	322	0.32	82.0%
CP7.06	CP7.07	23.50	23.50	600	300	UC	22.47	22.41	CP	18	1.35	0.39	A4	B4	0.5								3579.00	2.07	309	0.31	78.8%
CP7.07	CP7.08	23.50	23.50	600	300	UC	22.41	22.30	CP	31.1	1.35	0.39	A4	B4	B5								3922.60	2.29	305	0.33	85.1%
CP7.08	CP6.04	23.50	23.20	600	300	UC	22.30	22.24	CP	18.2	1.35	0.39	A4	B4	B5								3922.60	2.68	298	0.32	83.1%

#SP: Start Point
##:with 10% reduction in flow area
###: With 11.1% rainfall increase as per Table 28 of SDM Corrigendum No. 1/2022.

Capacity Checking of Existing 12m Width Rectangular Nullah



a1	1	П
b1	0.0	1
a2	1	1
b2	0.0	
Total Depth	3.20	m
Base Width	12.00	m
Assumed Water Depth	2.70	m
Freeboard	0.50	m

Assumed Water Depth	Freeboard	Base Width	Width of Water Surface	Flow Area	Wetted Perimeter	Hydralic Radius	Manning's Roughness	Gradient	Velocity	Capacity
m	m	m	m	m ²	m	m		1 in	m/s	m³/s
2.70	0.50	12.00	12.00	32.40	17.40	1.86	0.035	150	3.53	114.40

Total Flow from The Application Site 1.82 m³/s (See Appendix A1)

Utilization Rate = 1.6%
Total flowfrom proposed site only occupy 1.6% of the existing Nullah.

APPENDIX B - PROPOSED LAYOUT PLAN DEVELOPMENT PARAMETERS STRUCTURE COVERED AREA GROSS FLOOR AREA BUILDING HEIGHT (ABOUT) (ABOUT) (ABOUT) APPLICATION SITE AREA COVERED AREA : 18,031 m² : 2,961 m² B1 B2 WAREHOUSE (EXCL. D.G.G.) WAREHOUSE (EXCL. D.G.G.) 1,896 m² (ABOUT) 1,065 m² (ABOUT) 1,896 m² (ABOUT) 1,065 m² (ABOUT) 15 m (ABOUT)(1-STOREY) 15 m (ABOUT)(1-STOREY) : 15,070 m² UNCOVERED AREA PLOT RATIO SITE COVERAGE (ABOUT) (ABOUT) : 0.16 TOTAL 2,961 m² (ABOUT) 2,961 m² (ABOUT) : 16 % NO. OF STRUCTURE DOMESTIC GFA : 2 : NOT APPLICABLE *D.G.G. - DANGEROUS GOODS GODOWN NON-DOMESTIC GFA : 2,961 m² : 2,961 m² (ABOUT) TOTAL GFA BUILDING HEIGHT : 15 m : 1 (ABOUT) NO. OF STOREY OPEN STORAGE AREA : 6,318 m² (ABOUT) В1 MMM OPEN STORAGE AREA CIRCULATION SPACE APPLICATION SITE OPEN STORAGE AREA ACCESS ROAD OPEN STORAGE AREA

PARKING PROVISIONS

NO. OF PRIVATE CAR PARKING SPACE DIMENSION OF PARKING SPACE : 9 : 5 m (L) X 2.5 m (W)

LOADING/UNLOADING PROVISIONS

NO. OF L/UL SPACE FOR CONTAINER VEHICLE

DIMENSION OF L/UL SPACE

NO. OF L/UL SPACE FOR LIGHT GOODS VEHICLE DIMENSION OF L/UL SPACE

: 3 : 7 m (L) X 3.5 m (W)

: 16 m (L) X 3.5 m (W)

OPEN STORAGE AREA

<u>B2</u>

LEGEND

STRUCTURE

PLAN 10

APPLICATION SITE

PARKING SPACE (PRIVATE CAR) L/UL SPACE (LIGHT GOODS VEHICLE) L/UL SPACE (CONTAINER VEHICLE)

