

LEGEND

- CP Proposed CatchPit
- Proposed UC with Cast Iron Cover (size as shown)
- ⊠ Proposed Connection Catchpit to ex. 1000 dia pipe
- ≡ Existing 1000 dia pipe / Existing 2m(W)x 1.67m(D) Open Channel
- ① Photo Viewpot

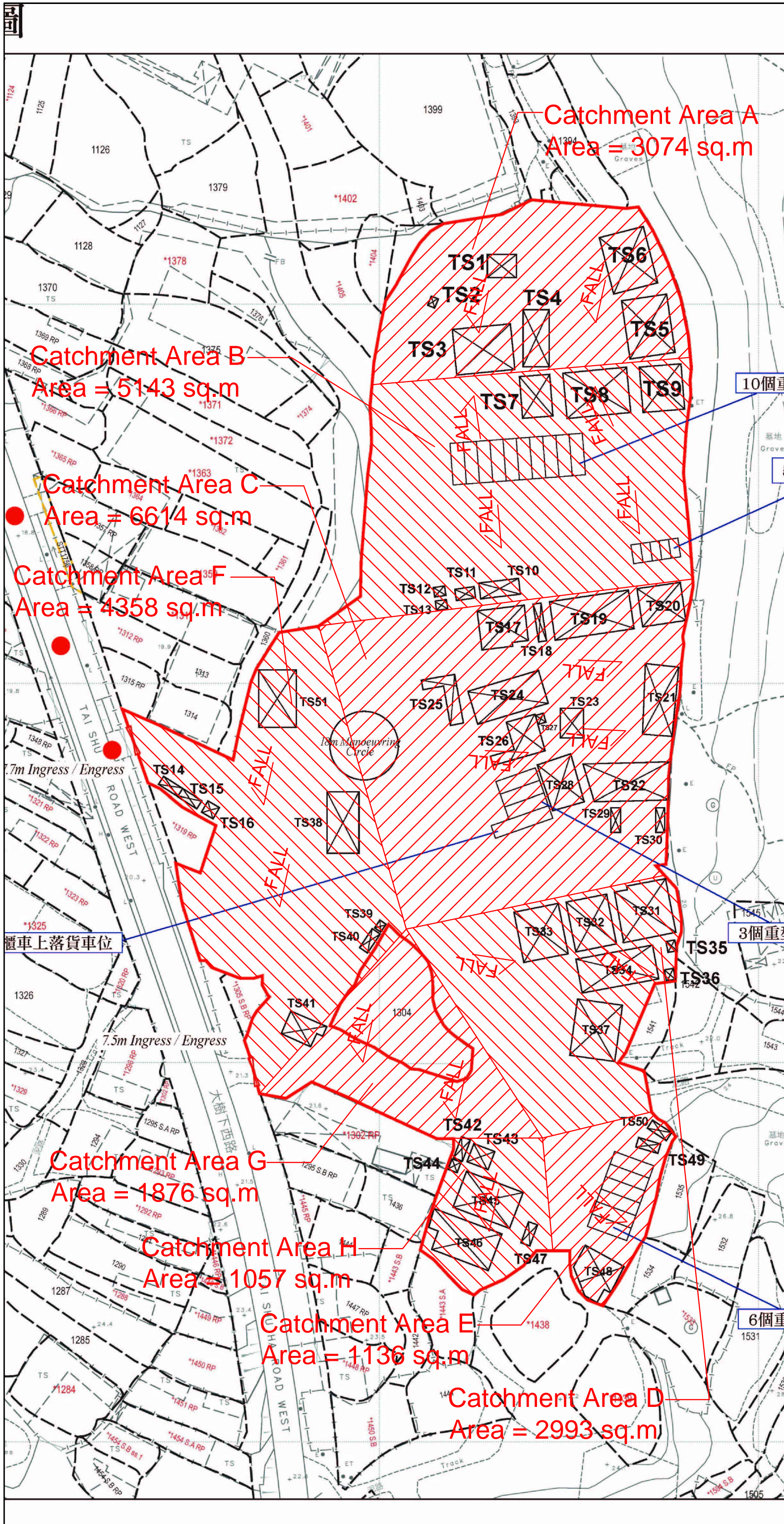
Company:
 正宏工程顧問公司
 Ching Wan Engineering
 Consultants Company

