
Appendix E
Traffic Impact Assessment

**Section 12A Planning Application for Proposed Amendments to the
Tung Chung Valley Outline Zoning Plan to Rezone “Residential
(Group C)2” Zone to “Residential (Group B)” Zone in Support of
Private Residential Development at Various Lots in D.D. 1 Tung
Chung and Adjoining Government Land, Tung Chung, Lantau
Island**

Traffic Impact Assessment Report

December 2024

AECOM

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1 INTRODUCTION

1.1 Background

- 1.1.1 The Application Site covers various lots and adjacent Government land in DD 1 TC and adjoining government land, Tung Chung Valley, New Territories. The Application Site is located to the west of the junction of Yu Tung Road / Chung Mun Road with an area of about 33,808m² (**Figure 1.1**).
- 1.1.2 The Application Site is zoned "Residential (Group C)2" under the current Approved Tung Chung Valley Outline Zoning Plan (OZP) no. S/I-TCV/2. In statutory planning terms, residential development with a maximum plot ratio of 1.0 and building height of 20 mPD is permitted as of right within the Site (hereinafter referred to as "Conforming Scheme").
- 1.1.3 The Application Site is near the planned Tung Chung West (TCW) MTR Station and Tung Chung West PTI. Under the current application, the Applicant proposes to rezone the Site to "Residential (Group B)" with a domestic ratio of 2.1 to provide about 1,783 nos. of residential units with an average flat size of about 39.8 m² (hereinafter referred to as "Proposed Scheme"). Also, a covered private transport lay-by and some local commercial facilities with a non-domestic plot ratio of 0.22 are proposed for the Site.
- 1.1.4 AECOM Asia Co. Ltd. was commissioned by the Applicant as the Traffic Consultant to prepare a TIA report in support of the Section 12A planning application.

1.2 Objectives

- 1.2.1 The main objectives of this report are as follows-
- Outline the proposed development parameters and internal transport facilities provision, vehicular access arrangement and pedestrian arrangement;
 - Review the current traffic condition in the vicinity of the Application Site;
 - Estimate the potential traffic generations and attractions of the Application Site;
 - Produce traffic forecasts on the surrounding road network at the adopted design year;
 - Assess traffic impact on the surrounding road network induced from the Application Site; and
 - Develop traffic improvement proposal(s), if necessary.

1.3 Structure of TIA Report

- 1.3.1 Following this introductory chapter, the TIA is structured as follows:
- **Chapter 2:** Proposed Development, describes the development schedule of the Application Site and its internal traffic facilities provisions, vehicular access arrangement and pedestrian arrangement.
 - **Chapter 3:** Existing Traffic Condition, reviews the current traffic conditions in the vicinity. The future road network serving Tung Chung West would also be discussed.

- **Chapter 4:** Traffic Forecasting describes the traffic forecasting methodology and presents the forecasted traffic flows in design year.
- **Chapter 5:** Traffic Impact Assessment, assesses the traffic impact induced on the surrounding road network and recommends improvement schemes, if necessary; and
- **Chapter 6:** Summary and Conclusion, summarizes the findings of the study and presents the conclusion of this TIA.

2 APPLICATION SITE

2.1 Development Schedule

2.1.1 **Table 2.1** summarizes the development schedule of the Proposed Scheme. The proposed indicative Master Layout Plan (MLP) under the current application is illustrated in **Figure 2.1** for reference.

Table 2.1 Indicative Development Schedule of the Application Site

	Proposed Development
Site Area	About 33,808m ²
GFA	About 78,292m ²
- Domestic Portion	About 70,997m ²
- Non-Domestic Portion	About 7,295m ²
Plot Ratio	Not more than 2.32
- Domestic Portion	Not more than 2.10
- Non-Domestic Portion	Not more than 0.22
Maximum Domestic Site Coverage	Not more than 33.3%
Maximum Building Height (main roof level)	
- Area (a)	Not more than 50mPD
- Area (b)	Not more than 80mPD
- Area (c)	Not more than 100mPD
No. of Storeys ⁽¹⁾	6 to 22 storeys above a 1 to 3 storey(s) podium
Domestic Portion	
Domestic GFA	About 70,997m ²
Domestic Plot Ratio	Not more than 2.10
No. of Blocks	9
No. of Units	About 1,783
Average Flat Size	About 39.8m ²
Anticipated Population ⁽²⁾	About 5,171
Private Open Space ⁽³⁾	Not less than 5,171m ²
Non-Domestic Portion – Commercial and Covered Private Transport Lay-by	
Commercial GFA ⁽⁴⁾	About 4,145m ²
Covered Private Transport Lay-by GFA	About 3,150m ²
Maximum Building Height	Not more than 19mPD
Residents' Clubhouses ⁽⁵⁾	
Clubhouse GFA	About 3,000m ²
No. of Storeys	1

Remarks:

- (1) Excluding basement floor(s) for car park and transfer plate; including above ground floors for commercial / covered private transport lay-by / ramp / E&M facilities / clubhouse / residential lobby / residential floors. The indicative typical floor-to-floor height is 3.25m which is subject to refinement at detailed design stage.
- (2) Adopting a person per flat ratio of 2.9 as per Tertiary Planning Units 950 – 951 under 2021 Population Census covering the Application Site
- (3) Not less than 1m² per person in accordance with Hong Kong Planning Standards and Guidelines (HKPSG) requirement
- (4) Commercial GFA refers to commercial uses ('Eating Place' and 'Shop and Services'), 'School' (kindergarten, nursery, language, computer, commercial and tutorial schools, art school, ballet and other types of schools providing interest / hobby related courses), 'Place of Entertainment' and 'Place of Recreation, Sports or Culture'. A kindergarten with a GFA of about 930m² is proposed.
- (5) Residents' clubhouse GFA is based on the maximum GFA concession for clubhouse according to Buildings Department's Practice Note APP-104 and shall be disregarded from the total GFA calculation

2.2 Access Arrangement and Public Transport Facilities

2.2.1 Refer to the **Section 3.5 Future Road Network**, a four-arm roundabout junction intersecting Yu Tung Road / Chung Mun Road (J5) is planned. The roundabout will include a vehicular access road to serve the Application Site. The vehicular access for the Application Site is shown in **Figure 2.1**. With about 20m long queuing space between the ingress/egress point of the proposed development and the main road, it would be sufficient to accommodate an average traffic demand of about one vehicle per minute during peak hours (Table 4.5).

2.3 Public Transport Facilities

2.3.1 As mentioned in **Table 2.1** and refer to **Figure 2.1 – Indicative Master Layout Plan**, even there will be a planned Tung Chung West MTR Station and Tung Chung West PTI, a covered private transport lay-by is proposed within the Application Site to supplement the provision of transport services for the future residents and visitors of the Application Site. The proposed covered private transport lay-by will be located to the southeast of the Application Site. It will consist of 3 double-width bays of 7.3m in width and 42m in length. All the 3 bays have been designed to allow manoeuvring of 12.8m buses. The management and maintenance responsibility of the covered private transport lay-by would be taken up the Applicant.

2.3.2 As mentioned in **Table 2.1**, the population of the Application Site will be 5,171. Reference was made to the published "Travel Characteristics Survey (TCS) 2011 Final Report". According to the TCS Final Report, the daily mechanised trip rate per population is 1.83 trips and the morning peak hour accounted for about 12% of the daily trips. The percentage of using public transport is about 73% of the total trips. By assuming a directional split of 90:10 in outbound/inbound direction, the estimated additional public transport demand in outbound direction in AM peak hour is about 746 passengers / hour (i.e. $5,171 \times 1.83 \times 0.12 \times 0.73 \times 0.9$).

2.3.3 Taking into consideration (1) some of the existing bus routes at Yu Tung Road and Tung Chung Road are running to / from Tung Chung Station; and (2) the Application Site is located in proximity of North Lantau Expressway which is the major strategic route to all destinations in Hong Kong, the possible bus routes are proposed as a circular bus route running between Tung Chung Station and the Application Site and a cross-district bus route running between Urban District (Kowloon or Hong Kong Island) and the Application Site.

2.3.4 With reference to the place of work and study in the Application Site and the modal split of the working population and students at Tung Chung District in 2021 by-census published by Census and Statistics Department, it is estimated that about 27% of the future residents would work/study in Tung Chung while 73% would work/study in other districts. For people who would go to other districts for work/study, around 57% of them would take MTR. Hence, it is assumed that 70% of the future residents (i.e. 522 passengers / hr) would take the local route for local trips/bus-rail interchange and another 30% of the future residents (i.e. 224 passengers / hr) would take the cross-district bus route directly.

2.3.5 To cater for the estimated public transport demand from the Application Site, the following public transport services are proposed as listed in **Table 2.2**.

Table 2.2 Proposed Public Transport Services

Public Transport Services	No. of Public Transport Route(s)	Frequency (min.)	Estimated Service Capacity (pax/hr) ⁽¹⁾
Local Bus Route (Circular)	1	10	522
Cross-District Bus Route	1	20	224

Note: The capacity of bus is assumed to be 100 passengers/bus.

- 2.3.6 One Bus Route (Circular): Running between Tung Chung Station and the Application Site, with terminating point at the Application Site and a bus stop at Tat Tung Road adjacent to Tung Chung Station for passenger boarding / alighting. The estimated round trip journey distance and journey time are about 5km and 10 minutes respectively. The round trip time is estimated about 15 minutes (5km / 30kph + 5 minutes boarding/alighting time). For proposed headway of 10 minutes, in total 2 buses would be required (i.e. 15/10) during peak hour. For non-peak period, it is assumed that the proposed circular bus service would be operated in headway of 20 minutes, in total 1 bus would be required (i.e. 15/20) during the non-peak hour. The proposed routing is shown in **Figure 2.2**.
- 2.3.7 One Cross-District Bus Route: running between the Application Site and Urban Districts (Kowloon or Hong Kong Island). Based on capacity of 100 persons/bus, this would be equivalent to 3 bus trips/hour. Hence 3 bus trips of cross-district bus routes to/from urban areas (such as Kowloon, Hong Kong Island) are proposed during the AM / PM peak hours.
- 2.3.8 With the proposed public transport services, the overall provided capacity is around 900 passengers / hour, which shall be sufficient to accommodate the estimated public transport demand.
- 2.3.9 Nonetheless, the proposed public transport services and associated operation arrangement (i.e. origin/destination, frequency, routings etc.) are subject to future reviewed by Transport Department, bus operators in the detailed design stage.
- 2.3.10 Subject to the future planning of public transport services, the provisions of covered private transport lay-by within the Application Site will allow adequate facility to serve the future public transport demand generated by the proposed development
- 2.3.11 As mentioned in **Table 2.1**, a covered private transport lay-by is proposed within the Application Site to cater for the provision of public transport services for the future residents and visitors of the Application Site. One bay is proposed for general pick-up / drop-off of passengers, including taxi. One bay is proposed for the circular bus route (between Tung Chung Station and the Application Site) while the remaining bay is proposed for the cross-district bus route during the AM/PM peak hours. The covered private transport laybys will be accessible to the nearby community during the operational hours of the two proposed bus routes.
- 2.3.12 The ingress/egress of the Covered Private Transport Lay-by are proposed to be located at Chung Mun Road as shown in **Figure 2.1**. To provide a quick and direct routing of the proposed covered transport lay-by leading to and from Yu Tung Road and Chung Mun Road, the ingress allowing the vehicles to turn right or left into the lay-by and left-out from the lay-by is proposed. The traffic impact of the proposed vehicular arrangement is considered insignificant, taking into account of the low Volume to Capacity ratio of Chung Mun Road (**Table 5.3**) and low usage of planned bus lay-by located at Chung Mun Road (11-16 buses during peak hours, **Table 3.4**) to the north

of the proposed ingress point. The indicative layout of the Covered Private Transport Lay-by together with the swept path analysis is shown in **Annex A**.

2.4 Pedestrian Facilities

2.4.1 Refer to **Figure 2.1 – Indicative Mater Layout Plan**, a possible pedestrian footbridge linking the podium of the Proposed Development with the adjacent Mun Tung Estate, the planned commercial sites in Planning Areas 38A and 38B and the planned TCW MTR Station is proposed. The construction of the possible pedestrian footbridge will be taken up by the Applicant. The management and maintenance responsibilities and allow of 24-hour public use with disabled facilities of the possible pedestrian footbridge will be subject to further review and liaison with relevant government departments at the subsequent stage.

2.5 Internal Parking and Servicing Provisions

2.5.1 The parking and loading/unloading facilities of the Application Site would be provided in accordance with the requirements of the Hong Kong Planning Standards and Guidelines (HKPSG). The respective requirements are summarized in **Table 2.3**.

Table 2.3 HKPSG Parking and Servicing Facilities Provisions Requirement

	HKPSG Requirements
Private Car Parking Spaces	
Private Housing – Private Car Parking Spaces	GPS x R1 x R2 x R3 For flat size ≤ 40m ² = 1 space per 4-7 flats x 0.5 x 0.75 x 1.00 = 0.375 spaces per 4-7 flats For flat size : 40m ² < flat size ≤70m ² , = 1 space per 4-7 flats x 1.2 x 0.75 x 1.00 = 0.9 spaces per 4-7 flats
Private Housing – Visitor Private Car Parking Spaces	5 spaces per block of more than 75 residential units
Commercial (Retail) – Private Car Parking spaces	1 car space per 150 – 300m ² GFA
Commercial (Kindergarten) – Private Car Parking Spaces	0 to 1 car parking space per 4 to 6 classrooms
Motorcycle Parking Spaces	
Private Housing – Motorcycle Parking Spaces	1 space per 100-150 flats
Retail – Motorcycle Parking Spaces	5% - 10% total provision for private cars
Bicycle Parking Spaces	
Private Housing – Bicycle Parking Spaces	1 space for every 15 flats with flat size smaller than 70m ²
Loading and Unloading Bay	

HKPSG Requirements	
Private Housing – Loading and Unloading Bay	1 space for every 800 flats subject to min. 1 bay per block
Retail – Loading and Unloading Bay	1 loading/ unloading bay for goods vehicles for every 800 to 1200m ² or part thereof, GFA
Lay-bys	
Kindergarten – Taxi / private cars lay-by	1 lay-by for taxis and private cars for every 5 to 8 classrooms
Kindergarten – Small Coaches lay-by	A minimum of 2 lay-bys for school buses or 5 lay-bys for small coaches (each 3m x 7m)

2.5.2 In light of the HKPSG requirements given in **Table 2.3**, the proposed provision for the Application Site according to the development schedules are summarized in **Table 2.4**. Taken into consideration the proximity to public transport services, availability of public car parking space, traffic conditions and the illegal parking condition in the vicinity, it is proposed to adopt a GPS of 6 for calculating the residential carparking provision according to HKPSG. The assessment the GPS under HKPSG adopted for the Application Site is shown in **Annex B**. The Application Site is within 500m radius of new Tung Chung West MTR Station to justify the Accessibility Adjustment Ratio of 0.85 is shown in **Figure 2.3** of the Planning Statement and extracted in **Annex C**. For retail private car parking space, the mid-range provision requirement has been adopted. For other facilities, higher end of provision has been adopted.

Table 2.4 Required and Proposed Parking and Servicing Facilities Provisions

	Parameters	Required Provision		Proposed Provision
Private Housing – Private Car Parking Spaces	934 Flats	FS ≤ 40m ²	51-88	187⁽¹⁾ spaces
	849 Flats	40m ² <FS≤70m ²	110-192	
	Total: 1,783 Flats	161-280 spaces		
Private Housing – Visitor Private Car Parking Spaces	9 Towers	45 spaces		45 ⁽²⁾ spaces
Retail – Private Car Parking Spaces	3,215 m ²	11 – 22 spaces		16 ⁽³⁾ spaces
Kindergarten – Private Car Parking Spaces	6 Classrooms	0 – 2 spaces		2 spaces
Private Car Parking Spaces				250 spaces
Private Housing – Motorcycle Parking Spaces	1,783 Flats	12 - 18 spaces		18 spaces
Retail – Motorcycle Parking Spaces	5% - 10% total provision for private cars	1 - 2 spaces		2 spaces
Motorcycle Parking Spaces				20 spaces

	Parameters	Required Provision	Proposed Provision
Private Housing – Bicycle Parking Spaces	1,783 Flats ⁽⁴⁾	119 spaces	119 spaces
Private Housing – Loading and Unloading Bay	9 Towers	9 bays	9 bays
Retail – Loading and Unloading Bay	3,215 m ²	3-4 bays	4 bays
Loading and Unloading Bays			13 bays
Kindergarten – Taxi / private cars lay-by	6 Classrooms	2 lay-bys for taxi / private cars	2 lay-bys
Kindergarten – Small Coaches lay-by	-	5 lay-bys for small coaches (each 3m x 7m)	5 lay-bys

Notes: Round up figures adopted.

- (1) GPS of 1 space per 6 flats is adopted.
- (2) All the towers have more than 75 units per block. Hence 5 visitor car parking spaces per block would be provided.
- (3) 1 car space per 200 m² GFA is adopted.
- (4) No. of flats with flat size smaller than 70 m².

3 EXISTING TRAFFIC CONDITION AND FUTURE ROAD NETWORK

3.1 Existing Traffic Arrangement

- 3.1.1 The Application Site is located to the west of the junction of Yu Tung Road / Chung Mun Road.
- 3.1.2 Yu Tung Road is a dual two district distributor road in east-west direction. It connects to Chung Mun Road at its western end and Tung Chung Eastern Interchange at its eastern end.
- 3.1.3 Tung Chung Eastern Interchange is a roundabout junction of Yu Tung Road / Yi Tung Road / North Lantau Highway. It serves as the major roundabout junction in Tung Chung area. It provides connection to the strategic highways - North Lantau Highway for all other destinations in Hong Kong.

3.2 Traffic Survey

- 3.2.1 A total of 6 existing critical junctions and 3 critical road links have been identified for assessment and listed in **Table 3.1** and shown in **Figure 3.1**. Existing layout of the critical junctions are presented in **Figure 3.2** to **Figure 3.7**.

Table 3.1 Surveyed Key Junctions for Assessment

Ref.	Junction	Type	Fig. No.
Junctions			
J1	Yu Tung Road / Yi Tung Road / North Lantau Highway	Roundabout	3.2
J2	Yu Tung Road / Shun Tung Road	Signal	3.3
J3	Yu Tung Road / Chung Yan Road	Signal	3.4
J4	Tung Chung Road / Chung Yan Road	Priority	3.5
J5	Yu Tung Road / Chung Mun Road	Priority	3.6
J6	Tung Chung Road / Shek Mun Kap Road	Roundabout	3.7
Road Links			
L1	Yu Tung Road (between J2 and J3)		
L2	Yu Tung Road (between J3 and J5)		
L3	Chung Mun Road (to the south of J5)		

- 3.2.2 To investigate the current traffic condition of the identified critical junctions and critical road links, manual classified traffic counts were conducted on a typical weekday in May 2023. The surveys were undertaken during 7:00am – 9:00am and 5:00pm – 7:00pm.
- 3.2.3 The identified morning (AM) and evening (PM) peak hour are from 7:30am to 8:30am and from 5:30pm to 6:30pm respectively. The 2023 observed AM and PM peak hour traffic flows are shown in **Figure 3.8**.

3.3 Junction and Link Assessment

- 3.3.1 Based on the 2023 observed traffic flows, capacity assessments were carried out in accordance with the methodology documented in the appendices of Transport Planning and Design Manual (TPDM) Volume 2 Chapter 4 for priority junction /

roundabout. Signal junction assessments were based on TPDM Volume 4.

3.3.2 The existing junction performance of the critical junctions are summarized in **Table 3.2**. The junction calculation spreadsheets are enclosed in **Annex D**.

Table 3.2 Existing Junction Performance

Ref.	Junction	Indicator*	2023 Observed	
			AM Peak	PM Peak
J1	Yu Tung Road / Yi Tung Road / North Lantau Highway	DFC	0.36	0.30
J2	Yu Tung Road / Shun Tung Road	RC	54%	>100%
J3	Yu Tung Road / Chung Yan Road	RC	67%	>100%
J4	Tung Chung Road / Chung Yan Road	DFC	0.40	0.17
J5	Yu Tung Road / Chung Mun Road	DFC	0.27	0.22
J6	Tung Chung Road / Shek Mun Kap Road	DFC	0.24	0.24

* RC = Reserve Capacity for signal junction; DFC = Design Flow / Capacity ratio for priority junction or roundabout

3.3.3 At present, the critical junctions are operating within capacity during the AM and PM peak periods.

3.3.4 Based on the observed traffic flows in **Figure 3.8**, the volume / capacity (V/C) ratios of the identified critical road links were assessed. The results are summarized in **Table 3.3**.

Table 3.3 Existing Road Link Performance

Ref.	Road Link	Direction	Capacity (pcu/hr)	2023 Traffic Flows (pcu/hr)		2023 V/C	
				AM Peak	PM Peak	AM Peak	PM Peak
L1	Yu Tung Rd (Between J2 and J3)	NB	3,050	1215	900	0.40	0.30
		SB	3,050	990	965	0.32	0.32
L2	Yu Tung Road (Between J3 and J5)	NB	2745 ⁽¹⁾	375	220	0.14	0.08
		SB	2745 ⁽¹⁾	355	220	0.13	0.08
L3	Chung Mun Road (To the south of J5)	NB	2350	215	85	0.09	0.04
		SB	2350	195	85	0.08	0.04

Note: (1) A 10% reduction in road capacity is assumed to account for bus activities on Yu Tung Road (between J3 and J5)

3.3.5 The assessment results in Table 3.3 indicate that all the critical road links are operating within capacities during the AM and PM peak periods.

3.4 Existing Public Transport Facilities

3.4.1 The existing franchised bus routes serving Yu Tung Road, Tung Chung Road and Chung Mun Road, where are located within 500m radius of the Application Site are summarized in the **Table 3.4** and presented in **Figure 3.9**.

Table 3.4 Existing Public Transport Services

Route No.	Origin / Destination	Frequency (min.)
Franchised Bus		
Bus Routes – Tung Chung Road		
11	Tai O ← → Tung Chung Station Bus Terminus	5 - 45
	Tai O (Sha Tsui) → Tung Chung Station Bus Terminus	07:10 (School Days Only), 14:05, 15:00, 15:45, 16:15, 17:00, 17:25
	Tai O (Shui Hau) → Tung Chung Station Bus Terminus	06:40 (Mon to Fri), 07:00 (Mon to Sat)
11A	Shek Pik ← → Tung Chung Station Bus Terminus	25 - 45
23	Tung Chung Tat Tung Road Bus Terminus ← → Ngong Ping	15 - 60
34	Tung Chung Tat Tung Road Bus Terminus ← → Shek Mun Kap	5 - 110
3M	Tung Chung Station Bus Terminus ← → Mui Wo Pier	5 - 60
	Pui O (Lo Wai Tsuen) → Tung Chung Station Bus Terminus	06:50, 07:20, 08:20
A35	Mui Wo Pier ← → HZMB Hong Kong Port	From HZMB HK Port : 06:15, 06:40, 08:30, 18:15, 23:30 From Mui Wo Pier: 05:30, 07:25, 17:00, 22:00, 00:15
N35	Mui Wo Pier ← → HZMB Hong Kong Pier	From Mui Wo Pier: 03:15, 04:20 From HZMB Hong Kong Pier: 01:30, 04:30
Bus Routes – Yu Tung Road		
36X	Mun Tung Estate → Disneyland	08:20
37H	Ying Tung Estate ← → North Lantau Hospital	20 - 30
37P	Yung Yat House (Yu Tung Road) ← → Caribbean Coast	3 - 5 (School Day)
39M	Tung Chung Station Bus Terminus ← → Mun Tung Estate	7 - 15
B6	HZMB Hong Kong Port ← → Mun Tung Estate	15 - 30
E11B	Tin Hau Station ← → Tung Chung (Mun Tung Estate)	12 - 40
E11S	Tin Hau Station ← → Tung Chung (Mun Tung Estate)	5 - 7
E21A	Ho Man Tin (Oi Man Estate) Tin Hau Station ← → Tung Chung (Yat Tung Estate)	20 - 30
E21B	Ho Man Tin (Oi Man Estate) Tin Hau Station ← → Tung Chung (Yat Tung Estate)	20 - 30
E21X	Tung Chung (Mun Tung Estate) → Hung Hom Station	07:48
E22S	Tung Chung (Mun Tung Estate) ← → Tseung Kwan O (Po Lam)	From Mun Tung Estate: 06:50, 07:05, 07:20 From Po Lam: 17:35
E31	Tsuen Wan (Discovery Park) ← → Tung Chng (Yat Tung Estate)	15 - 25
E36A	Tung Chung (Yat Tung Estate) ← → Yuen Long (Tak Yip Street)	25 - 60

Route No.	Origin / Destination	Frequency (min.)
N31	Tsuen Wan (Discovery Park) ← → Airport (Ground Transportation Centre)	30
S64X	Tung Chung (Mun Tung) ← → Airport	10 - 35
Bus Routes – Chung Mun Road		
37	Yat Tung Estate ← → Ying Tung Estate	10 - 20
38X	Yat Tung Estate (Yu Tung Road) ← → Tung Chung Station Bus Terminus	6 – 8 (School Day)

3.5 Future Road Network

3.5.1 According to PWP item No. 7786CL Tung Chung New Town Extension (Road Works at Yu Tung Road, Chung Mun Road, Road L29, Road L30 and Shek Mun Kap Road) and (Road Works at Road L22, Road L24, Road L25, Road L26 and Road L28), there will be new planned roads and road improvement works to serve the whole Tung Chung West. The road configuration and schematic layout of these planned roads are shown in **Table 3.5** and illustrated in **Figure 3.10**.

Table 3.5 Planned Road Improvement Works and Planned New Roads

Ref.	Road	Road Configuration	Types (New/Improvement)
1	Chung Mun Road	S-4	Improvement
2	Road L22	S-2	New
3	Road L23	S-2	New
4	Road L24	S-2	New
5	Road L25	S-2	New
6	Road L26	S-2	New
7	Road L28	S-2	New
8	Road L29	S-2	New
9	Road L30	S-2	New
10	Shek Mun Kap Road	S-2	Improvement

3.5.2 The critical junctions and identified road links for the traffic impact assessment in future are listed in **Table 3.6** and shown in **Figure 3.10**. The planned junction layouts are presented in **Figure 3.11** to **Figure 3.17**. The planned junction types (i.e. signalized, priority or roundabout) and junction improvement schemes for J2 and J3 are adopted under the Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02.)

3.5.3 Proposed Junction Improvement Scheme for J2 under REP-116-02 (**Figure 3.18**): (1) convert the left-turn traffic lane from eastbound Yu Tung Road to northbound Shun Tung Road into a free-flow lane and (2) make minor lane marking modification to form a separate lane for the left turn from eastbound of Yu Tung Road.

- 3.5.4 Proposed Junction Improvement Scheme for J3 under REP-116-02 (**Figure 3.19**): (1) change the lane marking of the middle entry lane of southbound Chung Yan Road from left turn + straight ahead to straight ahead + right turn; (2) change the lane marking of the rightmost entry lane of southbound Chung Yan Road from straight ahead + right turn to right turn; (3) change the lane marking of the middle entry lane of westbound Yu Tung Road from straight ahead + right turn to straight ahead; (4) widen the eastbound Yu Tung Road to provide three entry lanes, with leftmost lane for left turn, middle lane for straight ahead, rightmost land for straight ahead + right turn.