INGRESS / EGRESS



PRODECT PROPERTY WAREHOUSE AND OPEN STORAGE OF CONSTRUCTION MATERIALS, MACHINERY AND VEHICLES WITH ANCILLARY FACILITIES AND ASSOCIATED FILLING OF LAND FOR A PERIOD OF 3 YEARS

VARIOUS LOTS IN D.D. 112 AND ADJOINING GOVERNMENT LAND, SHEK KONG, YUEN LONG, NEW TERRITORIES

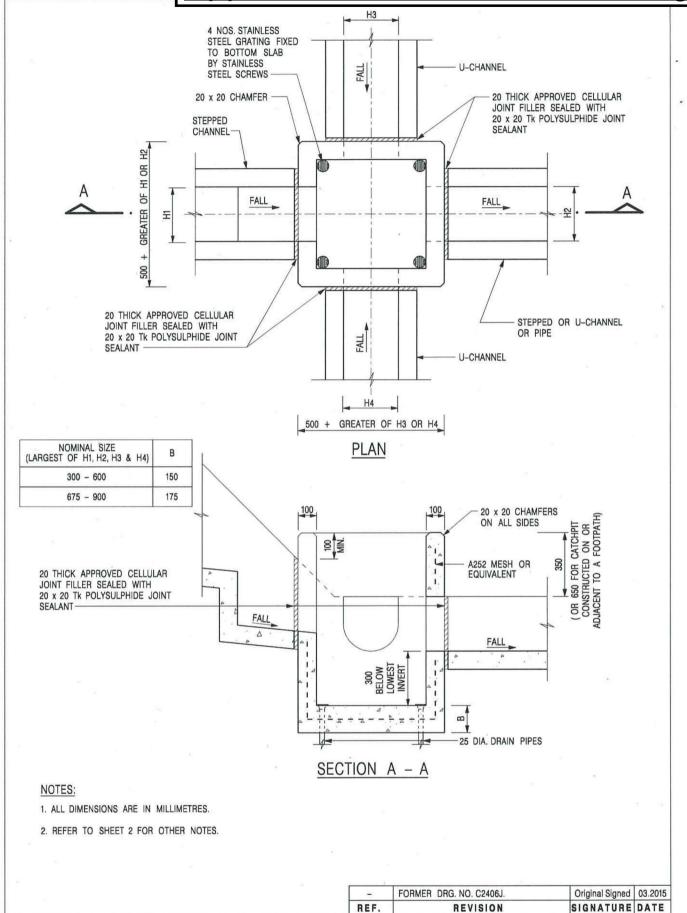
INGRESS / EGRESS 11 m (ABOUT)(W)

OPEN STORAGE AREA

1:1500 @ A4 12.2.2025



Appendix C - Reference Drawings



CATCHPIT WITH TRAP (SHEET 1 OF 2)

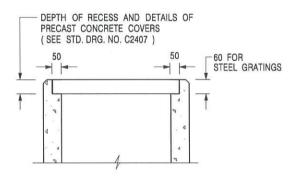
卓越工程 建設香港

CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 DATE JAN 1991

C2406 /1



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
- SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	REF.	REVISION	SIGNATURE	DATE
	-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
	Α	MINOR AMENDMENT.	Original Signed	04.2016

CATCHPIT WITH TRAP (SHEET 2 OF 2)