PROJECT:
**Proposed Temporary
 Vehicle Repair
 Workshop and Open
 Storage of Building
 Materials/Prefabricated
 Components,
 Recycling Materials,
 Construction
 Machinery and Used
 Electrical/Electronic
 Appliances with
 Ancillary Workshop
 and Packaging
 Activities for a Period
 of 3 Years at Various
 Lots in D.D. 118, Tai
 Tong, Yuen Long,**
 Application No.:

TITLE:
Drainage Layout Plan

File:	DWG NO.
Scale:	D01

Date:
 19-12-2024



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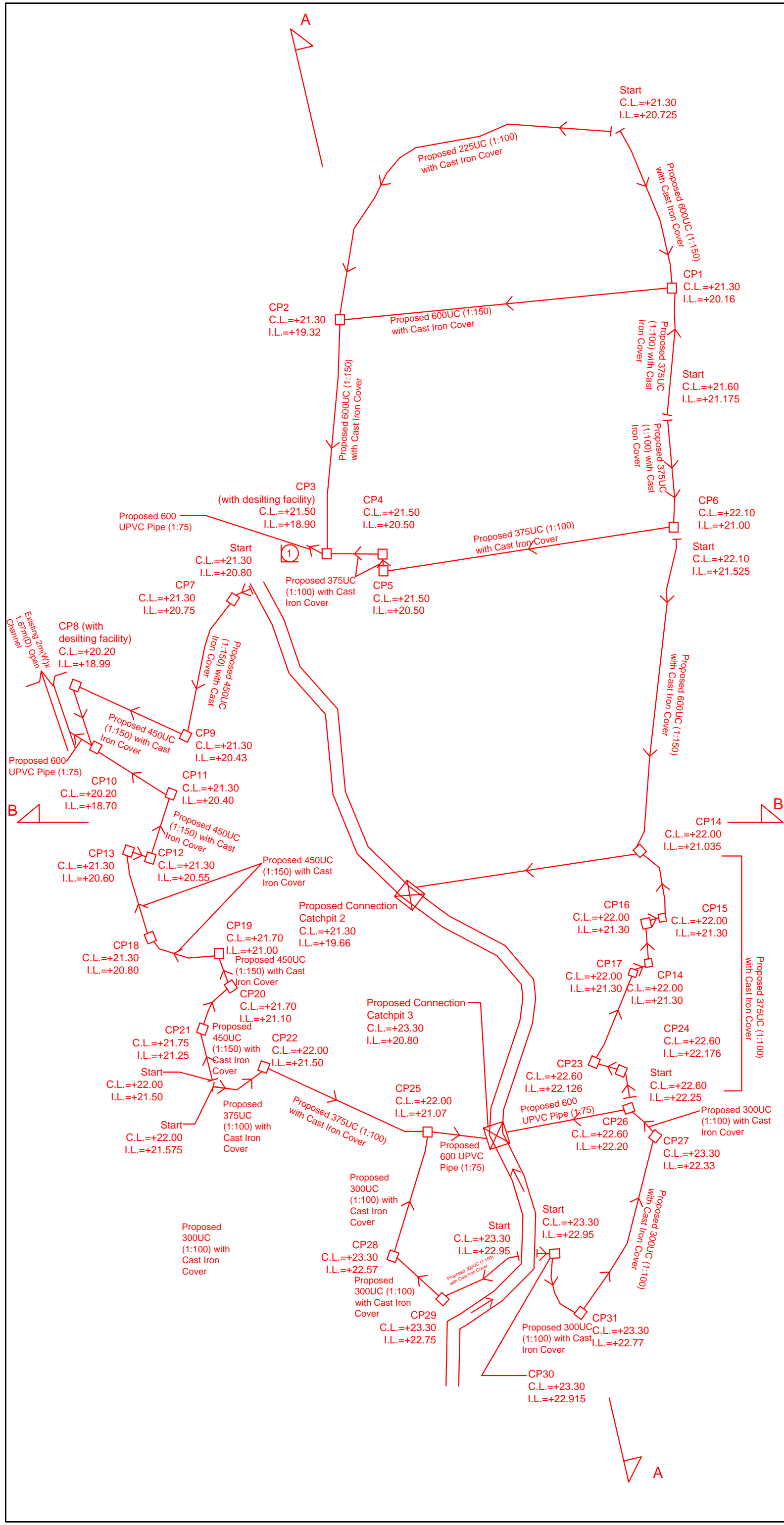
Company:
正宏工程顧問公司
Ching Wan Engineering Consultants Company