Table 3.6 Critical Junctions for Assessment

Ref.	Junction	Existing /Planned	Type ⁽²⁾	Fig. No.
Junctions				
J1	Yu Tung Road / Yi Tung Road / North Lantau Highway	Existing	Roundabout	3.2
J2 ⁽¹⁾	Yu Tung Road / Shun Tung Road	Planned	Signal	3.18 ⁽¹⁾
J3 ⁽¹⁾	Yu Tung Road / Chung Yan Road	Planned	Signal	3.19 ⁽¹⁾
J4	Tung Chung Road / Chung Yan Road	Existing	Priority	3.5
J5	Yu Tung Road / Chung Mun Road / Road L22 / Road L23	Planned	Roundabout	3.11
J6	Tung Chung Road / Shek Mun Kap Road	Planned	Priority	3.12
J7	Road L29 / Road L30	Planned	Priority	3.13
J8	Tung Chung Road / Road L30	Planned	Priority	3.14
J9	Road L25 / Road L29	Planned	Priority	3.15
J10	Shek Mun Kap Road / Road L28 / Road L29	Planned	Priority	3.16
J11	Chung Mun Road / Road 24 / Road L29	Planned	Signal	3.17
Road Links				
L1	Yu Tung Road (between J2 and J3)			
L2	Yu Tung Road (between J3 and J5)			
L3	Chung Mun Road (to the south of J5)			

Note: (1) Junction Improvement Works are adopted under Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02).

(2) The planned junction types are adopted based on Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02).

4 TRAFFIC FORECASTING

4.1 Design Year

4.1.1 The proposed development is tentatively scheduled for completion in 2030. Year 2033 is therefore selected as a design year for assessment purpose (i.e. 3 years after the planned completion).

4.2 Future Planned Developments

4.2.1 With the Application Site (Conforming Scheme), there are also several planned / potential developments in the vicinity, which have been taken into account in the background traffic forecast and are listed in **Table 4.1** and diagrammatically shown in **Figure 4.1**. The development parameters and average flat size in Table 4.1 are adopted based on the Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02.)

Table 4.1 Planned / Potential Future Developments in the Vicinity

Ref.	Lot	Proposed Use	Development Parameter ⁽¹⁾	Estimated Average Flat Size ⁽¹⁾
1	Application Site	Private Housing	236 units	140 m ²
2	Site A	Private Housing	212 units	140 m ²
3	Site B	Private Housing	818 units	100 m ²
4	Site C	Private Housing	124 units	100 m ²
5	Site D	Private Housing	245 units	140 m ²
6	Site E	Private Housing	53 units	140 m ²
7	Site F	Private Housing	126 units	140 m ²
8	Area 23	Public Housing	1908 units	50 m ²
9	Area 23	Commercial Facilities GFA	1,635 m ² GFA	50 m ²
10	Area 33	Private Housing	411 units	100 m ²
11	Area 38 (Area 38A & B)	Commercial Development	29,601m ² GFA	--
12	Area 38 (Area 38C)	Commercial Development	2,742m ² GFA	--
13	Area 42	Public Housing Commercial Facilities GFA	6,600 units Commercial Facilities GFA=16,000m ²	40 m ²
14	Area 46	Public Housing Commercial Facilities GFA	1,711 units Commercial Facilities GFA=4,480m ²	40m ²
15	Area 48	Private Housing	187 units	100 m ²

Notes:

(1) Development parameters and average flat size are adopted based on the Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02.)

4.3 Trip Generation of Planned / Potential Future Developments

4.3.1 For the Conforming Scheme, the development trip rates used in this report are adopted from the Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02.) and presented in **Table 4.2**.

Table 4.2 TPDM Trip Rates (pcu/hr)

Landuse	Average Flat Size (m ²)	Unit	TPDM Trip Rates (pcu/hr)			
			AM Peak		PM Peak	
			Gen.	Att.	Gen.	Att.
Subsidised Housing Public Rental	40	pcu/hr/flat	0.0325	0.0213	0.0196	0.0263
Subsidised Housing HOS	50	pcu/hr/flat	0.0483	0.0279	0.0244	0.0351
Private Housing / R(A)	60	pcu/hr/flat	0.0415	0.0141	0.0157	0.0276
	70	pcu/hr/flat	0.0659	0.0301	0.0258	0.0409
	80	pcu/hr/flat	0.0737	0.0305	0.0289	0.0491
Private Housing / R(B)	100	pcu/hr/flat	0.1572	0.0665	0.0609	0.0864
	120	pcu/hr/flat	0.189	0.0845	0.0783	0.1074
	140	pcu/hr/flat	0.2166	0.0988	0.0924	0.1237
Retail / Shopping Complex (Office + Retail)	--	pcu/hr/100m ² GFA	0.1285	0.1525	0.236	0.2622
Office	--	pcu/hr/100m ² GFA	0.1045	0.1646	0.1217	0.0840

Note:

- (1) the development trip rates (Conforming Scheme) used in this report are adopted from the Agreement No. CE 70/2015 (CE) Tung Chung New Town Extension (West) – Design and Construction Deliverable C31 – Final Detailed Traffic Impact Assessment Report (REP-116-02.)

4.3.2 According to the trip rate in **Table 4.2**, **Table 4.3** summarizes the estimated trip generations of the planned / potential future developments as listed in **Table 4.1**.

Table 4.3 Estimated Traffic Flows for Planned Future Developments in the Vicinity

Ref.		Estimated Trips (pcu/hr)			
		AM Peak		PM Peak	
		Generation	Attraction	Generation	Attraction
1 – Application Site	Estimated Flow (pcu/hr) (236 units)	51	23	22	29
2 – Site A	Estimated Flow (pcu/hr) (212 units)	53	30	33	42
3 – Site B	Estimated Flow (pcu/hr) (818 units)	129	54	50	71
4 – Site C	Estimated Flow (pcu/hr) (124 units)	19	8	8	11
5 – Site D	Estimated Flow (pcu/hr) (245 units)	53	24	23	30
6 – Site E	Estimated Flow (pcu/hr) (53 units)	11	5	5	7
7 – Site F	Estimated Flow (pcu/hr) (126 units)	27	12	12	16
8 – Area 23	Estimated Flow (pcu/hr) (1,908 units)	92	53	47	67
9 – Area 23	(commercial facilities GFA = 1,635 m ²)	2	2	4	4
10 – Area 33	Estimated Flow ⁽¹⁾ (pcu/hr) (411 units)	65	27	25	36
11 – Area 38 (Area 38A & B)	Estimated Flow (pcu/hr) (29,601m ² Retail GFA)	38	45	70	78
12 – Area 38 (Area 38C)	Estimated Flow (pcu/hr) (2,742m ² Retail GFA)	4	4	6	7
13 – Area 42	Estimated Flow (pcu/hr) ((6,600 units)	216	142	131	175
	Estimated Flow (pcu/hr) (commercial facilities GFA = 16,000m ²)	21	24	38	42
14 – Area 46	Estimated Flow (pcu/hr) (1,711 units)	56	36	34	45
	Estimated Flow (pcu/hr) (commercial facilities GFA = 4,480m ²)	6	7	11	12
15 – Area 48	Estimated Flow (pcu/hr) (187 units)	29	12	11	16

4.4 Reference Traffic Forecasts

- 4.4.1 For the future traffic forecasts, in-house local area models (LAM) would be developed on this area by making reference to TD’s 2019-based Base District Traffic Model (BDTM) “NTW3” covering Lantau Island.
- 4.4.2 The 2019 / 2026 / 2031 BDTM would be cordoned off to produce LAM for providing traffic flows within the study area. By proportional of 2019 / 2026 BDTM condoned matrix, the 2023 LAM matrix was derived. The 2019 LAM road network would also be refined for matching with existing road network in year 2023. The 2023 LAM matrix will be taken and assigned to the 2023 LAM road network by “SATURN” software to produce the year 2023 traffic flows. The cordoned 2023 LAM would be validated against 2023 observed traffic flows to ensure the base year LAM could satisfactorily replicate the traffic flow before the model is used to produce future year traffic forecasts.
- 4.4.3 The 2031 road network would be retrieved for matching with the planned network in Tung Chung West. The growth pattern demand from the produced 2023 / 2031 BDTM matrix were fed into the LAM for projecting the traffic flows from year 2023 to year

2033. In addition, the trip ends of traffic zones were adjusted and controlled to the estimated trips generated by the future planned development in the vicinity as listed in **Table 4.3**. The 2033 reference traffic flows with the Conforming Scheme are shown in **Figure 4.2**.

4.5 Design Traffic Forecasts

4.5.1 In the current proposal, the Applicant suggested a higher domestic plot ratio of 2.10, which would include 1,783 residential units with an average flat size of 39.8m². However, the TPDM has not provided any recommended trip generation rates for flats of this size. To address this, the adopted trip rates are determined by the trip rates for private housing of average flat size of 60 m² in Conforming Case in **Table 4.2** in proportion to the change in average flat size. For example, trip generation rate of private housing in AM peak with averaged flat size of 40m² would be adjusted as follows: 0.0415 pcu/hr/flat x 40m² / 60m². For the retail, the trip rate proposed in the Conforming Scheme in **Table 4.2** was adopted. For the kindergarten, the trip rate as stipulated in BDTM NT Final Report, Appendix P 2 – Proposed Trip Rates of kindergarten was adopted. The trip rate for the Proposed Scheme are summarized in **Table 4.4**.

Table 4.4 Trip Rates for Proposed Scheme

Land Use	Average Flat Size (m ²)	Trip Rates (pcu/hr/flat)			
		AM Peak		PM Peak	
		Gen.	Att.	Gen.	Att.
Private Housing	40	0.0277 ⁽¹⁾	0.0094 ⁽²⁾	0.0105 ⁽³⁾	0.0184 ⁽⁴⁾
Retail	pcu/hr/100m ²	0.1285	0.1525	0.236	0.2622
Kindergarten	pcu/hr/class operating	2.3056	2.3056	0.0286	0.0286

Note:

- (1) The adopted trip generation rate = adopted trip generation rate of private housing with average flat size of 60 sqm in AM peak x flat size of 40 sqm / flat size of 60 sqm = 0.0415 x 40 / 60 = 0.0277
- (2) The adopted trip attraction rate = adopted trip attraction rate of private housing with average flat size of 60 sqm in AM peak x flat size of 40 sqm / flat size of 60 sqm = 0.0141 x 40 / 60 = 0.0094
- (3) The adopted trip generation rate = adopted trip generation rate of private housing with average flat size of 60 sqm in PM peak x flat size of 40 sqm / flat size of 60 sqm = 0.0157 x 40 / 60 = 0.0105
- (4) The adopted trip attraction rate = adopted trip attraction rate of private housing with average flat size of 60 sqm in PM peak x flat size of 40 sqm / flat size of 60 sqm = 0.0276 x 40 / 60 = 0.0184

4.5.2 Based on the development schedule as mentioned in **Section 2**, the adopted trip rates from **Table 4.4** (Proposed Scheme) and the development trip generation and attraction under the Proposed Scheme for the Application Site are illustrated in **Table 4.5**. The traffic generation under the Conforming Scheme is also listed for comparison purpose.

Table 4.5 Estimated Traffic Flows for the Application Site

Subject Site	Estimated Trips (pcu/hr)			
	AM Peak		PM Peak	
	Gen.	Att.	Gen.	Att.
Conforming Scheme				
<i>Domestic Portion (PR = 1.0)</i>	51	23	22	29
Proposed Scheme				
<i>Domestic Portion (PR = 2.10)</i>	49	17	19	33
<i>Retail</i>	4	5	7	8
<i>Kindergarten</i>	14	14	1	1
Total Two-way Traffic (Conforming Scheme) (pcu/hr)	74		51	
Total Two-way Traffic (Proposed Scheme) (pcu/hr)	103		69	
Difference (pcu/hr)	29		18	

- 4.5.3 As shown in **Table 4.5**, as compared with the Conforming Scheme, the Proposed Scheme would generate additional two-way traffic of about 29 pcu/hr and 18 pcu/hr during the AM and PM peak hours.
- 4.5.4 The 2033 design traffic flows are produced by adding additional trip generated by the Application Site under the Proposed Scheme as estimated in **Table 4.5** to 2033 reference traffic flows. The 2033 design traffic flows (Proposed Scheme) are shown in **Figure 4.3**.

5 TRAFFIC IMPACT ASSESSMENT

5.1 Junction Capacity Assessment

5.1.1 The operational performance of 11 critical junctions based on year 2033 traffic forecasts as mentioned in **Section 4** have been assessed. The results of junction capacity analysis are summarized in **Table 5.1**. Junction capacity calculation sheets are attached in Annex B.

Table 5.1 Junction Performance in 2033

Ref.	Junction	Indicator ⁽¹⁾	2033			
			Reference (Conforming Scheme)		Design (Proposed Scheme)	
			AM Peak	PM Peak	AM Peak	PM Peak
J1	Yu Tung Road / Yi Tung Road / North Lantau Highway	DFC	0.60	0.52	0.61	0.53
J2	Yu Tung Road / Shun Tung Road	RC	21%	58%	20%	56%
J3	Yu Tung Road / Chung Yan Road	RC	0%	29%	-1%	28%
J4	Tung Chung Road / Chung Yan Road	DFC	0.49	0.21	0.49	0.21
J5	Yu Tung Road / Chung Mun Road / Road L22 / Road L23	DFC	0.31	0.26	0.31	0.27
J6	Tung Chung Road / Shek Mun Kap Road	DFC	0.15	0.16	0.15	0.16
J7	Road L29 / Road L30	DFC	0.30	0.33	0.30	0.33
J8	Tung Chung Road / Road L30	DFC	0.30	0.24	0.30	0.24
J9	Road L29 / Road L25	DFC	0.39	0.23	0.39	0.23
J10	Shek Mun Kap Road / Road L29 / Road L 28	DFC	0.16	0.15	0.16	0.15
J11	Chung Mun Road / Road L29 / Road L24	RC	68%	>100%	68%	>100%

Notes:

(1) RC = Reserve Capacity for signal junction; DFC = Design Flow / Capacity ratio for priority junction or roundabout

5.1.2 As shown in **Table 5.1**, all junctions will be operation within capacity in 2033 except J3 in AM peak period. It should be noted that the RC of J3 would be below 15% in 2033 Reference Case (i.e. Conforming Scheme). The proposed development has only induced minimal traffic impact of 1% on J3. However, to enhance junction capacity of J3, junction improvement schemes for J3 have been proposed for consideration.

5.2 Junction Improvement for Junction of Yu Tung Road / Chung Yan Road (J3)

5.2.1 To enhance junction capacity of J3, it is proposed to provide one additional westbound far-side flare traffic lane of about 60m on Yu Tung Road and revise the lane markings to optimize the junction performance. In addition, it is proposed to provide one additional receiving lane at Yu Tung Road eastbound on Yu Tung Road to cater the straight-ahead traffic of Yu Tung Road eastbound. The proposed junction layout for junction of Yu Tung Road / Chung Yan Road (J3) is shown in **Figure 5.1**. The junction performance is reassessed by taking into consideration the junction improvement and the junction would operate with sufficient capacity as shown in **Table 5.2**.

Table 5.2 2033 Junction Performance with Improvement Scheme

Ref.	Junction	Indicator*	2033 Design Case	
			AM Peak	PM Peak
J3	Yu Tung Road / Chung Yan Road	RC	20%	55%

Notes: RC = Reserve Capacity for signal junction

5.3 Road Link Assessment

5.3.1 The volume / capacity (V/C) ratios of the identified critical road links based on 2033 traffic forecasts Reference and Design Cases have been assessed. The results are summarized in **Table 5.3**.

Table 5.3 Road Link Performance in 2033

Ref.	Road Link	Direction	Capacity (pcu/hr)	2033							
				Reference Case				Design Case			
				Traffic Flows (pcu/hr)		V/C		Traffic Flows (pcu/hr)		V/C	
				AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
L1	Yu Tung Rd (Between J2 and J3)	NB	3,050	2105	1455	0.69	0.48	2130	1475	0.70	0.48
		SB	3,050	1550	1665	0.51	0.55	1560	1675	0.51	0.55
L2	Yu Tung Road (Between J3 and J5)	NB	2,745 ⁽¹⁾	1105	685	0.40	0.25	1125	700	0.41	0.26
		SB	2,745 ⁽¹⁾	775	740	0.28	0.27	785	755	0.29	0.28
L3	Chung Mun Road	NB	2350	725	390	0.31	0.17	725	390	0.31	0.17
		SB	2350	435	395	0.19	0.17	435	395	0.19	0.17

Note: (1) A 10% reduction in road capacity is assumed to account for bus activities on Yu Tung Road (between J3 and J5)

5.3.2 The assessment results in **Table 5.3** indicated that all the above road links would be operated within capacity in year 2033.

6 CONCLUSION

6.1 Summary

- 6.1.1 The Application Site covers various lots and adjacent Government land in DD 1 TC and adjoining government land, Tung Chung Valley, New Territories. The Site is located to the west of the junction of Yu Tung Road / Chung Mun Road with an area of about 33,808m².
- 6.1.2 The Application Site is zoned "Residential (Group C)2" under the current Approved Tung Chung Valley Outline Zoning Plan (OZP) no. S/I-TCV/2. In statutory planning terms, residential development with a maximum plot ratio of 1.0 and building height of 20 mPD is permitted as of right within the Site.
- 6.1.3 Refer to the **Section 3.5 Future Road Network**, a four-arm roundabout junction intersecting Yu Tung Road / Chung Mun Road (J5) is planned. The roundabout will include a vehicular access road to serve the Application Site.
- 6.1.4 The Application Site is close to the planned TCW MTR Station and Tung Chung West PTI. Under the current application, the Applicant proposes to rezone the Site with a domestic plot ratio of 2.10 to provide about 1,783 nos. of residential units (with an average flat size of about 39.8m²) on the Site.
- 6.1.5 The parking and loading/unloading facilities of the Application Site would be provided in accordance with the requirements as stipulated in the HKPSG.
- 6.1.6 In order to review the existing traffic condition, traffic count surveys were conducted at 6 identified critical junctions and 3 identified road links to investigate the traffic condition during commuting peak hours. At present, all the critical junctions and identified road links are operating within capacity.
- 6.1.7 To supplement the provision of transport services for the future residents and visitors of the Application Site, the proposed Covered Private Transport Lay-by will consist of 3 double-width bays of 7.3m in width and 42m in length. All the 3 bays have been designed to allow manoeuvring of 12.8m buses. One bay is proposed for general pick-up/drop-off of passengers, including taxi. The remaining two bays are proposed for bus services. The covered private transport laybys will be accessible to the nearby community during the operational hours of the two proposed bus routes. The management and maintenance responsibility of the covered private transport lay-by would be taken up the Applicant.
- 6.1.8 As compared with the Conforming Scheme, the Proposed Scheme would generate additional two-way traffic of about 29 pcu/hr and 18 pcu/hr during the AM and PM peak hour.
- 6.1.9 The Application Site is tentatively scheduled for completion in 2030. According to Guidelines and Requirements of TIA Studies, the TIA should assess at least 3 years after the planned completion of the Proposed Development. Hence, 2033 is adopted as the design year for this TIA.
- 6.1.10 Peak hour traffic forecasts in design year 2033 were generated by the local area model. In addition, the traffic generated by other key future developments and to/from the Application Site have been included.

6.1.11 Junction capacity assessment was conducted for both 2033 reference and design cases. The results revealed that all junctions would be operating within junction capacity $RC \geq 15\%$ or $DFC \leq 0.85$ under design case in 2033 except for J3 in AM peak period even in 2033 Reference Case (i.e. Conforming Scheme). In light of this, further junction improvement scheme at J3 (as shown in **Figure 5.1**) is formulated for improving the junction capacity. With the said junction improvement scheme, J3 would operate with sufficient capacity in 2033 design case.

6.1.12 The assessment results in **Table 5.3** indicated that all the identified road links operated within capacity in year 2033.

6.1.13 Conclusion

6.1.14 In light of the findings of this TIA, it is concluded that there is no adverse traffic impact imposed on the surrounding road network due to the Application Site. With the proposed mitigation measures in place, the Application Site is technically feasible in traffic terms.

Figure

TUNG CHUNG BAY

LEGEND:



APPLICATION SITE



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PROJECT

SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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FIGURE 1.1



PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND



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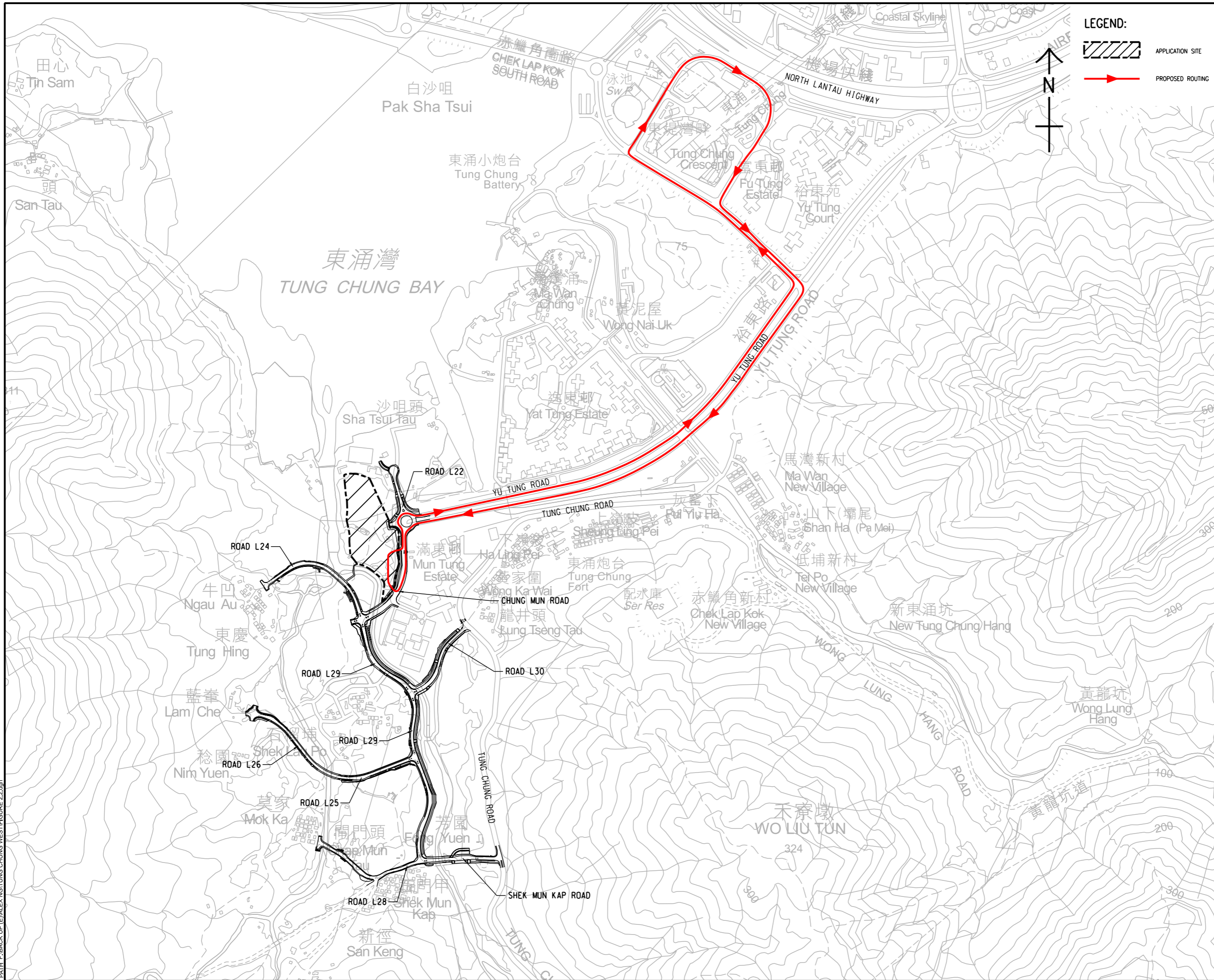
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 FIGURE 2.1

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APPLICATION SITE



PROPOSED ROUTING



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SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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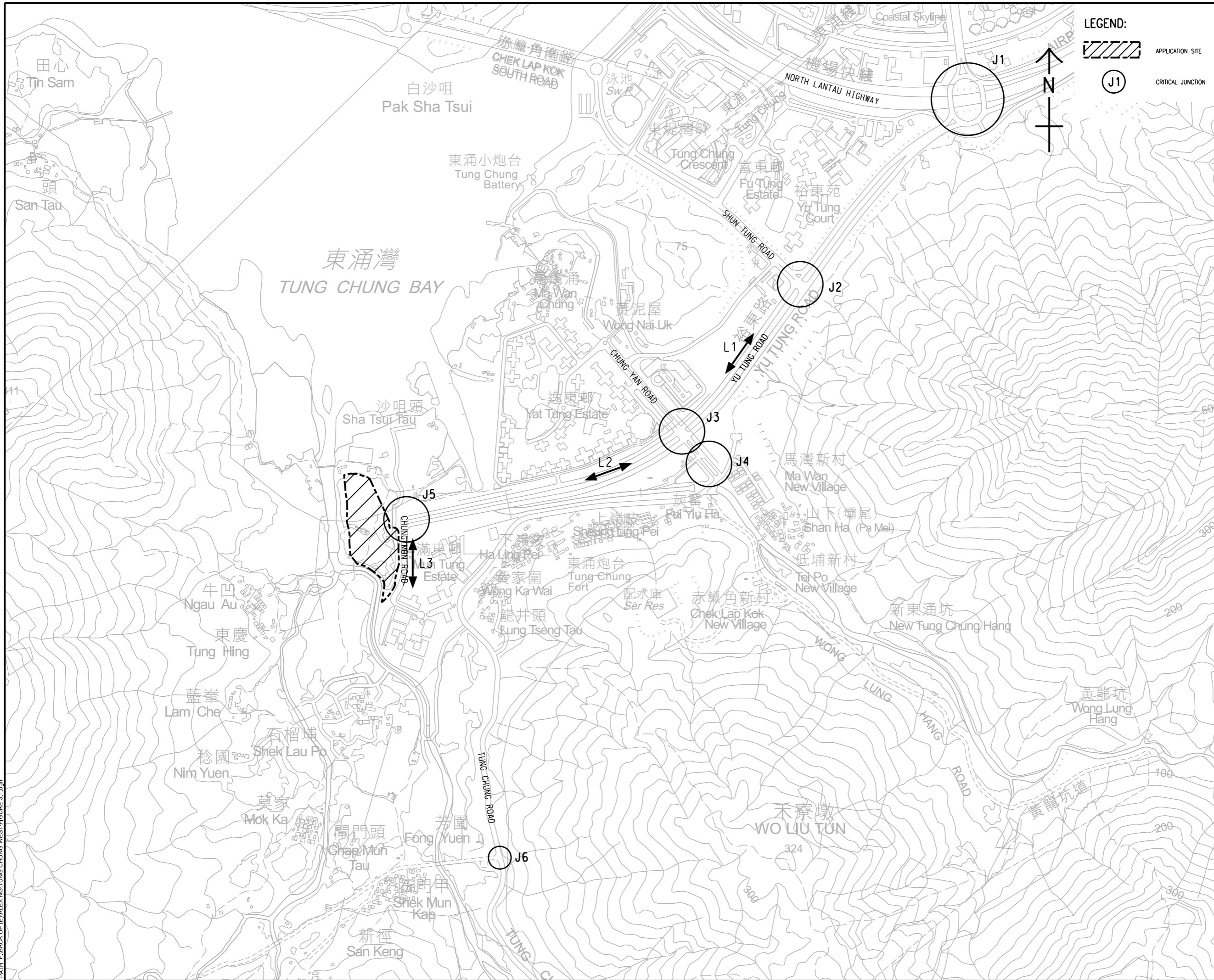
SHEET TITLE

ROUTING OF BUS SERVICE BETWEEN APPLICATION SITE AND TUNG CHUNG STATION

SHEET NUMBER

FIGURE 2.2

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LEGEND:



APPLICATION SITE



CRITICAL JUNCTION



PROJECT

SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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DIMENSION UNIT

公尺/米

KEY PLAN

索引圖

PROJECT NO.

CONTRACT NO.

