Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years

At Various Lots in D.D. 117 and Adjoining Government Land, Tai Tong, Yuen Long, New Territories

Drainage Assessment Report

Applicant:

R-riches Property Consultants Limited

Oct 2024

Consultant: C & H Consulting Co. Ltd

DRAINAGE IMPACT ASSESSMENT

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1. Introduction

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years at various Lots in D.D. 117 and Adjoining Government Land, Tai Tong, Yuen Long, New Territories (A/YL-TT/635), is applied for planning permission. This report is a Drainage Assessment Report to support the submission.

2. Site Description

The site is located on the left in western side of Tai Tong Shan Road. The site has a higher level on the northern side and lower level on the southern side. The site has partly developed, and the open storage has been constructed at the site. The topography level of the site is lower than the eastern side of site and higher than the western side of the site. Site Area is about 10,313 sq. m (Includes Government Land of about 1,794 sq. m) (Appendix A shows the Location Plan) The ground profile in the further west is sloping downward towards the west direction.

There are some existing drainage facilities that existing Stream (5m width x 5 m depth) along northern side of the site connected to existing open channel with critical size 13m width x 6m depth along western side of the site. Existing Stream is natural-stream channels (conservatively take n=0.04). and the open channel is made of concrete (conservatively take n=0.018). The final discharge is managed by DSD. " n" value is a coefficient which represents the roughness or friction applied to the flow by the channel in manning's equation.

3. Drainage Assessment

Peripheral channel is designed to collect the runoff generated from and passed through the site (Appendix B shows the Outside Catchment Area Plan). The final discharge point is the existing 13m width x 6m depth open channel in the western side of the site. The runoff intensity is 180mm/hr. The runoff coefficient

of the site to be 0.95, while that of the part of outside catchment area is 0.95 and part of area is 0.40. (Appendix C shows the Drainage Layout Plan of the site).

The events is an end over for the evicting 1 Om/(M/N) Fm/(D) even showned is a recented in Annandiv D. The total even including the site is E16 72Eca m. The variant intensity is 100mm /br. It is concernatively accounted that FOO/ to be band payred (C=0.2E).

For the site itself, the runoff including the site and the outside catchment area is collected by proposed 375UC and finally discharge to the existing 1.8m(W)x1.5m(D) open channel via the proposed 525pipe. For checking the existing 1.8m(W)x1.5m(D) open channel, since the site is currently full of vegetation, in drainage point of view, the runoff coefficient is changed from 0.25 to 0.95. It is found that the 1.8m(W)x1.5m(D) open channel is adequate to cater the extra runoff due the proposed development. Detailed calculation is presented in Appendix E,

It is found that the existing open channel is capable to cater the runoff without flooding risk.

4. Conclusion

Proposed Temporary Warehouse (Excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years at various Lots in D.D. 117 and Adjoining Government Land, Tai Tong, Yuen Long, New Territories (A/YL-TT/635), is applied for planning permission. The runoff generated from the site is collected and discharged to the existing open channel in the south that finally discharge to Existing open channel (SCP1011280) maintained by DSD. The overall catchment of the existing open channel and the corresponding runoff is investigated and found that it is capable of catering to the extra runoff from the proposed development. (Appendix E shows the detailed calculation) There is no flooding risk for the proposed development.

Appendix A: Site Location Plan





Appendix B: Outside Catchment Plan



Appendix C: Dr ainage Layout Plan







Appendix D: Overall Catchment Area for Existing Channel

Appendix D1: Overall Catchment Area for Existing 5m (W) x 5m (H) Channel



Catchment Area = 24699.05m2 Coefficient of surface runoff =0.25

Appendix D2: Overall Catchment Area for Existing 13m (W) x 6m (H) Channel

前往地圖: https://www.map.gov.hk/gm/geo:22.4081,114.0166?z=18056



由「地理資訊地圖」網站提供: https://www.map.gov.hk 注意: 使用此地圖受「地理資訊地圖」的使用條款及條件以及知識產權告示約束。

O GEOINFO MAP 地理資訊地圖

前往地圖: https://www.map.gov.hk/gm/geo:22.4046,114.0032?z=36112



由「地理資訊地圖」網站提供: https://www.map.gov.hk 注意:使用此地圖受「地理資訊地圖」的使用條款及條件以及知識產權告示約束。

Appendix E: Calculation

Appendix E1: Calculation of 1m x 1m channel and 1000mm dia. pipe and 900mm width stepped channel