PROJECT:
Proposed Temporary Vehicle Repair Workshop and Open Storage of Building Materials/Prefabricated Components, Recycling Materials, Construction Machinery and Used Electrical/Electronic Appliances with Ancillary Workshop and Packaging Activities for a Period of 3 Years at Various Lots in D.D. 118, Tai Tong, Yuen Long,

Application No.:

TITLE:
Catchment Area Plan

File:	DWG NO.
Scale:	D02
Date:	19-12-2024



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




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 Lots in D.D. 118, Tai
 Tong, Yuen Long,**

Application No.:

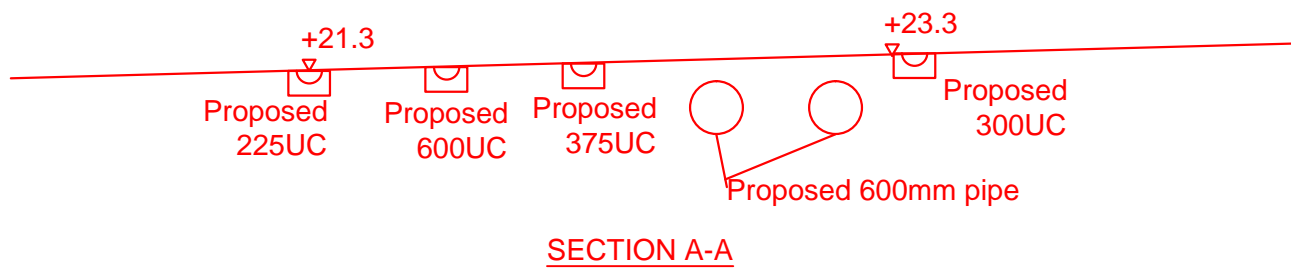
TITLE:
**Drainage Layout Plan
 (without base map)**

File:	DWG NO.
Scale:	D03
Date: 19-12-2024	

LEGEND

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-  Photo Viewpot

THE SITE



Company:

正宏工程顧問公司
Ching Wan Engineering
Consultants Company

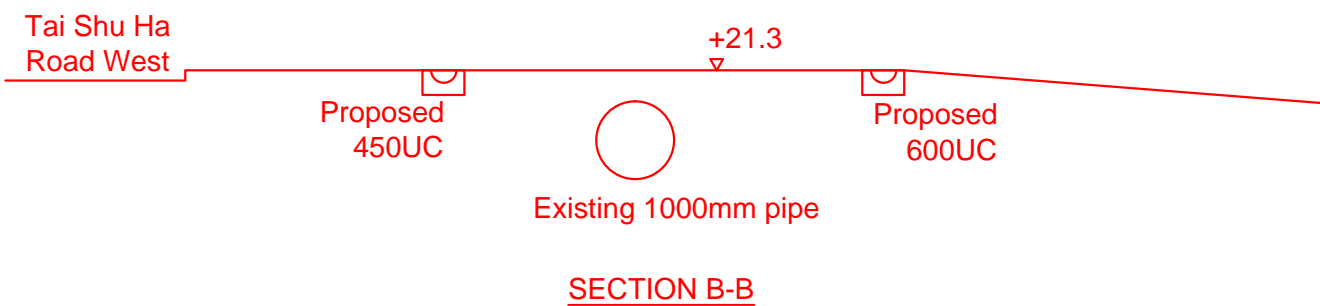
PROJECT:
**Proposed Temporary
 Vehicle Repair
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 Electrical/Electronic
 Appliances with
 Ancillary Workshop
 and Packaging
 Activities for a Period
 of 3 Years at Various
 Lots in D.D. 118, Tai
 Tong, Yuen Long,**