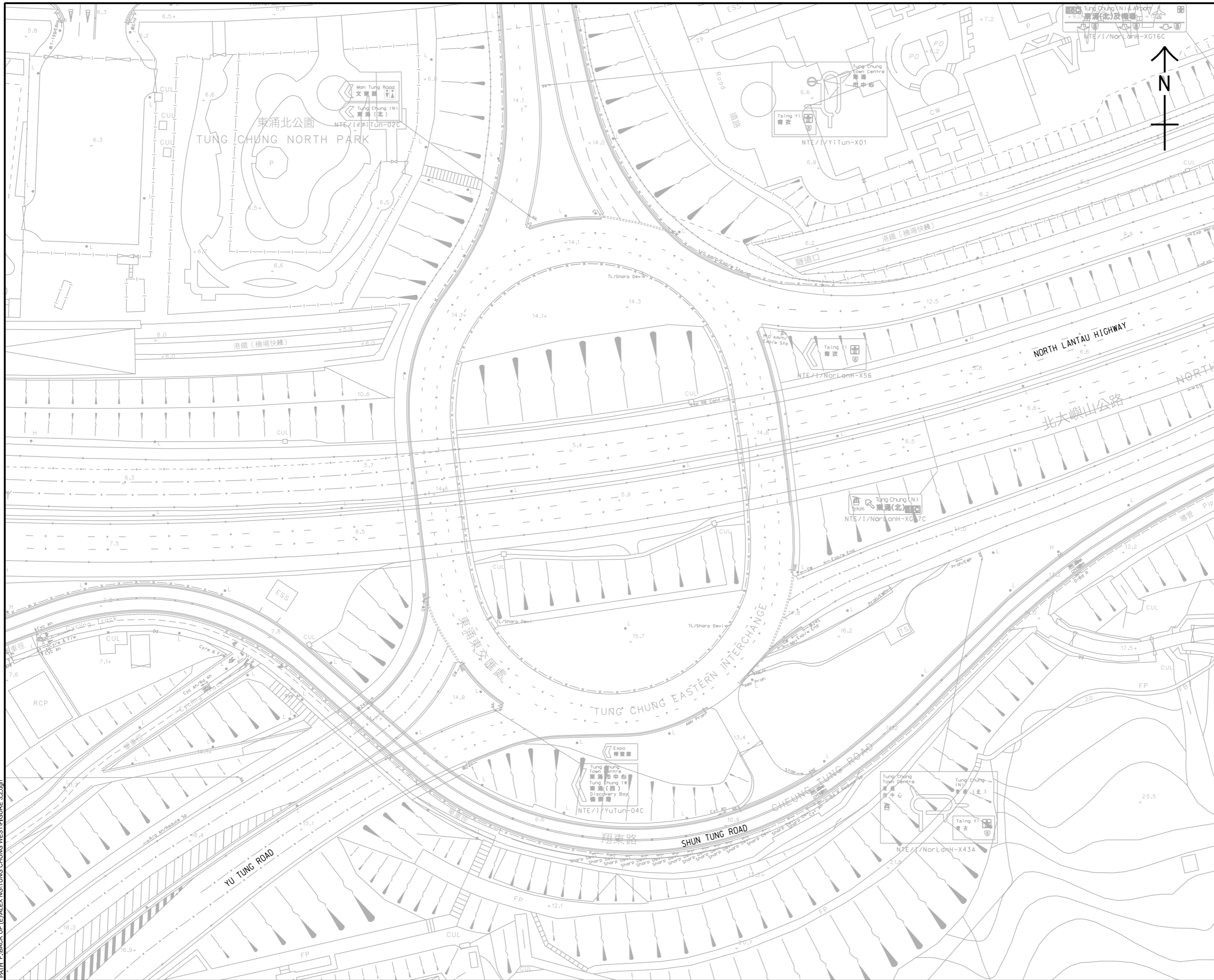
SHEET TITLE

LOCATION OF SURVEYED JUNCTIONS

SHEET NUMBER

FIGURE 3.1

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KEY PLAN
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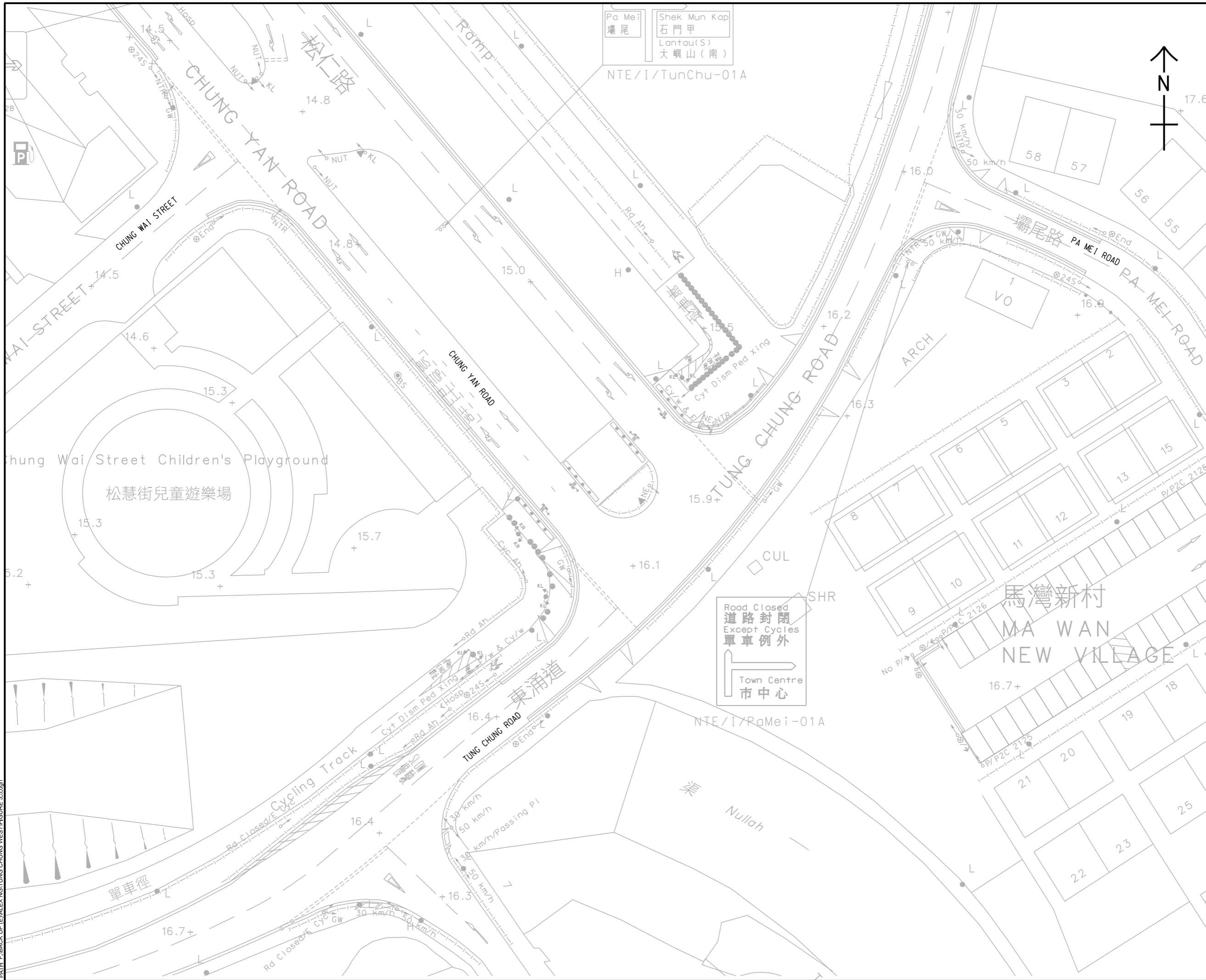
CONTRACT NO.
 合約編號

SHEET TITLE
 EXISTING JUNCTION LAYOUT OF YU TUNG ROAD / YI TUNG ROAD / NORTH LANTAU HIGHWAY

SHEET NUMBER
 圖號

FIGURE 3.2

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SCALE
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 A3 1:4000

KEY PLAN
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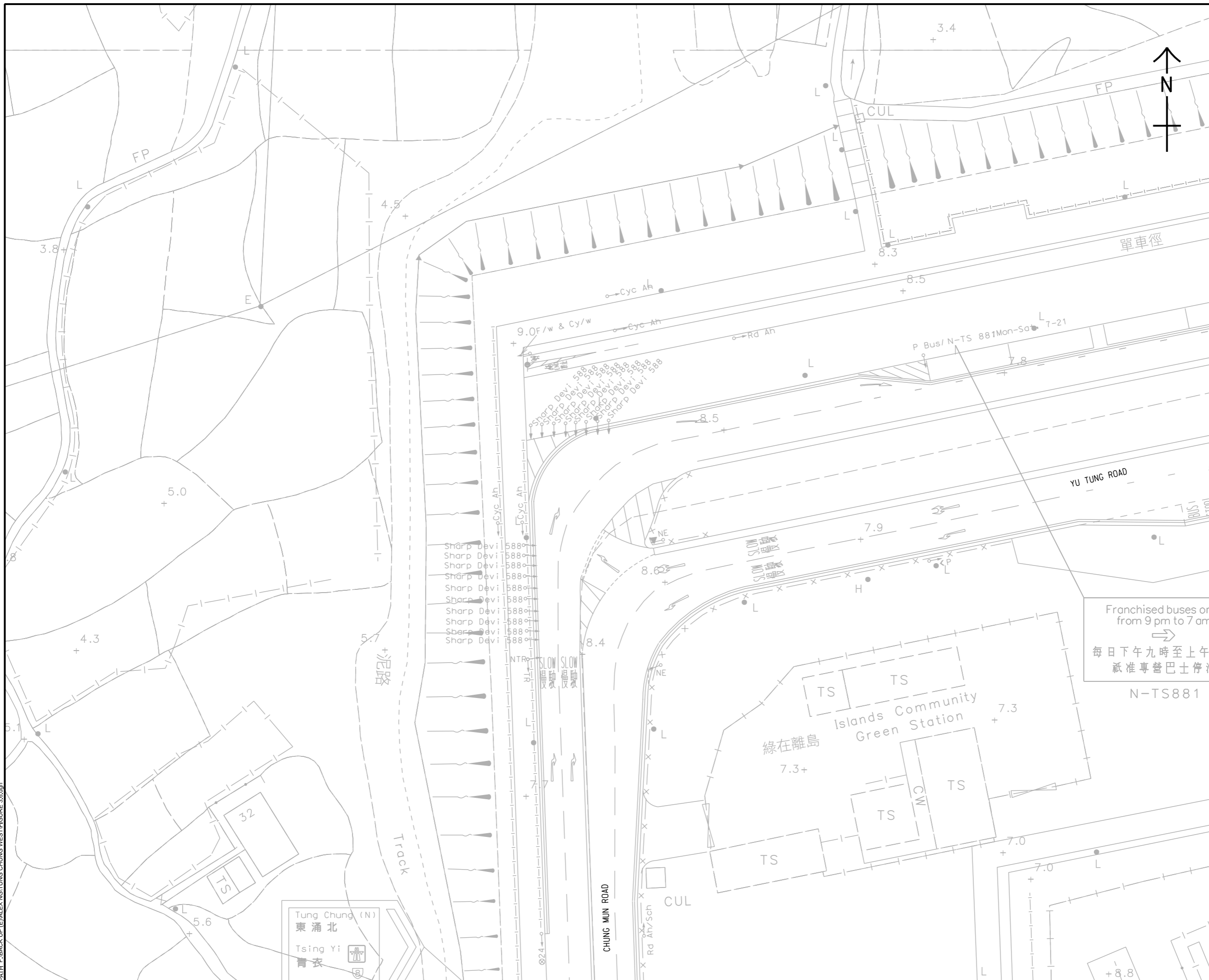
PROJECT NO.
 項目編號

CONTRACT NO.
 合約編號

SHEET TITLE
 圖則名稱
 EXISTING JUNCTION LAYOUT OF TUNG CHUNG ROAD / CHUNG YAN ROAD (J4)

SHEET NUMBER
 圖則編號
 FIGURE 3.5

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SCALE
 A3 1: 500

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KEY PLAN
 索引圖

PROJECT NO.
 項目編號

CONTRACT NO.
 合約編號

SHEET TITLE
 圖則名稱

EXISTING JUNCTION LAYOUT OF YU TUNG ROAD / CHUNG MUN ROAD (J5)

SHEET NUMBER
 圖則編號

FIGURE 3.6

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SCALE DIMENSION UNIT

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KEY PLAN

索引圖

PROJECT NO. CONTRACT NO.

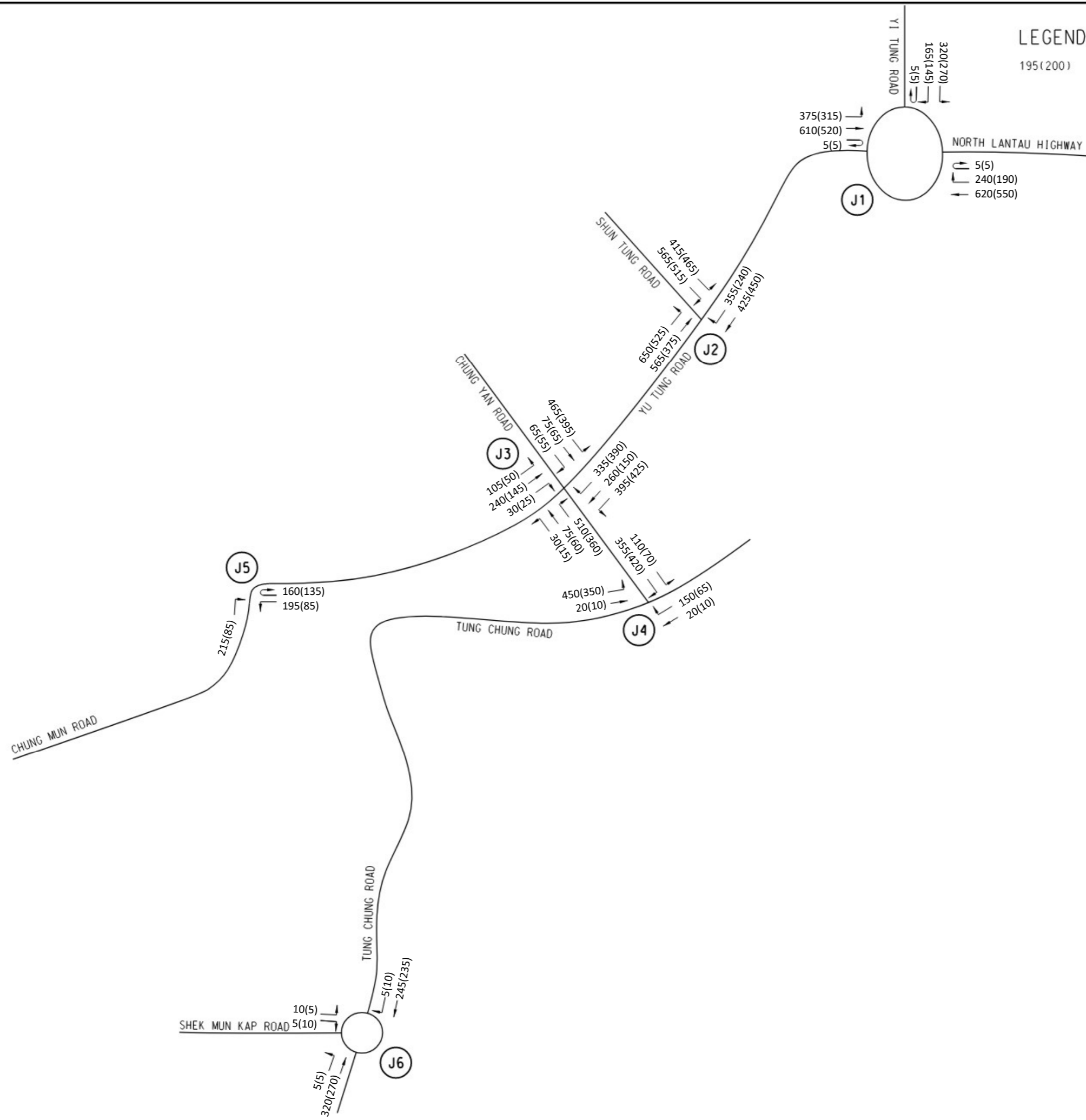
SHEET TITLE

EXISTING JUNCTION LAYOUT OF TUNG CHUNG ROAD / SHEK MUN KAP ROAD (J6)

SHEET NUMBER

FIGURE 3.7

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DIMENSION UNIT

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KEY PLAN

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CONTRACT NO.

SHEET TITLE

2023 OBSERVED TRAFFIC FLOWS

SHEET NUMBER

FIGURE 3.8

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TUNG CHUNG BAY

LEGEND:

 SITE LOCATION

 BUS STOP



NLB : 37 37H 38X 39M B6
CTB : E11B E21A E21B E31 E36A N31 S64X

NLB : 36X 37 37H 37P 38X 39M B6
CTB : E11B E11S E21A E21B E21X E22S
LWB : E31 E36A N31 S64X

CTB : E11B, E11S, E21A,
E21B, E21X, E22S



NLB : 36, 37, 37H, 39M, B6
LWB : E31, E36A, N31, S64X

NLB : 37, 38X

NLB : 11 11A 23
34 3M A35 N35



PROJECT
SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND



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SCALE **DIMENSION UNIT**

A3 1:4000 METER

KEY PLAN

PROJECT NO. **CONTRACT NO.**

SHEET TITLE

EXISTING PUBLIC TRANSPORT IN THE VICINITY OF APPLICATION SITE

SHEET NUMBER

FIGURE 3.9

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LEGEND:



APPLICATION SITE



CRITICAL JUNCTION



PROJECT

SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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STATUS

SCALE

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DIMENSION UNIT

公尺/米

KEY PLAN

索引圖

PROJECT NO.

CONTRACT NO.

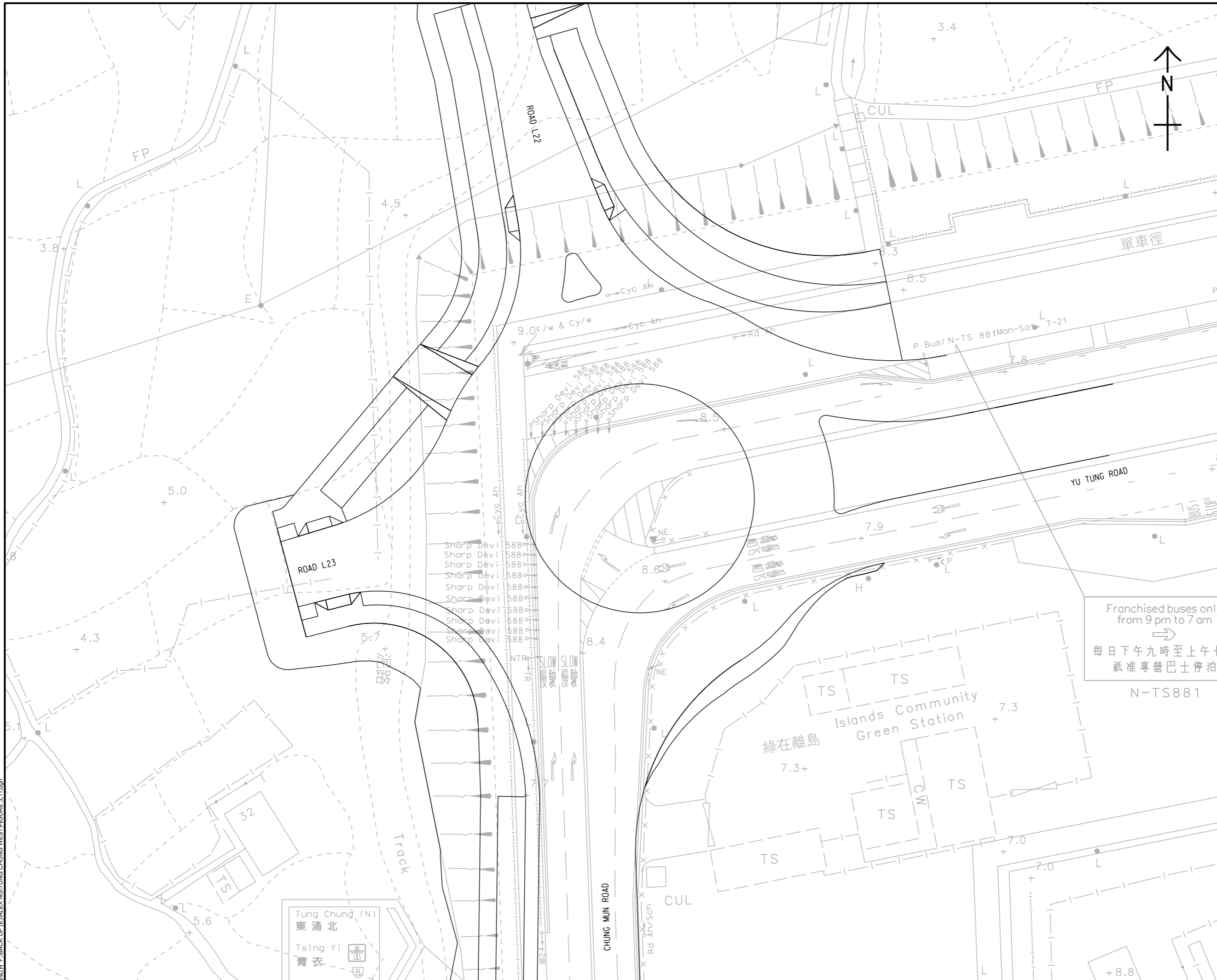
SHEET TITLE

SCHEMATIC LAYOUT OF PLANNED ROAD NETWORK AND PLANNED CRITICAL JUNCTIONS

SHEET NUMBER

FIGURE 3.10

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SCALE DIMENSION UNIT

A3 1: 500

KEY PLAN

PROJECT NO.

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CONTRACT NO.

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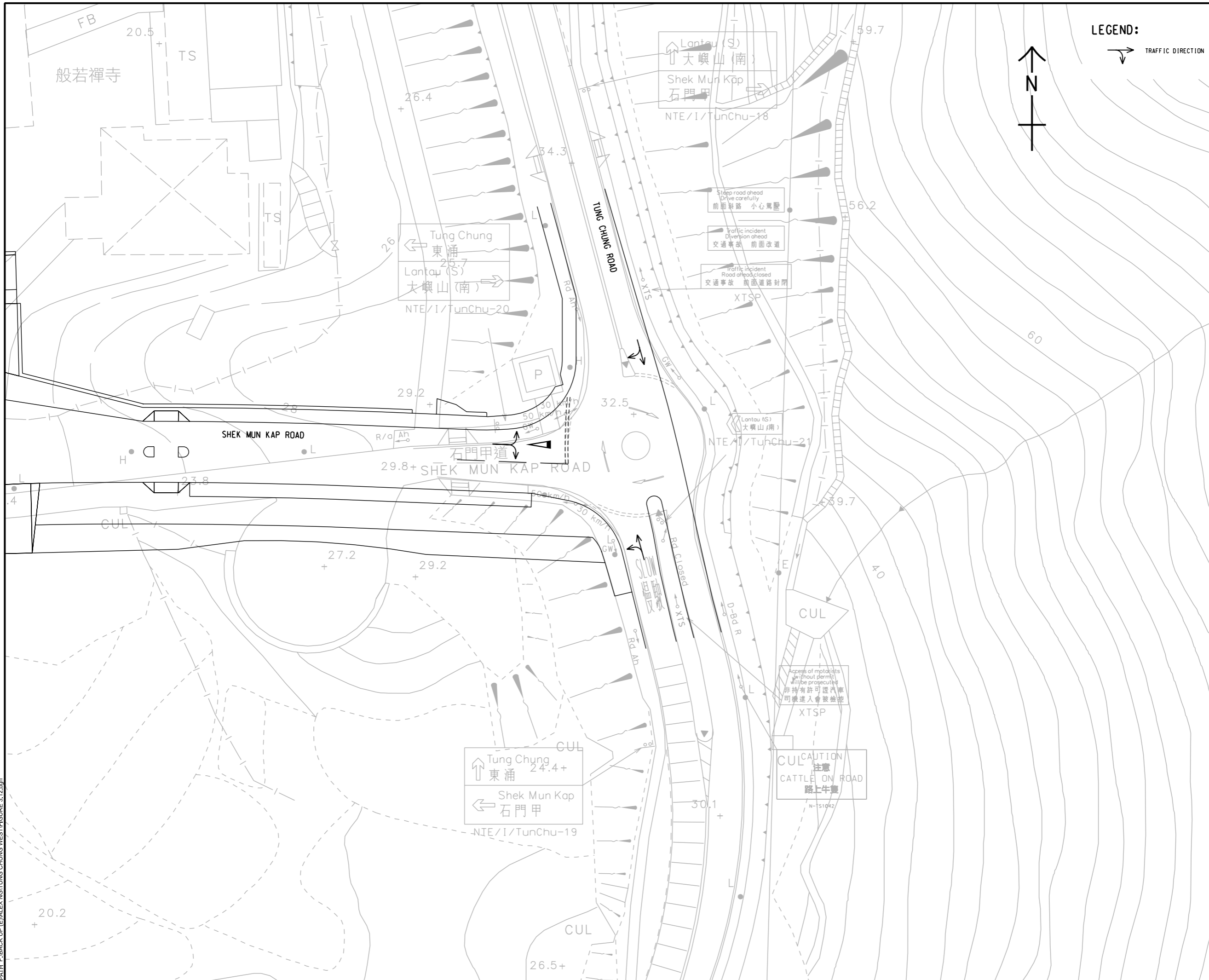
SHEET TITLE

PLANNED JUNCTION LAYOUT OF YU TUNG ROAD / CHUNG MUN ROAD / ROAD L23 (J5)

SHEET NUMBER

FIGURE 3.11

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PROJECT NO.
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CONTRACT NO.
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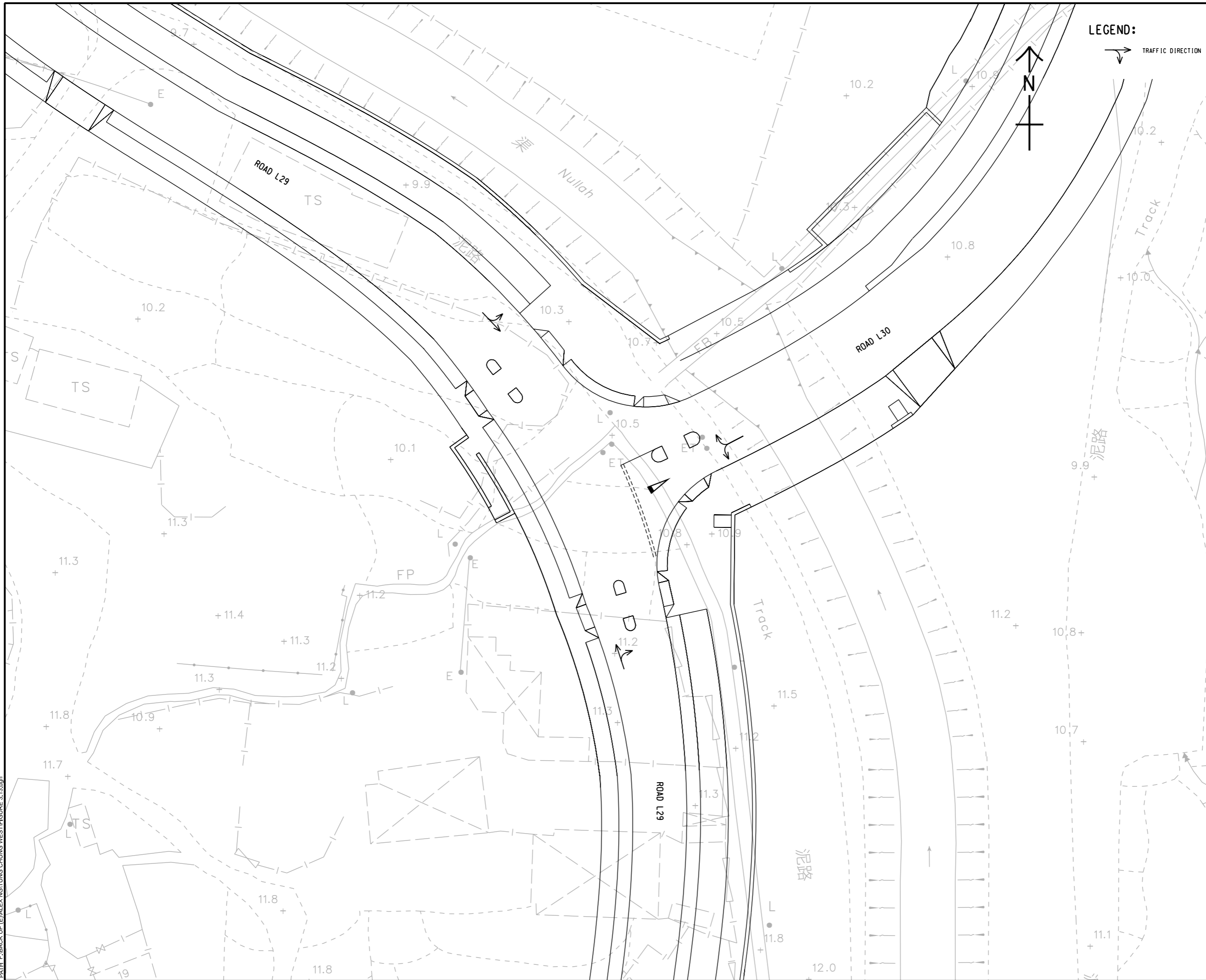
SHEET TITLE
 圖紙名稱