Company:

Project :

Date:

8/10/2024

Site Area = Outside Catchment Area = Total Catchmnet Area=
 10313
 m²

 14289
 m²

 24602
 m²

(C=0.95, hard-paved) (C=0.6, Grassland, heavy soil, Steep Slope) (Ratio of concrete paved: ratio of soil paved = 1:2)

 $t_o = \frac{0.14465 L}{H^{0.2} A^{0.1}}$

where $t_o =$ time of concentration of a natural catchment (min.)

- A = catchment area (m^2)
- H = average slope (m per 100 m), measured along the line of natural flow, from the summit of the catchment to the point under consideration
- L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m)

0.6 t= 0.14475*100/1.5^0.2/38338^0.1 = 4.646 min

$$i = \frac{a}{(t_d + b)^c}$$

where i = extreme mean intensity in mm/hr, t_d = duration in minutes ($t_d \le 240$), and a, b, c = storm constants given in Tables 3a, 3b, 3c and 3d.

Assume 10 yrs return period

Therefore,

a = 485 b= 3.11 c = 0.397		
i = 215.062891	mm/hr	
take	i = 220	mm/hr

Company: Project :

Date:

8/10/2024

Calculation for channels:

	14289		(C=0.6, Gra	(C=0.6, Grassland, heavy soil, Steep Slo					(Ratio of c	crete paved: ratio of soil paved = $1:2$		
Catchment Area of site												
Site Area	= =	10313 0.010313	m ² km ²									
Peak runoff in m ³ /s	= = =	0.278 0.599206 35952	x m^3/s liter/min	0.95	X	220	mm/hr	Х	0.010313	km^2		
Outside Area	= =	14289 0.014289	m² km²									
Peak runoff in m^3/s	= = =	0.278 0.524349 31461	x m^3/s liter/min	0.6	Х	220	mm/hr	Х	0.014289	km^2		
Total Peak Runoff for Site	=	1.123555	m^3/s	=		67413.3	042		liter/min			

Check 1000mm dia. Pipe by Colebrook-White Equation

$$V = -\sqrt{(8gDs)} \log(\frac{ks}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}})$$

where :											
V	=			mean	velocity (m/s)						
g	=	9.81	m/s2	gravit	tational acceleration (m/	/s2)					
D	=	1	m	intern	al pipe diameter (m)						
ks	=	0.000003	m	hydra	hydraulic pipeline roughness (m) (Table 5, from DSD Sewerage Manual.						
V	=	1.14E-06	m2/s	kinen	natic viscosity of fluid (m2/s)					
S	=	0.01		hydra	ulic gradient						
Area	=	0.70685835	m2/s								
Therefore,	=	4.5521	m/s	>	Design velocity	=	1.1236	m3/s	/	(1^2 * pi/4 x 0.9)	
design V of					from catchment area	=	1.5895053	m/s		===>O.K.	

For 1m x 1m channel in the site,



Proposed 1.0m (W) x 1.0m (H) open channel (1:150) in the site can cater the surface runoff from proposed development



ANNEX TGN

27 A1 (2/4)

The Government of the Hong Kong Special Administrative Region Geotechnical Engineering Office, Civil Engineering and Development Department

Hydraulic Design of Stepped Channels **GEO Technical Guidance Note No. 27**

On

Slopes **TGN 27**)

Appendix E2: Calculation of 5m(W) x 5m (H) Existing channel

Company:

Project :

Date:

8/10/2024

Site Area =	10313	m ²
Outside Catchment Area =	24699	m ²
Total Catchmnet Area=	35012	m ²

$$t_o = \frac{0.14465L}{H^{0.2} A^{0.1}}$$

where $t_o =$ time of concentration of a natural catchment (min.)