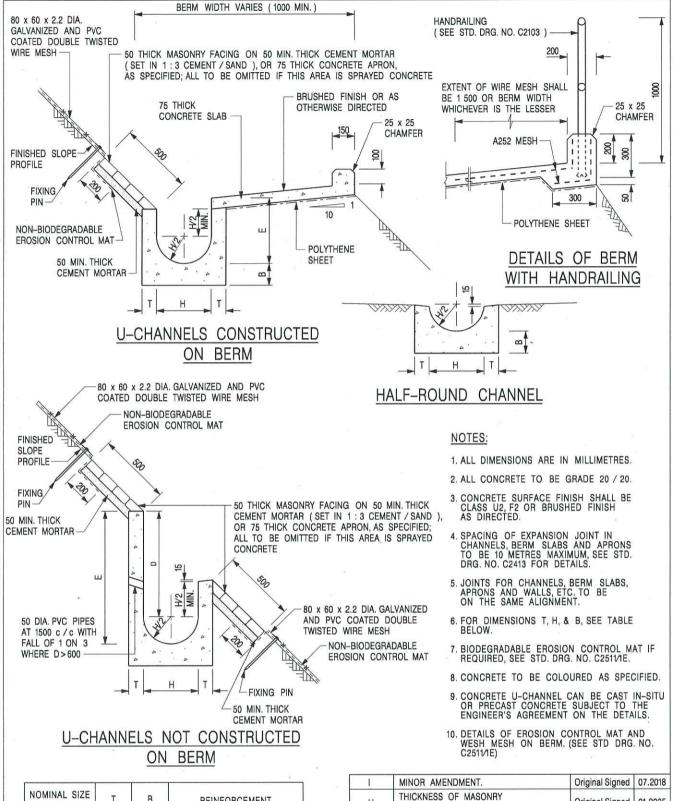


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:20 **DATE** JAN 1991

drawing no. C2406 /2A

卓越工程 建設香港



NOMINAL SIZE H	T	В	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100
375 - 600	100	150	WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
В	MINOR AMENDMENTS.	Original Signed	3.94
С	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
D	MINOR AMENDMENT.	Original Signed	08.2001
E	DRAWING TITLE AMENDED.	Original Signed	11.2001
F	GENERAL REVISION.	Original Signed	12.2002
G	MINOR AMENDMENT.	Original Signed	01.2004
Н	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
1	MINOR AMENDMENT.	Original Signed	07.2018

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A -WITH MASONRY APRON)

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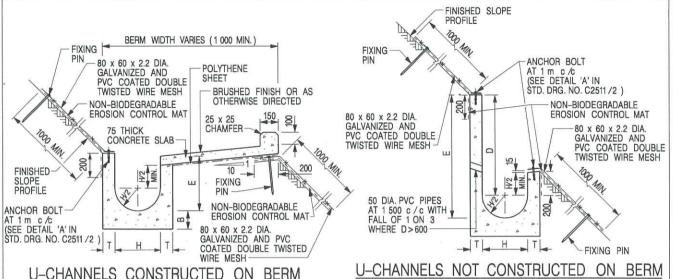
CEDD

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE 1:25

DATE JAN 1991

C2409l



U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT

BIODEGRADABLE

EROSION CONTROL MAT

07.2018

12.2017

01.2005

12.2002

08 2001

6.99

3.94

10.92

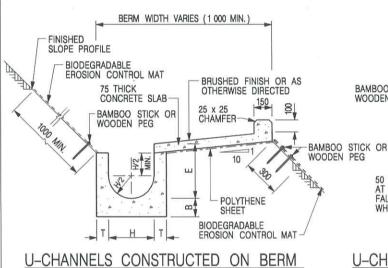
DATE

Original Signed

SIGNATURE

FINISHED SLOPE PROFILE

ш



WITH BIODEGRADABLE

EROSION CONTROL MAT

BAMBOO STICK OR WOODEN PEG

U-CHANNELS NOT CONSTRUCTED ON BERM

WITH BIODEGRADABLE

EROSION CONTROL MAT

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE TO BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- 5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- 6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- 7. FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- 8. MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- 10. THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	Ţ	В	REINFORCEMENT
300	80	100	A252 MESH PLACED
375 - 600	100	150	CENTRALLY AND T=100 WHEN E>650
675 - 900	125	175	A252 MESH PLACED CENTRALLY

	DETAILS	OF	HALF-I	ROUN	D AND	
	U-CHAN	NELS	S (TYP	ЕВ-	- WITH	
I	FROSION	COI	NTROI	MAT	APRON	1)

6
CEDD
CEDD
nac

Н

G

F

E

D

C

В

A

REF.

BAMBOO STICK OR WOODEN PEG

50 DIA. PVC PIPES AT 1 500 c/c WITH FALL OF 1 ON 3

WHERE D>600

CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC
DATE JAN 1991

MINOR AMENDMENT.

MINOR AMENDMENT

GENERAL REVISION.