PLANNED JUNCTION LAYOUT OF TUNG CHUNG ROAD / SHEK MUN KAP ROAD (J6)

SHEET NUMBER
 圖紙編號

FIGURE 3.12

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 公尺/米

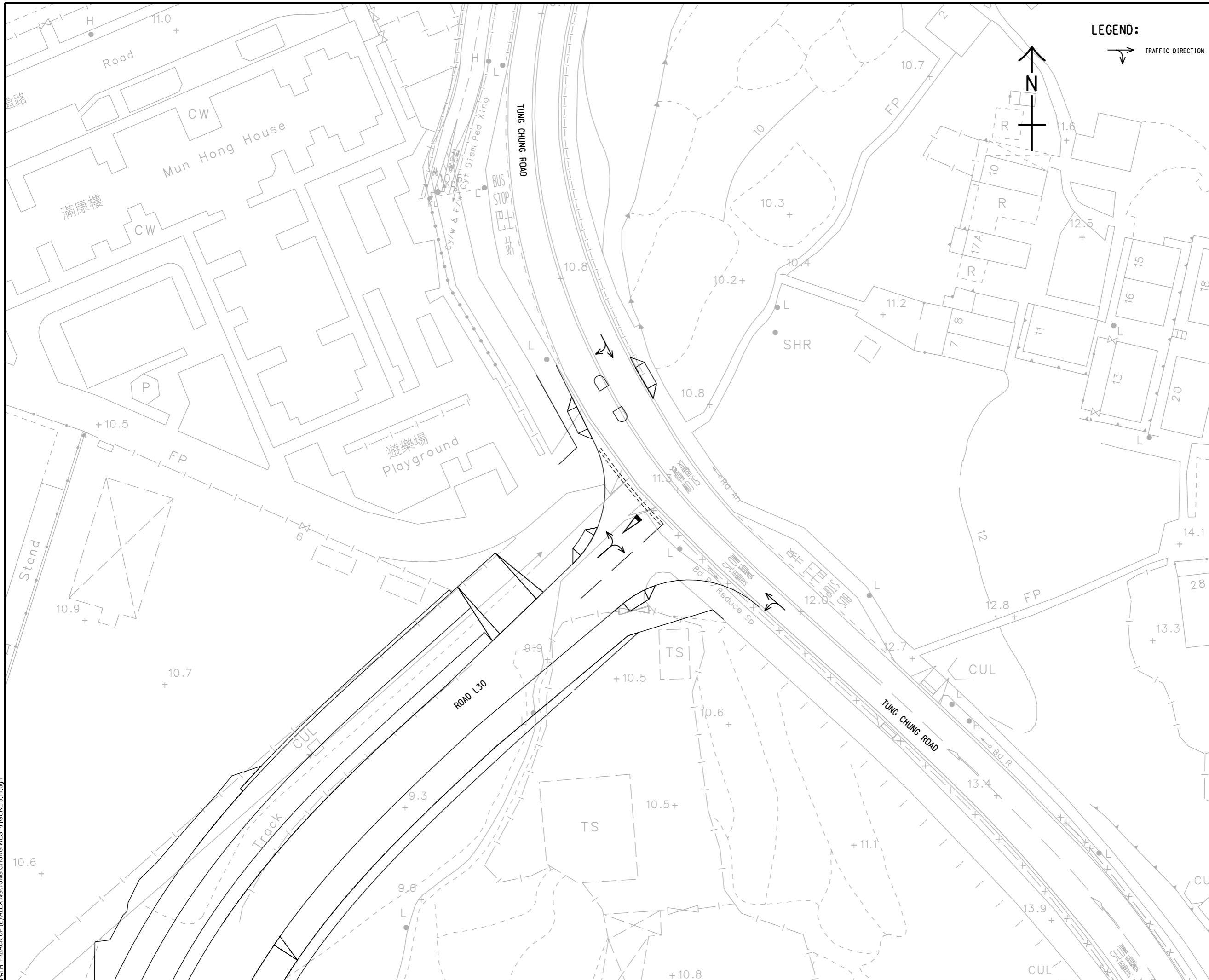
KEY PLAN
 索引圖

PROJECT NO.
 項目編號

CONTRACT NO.
 合約編號

SHEET TITLE
 圖則名稱
 PLANNED JUNCTION LAYOUT OF ROAD L29 / ROAD L30 (J7)

SHEET NUMBER
 圖則編號
 FIGURE 3.13



PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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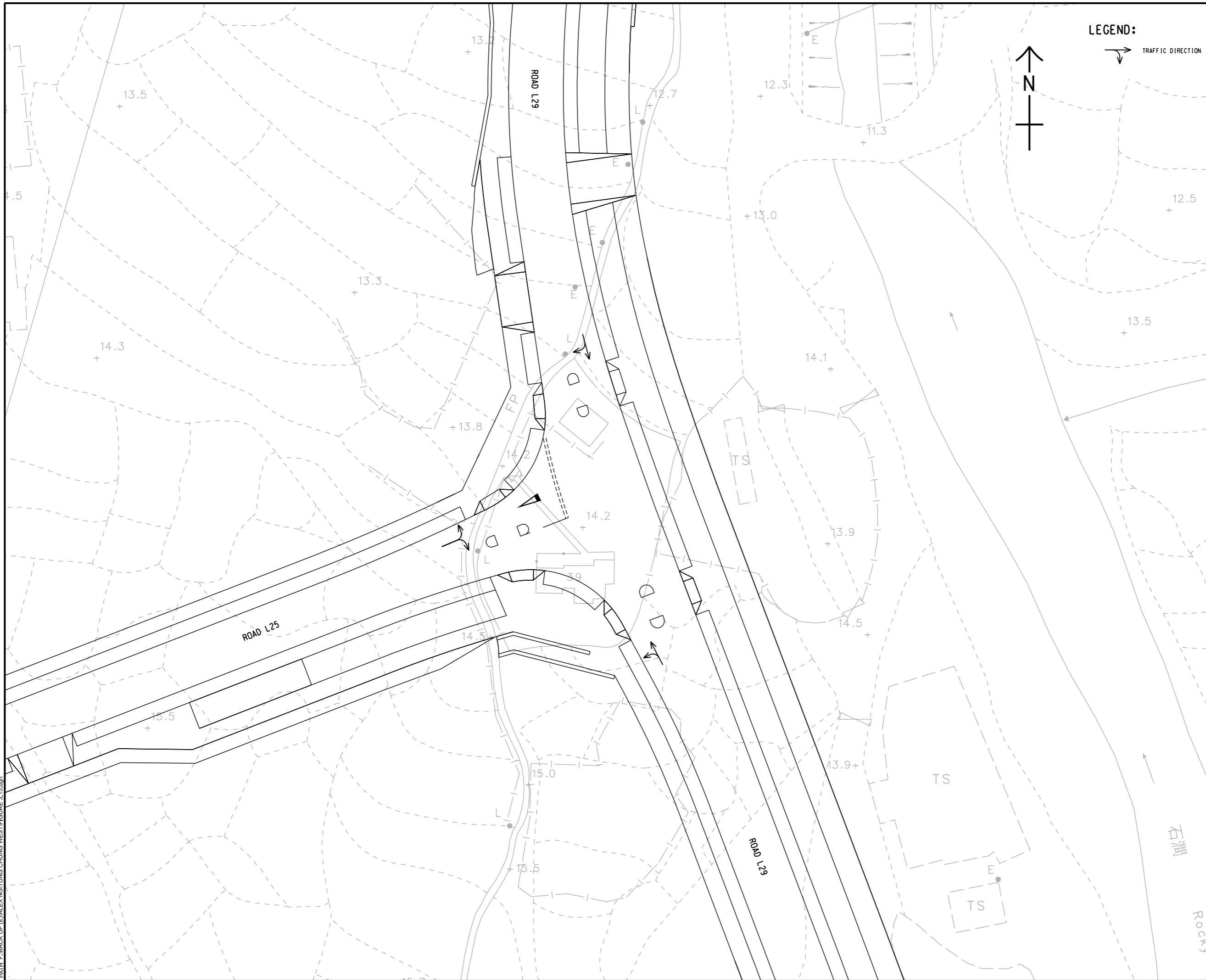
KEY PLAN

PROJECT NO. **CONTRACT NO.**

SHEET TITLE
 PLANNED JUNCTION LAYOUT OF TUNG CHUNG ROAD / ROAD L30 (J8)

SHEET NUMBER
 FIGURE 3.14

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SHEET TITLE

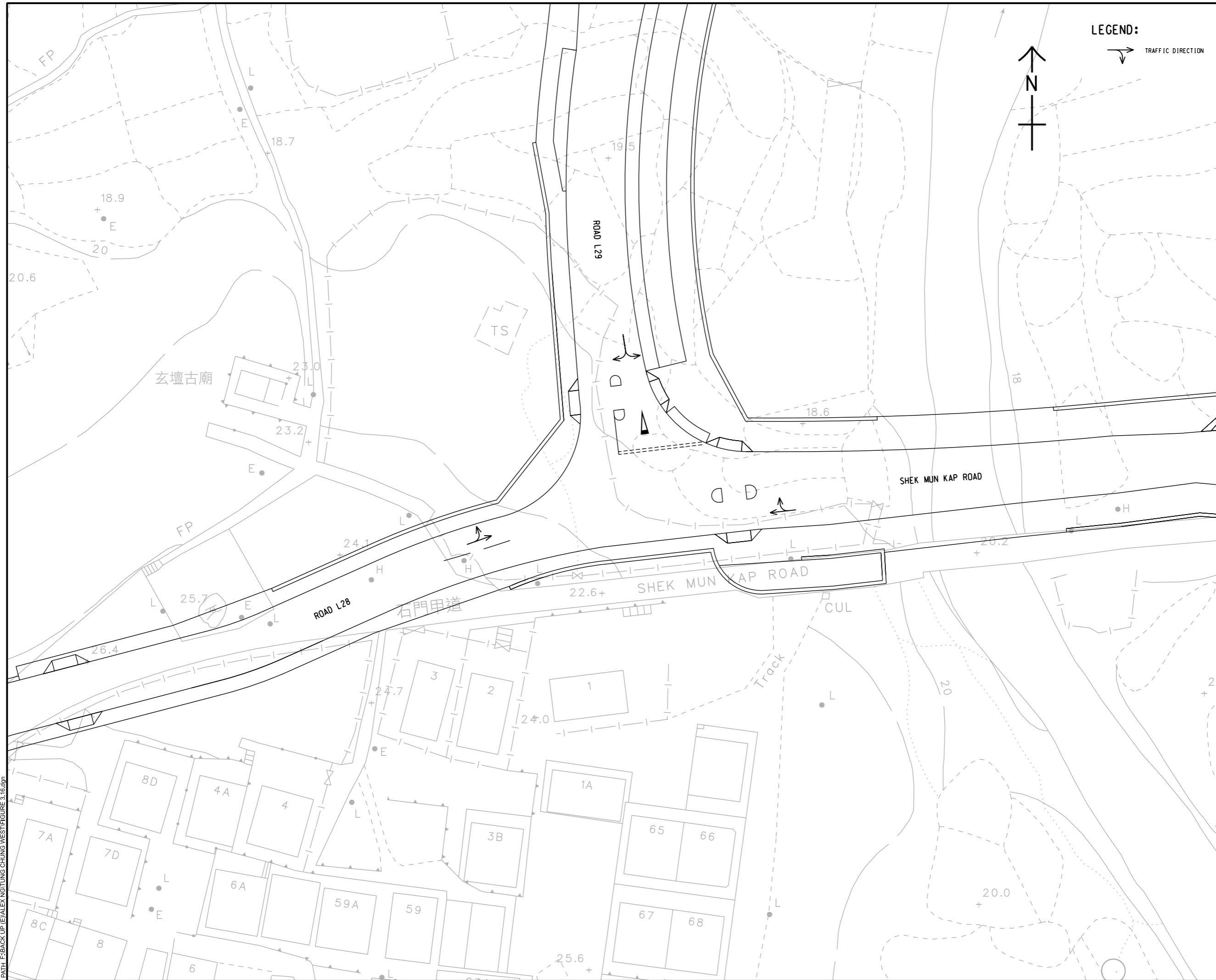
PLANNED JUNCTION LAYOUT OF ROAD L25 / ROAD L29 (J9)

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FIGURE 3.15

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石澗
Rocky Stream



LEGEND:



TRAFFIC DIRECTION



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KEY PLAN

PROJECT NO. **CONTRACT NO.**

SHEET TITLE

PLANNED JUNCTION LAYOUT OF SHEK MUN KAP ROAD / ROAD L28/ ROAD L29 (J10)

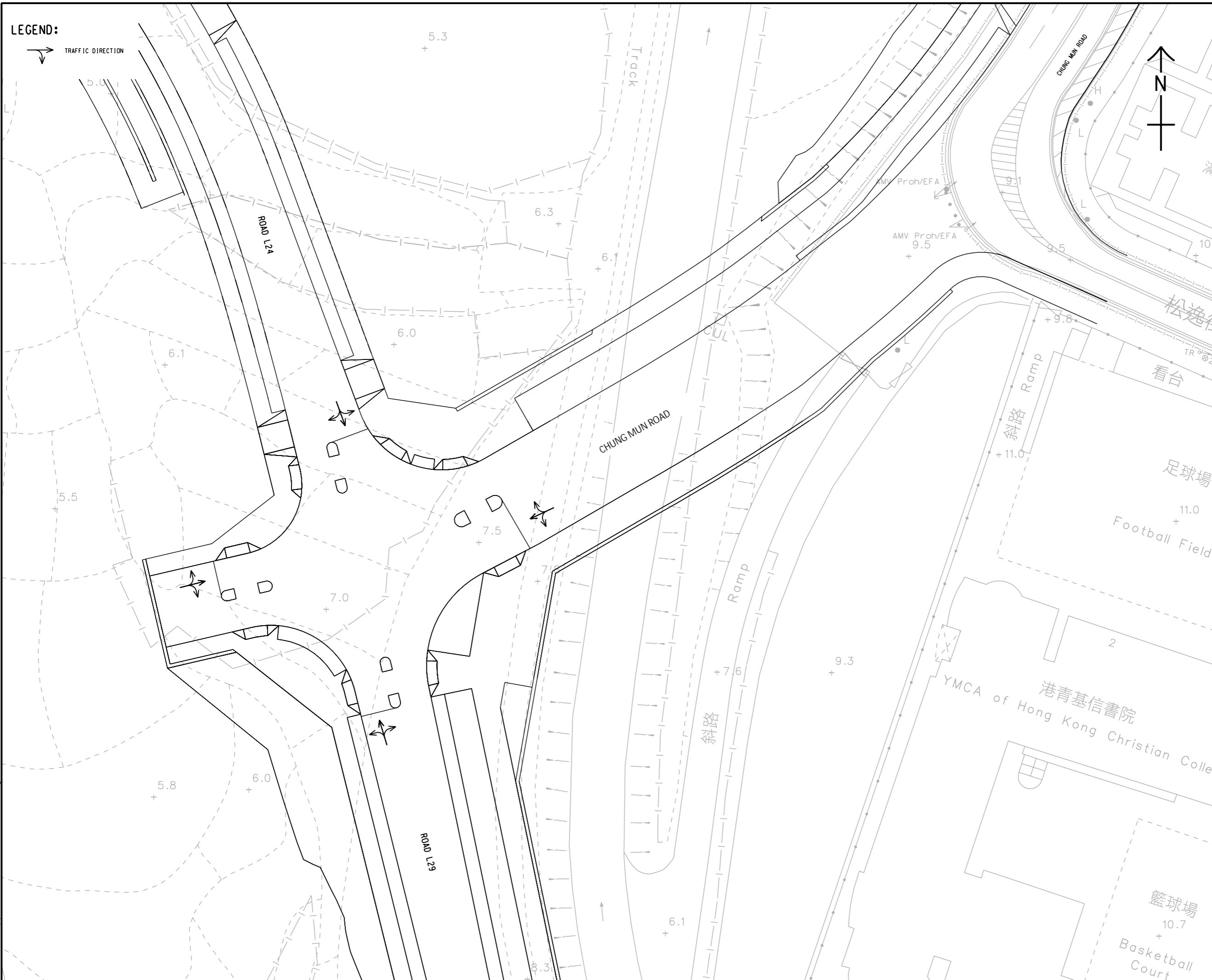
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FIGURE 3.16

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 Checked:
 Designer:
 Project Management Initials:
 Plot File by: chaniowai 11/10/2023
 PATH_F:\BACK UP\EA\ALEX\NOSTUNG CHUNG WEST\FIGURE 3.17.dgn

LEGEND:



PROJECT
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DIMENSION UNIT

KEY PLAN

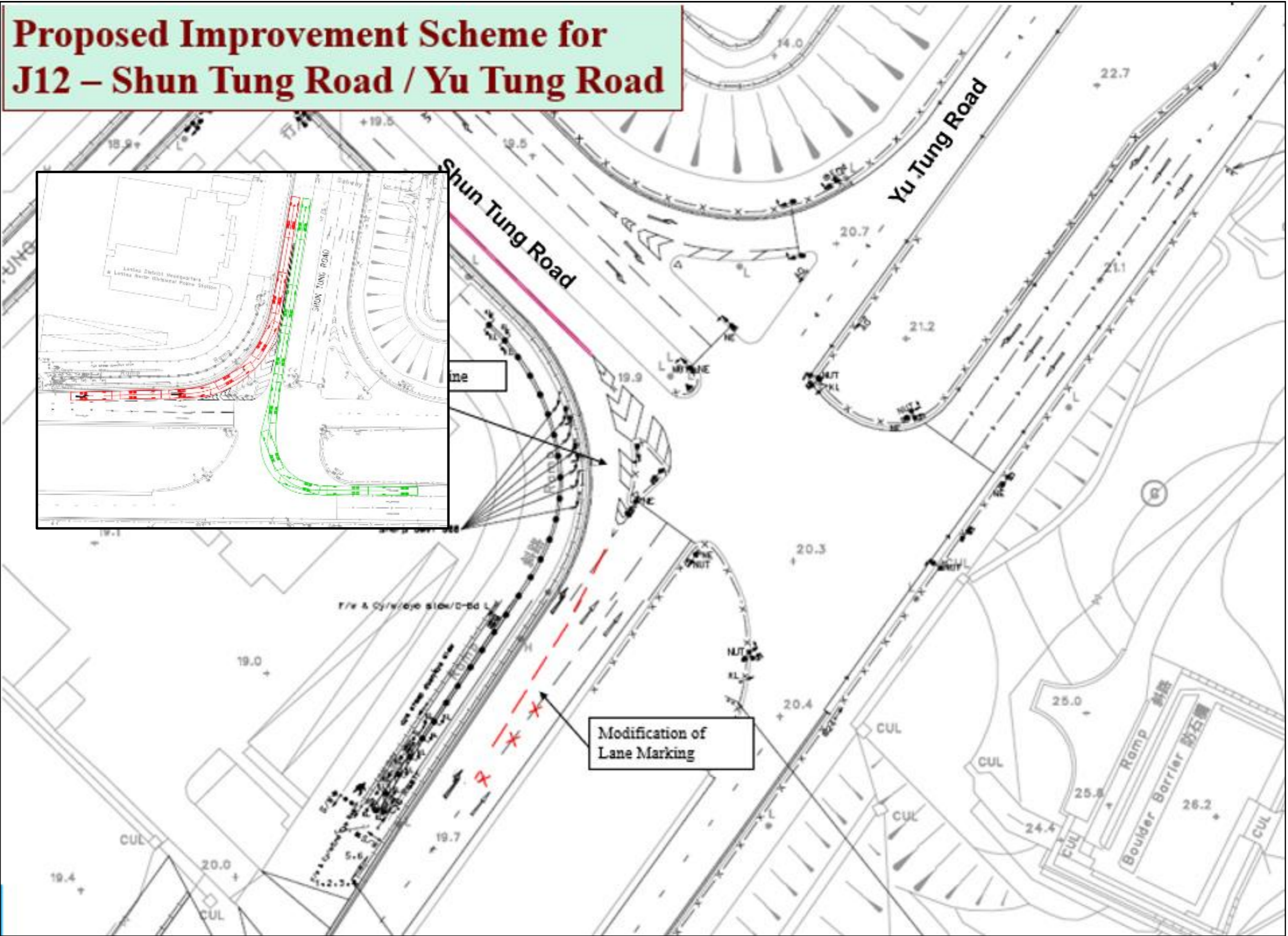
PROJECT NO.
CONTRACT NO.

SHEET TITLE
 PLANNED JUNCTION LAYOUT OF CHUNG MUN ROAD / ROAD 24 / ROAD L29 (J11)

SHEET NUMBER
 FIGURE 3.17

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Proposed Improvement Scheme for J12 – Shun Tung Road / Yu Tung Road



PROJECT
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KEY PLAN

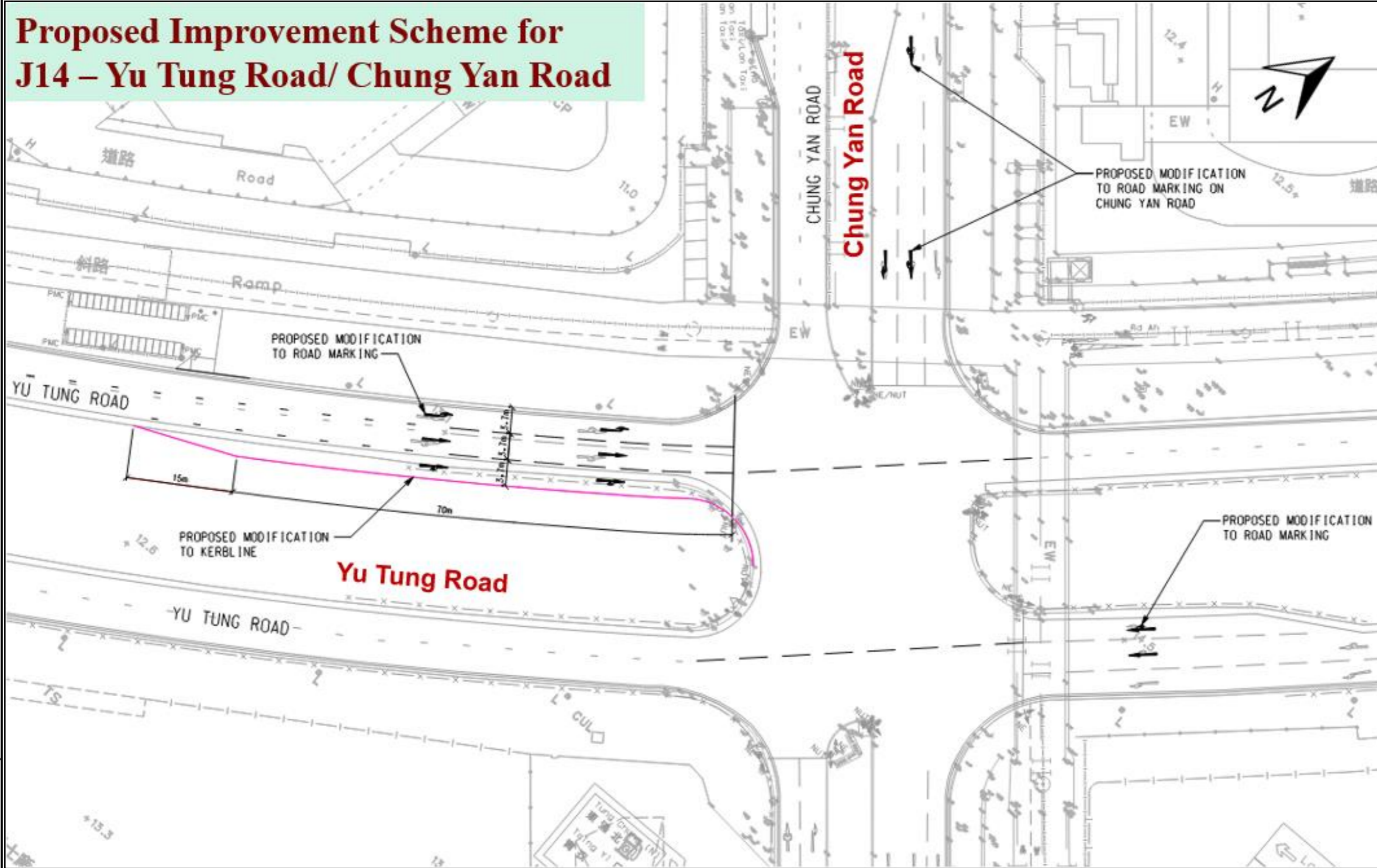
PROJECT NO.
 CONTRACT NO.

SHEET TITLE
 PROPOSED JUNCTION IMPROVEMENT SCHEME OF J2 UNDER TUNG CHUNG NEW TOWN EXTENSION (WEST) TIA (REP-116-02)

SHEET NUMBER
 FIGURE 3.18

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Proposed Improvement Scheme for J14 – Yu Tung Road/ Chung Yan Road



PROJECT
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PROJECT NO. **CONTRACT NO.**

SHEET TITLE
PROPOSED JUNCTION IMPROVEMENT SCHEME OF J3 UNDER TUNG CHUNG NEW TOWN EXTENSION (WEST) TIA (REP-116-02)

SHEET NUMBER

FIGURE 3.19

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LEGEND:



APPLICATION SITE



PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND



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ISSUE/REVISION

I/R	DATE	DESCRIPTION	CHK.