- A = catchment area (m^2)
- H = average slope (m per 100 m), measured along the line of natural flow, from the summit of the catchment to the point under consideration

(C=0.95, hard-paved) (C=0.6, Grassland)

L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m)

> t= 0.14475*100/1.5^0.2/24699^0.1 = 4.854 min

$$i = \frac{a}{\left(t_d + b\right)^c}$$

where i = extreme mean intensity in mm/hr, t_d = duration in minutes ($t_d \le 240$), and a, b, c = storm constants given in Tables 3a, 3b, 3c and 3d.

Assume 10 yrs return period

Therefore,

a = 485 b= 3.11 c = 0.397		
i = 212.806427	mm/hr	
take	i = 220	mm/hr

Company: Project :

Data	9/10/2024
Date:	8/10/2024

Calculation for channels:

Catchment Area of site

Site Area	= =	10313 0.010313	m² km²							
Peak runoff in m ³ /s	= = =	0.278 0.599206 35952	x m^3/s liter/min	0.95	Х	220	mm/hr	Х	0.010313	km^2
Outside Area	= =	24699 0.024699	m ² km ²							
Peak runoff in m ³ /s	= = =	0.278 0.906355 54381	x m^3/s liter/min	0.6	Х	220	mm/hr	Х	0.024699	km^2
Total Peak Runoff for Site	=	1.50556	m^3/s	=		90333.6	258		liter/min	

By Marning's equation (5m x 5m (lepth) channel is adopted)



Existing 5m (W) x 5m (H) open channel (1:150) in the site can cater the surface runoff from proposed development

Appendix E3: Calculation of 13m(W) x 6m (H) Existing channel

Company:

Project :

Date:

8/10/2024

10313

614642

624955

Site Area = Outside Catchment Area = Total Catchmnet Area=

m ²	
m^2	
m ²	

$$t_o = \frac{0.14465L}{H^{0.2} A^{0.1}}$$

time of concentration of a natural catchment (min.) where to =

- catchment area (m²) A =
- Н = average slope (m per 100 m), measured along the line of natural flow, from the summit of the catchment to the point under consideration

(C =0.95, hard-paved)

(C=0.6, Grassland)

L = distance (on plan) measured on the line of natural flow between the summit and the point under consideration (m)

> t= 0.14475*100/1.5^0.2/624955^0.1 = 3.514 min

$$i = \frac{a}{(t_d + b)^c}$$

where = extreme mean intensity in mm/hr, i td = duration in minutes ($t_d \le 240$), and a, b, c =storm constants given in Tables 3a, 3b, 3c and 3d.

Assume 10 yrs return period

a = b= c =	485 3.11 0.397		
i =	228.957085	mm/hr	
	take		i = 230

Therefore,

i = 230

mm/hr

Company: Project :

Data	9/10/2024
Date:	8/10/2024

Calculation for channels:

Catchment Area of site

Site Area	= =	10313 0.010313	m ² km ²							
Peak runoff in m^3/s	= = =	0.278 0.626443 37587	x m^3/s liter/min	0.95	Х	230	mm/hr	х	0.010313	km^2
Outside Area	= =	624955 0.624955	m ² km ²							
Peak runoff in m^3/s	= = =	0.278 23.97577 1438546	x m^3/s liter/min	0.6	X	230	mm/hr	Х	0.624955	km^2
Total Peak Runoff for Site	=	24.60222	m^3/s	=		1476132	2.971		liter/min	



Existing 13m (W) x 6m (H) open channel (1:150) in the site can cater the surface runoff from proposed development

Appendix F: Site Photo













Slope Gradient = 25 degrees

Dimension of Stepped Channel

Appendix G: Standard Drawing





ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- 8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'G' ON STD. DRG. NO. C2405; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 ¢ STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'F' ON STD. DRG. NO. C2405.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	– FORMER DRG. N	O. C2406J. Original Signed 03.2015	
	REF. RE	VISION SIGNATURE DATE	
CATCHPIT WITH TRAP	CIV CEDD DEVE	IL ENGINEERING AND	
(SHEET 2 OF 2)	SCALE 1:20 DRAWING NO.		
х <i>1</i>	DAIE JAN 199	1 02400 / 2	
卓越工程 建設香港	We Engineer Hong Kong's Development		