Application No.:

TITLE:

SECTIONS

THE SITE



File:

DWG NO.

Scale:

D05

Date:

19-12-2024



PHOTO 1

CATCHMENT A

Catchment Area = 3074 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 3074 \quad \text{m}^2$$
$$= 0.003074 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.003074 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$

$$= 0.226 \quad \text{m}^3/\text{sec}$$
$$= \underline{13531} \quad \text{lit/min}$$

CATCHMENT B

Catchment Area = 5143 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 5143 \quad \text{m}^2$$
$$= 0.005143 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.005143 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$

$$= 0.377 \quad \text{m}^3/\text{sec}$$
$$= \underline{22638} \quad \text{lit/min}$$

CATCHMENT C

Catchment Area = 6614 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 6614 \quad \text{m}^2$$
$$= 0.006614 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.006614 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$

$$= 0.485 \quad \text{m}^3/\text{sec}$$
$$= \underline{29113} \quad \text{lit/min}$$

CATCHMENT D

Catchment Area = 2993 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 2993 \quad \text{m}^2 \\ = 0.002993 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.002993 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$
 $= 0.220 \quad \text{m}^3/\text{sec}$
 $= \underline{13174} \quad \text{lit/min}$

CATCHMENT E

Catchment Area = 1136 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 1136 \quad \text{m}^2 \\ = 0.001136 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.001136 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$
 $= 0.083 \quad \text{m}^3/\text{sec}$
 $= \underline{5000} \quad \text{lit/min}$

CATCHMENT F

Catchment Area = 4358 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 4358 \quad \text{m}^2 \\ = 0.004358 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.004358 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$
 $= 0.320 \quad \text{m}^3/\text{sec}$
 $= \underline{19182} \quad \text{lit/min}$

CATCHMENT G

Catchment Area = 1876 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 1876 \quad \text{m}^2 \\ = 0.001876 \quad \text{km}^2$$

take $i = 250 \quad \text{mm/hr}$

Therefore, $Q = 0.278 * 0.95 * 250 * 0.001876 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$
 $= 0.138 \quad \text{m}^3/\text{sec}$
 $= \underline{8258} \quad \text{lit/min}$

CATCHMENT H

Catchment Area = 1057 m²

Calculation of Runoff from the Proposed Development,

$$Q = 0.278 C i A$$

$$C = 0.95 \quad (\text{P.42 of Stormwater Drainage Manual})$$

$$A = 1057 \quad \text{m}^2$$
$$= 0.001057 \quad \text{km}^2$$

$$\text{take } i = 250 \quad \text{mm/hr}$$

$$\text{Therefore, } Q = 0.278 * 0.95 * 250 * 0.001057 / 0.9 \quad (\text{0.9 factor according to Section 9.3 of SDM})$$
$$= 0.078 \quad \text{m}^3/\text{sec}$$
$$= \underline{4653} \quad \text{lit/min}$$

Design Drain in Catchment Area A

$$\text{Design Q} = \text{Area A} + 0.5 \text{ Area B}$$
$$= 13531 + 11318.89$$
$$= 24850 \quad \text{lit/min}$$