MINOR AMENDMENT.

MINOR AMENDMENT.

MINOR AMENDMENT

FIXING DETAILS OF BIODEGRADABLE

150 x 100 UPSTAND ADDED AT BERM

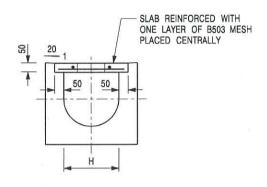
REVISION

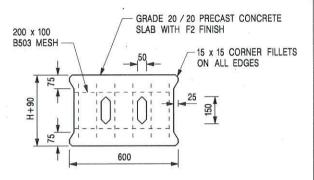
EROSION CONTROL MAT ADDED.

DIMENSION TABLE AMENDED

C2410

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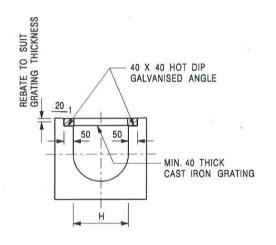


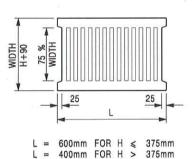
<u>PLAN OF SLAB</u>

TYPICAL SECTION

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)





TYPICAL SECTION

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. H=NOMINAL CHANNEL SIZE.
- ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
- 4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

REF.	REVISION	SIGNATURE	DATE
Α	CAST IRON GRATING AMENDED.	Original Signed	
В	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
С	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
D	NOTE 4 ADDED.	Original Signed	06.2008
E	NOTES 3 & 4 AMENDED.	Original Signed	

COVER SLAB AND CAST IRON GRATING FOR CHANNELS

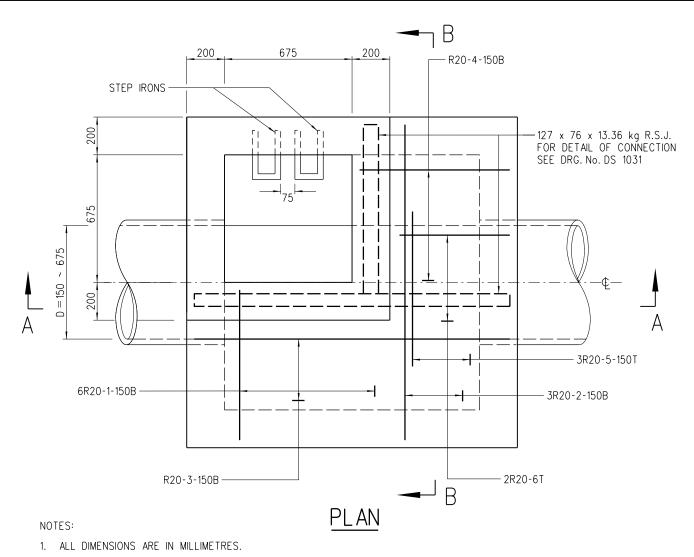


CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

 SCALE
 1:20
 DRAWING NO.

 DATE
 JAN 1991
 C2412E

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- NOTATION OF REINFORCEMENT :THE SEQUENCE OF DESCRIPTION OF IDENTIFICATION MARKS ON DRAWINGS FOR STEEL REINFORCING BARS FOR CONCRETE WORK IS AS FOLLOWS (NUMBER, TYPE, SIZE, MARK, SPACING, LOCATION OR COMMENT)
- 3. B DENOTES GRADE 500B RIBBED REINFORCEMENT.
- 4. R DENOTES GRADE 250 PLAIN REINFORCEMENT.
- 5. PIPE DIAMETER
- : 150 TO 675 mm
- NORMAL R. OF DEPTH RANGE
- :1500 TO 3 000 mm (MEASURED FROM ROAD LEVEL TO LOWEST INVERT)
- 7. USED IN
- :STORMWATER DRAIN AND SEWER
- JUNCTION
- : POSITION OF JUNCTION TO BE DETERMINED IN EACH INDIVIDUAL CASE. CHANNELS IMMEDIATELY UNDER ACCESS TO MANHOLE SHOULD BE AVOIDED.
- 9. TOP TREATMENT
- : SEE DRG. No. DS 1032
- 10. FOUNDATION
- : FOUNDATION OF MANHOLE VARIES WITH SITE CONDITION. THEREFORE, IT SHOULD BE DETERMINED ON SITE BY THE ENGINEER.