Figure 8.10 - Typical Details of Catchpits



Figure 8.11 - Typical U-channel Details





FIRE SERVICES NOTES:

1. <u>Hose reel system</u>

- 1.1 HR SYSTEM TO BE PROVIDED AND INSTALLED FOR THE STRUCTURE <u>B1</u> IN ACCORDANCE WITH THE CODE OF PRACTICE FOR MINIMUM FIRE SERVICE INSTALLATIONS AND EQUIPMENT.
- 1.2 HOSE REELS SHALL BE PROVIDED AT THE POSITIONS INDICATED ON PLAN.
- 1.3 SUFFICIENT HOSE REELS TO BE PROVIDED TO ENSURE THAT EVERY PART OF THE AREA CAN BE REACHED BY A LENGTH OF NOT MORE THAN 30m OF HOSE REEL TUBING.
- 1.4 AN MODIFIED HOSE REEL SYSTEM WITH 2000L F.S. WATER TANK TO BE PROVIDED AND TO BE SINGLE END FEED FROM TOWN MAIN. THE LOCATION OF THE FS WATER TANK AND FS PUMP ROOM ARE CLEARLY MARKED ON PLANS.
- 1.5 TWO FIXED FIRE PUMPS (DUTY & STANDBY) TO BE PROVIDED IN THE PUMP ROOM.
- 1.6 NO FIRE SERVICES INLET TO BE PROVIDED FOR THE MODIFIED HOSE REEL SYSTEM.
- 1.7 AN INSTRUCTION PLATE SHALL BE PROVIDED NEXT TO THE BREAK GLASS UNIT FOR OPERATION OF HOSE REEL
- AUTOMATIC SPRINKLER SYSTEM
- 2.1 AN AUTOMATIC SPRINKLER SYSTEM TO BE PROVIDED TO THE ENTIRE STRUCTURE <u>B1</u> IN ACCORDANCE WITH LPC RULES INCORPORATING BS EN12845 : 2015 AND FSD CIRCULAR LETTER 5/2020. THE SPRINKLER TANK, SPRINKLER PUMP ROOM, SPRINKLER INLET AND SPRINKLER CONTROL VALVE GROUP SHALL BE CLEARLY MARKED ON PLANS.
- 2.2 THE CLASSIFICATION OF THE AUTOMATIC SPRINKLER INSTALLATION TO BE ORDINARY HAZARD GROUP 3.
- 2.3 A 135,000 LITERS SPRINKLER WATER TANK TO BE PROVIDED AS INDICATED ON PLAN.
- 2.4 SPRINKLER CONTROL VALVE SET AND SPRINKLER INLET TO BE PROVIDE AT GROUND FLOOR AND THE LOCATION AS INDICATED ON PLAN.
- 2.5 TWO SPRINKLER PUMPS (DUTY & STANDBY) AND ONE SPRINKLER JOCKEY PUMP TO BE PROVIDED FOR SERVING THE STRUCTURE AND LOCATED IN PUMP ROOM.
- 2.6 ALL SPRINKLER PIPE SIZE SHOULD BE Ø32mm UNLESS SPECIFY.