Provide 600UC (1:150) is OK

Design Drain in Catchment Area B

$$\text{Design Q} = 0.5 \text{ Area B}$$
$$= 11319 \quad \text{lit/min}$$

Provide 375UC (1:100) is OK

Design Drain in Catchment Area C

$$\text{Design Q} = 29113 \quad \text{lit/min}$$

Provide 600UC (1:150) is OK

Design Drain in Catchment Area D

$$\text{Design Q} = 13174 \quad \text{lit/min}$$

Provide 375UC (1:100) is OK

Design Drain in Catchment Area E

$$\text{Design Q} = 5000 \quad \text{lit/min}$$

Provide 375UC (1:100) is OK

Design Drain in Catchment Area F

$$\text{Design Q} = 19182 \quad \text{lit/min}$$

Provide 450UC (1:150) is OK

Design Drain in Catchment Area G

$$\text{Design Q} = 8258 \quad \text{lit/min}$$

Provide 375UC (1:100) is OK

Design Drain in Catchment Area H

$$\text{Design Q} = 4653 \quad \text{lit/min}$$

Provide 300UC (1:100) is OK

Calculation Maximum Capacity of Proposed 1000mm dia. Underground pipe.

Manning Equation $V = R^{2/3} \cdot S_f^{0.5} / n$

where $R = \frac{\pi r^2}{2 \pi r} = \frac{r}{2} = 0.25$ m
 dia = 1000 mm
 r = 0.5 m

$n = 0.012$ s/m^{1/3} (Talbe 13 of Stormwater Drainage Manual)

1/ 100 $S_f = 0.01$

Therefore, $V = \frac{0.25^{2/3} \cdot 0.01^{0.5}}{0.012} = 3.31$ m/sec

Maximum Capacity (Q_{max}) = $0.8 \cdot V \cdot A$ (0.8 factor for sedimentation)
 = $0.8 \cdot 3.31 \cdot \pi r^2$
 = 2.08 m³/sec
 = 124674 lit/min
 > 60197 lit/min (Area C+D+E+G+H)

Existing 1000 dia. pipe is OK

Calculation Maximum Capacity of Proposed 600dia(CP3 outfall)

Manning Equation $V = R^{2/3} \cdot S_f^{0.5} / n$

where $R = \frac{\pi r^2}{2 \pi r} = \frac{r}{2} = 0.15$ m
 dia = 600 mm
 r = 0.3 m

$n = 0.012$ s/m^{1/3} (Talbe 13 of Stormwater Drainage Manual)

1/ 75 $S_f = 0.0133333$

Therefore, $V = \frac{0.15^{2/3} \cdot 0.0133333^{0.5}}{0.012} = 2.72$ m/sec

Maximum Capacity (Q_{max}) = $0.8 \cdot V \cdot A$ (0.8 factor for sedimentation)
 = $0.8 \cdot 2.72 \cdot \pi r^2$
 = 0.61 m³/sec
 1 nos of pipe = 0.61 m³/sec
 = 36868 lit/min
 > 36168 lit/min

Proposed 600 dia pipe (1:75) is OK

Calculation Maximum Capacity of Existing 2000dia 1670(D) Open Channel

Manning Equation $V = R^{2/3} \cdot S_f^{0.5} / n$

where $R = \frac{(2 \cdot 1.67)}{(2 + 2 \cdot 1.67)} = 0.625$ m
 W = 2000 mm
 D = 1670 mm

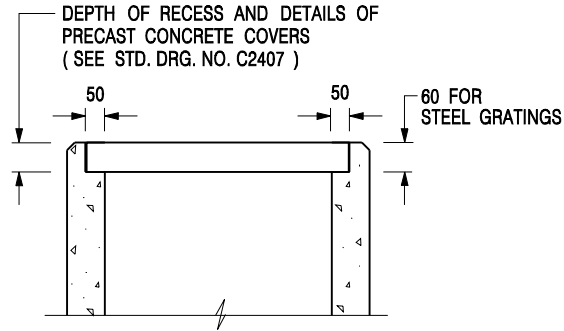
$n = 0.018$ s/m^{1/3} (Talbe 13 of Stormwater Drainage Manual)