STATUS

SCALE **DIMENSION UNIT**

A3 1: 10000

KEY PLAN

PROJECT NO. **CONTRACT NO.**

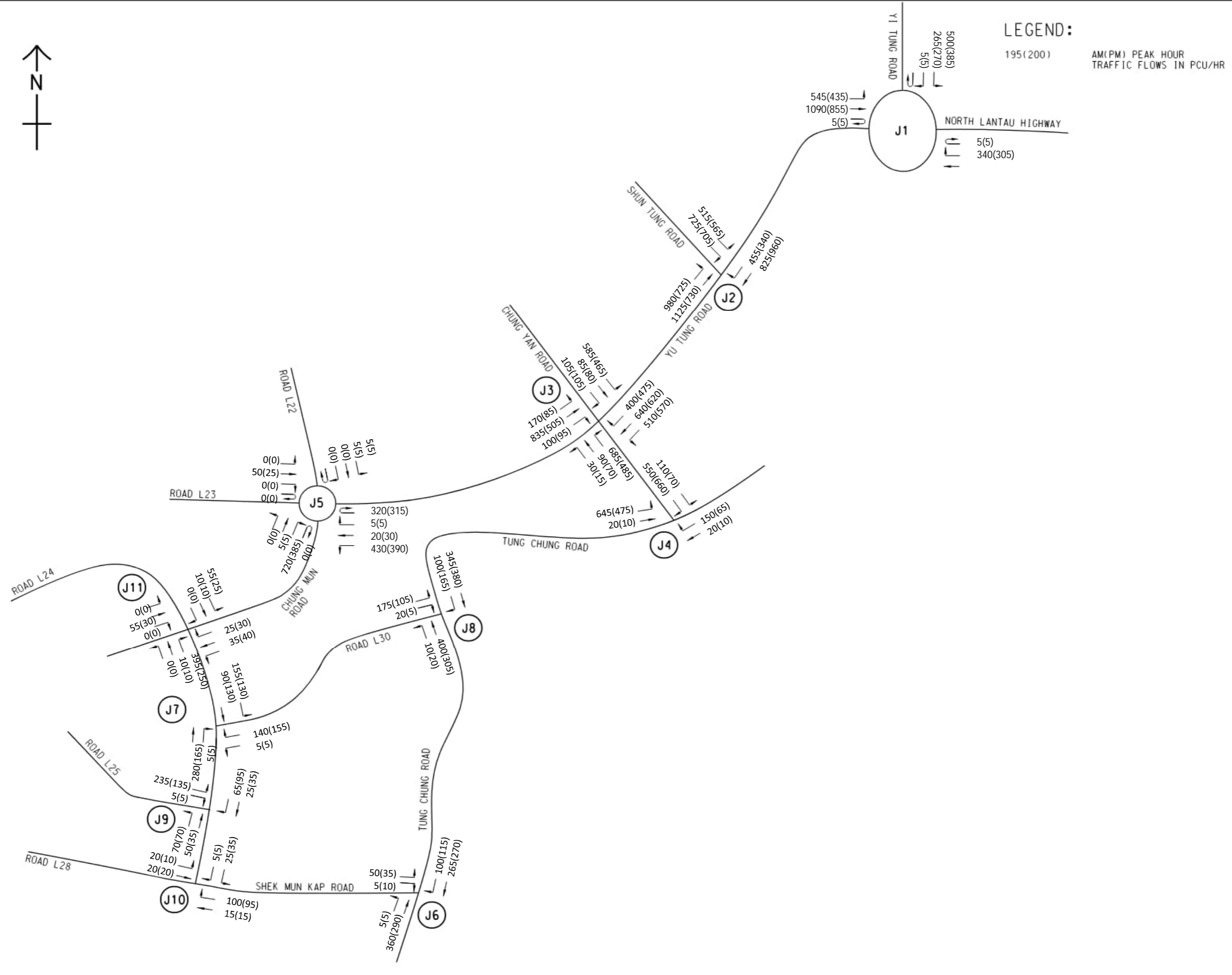
SHEET TITLE

PLANNED / POTENTIAL DEVELOPMENTS IN THE VICINITY OF APPLICATION SITE

SHEET NUMBER

FIGURE 4.1

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LEGEND:
 195(200) AM(PM) PEAK HOUR TRAFFIC FLOWS IN PCU/HR

AECOM

PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP B)" ZONE TO "RESIDENTIAL (GROUP C)2" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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STATUS

SCALE
 A3 N.T.S.

DIMENSION UNIT
 METRES

KEY PLAN

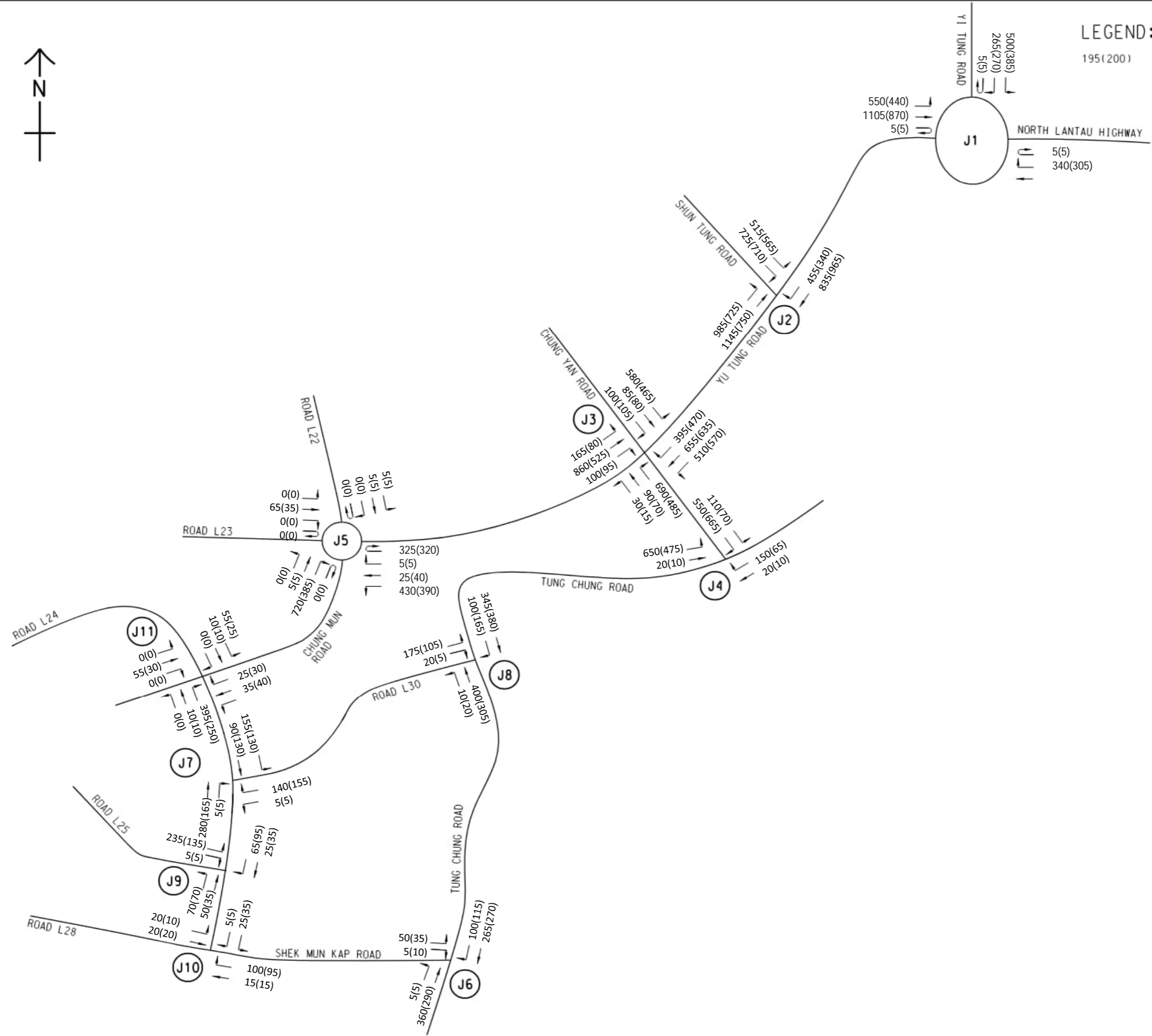
PROJECT NO.
 2033

CONTRACT NO.
 2033

SHEET TITLE
 2033 REFERENCE TRAFFIC FLOWS (CONFORMING SCHEME)

SHEET NUMBER
 4.2

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LEGEND:

195(200)
 AM(PM) PEAK HOUR
 TRAFFIC FLOWS IN PCU/HR



PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE 'RESIDENTIAL (GROUP C)2' ZONE TO 'RESIDENTIAL (GROUP B)2' ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND
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NO.	DATE	DESCRIPTION	CHK.

NO.	DATE	DESCRIPTION	CHK.

STATUS

SCALE **DIMENSION UNIT**
 A3 N.T.S. METRES

KEY PLAN

PROJECT NO. **CONTRACT NO.**

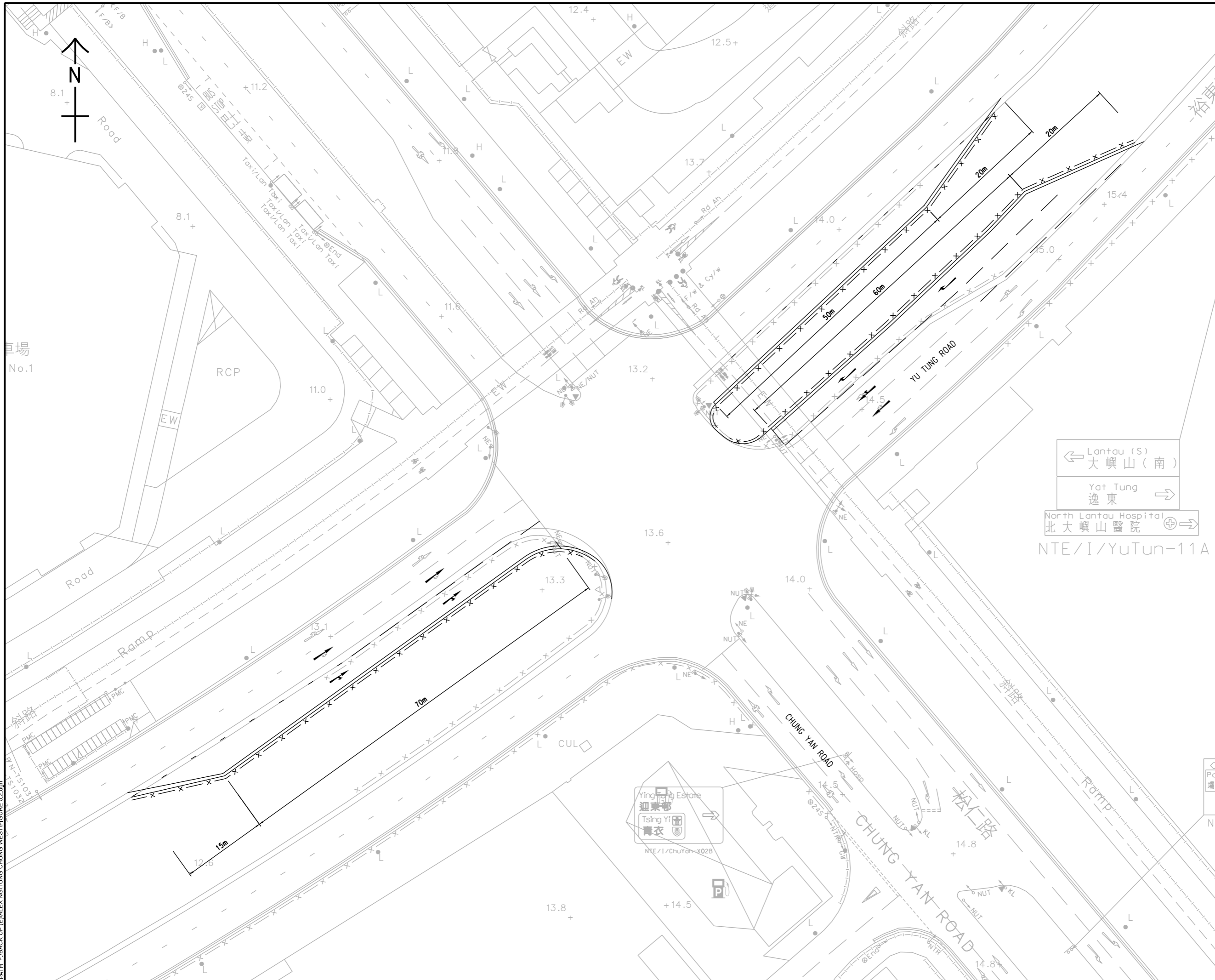
SHEET TITLE

2033 DESIGN TRAFFIC FLOWS (PROPOSED SCHEME)

SHEET NUMBER
 2018/20

FIGURE 4.3

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PROJECT
 SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

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ISSUE/REVISION

I/R	DATE	DESCRIPTION	CHK.

STATUS

SCALE **DIMENSION UNIT**
 A3 1: 600 公尺/米

KEY PLAN

PROJECT NO. **CONTRACT NO.**

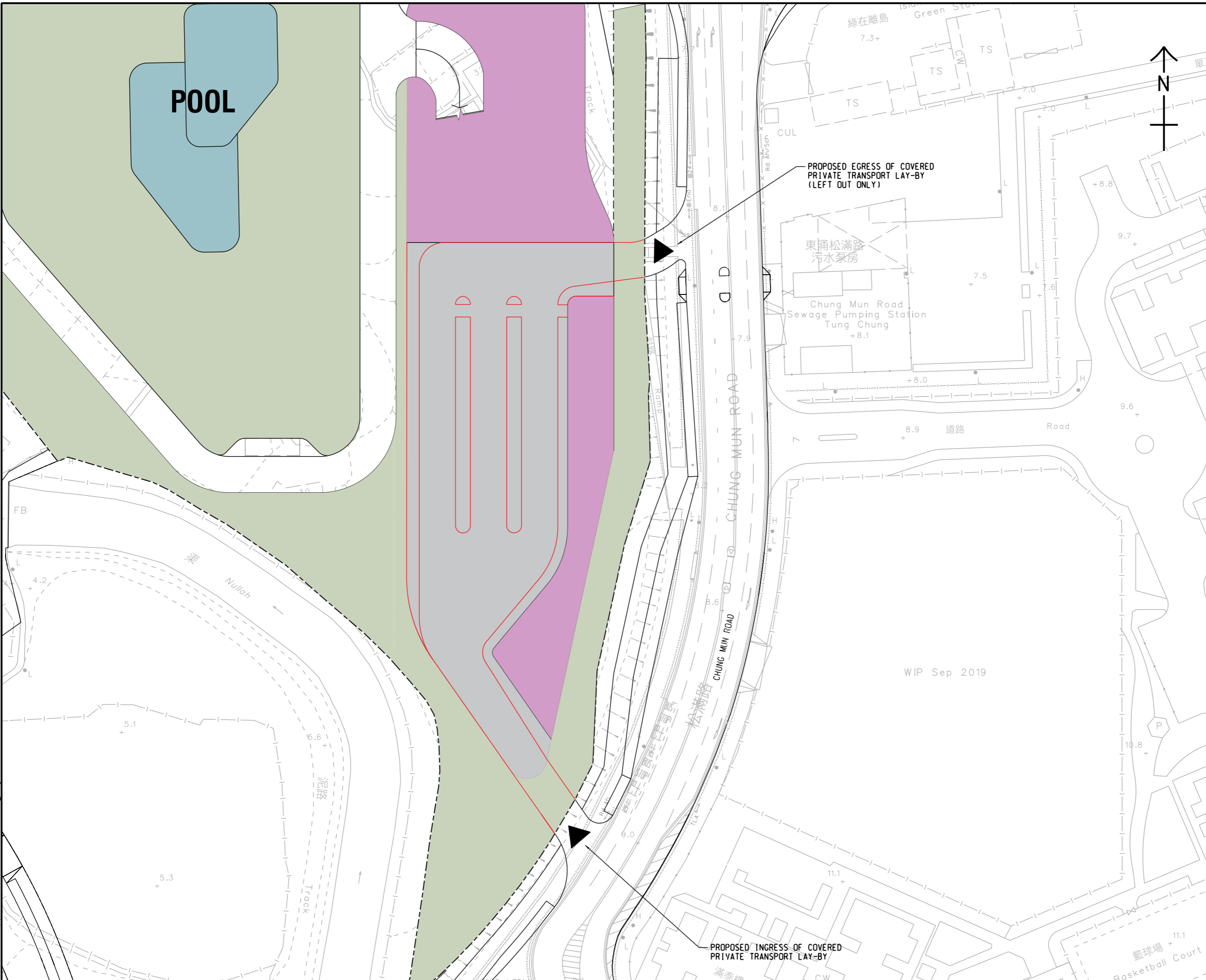
SHEET TITLE
 PROPOSED JUNCTION IMPROVEMENT SCHEME AT J3 (YU TUNG ROAD / CHUNG YAN ROAD)

SHEET NUMBER
 FIGURE 5.2

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Annex A

Indicative Covered Private Transport Lay-by Layout Plan & Swept Path Analysis



PROJECT
項目

SECTION 12A PLANNING APPLICATION FOR PROPOSED AMENDMENTS TO THE TUNG CHUNG VALLEY OUTLINE ZONING PLAN TO REZONE "RESIDENTIAL (GROUP C)2" ZONE TO "RESIDENTIAL (GROUP B)" ZONE IN SUPPORT OF PRIVATE RESIDENTIAL DEVELOPMENT AT VARIOUS LOTS IN D.D. 1 TC AND ADJOINING GOVERNMENT LAND, TUNG CHUNG, LANTAU ISLAND

CLIENT
業主



CONSULTANT
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修訂

I/R	DATE	DESCRIPTION	CHK.

STATUS
階段

SCALE
比例

DIMENSION UNIT
尺寸單位

A3 1: 700

KEY PLAN
索引圖

PROJECT NO.
項目編號

CONTRACT NO.
合約編號

SHEET TITLE
圖紙名稱

INDICATIVE COVERED PRIVATE TRANSPORT LAY-BY LAYOUT PLAN

SHEET NUMBER
圖紙編號

ANNEX A

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Annex B

Assessment Results of Global Parking Standard under HKPSG Adopted for the Application Site

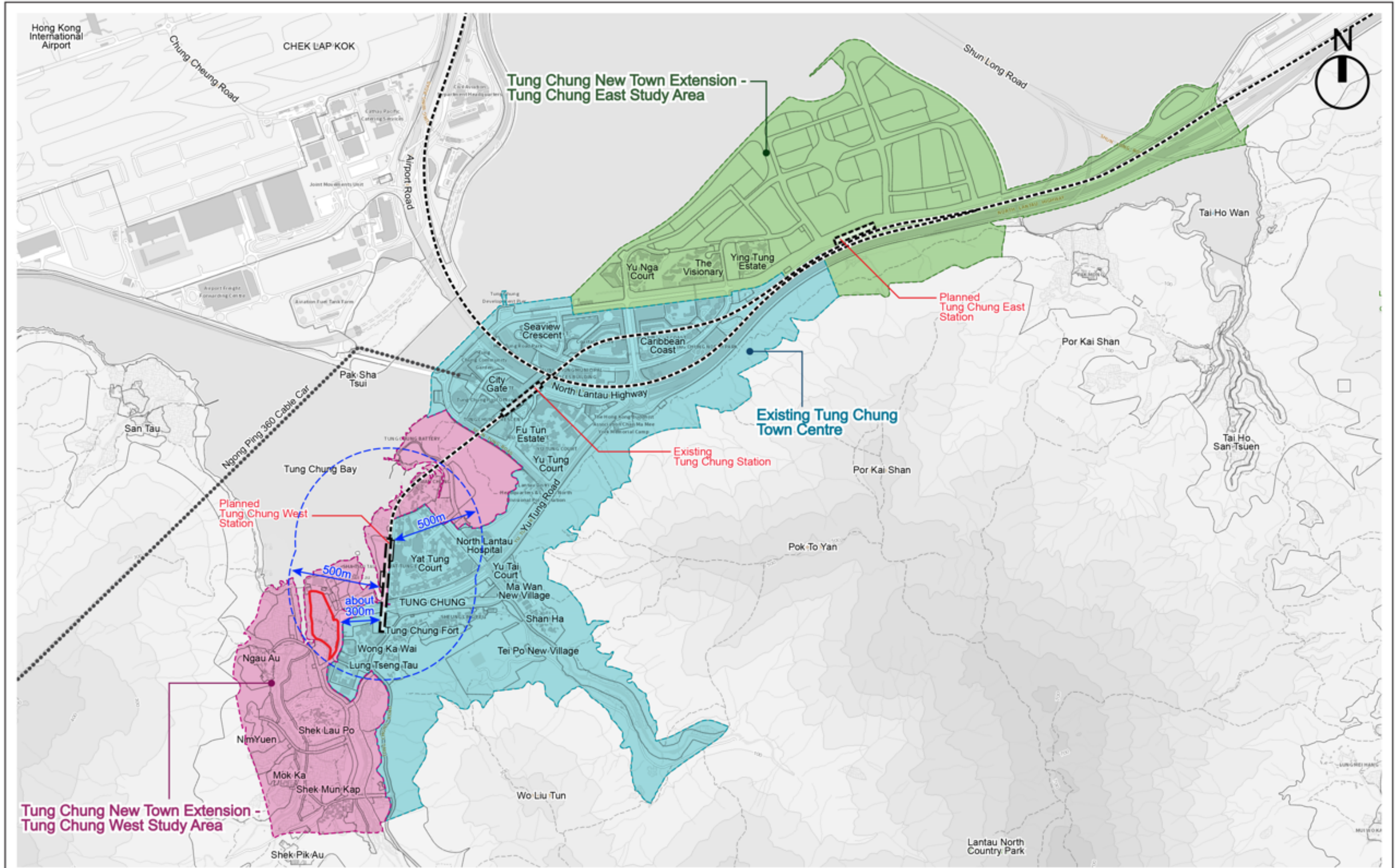
The following Table summarizes our assessment the Global Parking Standard (GPS) under HKPSG adopted for the Subject Site.

	Assessment	Adopted Value	Weighting
Proximity and convenience for access to PT services	There are 3 on-street bus terminus/en-route bus stop covering 15 bus routes at Yu Tung Road and an enroute bus stop covering 2 bus routes at Chung Mun Road. The walking distance from the Subject Site to those stops are about 200m. As stated in TPDM, the ideal walking distance to a bus stop should not exceed 400m. In addition, the proposed transport facilities to the southeast of the Subject Site will be provided to enhance road-based public transport. Hence the development site is considered to be <u>quite accessible</u> to the public transport services.	0.25	20%
Availability of public car parking spaces during peak hours	The nearest Mun Tung Estate car park provides 29 public private car parking spaces, including one designated for disabled drivers, which are available for use by the local community. It is located about 250m from the Subject Site. A recent observation of the Mun Tung Estate car park found that about 30% or more public parking spaces were available during AM and PM peak hours. It indicates that the unused capacity can be allocated to meet additional parking demands from nearby areas. Hence the public car parking spaces are considered to be <u>moderately available</u> for the development site.	0.50	30%
Traffic conditions	The nearby critical junctions in the vicinity has been assess in the TIA. All the assessed junctions including the proposed junction improvement scheme of Junction J3 Yu Tung Road / Chung Yan Road would operate within capacities. Hence the traffic conditions is considered to be <u>moderately congested</u> .	0.50	10%
Level of illegal parking	Illegal parking is observed in the vicinity but the situation is not common. Hence, level of illegal parking is considered <u>slightly severe</u> .	0.25	40%

Based on the above assessment and the adopted values for respective factors, the GPSI is determined as 0.35 (i.e. $0.25 \times 20\% + 0.50 \times 30\% + 0.5 \times 10\% + 0.25 \times 40\%$). Referring to Table -GPS value based on GPSI under HKPSG GPS is 6 if ($0.2 \leq \text{GPSI} < 0.4$), a GPSI of 6 would be appropriate for calculating the carparking provision at the proposed development according to HKPSG requirements.

Annex C

Tung Chung New Town Extension Extracted from Planning Statement



Application Site

100 0 400m

**Ilewelyn
davies**

Title

Tung Chung New Town Extension

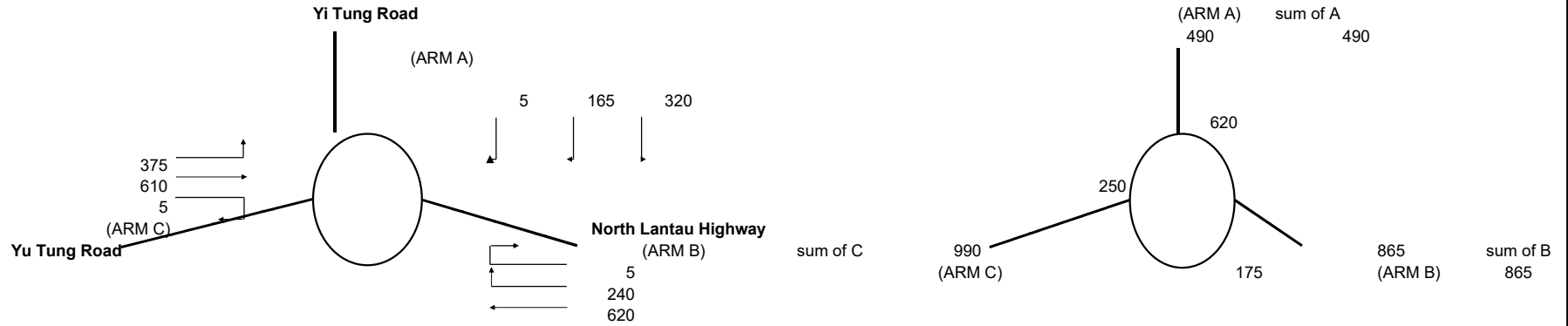
Checked	DH	Drawn	PW
Rev	0	Date	Sep 2024
Scale	Figure		2.3

Annex D

Junction Calculation Sheet

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2023 - Observed Traffic Flows (AM)	Project No.	Prepared By	Checked By	Date
----------	----------------------------------	----------	------------------------------------	-------------	-------------	------------	------



ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	490	865	990
Qc = Circulating flow across entry (pcu/h)	620	175	250
OUTPUT PARAMETERS:			
S = Sharpness of flare = 1.6(E-V)/L	0.64	0.65	0.53
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.00	0.98	0.97
X2 = V + ((E-V)/(1+2S))	9.75	9.22	9.94
M = EXP((D-60)/10)	109.95	109.95	109.95
F = 303*X2	2956	2795	3010
Td = 1+(0.5/(1+M))	1.00	1.00	1.00
Fc = 0.21*Td(1+0.2*X2)	0.62	0.60	0.63
Qe = K(F-Fc*Qc)	2560	2638	2774
DFC = Design flow/Capacity = Q/Qe	0.19	0.33	0.36

TOTAL ENTRY FLOWS = 2345 PCU
CRITICAL DFC = 0.36

JUNCTION CAPACITY CALCULATION

Junction J2 - Yu Tung Road / Shun Tung Road

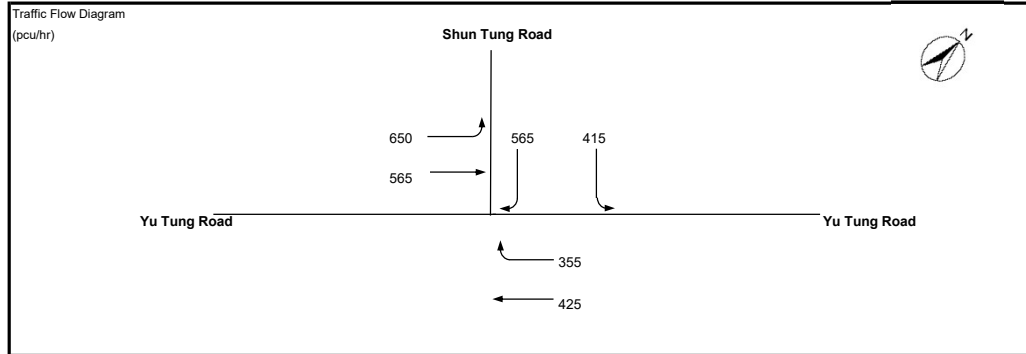
2023 AM Observed Traffic Flows

DESIGN:

CHECK:

JOB NO:

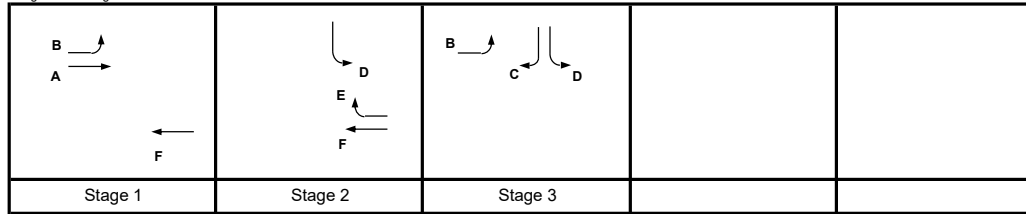
DATE: Nov 20



No. of stages per cycle	N =	3
Cycle time	C =	120 sec
Sum(y)	Y =	0.510
Lost time	L =	15 sec
Total Flow	=	18,780 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	56 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	31 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.788
R.C. _{ult}	= $(Y_{ult} - Y) / Y \times 100\%$	54.5 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	35 sec
Y_{max}	= $1 - L / C$	0.875

J2

Stage/Phase Diagrams



Critical Case : B,E

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 54\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road NB	B A							1,3 1	4.000 4.000	1 2		50				
Shun Tung Road EB	D C	2,3 3	3.700 3.700	1 2	25		0 0				1985 4250	415		415 565	100%	100%	1873 4048	0.222 0.140			
Yu Tung Road SB	F E	1,2 2	3.650 3.650	2 1			0 0				4100 2120		425	425 355		100%	4100 2000	0.104 0.178	0.178		

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road

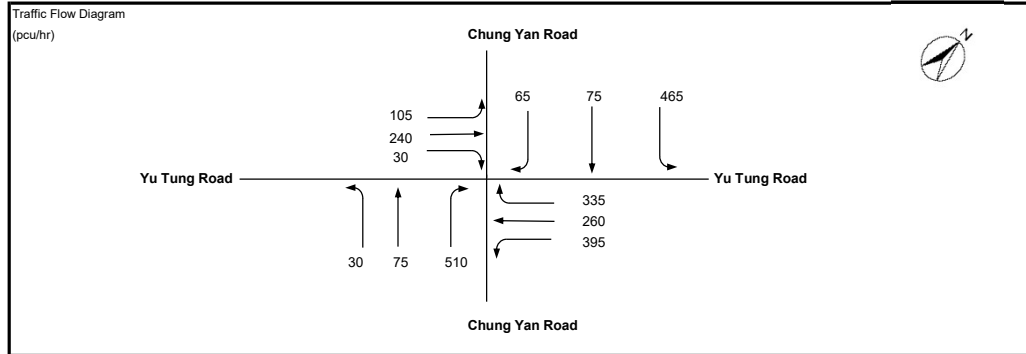
2023 AM Observed Traffic Flows

DESIGN:

CHECK:

JOB NO:

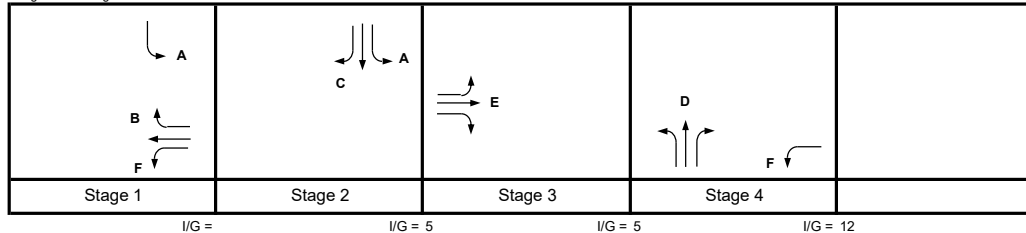
DATE: Nov 20



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.455
Lost time	L =	19 sec
Total Flow	=	20,510 pcu
Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	61 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	35 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.758
R.C. _{ult}	$= (Y_{ult} - Y) / Y \times 100\% =$	66.5 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	38 sec
Y_{max}	$= 1 - L/C =$	0.842

J3

Stage/Phase Diagrams



Critical Case : A,E,D

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 67\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		15				
	B	1	3.600	1		20	0	0			2115			66	316		18%	2087	0.152		
	B	1	3.600	1		10	0	0			2115			279	279		100%	1839	0.152		
Chung Yan Road WB	D	4	3.500	1	25	25	0	1		0	1965	30	75	194	299	10%	65%	1880	0.159		
	D	4	3.500	1		25	0	0		0	2105			316	316		100%	1986	0.159	0.159	
Yu Tung Road NB	E	3	3.500	1	15		0	1		0	1965	105	72		177	59%		1855	0.095		
	E	3	3.500	1		15	0	0		0	2105		168	30	198		15%	2074	0.095	0.095	
Chung Yan Road EB	A	1,2	3.500	1	16			1		0	1965	360			360	100%		1797	0.200	0.200	
	C	2	3.500	1	18					0	2105	105	15		120	88%		1962	0.061		
	C	2	3.500	1		25	0	0		0	2105		60	65	125		52%	2041	0.061		

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

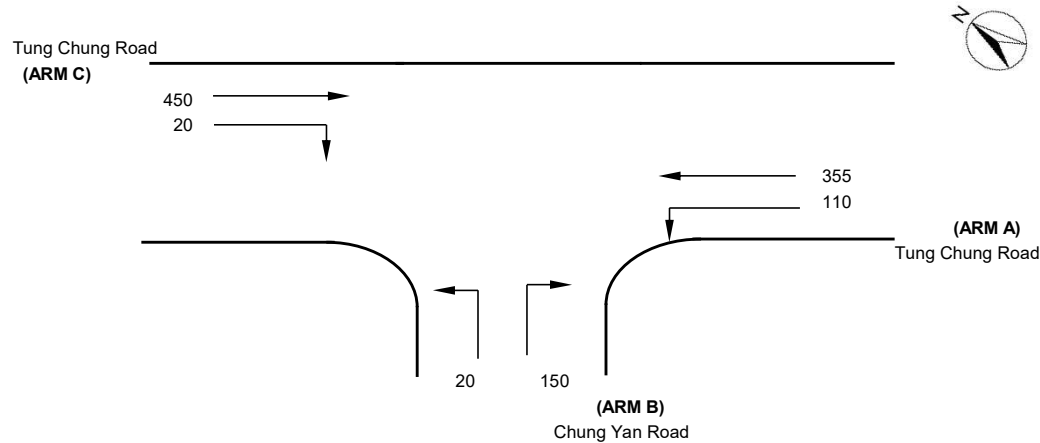
2023 AM Observed Traffic Flows

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	110 (pcu/hr)
q a-c	=	355 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	450 (pcu/hr)
q c-b	=	20 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	150 (pcu/hr)
q b-c	=	20 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	411
Q b-c	=	622
Q c-b	=	604
Q b-ac	=	428

CRITICAL DFC = 0.40

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.36
DFC b-c	=	0.03
DFC c-b	=	0.03
DFC b-ac	=	0.40

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J5 - Chung Mun Road / Yu Tung Road

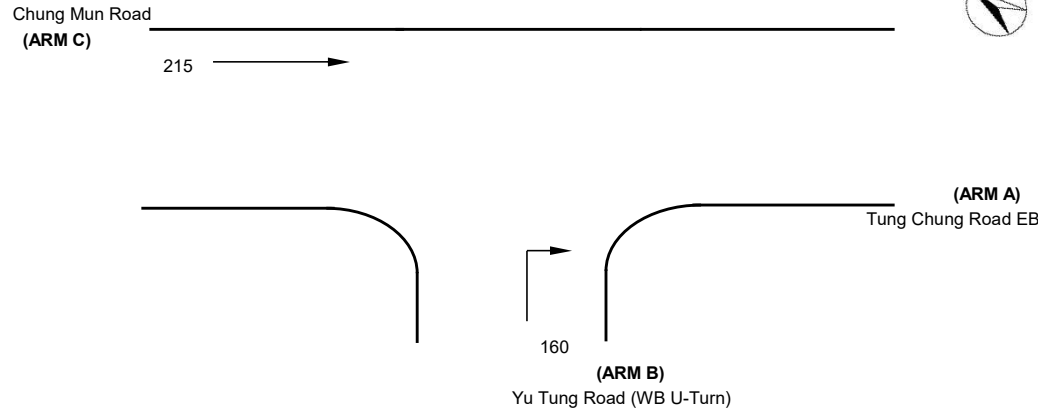
2023 AM Observed Traffic Flows

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J5

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	8.8 (metres)
W cr	=	0 (metres)
q a-b	=	0 (pcu/hr)
q a-c	=	0 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	4 (metres)
Vr c-b	=	200 (metres)
q c-a	=	215 (pcu/hr)
q c-b	=	0 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	50 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	17 (metres)
q b-a	=	160 (pcu/hr)
q b-c	=	0 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.002944
E	=	0.903036
F	=	1.107269
Y	=	0.696400

THE CAPACITY OF MOVEMENT :

Q b-a	=	594
Q b-c	=	673
Q c-b	=	825
Q b-ac	=	594

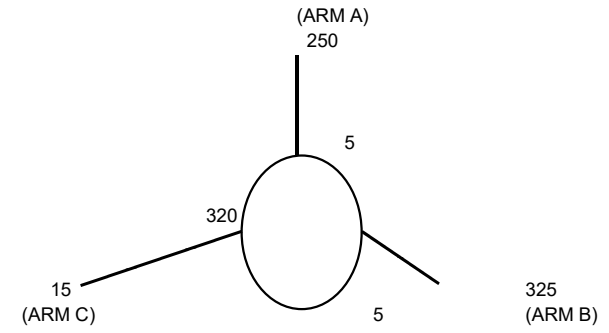
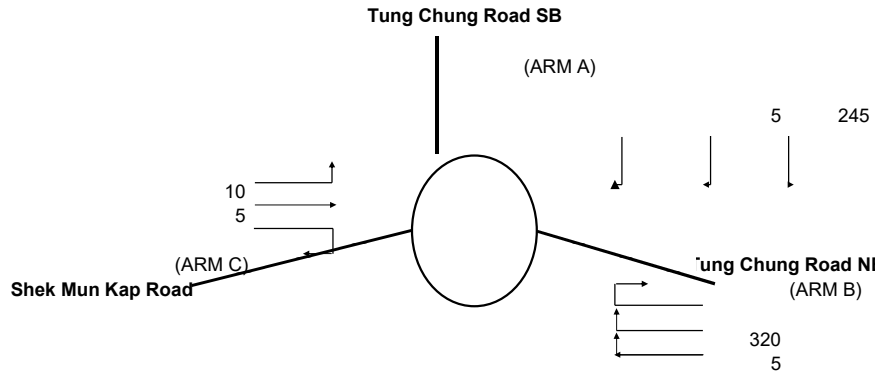
CRITICAL DFC = 0.27

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.27
DFC b-c	=	0.00
DFC c-b	=	0.00
DFC b-ac	=	0.27

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung Road / Shek Mun Kap Road (J6)	Scenario	2023 - Observed Traffic Flows (AM)	Project No.	Prepared By	Checked By	Date
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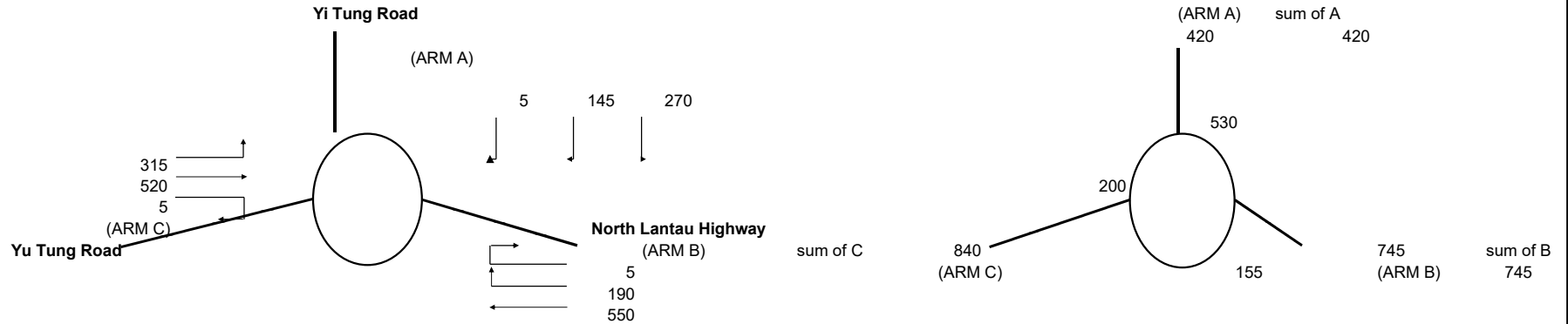
ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	3.50	3.50	5.00
E = Entry width (m)	3.50	6.00	4.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	20.00	20.00	20.00
D = Inscribed circle diameter (m)	22.00	22.00	22.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	250	325	15
Qc= Circulating flow across entry (pcu/h)	5	5	320
OUTPUT PARAMETERS:			
S = Sharpness of flare = $1.6(E-V)/L$	0.00	0.33	-0.13
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.97	0.95	0.95
X2= $V + ((E-V)/(1+2S))$	3.50	5.00	3.64
M = $EXP((D-60)/10)$	0.02	0.02	0.02
F = $303 \times X2$	1061	1515	1102
Td= $1+(0.5/(1+M))$	1.49	1.49	1.49
Fc= $0.21 \times Td(1+0.2 \times X2)$	0.53	0.63	0.54
Qe= $K(F-Fc \times Qc)$	1021	1433	881
DFC = Design flow/Capacity = Q/Qe	0.24	0.23	0.02

TOTAL ENTRY FLOWS = 590 PCU

CRITICAL DFC = 0.24

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2023 - Observed Traffic Flows (AM)	Project No.	Prepared By	Checked By	Date
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ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	420	745	840
Qc = Circulating flow across entry (pcu/h)	530	155	200
OUTPUT PARAMETERS:			
S = Sharpness of flare = 1.6(E-V)/L	0.64	0.65	0.53
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.00	0.98	0.97
X2 = V + ((E-V)/(1+2S))	9.75	9.22	9.94
M = EXP((D-60)/10)	109.95	109.95	109.95
F = 303*X2	2956	2795	3010
Td = 1+(0.5/(1+M))	1.00	1.00	1.00
Fc = 0.21*Td(1+0.2*X2)	0.62	0.60	0.63
Qe = K(F-Fc*Qc)	2616	2649	2805
DFC = Design flow/Capacity = Q/Qe	0.16	0.28	0.30

TOTAL ENTRY FLOWS = 2005 PCU
CRITICAL DFC = 0.30

JUNCTION CAPACITY CALCULATION

Junction J2 - Yu Tung Road / Shun Tung Road

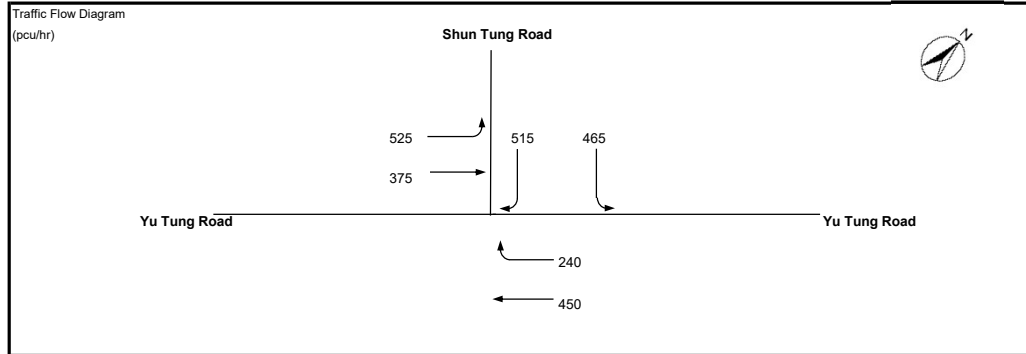
2023 AM Observed Traffic Flows

DESIGN:

CHECK:

JOB NO:

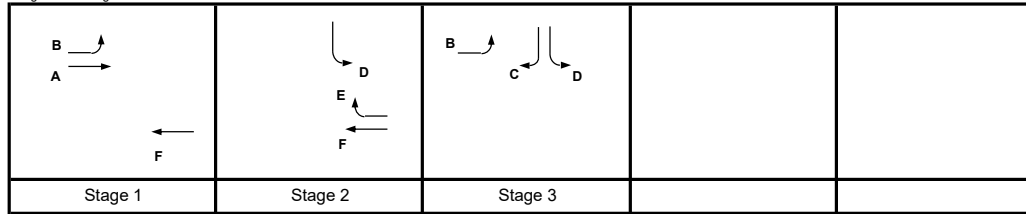
DATE: Nov 20



No. of stages per cycle	N =	3
Cycle time	C =	120 sec
Sum(y)	Y =	0.388
Lost time	L =	15 sec
Total Flow	=	18,780 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	45 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	25 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.788
$R.C._{ult}$	= $(Y_{ult} - Y) / Y \times 100\%$	102.8 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	26 sec
Y_{max}	= $1 - L/C$	0.875

J2

Stage/Phase Diagrams



I/G = 5

I/G = 12

I/G =

Critical Case : B,E

$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 103\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road NB	B A							1,3 1	4.000 4.000	1 2		50				
Shun Tung Road EB	D C	2,3 3	3.700 3.700	1 2	25		0 0	1 0				1985 4250	465		465 515	100%	100%	1873 4048	0.248 0.127		
Yu Tung Road SB	F E	1,2 2	3.650 3.650	2 1			0 0	1 0				4100 2120		450		450 240		100%	4100 2000	0.110 0.120	0.120

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road

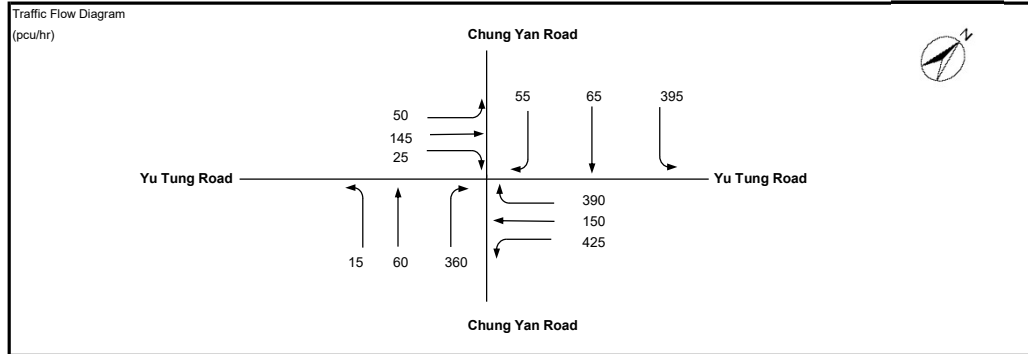
2023 AM Observed Traffic Flows

DESIGN:

CHECK:

JOB NO:

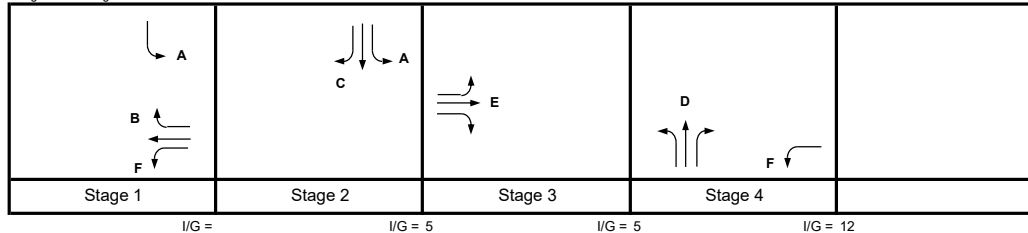
DATE: Nov 20



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.346
Lost time	L =	19 sec
Total Flow	=	20,510 pcu
Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	51 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	29 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.758
$R.C._{ult}$	$= (Y_{ult} - Y) / Y \times 100\% =$	118.7 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	31 sec
Y_{max}	$= 1 - L/C =$	0.842

J3

Stage/Phase Diagrams



Critical Case : A,E,D

$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 119\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		15				
	B	1	3.600	1	20	0	0	0			2115			284	284	47%	2043	0.139			
	B	1	3.600	1	10	0	0	0			2115		256	256	100%	1839	0.139				
Chung Yan Road WB	D	4	3.500	1	25	25	0	1	0		1965	15	60	137	212	7%	65%	1884	0.112	0.112	
	D	4	3.500	1	25	25	0	0	0		2105		223	223	100%	1986	0.112				
Yu Tung Road NB	E	3	3.500	1	15		0	1	0		1965	50	55		105	48%		1876	0.056		
	E	3	3.500	1	15	15	0	0	0		2105		90	25	115	22%	22%	2060	0.056	0.056	
Chung Yan Road EB	A	1,2	3.500	1	16		0	1	0		1965	320	21		320	100%		1797	0.178	0.178	
	C	2	3.500	1	18		0	0	0		2105	75	21		96	78%		1976	0.049		
	C	2	3.500	1	25	0	0	0	0		2105		44	55	99	56%	56%	2037	0.049		

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

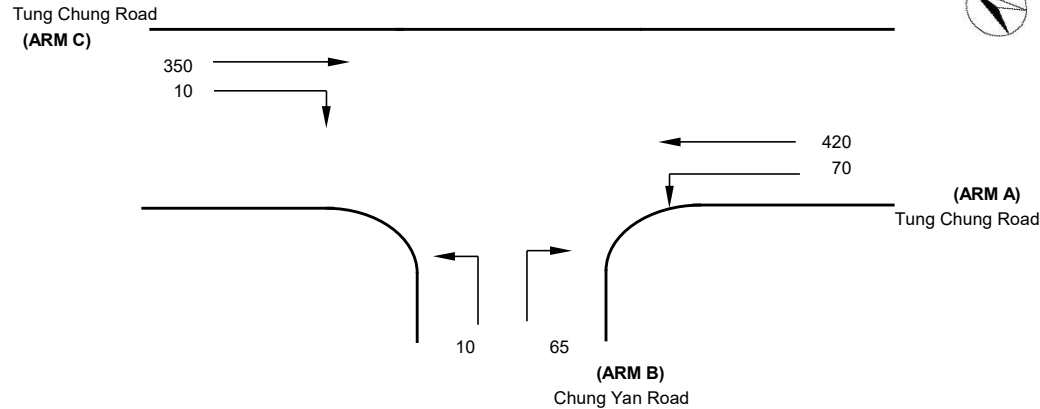
2023 AM Observed Traffic Flows

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	420 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	350 (pcu/hr)
q c-b	=	10 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	65 (pcu/hr)
q b-c	=	10 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	418
Q b-c	=	609
Q c-b	=	598
Q b-ac	=	437

CRITICAL DFC = 0.17

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.16
DFC b-c	=	0.02
DFC c-b	=	0.02
DFC b-ac	=	0.17

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J5 - Chung Mun Road / Yu Tung Road

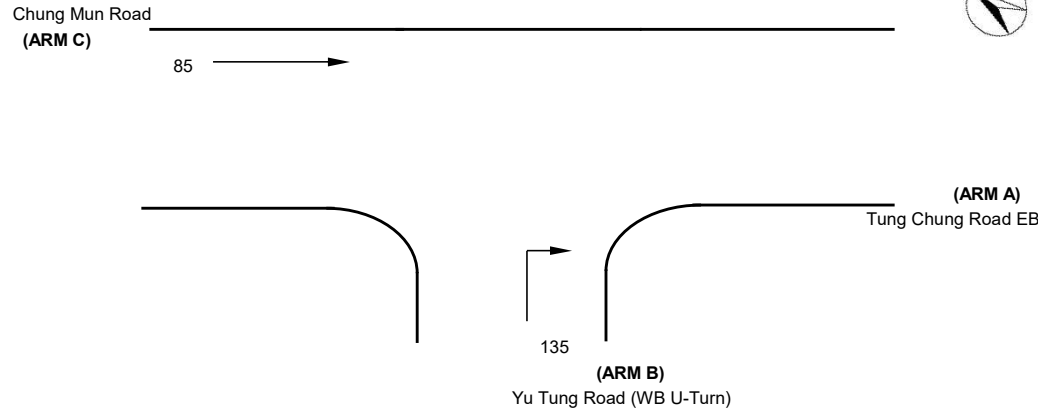
2023 AM Observed Traffic Flows

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J5

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	8.8 (metres)
W cr	=	0 (metres)
q a-b	=	0 (pcu/hr)
q a-c	=	0 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	4 (metres)
Vr c-b	=	200 (metres)
q c-a	=	85 (pcu/hr)
q c-b	=	0 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	50 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	17 (metres)
q b-a	=	135 (pcu/hr)
q b-c	=	0 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.002944
E	=	0.903036
F	=	1.107269
Y	=	0.696400

THE CAPACITY OF MOVEMENT :

Q b-a	=	615
Q b-c	=	673
Q c-b	=	825
Q b-ac	=	615

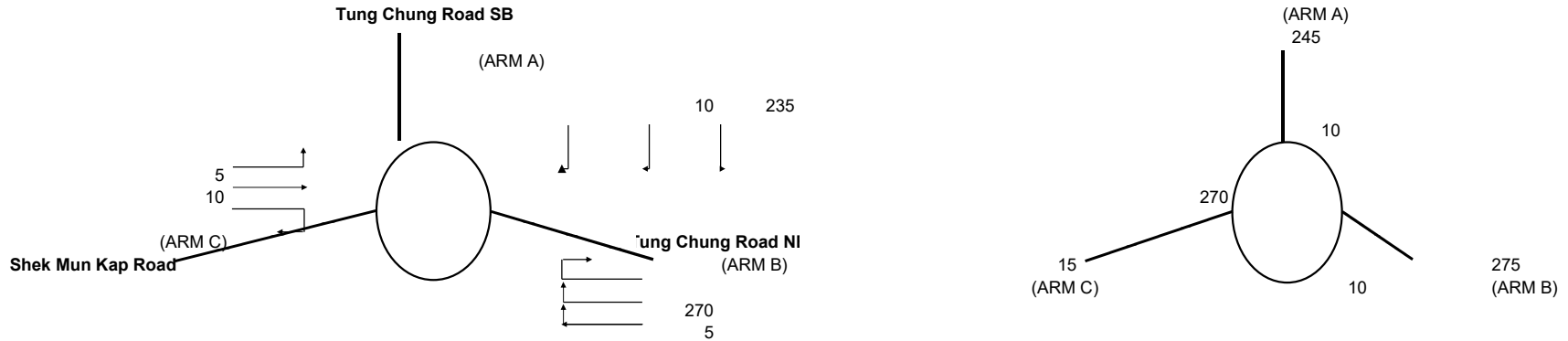
CRITICAL DFC = 0.22

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.22
DFC b-c	=	0.00
DFC c-b	=	0.00
DFC b-ac	=	0.22

ROUNDAABOUT CAPACITY CALCULATION

Junction	Tung Chung Road / Shek Mun Kap Road (J6)	Scenario	2023 - Observed Traffic Flows (AM)	Project No.	Prepared By	Checked By	Date
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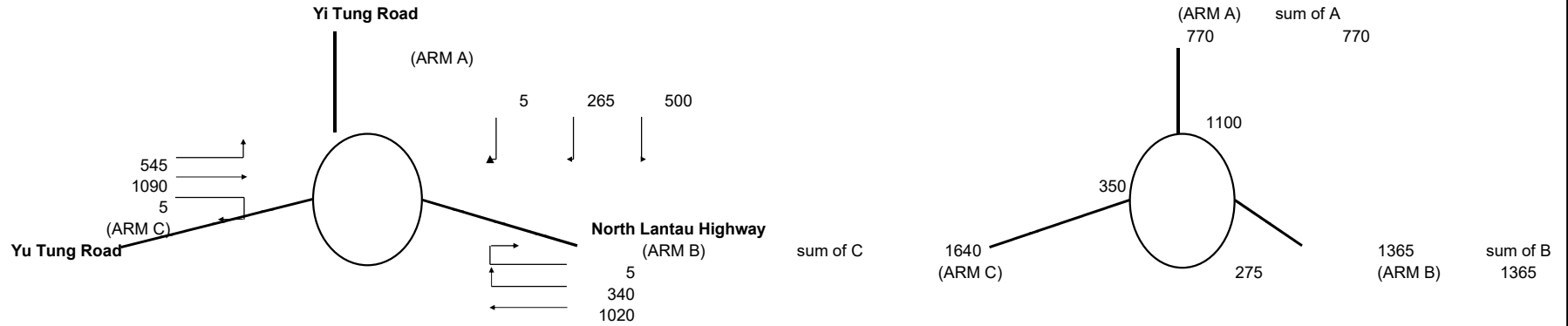


ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	3.50	3.50	5.00
E = Entry width (m)	3.50	6.00	4.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	20.00	20.00	20.00
D = Inscribed circle diameter (m)	22.00	22.00	22.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	245	275	15
Qc = Circulating flow across entry (pcu/h)	10	10	270
OUTPUT PARAMETERS:			
S = Sharpness of flare = $1.6(E-V)/L$	0.00	0.33	-0.13
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	0.97	0.95	0.95
X2 = $V + ((E-V)/(1+2S))$	3.50	5.00	3.64
M = $EXP((D-60)/10)$	0.02	0.02	0.02
F = $303 \times X2$	1061	1515	1102
Td = $1+(0.5/(1+M))$	1.49	1.49	1.49
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.53	0.63	0.54
Qe = $K(F-Fc \times Qc)$	1019	1430	906
DFC = Design flow/Capacity = Q/Qe	0.24	0.19	0.02

TOTAL ENTRY FLOWS = 535 PCU
CRITICAL DFC = 0.24

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2033 - AM - Reference Traffic Flows (Conforming Scheme)	Project No.	Prepared By	Checked By	Date
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ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	770	1365	1640
Qc = Circulating flow across entry (pcu/h)	1100	275	350
OUTPUT PARAMETERS:			
S = Sharpness of flare = 1.6(E-V)/L	0.64	0.65	0.53
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.00	0.98	0.97
X2 = V + ((E-V)/(1+2S))	9.75	9.22	9.94
M = EXP((D-60)/10)	109.95	109.95	109.95
F = 303*X2	2956	2795	3010
Td = 1+(0.5/(1+M))	1.00	1.00	1.00
Fc = 0.21*Td(1+0.2*X2)	0.62	0.60	0.63
Qe = K(F-Fc*Qc)	2263	2579	2713
DFC = Design flow/Capacity = Q/Qe	0.34	0.53	0.60

TOTAL ENTRY FLOWS = 3775 PCU
CRITICAL DFC = 0.60

JUNCTION CAPACITY CALCULATION

Junction J2 - Yu Tung Road / Shun Tung Road - Improved Scheme

2033 AM Reference Traffic Flows (Conforming Scheme) (With CEDD Improvements)

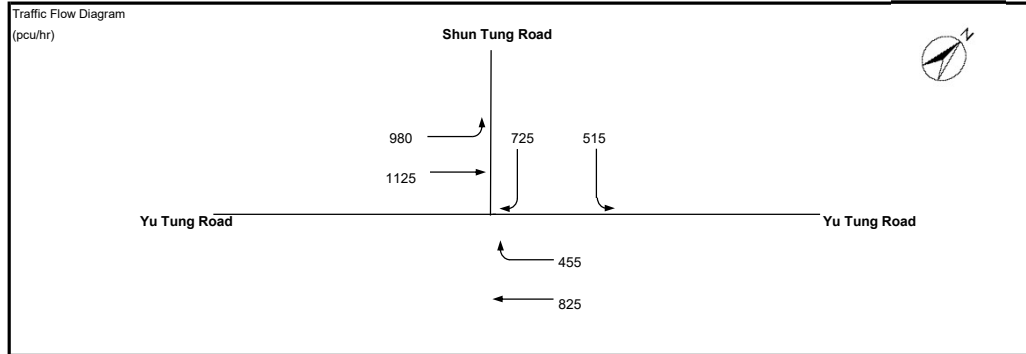
DESIGN:

CHECK:

JOB NO:

DATE: Nov 20

Traffic Flow Diagram
(pcu/hr)

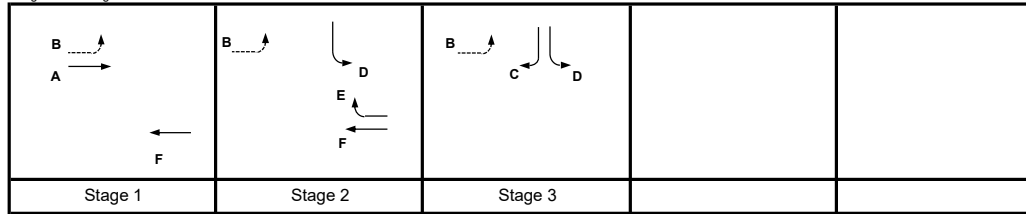


No. of stages per cycle	N =	3
Cycle time	C =	120 sec
Sum(y)	Y =	0.668
Lost time	L =	12 sec
Total Flow	=	18,780 pcu

J2

Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	=	69	sec
Min. Cycle Time C_m	= $L / (1 - Y)$	=	36	sec
Y_{ult}	= $0.9 - 0.0075 \times L$	=	0.810	
R.C. _{ult}	= $(Y_{ult} - Y) / Y \times 100\%$	=	21.3	%
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	=	46	sec
Y_{max}	= $1 - L / C$	=	0.900	

Stage/Phase Diagrams



Critical Case : A,E,C

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 21\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road NB	A							1	4.000	2		50				
Shun Tung Road EB	D	2,3	3.700	1	25		0	0			1985	515		515	100%		1873	0.275	0.179		
	C	3	3.700	2		30	0	0			4250		725	725		100%	4048	0.179	0.179		
Yu Tung Road SB	F	1,2	3.650	2			0	0			4100		825	825		100%	4100	0.201	0.228		
	E	2	3.650	1		25	0	0			2120		455	455		100%	2000	0.228	0.228		

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

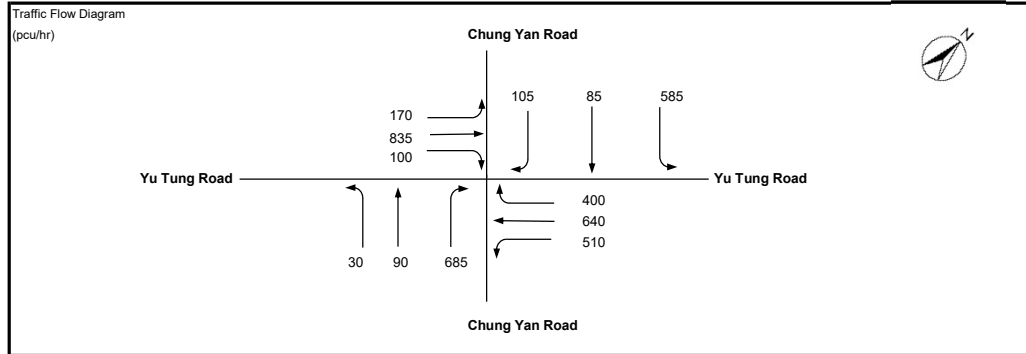
2033 AM Reference Traffic Flows (Conforming Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

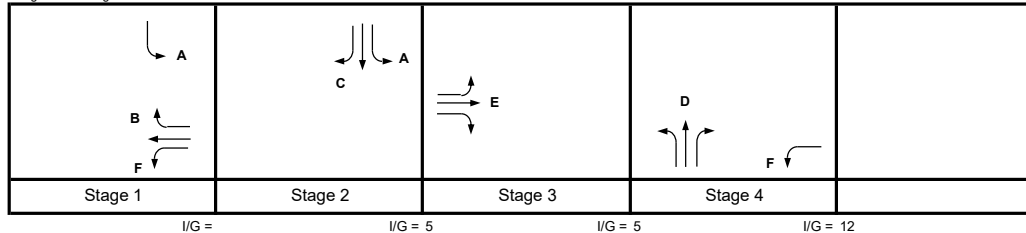
DATE: Nov 20



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.757
Lost time	L =	19 sec
Total Flow	=	22,595 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	138 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	78 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.758
R.C. _{ult}	= $(Y_{ult} - Y) / Y \times 100\%$	0.0 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	120 sec
Y_{max}	= $1 - L/C$	0.842

J3

Stage/Phase Diagrams



Critical Case : A,E,D

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 0\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		20				
	B	1	3.500	1								2105		531		531			2105	0.252	
	B	1	3.500	1		28	0	0				2105			109	400	509	79%	2020	0.252	
Chung Yan Road WB	D	4	3.500	1	25	25	0	1				1965	30	90	271	391	8%	69%	1878	0.208	0.208
	D	4	3.500	1		25	0	0				2105			414	414	100%	100%	1986	0.208	
Yu Tung Road NB	E	3	3.500	1	15			1				1965	170			170	100%		1786	0.095	
	E	3	3.500	1				0				2105		471		471			2105	0.224	0.224
	E	3	3.500	1		25	0	0				2105		365	100	465	22%		2078	0.224	
Chung Yan Road EB	A	1,2	3.500	1	16			1				1965	585			585	100%		1797	0.326	0.326
	C	2	3.500	1		18		0				2105		85	12	97	13%		2083	0.047	
	C	2	3.500	1		25	0	0				2105			93	93	100%		1986	0.047	

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

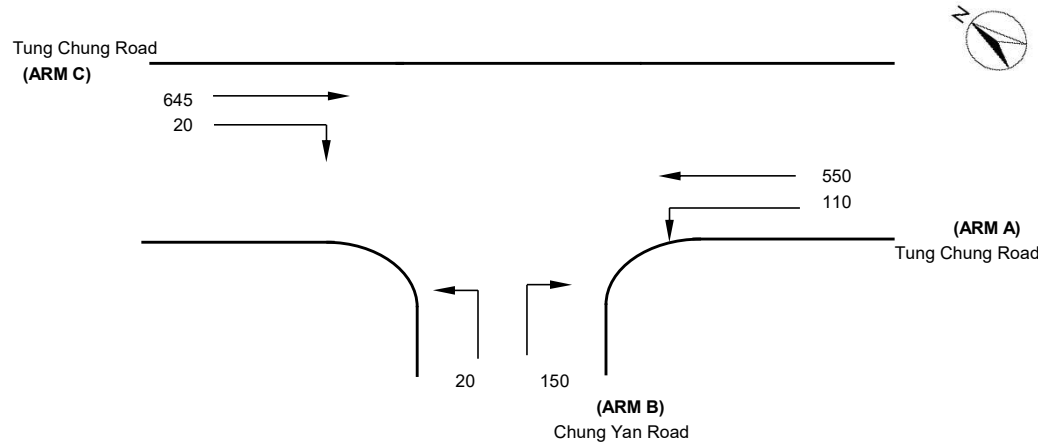
2033 AM Reference Traffic Flows (Conforming Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	110 (pcu/hr)
q a-c	=	550 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	645 (pcu/hr)
q c-b	=	20 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	150 (pcu/hr)
q b-c	=	20 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	329
Q b-c	=	570
Q c-b	=	552
Q b-ac	=	346

CRITICAL DFC = 0.49

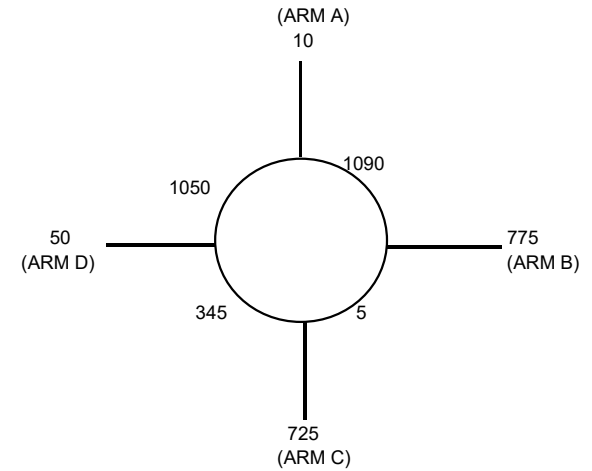
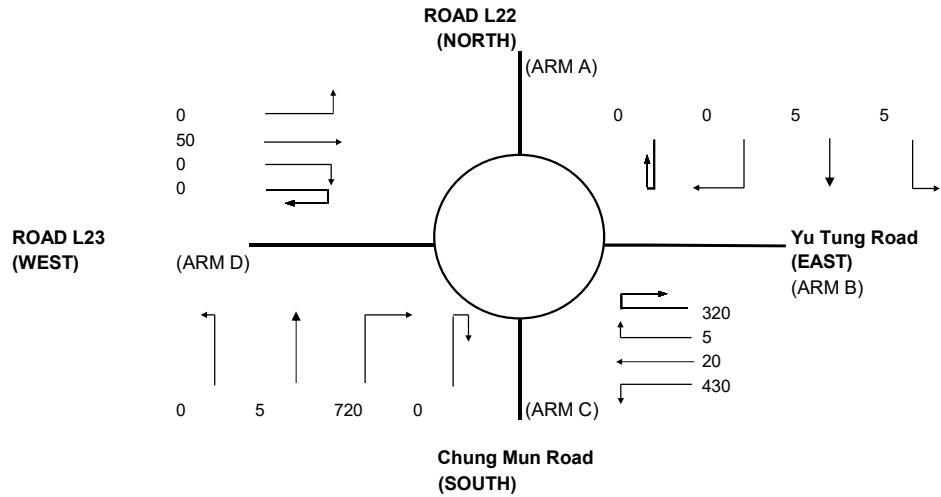
COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.46
DFC b-c	=	0.04
DFC c-b	=	0.04
DFC b-ac	=	0.49

ROUNDAABOUT CAPACITY CALCULATIO

AECOM

Junction	J5 - Yu Tung Road / Chung Mun Road	Scenario	2033 - AM - Reference Traffic Flows (Conforming Scheme)	Project No.	Prepared By	Checked By	Date
				-	-	-	Nov 2024



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	5.75	8.80	7.30	4.85
E = Entry width (m)	6.50	9.00	8.50	6.00
L = Effective length of flare (m)	10.00	7.00	15.00	15.00
R = Entry radius (m)	25.00	25.00	40.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00
A = Entry angle (degree)	20.00	20.00	30.00	20.00
Q = Entry flow (pcu/h)	10	775	725	50
Qc= Circulating flow across entry (pcu/h)	1090	5	345	1050
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.12	0.05	0.13	0.12
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.04	1.04	1.02	1.04
X2= V + ((E-V)/(1+2S))	6.35	8.98	8.26	5.77
M = EXP((D-60)/10)	1.00	1.00	1.00	1.00
F = 303*X2	1926	2722	2501	1749
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.60	0.73	0.70	0.57
Qe= K(F-Fc*Qc)	1332	2839	2317	1207
DFC = Design flow/Capacity = Q/Qe	0.01	0.27	0.31	0.04

TOTAL ENTRY FLOWS = 1560 PCU

CRITICAL DFC = 0.31

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J6 - Tung Chung Road / Shek Mun Kap Road

2033 AM Reference Traffic Flows (Conforming Scheme)

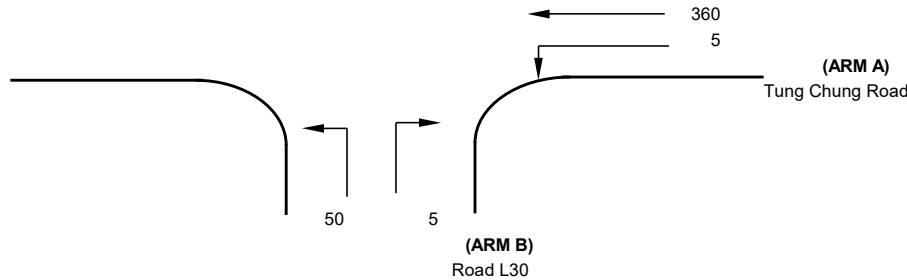
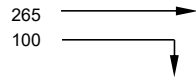
Designed By :

Checked By :

Job No. :

Date : Nov 24

Tung Chung Road
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J6

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	5 (pcu/hr)
q a-c	=	360 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	265 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	50 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	488
Q b-c	=	690
Q c-b	=	689
Q b-ac	=	665

CRITICAL DFC = 0.15

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.07
DFC c-b	=	0.15
DFC b-ac	=	0.08

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J7 - Road L29 / Road L30

2033 AM Reference Traffic Flows (Conforming Scheme)

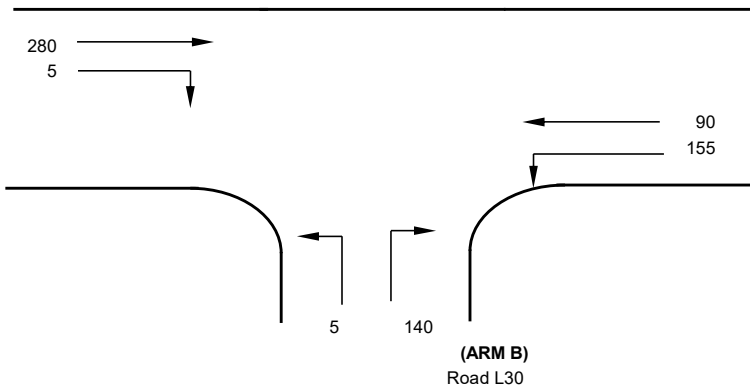
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J7

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	11.7 (metres)
W cr	=	1.5 (metres)
q a-b	=	155 (pcu/hr)
q a-c	=	90 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	280 (pcu/hr)
q c-b	=	5 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	140 (pcu/hr)
q b-c	=	5 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.857384
Y	=	0.596350

THE CAPACITY OF MOVEMENT :

Q b-a	=	479
Q b-c	=	611
Q c-b	=	593
Q b-ac	=	482

CRITICAL DFC = 0.30

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.29
DFC b-c	=	0.01
DFC c-b	=	0.01
DFC b-ac	=	0.30

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J8 - Tung Chung Road / Road L30

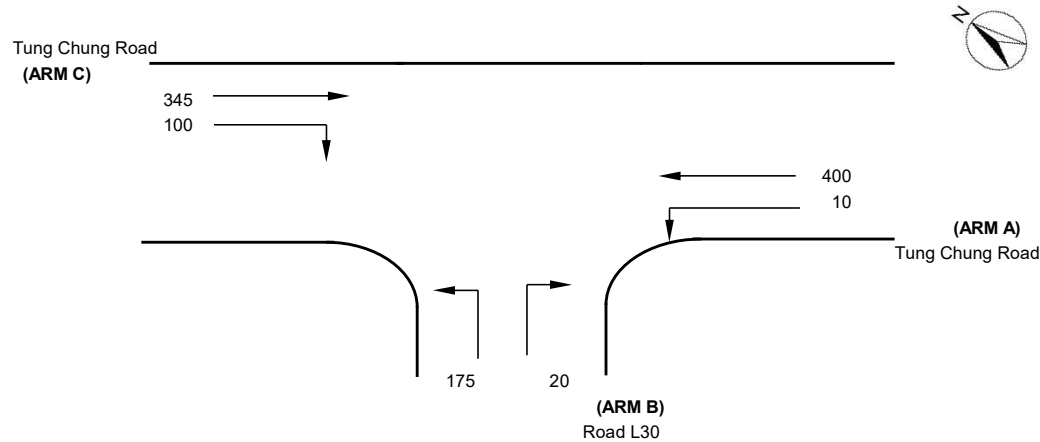
2033 AM Reference Traffic Flows (Conforming Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J8

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	10 (pcu/hr)
q a-c	=	400 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	345 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	20 (pcu/hr)
q b-c	=	175 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	460
Q b-c	=	678
Q c-b	=	676
Q b-ac	=	646

CRITICAL DFC = 0.30

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.04
DFC b-c	=	0.26
DFC c-b	=	0.15
DFC b-ac	=	0.30

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J9 - Road L29 / Road L25

2033 AM Reference Traffic Flows (Conforming Scheme)

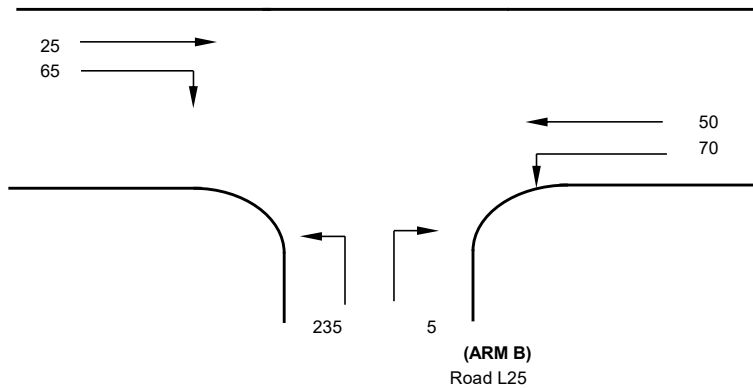
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J9

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	10.3 (metres)
W cr	=	1.5 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	50 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.1 (metres)
Vr c-b	=	100 (metres)
q c-a	=	25 (pcu/hr)
q c-b	=	65 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	235 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.838923
Y	=	0.644650

THE CAPACITY OF MOVEMENT :

Q b-a	=	503
Q b-c	=	623
Q c-b	=	601
Q b-ac	=	620

CRITICAL DFC = 0.39

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.38
DFC c-b	=	0.11
DFC b-ac	=	0.39

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J10 - Road L28 / Road L29 / Shek Mun Kap Road

2033 AM Reference Traffic Flows (Conforming Scheme)

Designed By :

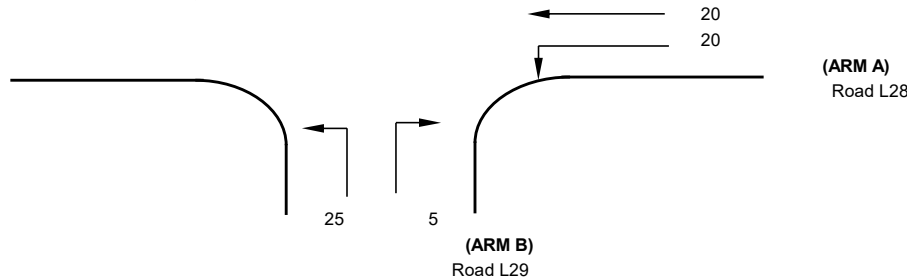
Checked By :

Job No. :

Date : Nov 24

Shek Mun Kap Road
(ARM C)

15
100



NOTES : (GEOMETRIC INPUT DATA)

J10

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	20 (pcu/hr)
q a-c	=	20 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	15 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.7 (metres)
W b-c	=	2.7 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	25 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.867478
E	=	0.894307
F	=	0.857384
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	501
Q b-c	=	659
Q c-b	=	629
Q b-ac	=	627

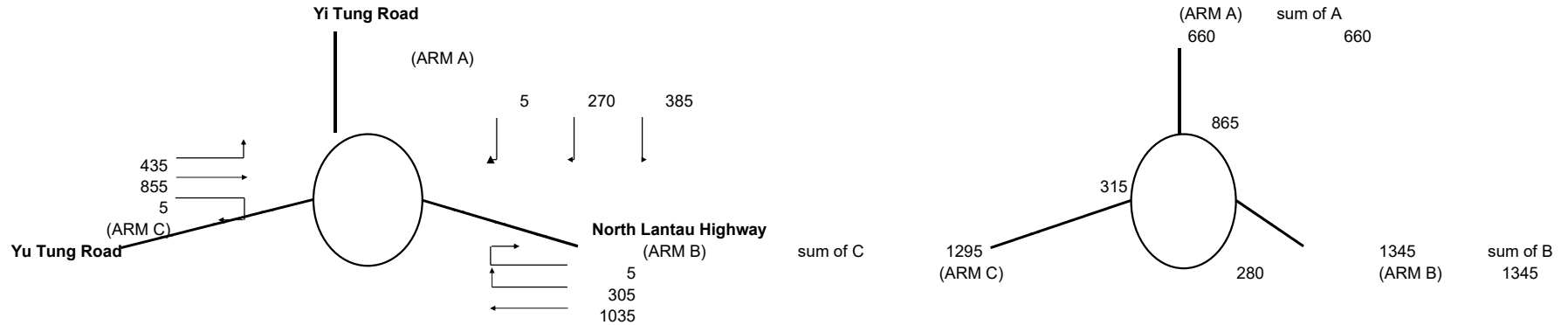
CRITICAL DFC = 0.16

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.04
DFC c-b	=	0.16
DFC b-ac	=	0.05

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2033 - PM - Reference Traffic Flows (Conforming Scheme)	Project No.	Prepared By	Checked By	Date
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ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	660	1345	1295
Qc= Circulating flow across entry (pcu/h)	865	280	315
OUTPUT PARAMETERS:			
S = Sharpness of flare = 1.6(E-V)/L	0.64	0.65	0.53
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.00	0.98	0.97
X2= V + ((E-V)/(1+2S))	9.75	9.22	9.94
M = EXP((D-60)/10)	109.95	109.95	109.95
F = 303*X2	2956	2795	3010
Td= 1+(0.5/(1+M))	1.00	1.00	1.00
Fc= 0.21*Td(1+0.2*X2)	0.62	0.60	0.63
Qe= K(F-Fc*Qc)	2408	2576	2734
DFC = Design flow/Capacity = Q/Qe	0.27	0.52	0.47

TOTAL ENTRY FLOWS = 3300 PCU
CRITICAL DFC = 0.52

JUNCTION CAPACITY CALCULATION

AECOM

Junction J2 - Yu Tung Road / Shun Tung Road - Improved Scheme

2033 PM Reference Traffic Flows (Conforming Scheme) (With CEDD Improvements)

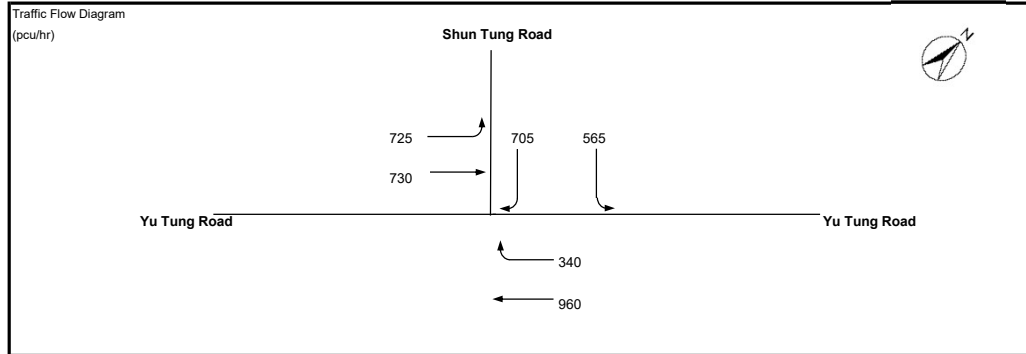
DESIGN:

CHECK:

JOB NO:

DATE: Nov 20

Traffic Flow Diagram
(pcu/hr)

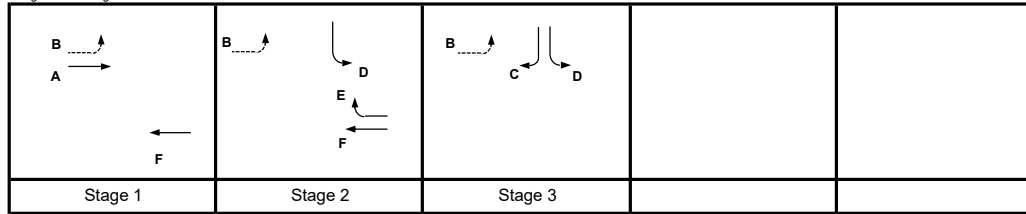


No. of stages per cycle N = 3
 Cycle time C = 120 sec
 Sum(y) Y = 0.514
 Lost time L = 12 sec
 Total Flow = 18,780 pcu