- 2.7 TYPE OF STORAGE METHOD FOR THE BUILDING IS AS FOLLOWS: (A) STORAGE CATEGORY : CATEGORY (I) (B) STORAGE HEIGHT : NOT EXCEEDING 4M (C) STORAGE : ST1
- 2.8 STORAGE BLOCK SHOULD BE SEPARATED BY AISLES NO LESS THAN 2.4 WIDE.
- 2.9 STORAGE SHOULD BE CONFINED TO BLOCKS NOT EXCEEDING 50M² IN PLAN AREA FOR CATEGORY I.
- 2.10 ALL INSTALLED SPRINKLER SHOULD BE PENDENT TYPE AND THE TEMPERATURE RATING OF SPRINKLER HEAD SHALL BE 68°C UNLESS OTHERWISE SPECIFIED.
- 3. <u>FIRE ALARM SYSTEM</u>
- 3.1 FIRE ALARM SYSTEM SHALL BE PROVIDED THROUGHOUT THE STRUCTURE A IN ACCORDANCE WITH BS 5839-1 : 2017 AND FSD INITIATION. MANUAL CALL POINT SHOULD BE PROVIDED ADJACENT TO ALL EXITS TO OPEN AIR ON G/F.
- 3.2 AN ADDRESSABLE TYPE FIRE ALARM PANEL TO BE PROVIDED AND LOCATED INSIDE PUMP ROOM.
- 4. <u>Emergency lighting</u>
- 4.1 SUFFICIENT EMERGENCY LIGHTING SHALL BE PROVIDED THROUGHOUT THE ENTIRE BUILDINGS/STRUCTURES IN ACCORDANCE WITH BS 5266: PART 1 AND BS EN 1838 AND FSD CIRCULAR LETTER 4/2021.
- 5. <u>EXIT SIGN</u>
- 5.1 SUFFICIENT DIRECTIONAL AND EXIT SIGN SHALL BE PROVIDED IN ACCORDANCE WITH BS 5266: PART 1 AND FSD CIRCULAR LETTER 5/2008.
- MISCELLANEOUS F.S. INSTALLATION
- 6.1 PORTABLE FIRE EXTINGUISHERS WITH SPECIFIED TYPE AND CAPACITY TO BE PROVIDED AT LOCATION AS INDICATED ON PLANS.
- 6.2 NO EMERGENCY GENERATOR TO BE PROVIDED FOR SERVING THE EMERGENCY POWER. DUPLICATED POWER SUPPLIES FOR ALL
- 6.2 WHEN A VENTILATION / AIR CONDITIONING CONTROL SYSTEM TO A BUILDING IS PROVIDED, IT SHALL STOP MECHANICALLY INDUCED AIR MOVEMENT WITHIN A DESIGNATED FIRE COMPARTMENT.
- 6.3 NO DYNAMIC SMOKE EXTRACTION SYSTEM SHALL BE PROVIDED SINCE FIRE COMPARTMENT OF STRUCTURE A & B NOT EXCEEDING 7000 CUBIC METERS.
- 6.4 <u>NO</u> AUDIO/VISUAL ADVISORY SYSTEM SHALL BE PROVIDED SINCE FIRE COMPARTMENT OF STRUCTURE <u>A & B</u> NOT EXCEEDING 2000 SPUARE METERS.
- 6.5 NO DANGEROUS GOODS WILL BE STORED AT ALL STRUCTURES.