NOTE 15 ADDED

- 11. CONCRETE
- : GRADE 30/20
- ALL BAR MARKS APPEARED HEREON ARE USED FOR REFERENCE IN THIS DRAWING ONLY.
- 13. MINIMUM COVER AT END OF BARS 40 mm
- 14. COVER AND FRAME NOT SHOWN ON PLAN FOR CLARITY.
- 15. RECESS WITH SQUARE STEEL ROD SHALL BE PROVIDED AT TOP OF MANHOLE CHAMBER FOR INSTALLING MONITORING DEVICE(S). DETAILS REFER TO DSD STANDARD DRAWING NO. DS 1099.

REV.	DESCRIPTION	SIGNATURE	DATE
	NEW ISSUE	ORIGINAL SIGNED	15.8.2007
А	NOTE 11 REVISED	ORIGINAL SIGNED	24.11.2014
В	NOTE 11 DELETED NOTES 2, 3 & 4 ADDED	ORIGINAL SIGNED	29.4.2015
ن	NOTE IS ADDED	URIGINAL SIGNED	2.8.2022

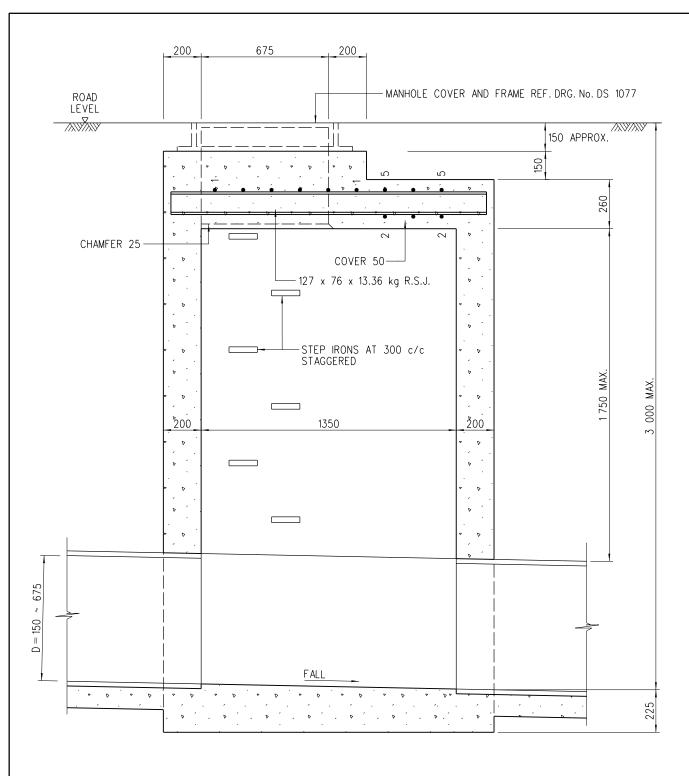
STANDARD MANHOLE TYPF F 1

DRAINAGE SERVICES DEPARTMENT REFERENCE DRAWING No.

1080C SCALE

1:20 (SHEET 1 OF 3)

ODICINAL CICNED 2 0 2022



SECTION A-A

BAR MARKS	SHAPE CODE 🔾
1 & 4	99
2, 3 & 6	(35)
5	20

REV.	DESCRIPTION	SIGNATURE	DATE
	NEW ISSUE	ORIGINAL SIGNED	15.8.2007
А	NOTE 11 REVISED	ORIGINAL SIGNED	24.11.2014
В	NOTE 11 DELETED NOTES 2, 3 & 4 ADDED	ORIGINAL SIGNED	29.4.2015
С	NOTE 15 ADDED	ORIGINAL SIGNED	2.8.2022

STANDARD MANHOLE
TYPE E1

DRAINAGE SERVICES DEPARTMENT

REFERENCE DRAWING NO.

SCALE DS 1080C (SHEET 2 OF 3)