1/ 50 $S_f = 0.02$

Therefore, $V = \frac{0.625^{2/3} \cdot 0.02^{0.5}}{0.018} = 5.75$ m/sec

Maximum Capacity (Q_{max}) = $0.8 \cdot V \cdot A$ (0.8 factor for sedimentation)
 = $0.8 \cdot 5.75 \cdot W \cdot D$
 = 15.35 m³/sec
 = 921228 lit/min
 > 19182 lit/min

Existing 2000dia 1670(D) Open Channel is OK




**ALTERNATIVE TOP SECTION
FOR PRECAST CONCRETE COVERS / GRATINGS**

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'G' ON STD. DRG. NO. C2405; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'F' ON STD. DRG. NO. C2405.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

**CATCHPIT WITH TRAP
(SHEET 2 OF 2)**

 CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT	
SCALE 1 : 20	DRAWING NO.
DATE JAN 1991	C2406 /2

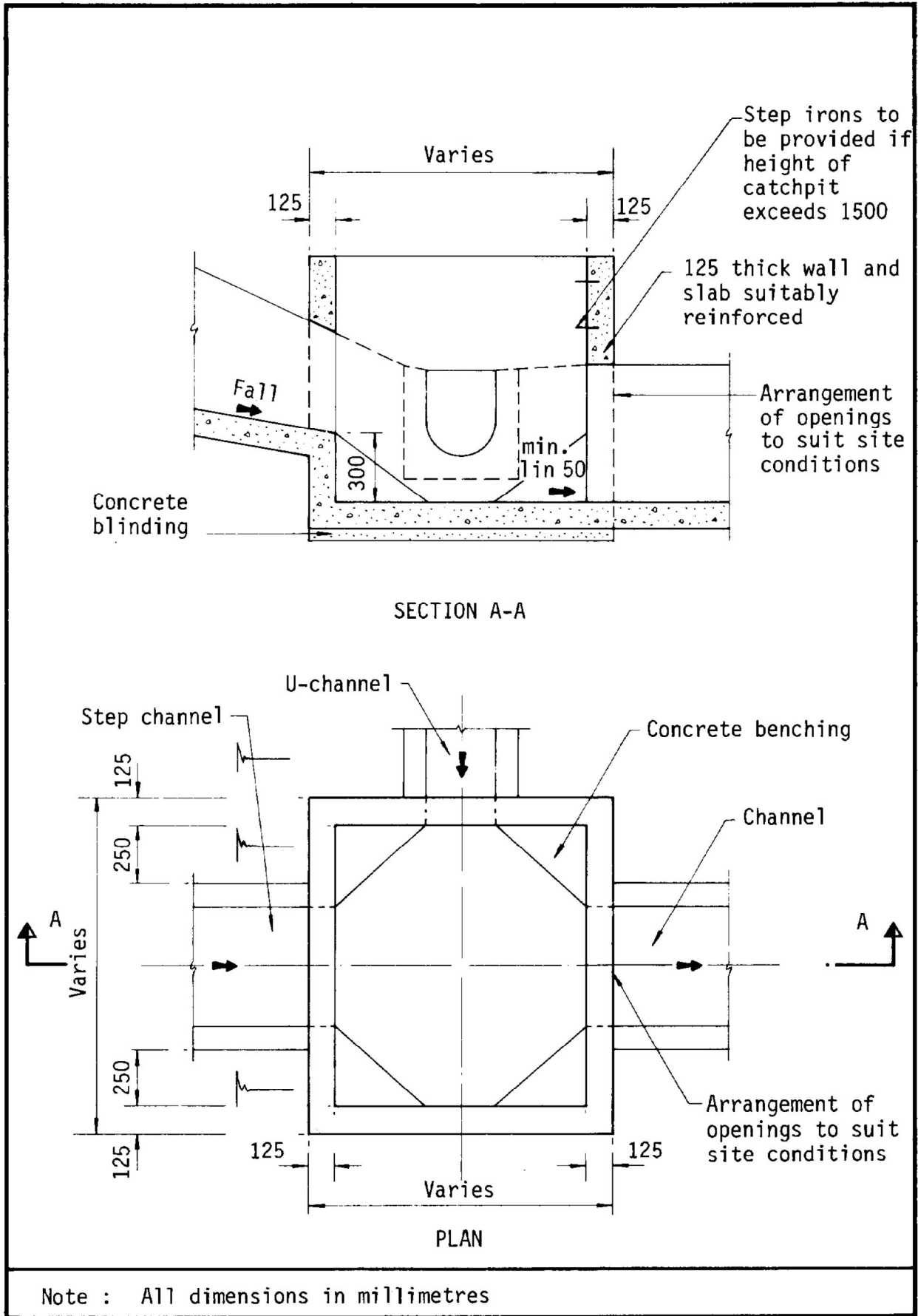
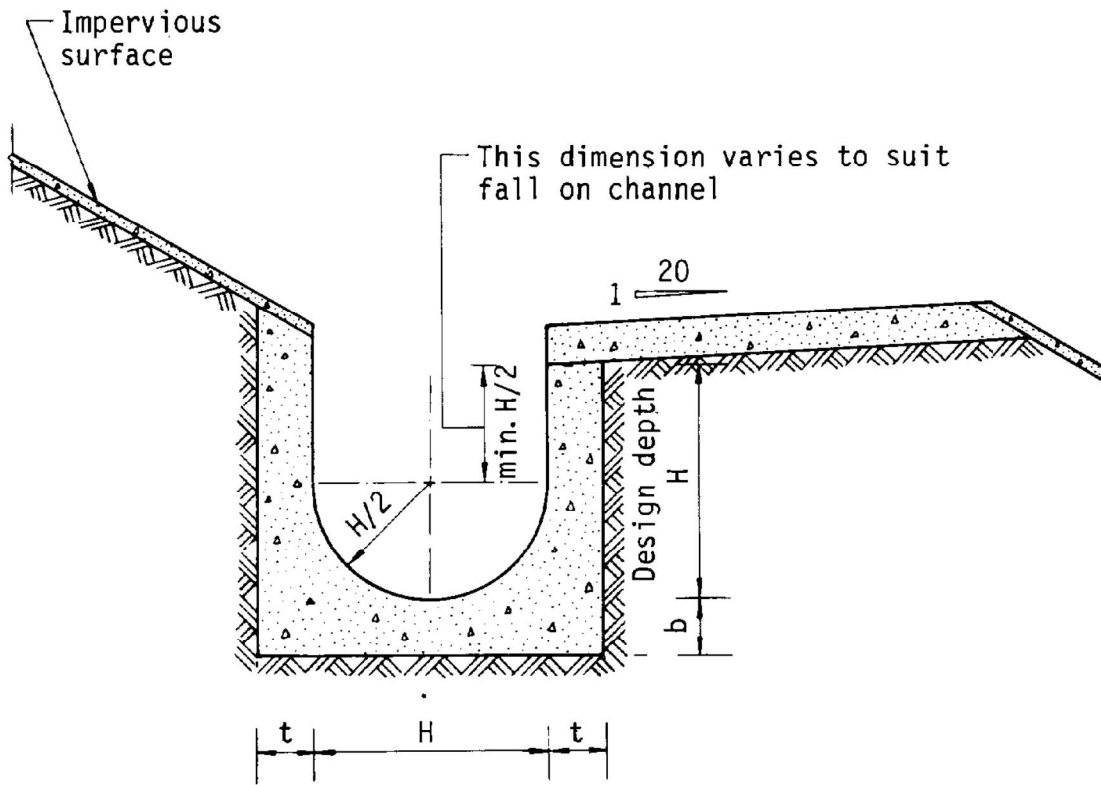


Figure 8.10 - Typical Details of Catchpits



Dimensions of U - channel

Nominal size of channel H (mm)	Thickness t (mm)	Thickness b (mm)
225 to 600	150	150
675 to 1200	175	225

Figure 8.11 - Typical U-channel Details