CIRCULAR LETTER NO.6/2021. ONE ACTUATING POINT AND ONE AUDIO WARNING DEVICE SHOULD BE LOCATED AT EACH HOSE REEL POINT. THE ACTUATION POINT SHOULD INCLUDE FACILITIES FOR FIRE PUMP START AND AUDIO / VISUAL WARNING DEVICE

FIRE SERVICES INSTALLATIONS COMPRISING A CABLE CONNECTED FROM ELECTRICITY MAINS DIRECTLY BEFORE THE MAIN SWITCH.

EGENIN (END LAVITIT DLAN)

	KFUK LAIUUI FLANZ	ADDIL	<u> </u>
H.R.	HOSE REEL W/ LOCKABLE GLASS FRONTED NOZZLE BOX, STRIKER, C/W FIRE ALARM BELL & BREAK GLASS UNIT	SPR. H.R.	SF H(
Ð	150mm FIRE ALARM BELL	F.E.	FIF
0	BREAK GLASS UNIT	CO_2	
———	SPRINKLER HEAD	F.S.I.	FIF
(F)	FLOW SWITCH	H/L	- (
$\langle M \rangle$		M/L	MI
	MONITORED GATE VALVE	L/L	LC
F F	SPRINKLER ZONE SUBSIDIARY CONTROL VALVE ASSEMBLY INCLUDES ZONE SUBSIDIARY CONTROL VALVE, FLOW SWITCH,	F/A	FF
	iest gate valve and drain valve	F/B	FF
\$ \$	GATE VALVE	T/A	TC
\bowtie	NON RETURN VALVE	T/B	TC
\bigcirc	VORTEX INHIBITOR	U/G	UN
	BALL FLOAT VALVE	F.S.	-
Ρ	PRESSURE SWITCH		
	SPRINKLER / HOSE REEL PIPE		
	SPRINKLER CONTROL VALVE SET		
-	CHECK METER POSITION		
	SPRINKLER / F.S. INLET		
(F.E) _{Co2}	5Kg CO2 TYPE FIRE EXTINGUISHER		
(F.E) P	4Kg DRY POWDER TYPE FIRE EXTINGUISHER		
	PUMP		
	150mm WATER ALARM GONG		
AFA	ADDRESSABLE TYPE FIRE ALARM PANEL		
	PUMP CONTROL PANEL		
$\bigcirc \bigcirc$	EMERGENCY LIGHT		
EXIT	EXIT SIGN		

DRAWING LIST:

DRAWING NO.	REVISION	DRAWING TITLE
FS-01	A	F.S. NOTES., BLOCK PLAN, LEGEND, ABBREVIATION, DRAWING LIST, PIPE MATERIAL SCHEDULE
FS-02	А	FIRE SERVICES INSTALLATION LAYOUT PLAN

ABBREVIATION PRINKLER OSE REEL IRE EXTINGUISHER ARBON DIOXIDE OSS PREVENTION COUNCIL IRE SERVICES INSTALLATION IGH LEVEL 11D LEVEL OW LEVEL ROM ABOVE ROM BELOW O ABOVE O BELOW INDERGROUND

IRE SERVICES

Α	P.D SUBMISSION	22-01-2024	WC
REV	DESRIPTION	DATE	BY
FSI C	ONTRACTOR		

East Power Engineering Limited Flat A, 7/F., Hop Shing Commercial Building 41 Chi Kiang Street, Tokwawan, Kowloon Fax. : 2394–3772 Tel. : 2397–3238

PROPOSED TEMPORARY WAREHOUSE (EXCLUDIN DANGEROUS GOODS GODOWN) WITH ANCILLARY FACILITIE FOR A PERIOD OF 3 YEARS FIRE SERVICES INSTALLATION WORK AT IN D.D. 117 AND ADJOINING GOVERNMENT LAND, TAI TONG, YUEN LONG, NE

DRAWING TITLE .S. NOTES, ABBREVIATION, DRAWING LIST, PIPE

MATERIAL SCHEDULE

	INITIAL	DESIGNATION	DATE	
DRAWN BY	CAD	CAD	27-2-2022	
DESIGN BY	WINKLE	S.ENG	27-2-2022	
CHECK BY	СМ	PM	27-2-2022	
APPROVED BY	-	-	-	
PROJECT NO.	_			
PAPER SIZE	A1	PLOT SCALE	1 : 1	
DRAWING NO.				
EP-21002-FS01				
SCALE	N. T. S.	REVISION	A	

COVERED AREA B2 BЗ B4 B5 B6 Β7 B8

А	P.D	SUBMISSION		22-01-	-2024	WC		
REV FSL CON		SCRIPTION		DA	TE	BY		
Eas	t Po	ower En	ginee	ring		nited		
		Flat A, 7/F.,	Hop Shir	ng Comr	mercial	Building		
	$\bigcirc)$	Fax. : $2394-$	-3772 Te	I. : 239	97-323	8		
PROJEC	T							
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DRAWIN	G TITLE							
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DRAWING FIRE S DRAWN DESIGNE	G TITLE ERVICE BY ED BY	INSTALLATION INITIAL CAD JACKIE	N LAYOU DESIGN CA S.E	t plan Nation Ng	07-1 07-1	DATE 2-2020 2-2020		
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