J2

Optimum Cycle $C_o = (1.5 \times L + 5) / (1 - Y) = 47$ sec
 Min. Cycle Time $C_m = L / (1 - Y) = 25$ sec
 $Y_{ult} = 0.9 - 0.0075 \times L = 0.810$
 $R.C._{ult} = (Y_{ult} - Y) / Y \times 100\% = 57.7\%$
 Practical Cycle Time $C_p = 0.9 \times L / (0.9 - Y) = 28$ sec
 $Y_{max} = 1 - L / C = 0.900$

Stage/Phase Diagrams



Critical Case : A,E,C

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 58\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
Yu Tung Road NB	A	1	4.000	1	50		0	1		0		2015	725	730		725	100%		1956	0.371	0.169
			4.000	2																	
Shun Tung Road EB	D	2,3	3.700	1	25		0	1		0		1985	565	705		565	100%	1873	0.302	0.174	
	C	3	3.700	2									30			0	0				4250
Yu Tung Road SB	F	1,2	3.650	2	25		0	1		0		4100	960	340		960	100%	4100	0.234	0.170	
	E	2	3.650	1												0					0

JUNCTION CAPACITY CALCULATION

AECOM

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

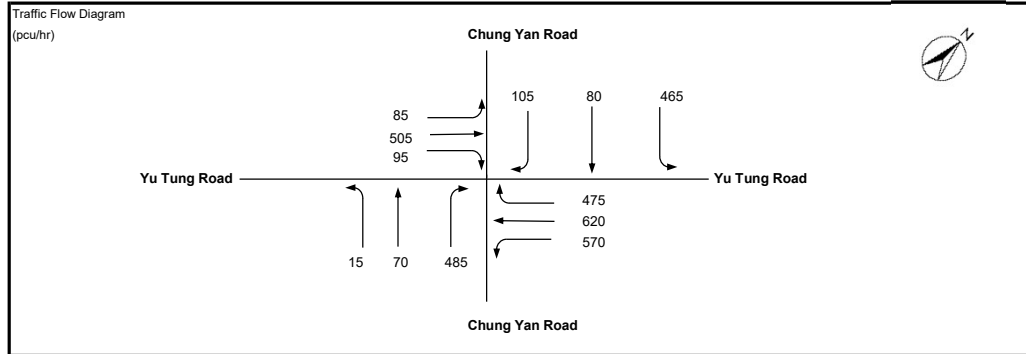
2033 PM Reference Traffic Flows (Conforming Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

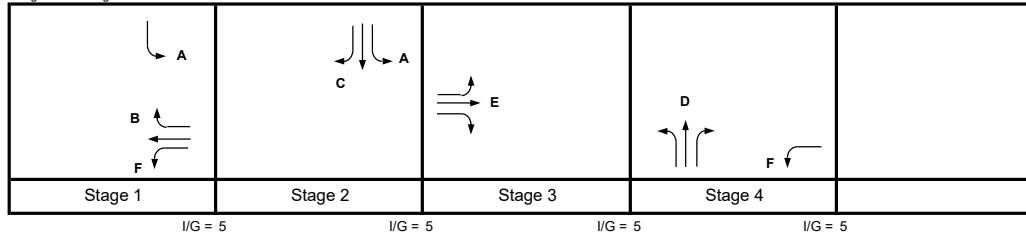
DATE: Nov 20



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.603
Lost time	L =	16 sec
Total Flow	=	22,595 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	73 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	40 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.780
R.C. _{ult}	= $(Y_{ult} - Y) / Y \times 100\%$	29.4 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	48 sec
Y_{max}	= $1 - L/C$	0.867

J3

Stage/Phase Diagrams



Critical Case : B,C,E,D

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 29\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
Yu Tung Road SB																					
↑	F	1,4	3.500	1	20					0		1965	570			570	100%		1828	0.312	0.266
↑	B	1	3.500	1						0		2105		560		560			2105	0.266	
↑	B	1	3.500	1		28	0	0		0		2105		60	475	535	89%	2009	0.266		
Chung Yan Road WB																					
←	D	4	3.500	1	25	25	0	1		0		1965	15	70	192	277	5%	69%	1881	0.147	0.147
←	D	4	3.500	1		25	0	0		0		2105		293		293	100%	1986	0.147		
Yu Tung Road NB																					
↘	E	3	3.500	1	15			1		0		1965	85			85	100%		1786	0.048	0.144
↑	E	3	3.500	1						0		2105		303		303			2105	0.144	
↑	E	3	3.500	1		25	0	0		0		2105		202	95	297	32%	2065	0.144		
Chung Yan Road EB																					
↘	A	1,2	3.500	1	16			1		0		1965	465			465	100%		1797	0.259	0.046
↘	C	2	3.500	1		18				0		2105		80	15	95	15%	2078	0.046		
↘	C	2	3.500	1		25	0	0		0		2105		90	90	90	100%	1986	0.046		

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

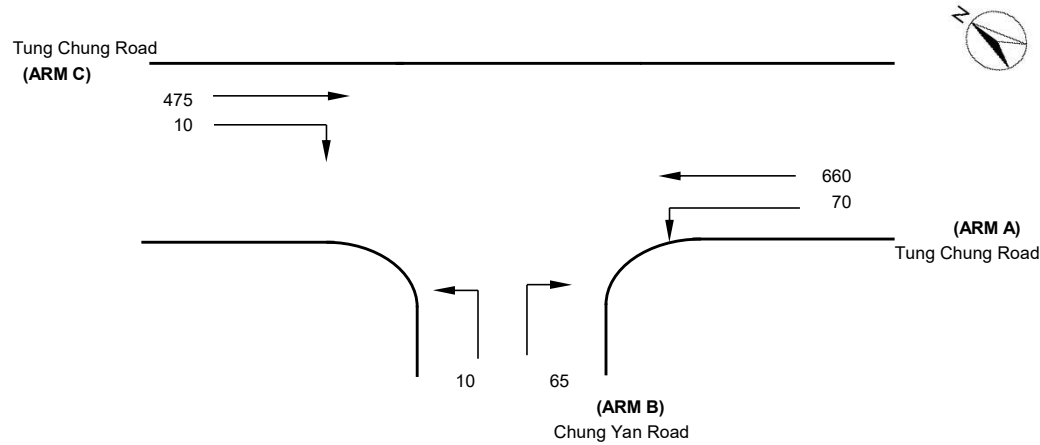
2033 PM Reference Traffic Flows (Conforming Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	660 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	475 (pcu/hr)
q c-b	=	10 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	65 (pcu/hr)
q b-c	=	10 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	336
Q b-c	=	545
Q c-b	=	534
Q b-ac	=	354

CRITICAL DFC = 0.21

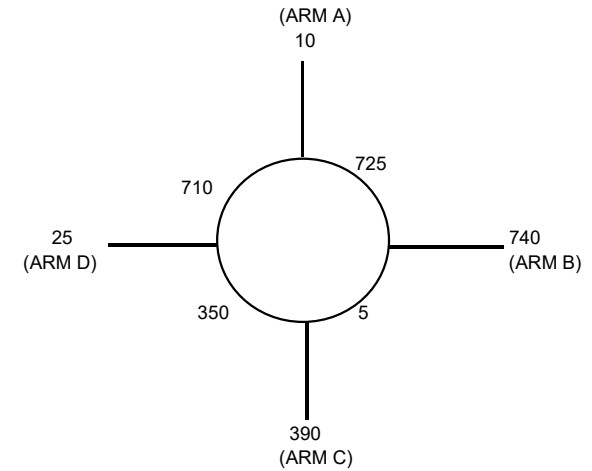
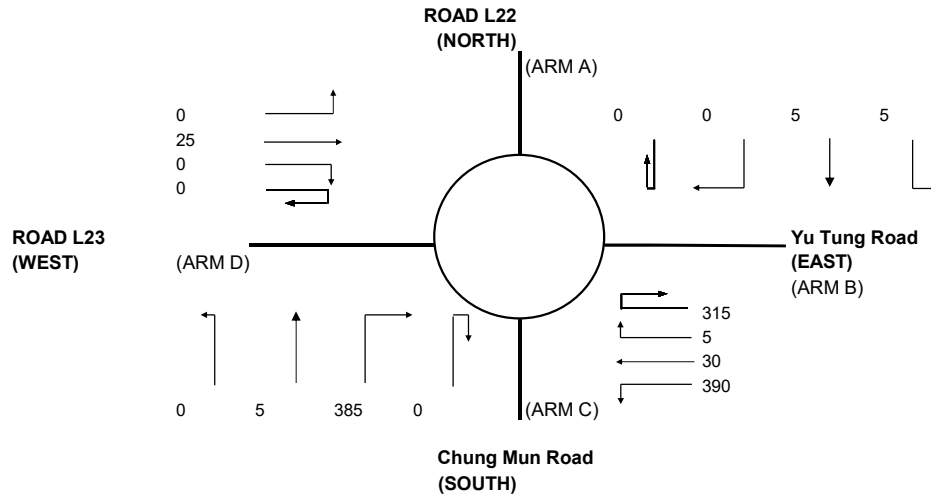
COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.19
DFC b-c	=	0.02
DFC c-b	=	0.02
DFC b-ac	=	0.21

ROUNDAABOUT CAPACITY CALCULATIO

AECOM

Junction	J5 - Yu Tung Road / Chung Mun Road	Scenario	2033 - PM - Reference Traffic Flows (Conforming Scheme)	Project No.	Prepared By	Checked By	Date
				-	-	-	Nov 2024



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	5.75	8.80	7.30	4.85
E = Entry width (m)	6.50	9.00	8.50	6.00
L = Effective length of flare (m)	10.00	7.00	15.00	15.00
R = Entry radius (m)	25.00	25.00	40.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00
A = Entry angle (degree)	20.00	20.00	30.00	20.00
Q = Entry flow (pcu/h)	10	740	390	25
Qc= Circulating flow across entry (pcu/h)	725	5	350	710
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.12	0.05	0.13	0.12
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.04	1.04	1.02	1.04
X2= V + ((E-V)/(1+2S))	6.35	8.98	8.26	5.77
M = EXP((D-60)/10)	1.00	1.00	1.00	1.00
F = 303*X2	1926	2722	2501	1749
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.60	0.73	0.70	0.57
Qe= K(F-Fc*Qc)	1560	2839	2313	1408
DFC = Design flow/Capacity = Q/Qe	0.01	0.26	0.17	0.02

TOTAL ENTRY FLOWS = 1165 PCU

CRITICAL DFC = 0.26

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J6 - Tung Chung Road / Shek Mun Kap Road

2033 PM Reference Traffic Flows (Conforming Scheme)

Designed By :

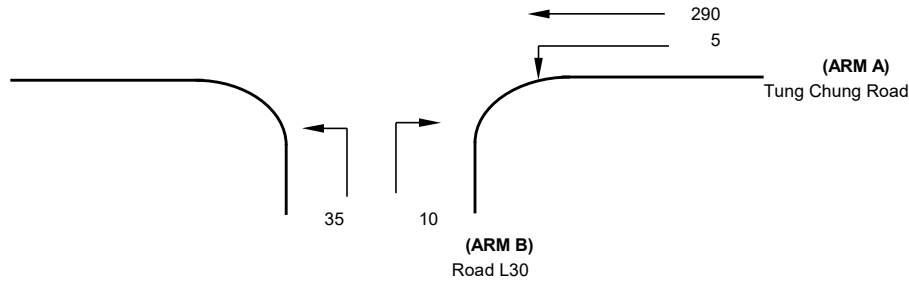
Checked By :

Job No. :

Date : Nov 24

Tung Chung Road
(ARM C)

270
115



NOTES : (GEOMETRIC INPUT DATA)

J6

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	5 (pcu/hr)
q a-c	=	290 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	270 (pcu/hr)
q c-b	=	115 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	10 (pcu/hr)
q b-c	=	35 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	502
Q b-c	=	710
Q c-b	=	709
Q b-ac	=	650

CRITICAL DFC = 0.16

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.02
DFC b-c	=	0.05
DFC c-b	=	0.16
DFC b-ac	=	0.07

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J7 - Road L29 / Road L30

2033 PM Reference Traffic Flows (Conforming Scheme)

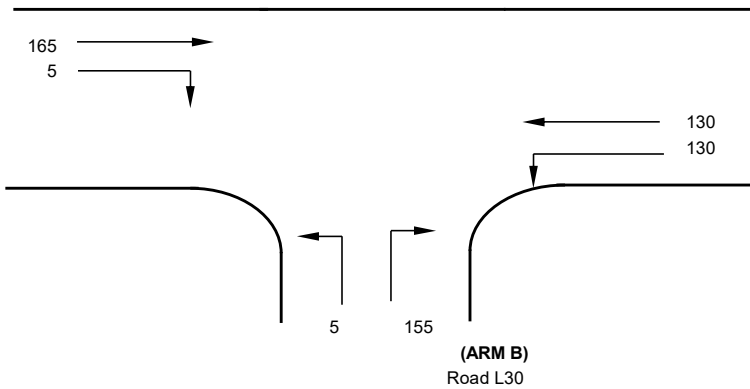
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



(ARM A)
Road L29

(ARM B)
Road L30

NOTES : (GEOMETRIC INPUT DATA)

J7

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	11.7 (metres)
W cr	=	1.5 (metres)
q a-b	=	130 (pcu/hr)
q a-c	=	130 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	165 (pcu/hr)
q c-b	=	5 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	155 (pcu/hr)
q b-c	=	5 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.857384
Y	=	0.596350

THE CAPACITY OF MOVEMENT :

Q b-a	=	486
Q b-c	=	605
Q c-b	=	590
Q b-ac	=	489

CRITICAL DFC = 0.33

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.32
DFC b-c	=	0.01
DFC c-b	=	0.01
DFC b-ac	=	0.33

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J8 - Tung Chung Road / Road L30

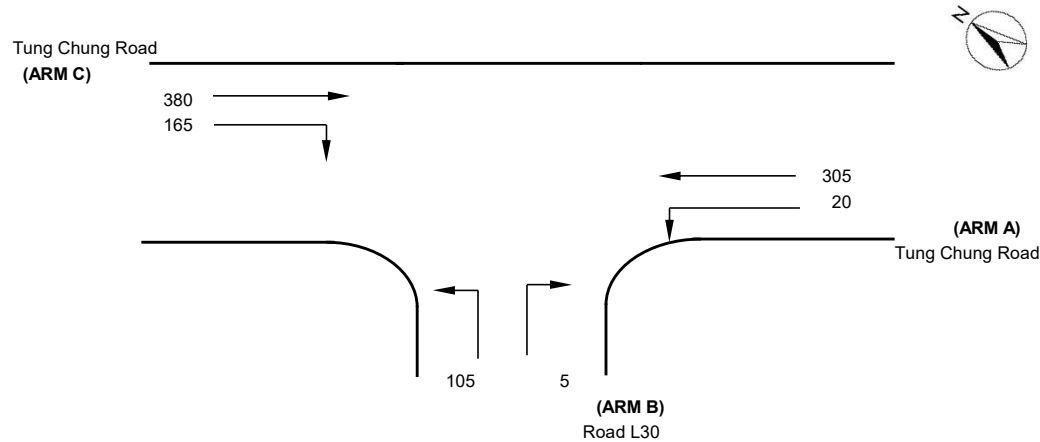
2033 PM Reference Traffic Flows (Conforming Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J8

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	20 (pcu/hr)
q a-c	=	305 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	380 (pcu/hr)
q c-b	=	165 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	105 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	453
Q b-c	=	704
Q c-b	=	700
Q b-ac	=	687

CRITICAL DFC = 0.24

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.15
DFC c-b	=	0.24
DFC b-ac	=	0.16

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J9 - Road L29 / Road L25

2033 PM Reference Traffic Flows (Conforming Scheme)

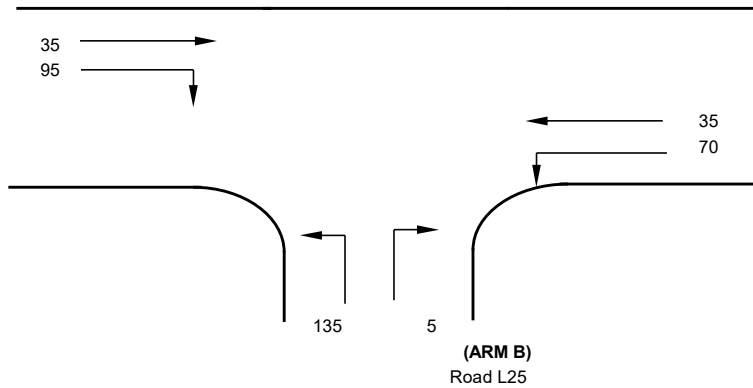
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J9

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	10.3 (metres)
W cr	=	1.5 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	35 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.1 (metres)
Vr c-b	=	100 (metres)
q c-a	=	35 (pcu/hr)
q c-b	=	95 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	135 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.838923
Y	=	0.644650

THE CAPACITY OF MOVEMENT :

Q b-a	=	496
Q b-c	=	626
Q c-b	=	604
Q b-ac	=	620

CRITICAL DFC = 0.23

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.22
DFC c-b	=	0.16
DFC b-ac	=	0.23

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J10 - Road L28 / Road L29 / Shek Mun Kap Road

2033 PM Reference Traffic Flows (Conforming Scheme)

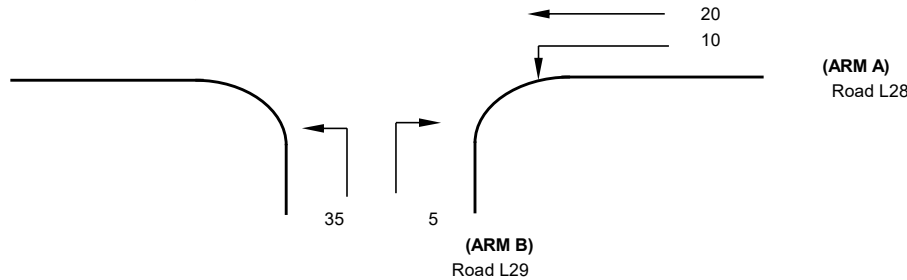
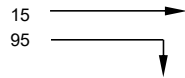
Designed By :

Checked By :

Job No. :

Date : Nov 24

Shek Mun Kap Road
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J10

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	10 (pcu/hr)
q a-c	=	20 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	15 (pcu/hr)
q c-b	=	95 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.7 (metres)
W b-c	=	2.7 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	35 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.867478
E	=	0.894307
F	=	0.857384
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	504
Q b-c	=	660
Q c-b	=	632
Q b-ac	=	636

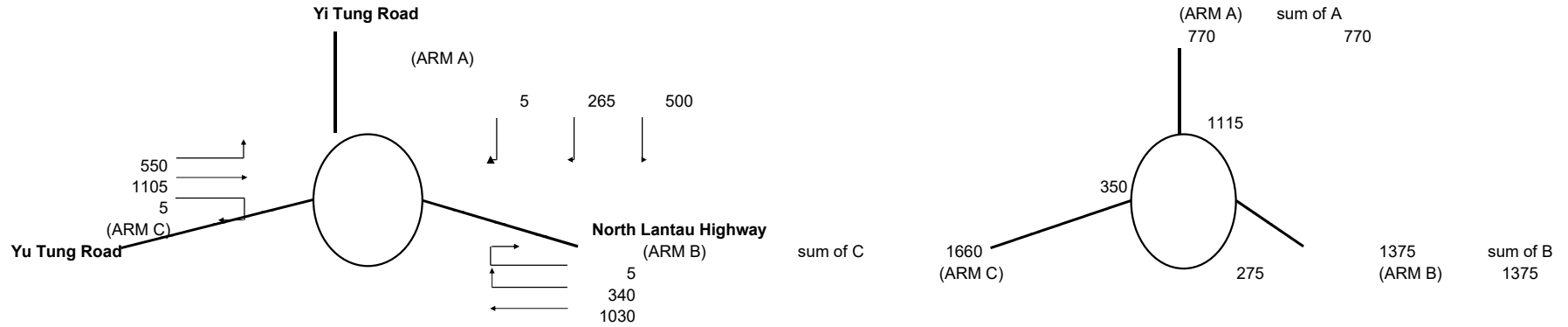
CRITICAL DFC = 0.15

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.05
DFC c-b	=	0.15
DFC b-ac	=	0.06

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2033 - AM - Design Traffic Flows (Proposed Scheme)	Project No.	Prepared By	Checked By	Date
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ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	770	1375	1660
Qc = Circulating flow across entry (pcu/h)	1115	275	350
OUTPUT PARAMETERS:			
S = Sharpness of flare = $1.6(E-V)/L$	0.64	0.65	0.53
K = $1-0.00347(A-30)-0.978(1/R-0.05)$	1.00	0.98	0.97
X2 = $V + ((E-V)/(1+2S))$	9.75	9.22	9.94
M = $EXP((D-60)/10)$	109.95	109.95	109.95
F = $303 \times X2$	2956	2795	3010
Td = $1+(0.5/(1+M))$	1.00	1.00	1.00
Fc = $0.21 \times Td(1+0.2 \times X2)$	0.62	0.60	0.63
Qe = $K(F-Fc \times Qc)$	2253	2579	2713
DFC = Design flow/Capacity = Q/Qe	0.34	0.53	0.61

TOTAL ENTRY FLOWS = 3805 PCU

CRITICAL DFC = 0.61

JUNCTION CAPACITY CALCULATION

AECOM

Junction J2 - Yu Tung Road / Shun Tung Road - Improved Scheme

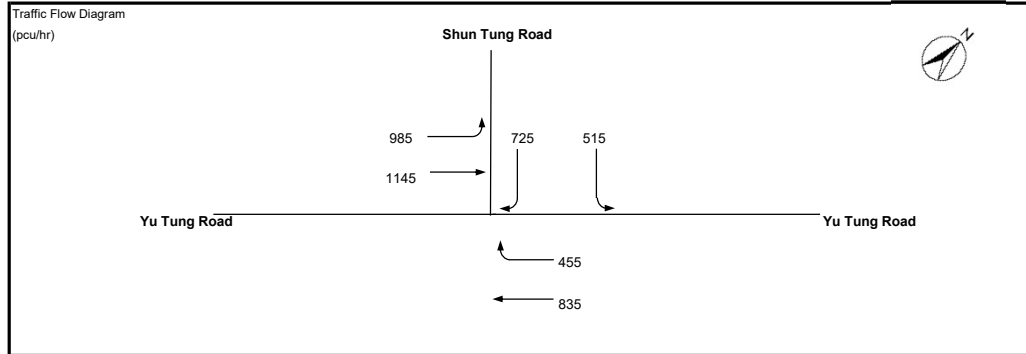
2033 AM Design Traffic Flows (Proposed Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

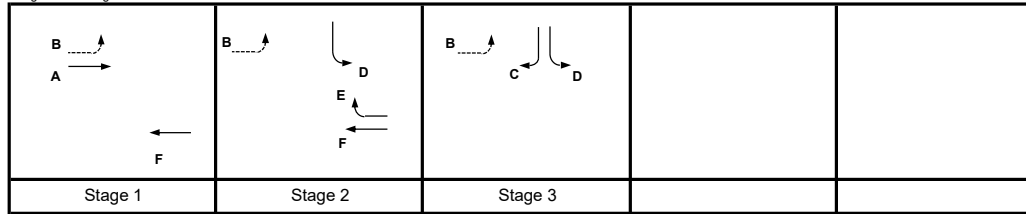
DATE: Nov 20



No. of stages per cycle	N =	3
Cycle time	C =	120 sec
Sum(y)	Y =	0.672
Lost time	L =	12 sec
Total Flow	=	18,780 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	70 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	37 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.810
$R.C._{ult}$	= $(Y_{ult} - Y) / Y \times 100\%$	20.5 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	47 sec
Y_{max}	= $1 - L / C$	0.900

J2

Stage/Phase Diagrams



Critical Case : A,E,C

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 20\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road NB	A							1	4.000	2		50				
Shun Tung Road EB	D	2,3	3.700	1	25		1	0				1985	515		515	100%		1873	0.275		
	C	3	3.700	2		30	0	0				4250		725	725		100%	4048	0.179	0.179	
Yu Tung Road SB	F	1,2	3.650	2			1	0				4100		835	835		100%	4100	0.204		
	E	2	3.650	1		25	0	0				2120		455	455		100%	2000	0.228	0.228	

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

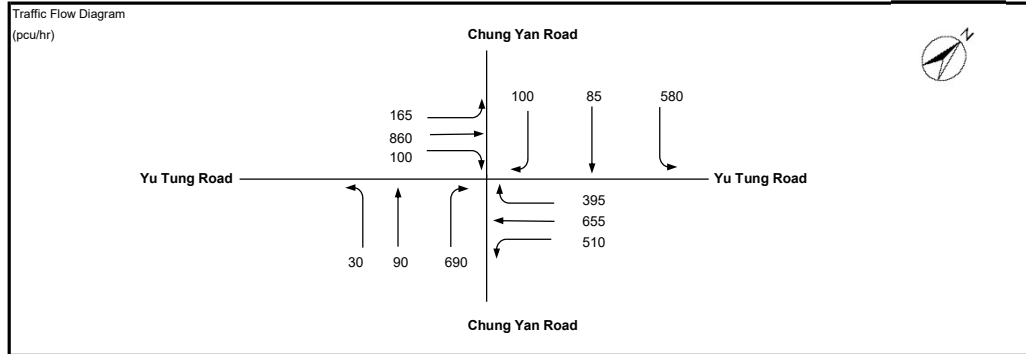
2033 AM Design Traffic Flows (Proposed Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

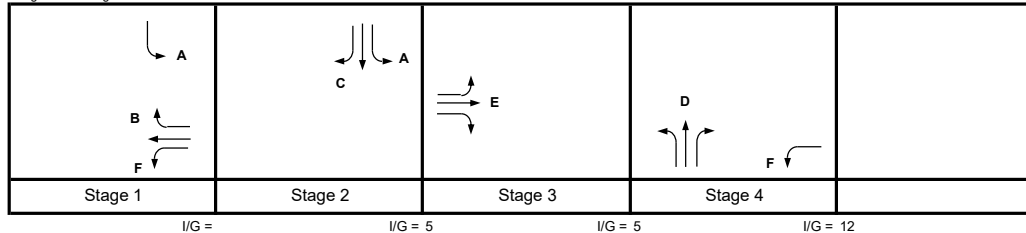
DATE: Nov 20



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.762
Lost time	L =	19 sec
Total Flow	=	22,595 pcu
Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	141 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	80 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.758
$R.C._{ult}$	$= (Y_{ult} - Y) / Y \times 100\% =$	-0.6 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	124 sec
Y_{max}	$= 1 - L/C =$	0.842

J3

Stage/Phase Diagrams



Critical Case : A,E,D

$$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = -1\%$$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		20				
	B	1	3.500	1								2105		536		536			2105	0.254	
	B	1	3.500	1		28	0	0				2105		119	395	514	77%		2022	0.254	
Chung Yan Road WB	D	4	3.500	1	25	25	0	1				1965	30	90	274	394	8%	70%	1878	0.210	0.210
	D	4	3.500	1		25	0	0				2105		416	416	416	100%		1986	0.210	
Yu Tung Road NB	E	3	3.500	1	15			1				1965	165			165	100%		1786	0.092	
	E	3	3.500	1				0				2105		483		483			2105	0.229	0.229
	E	3	3.500	1		25	0	0				2105		377	100	477	21%		2079	0.229	
Chung Yan Road EB	A	1,2	3.500	1	16			1				1965	580			580	100%		1797	0.323	0.323
	C	2	3.500	1		18		0				2105		85	10	95	10%		2087	0.045	
	C	2	3.500	1		25	0	0				2105		90	90	90	100%		1986	0.045	

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

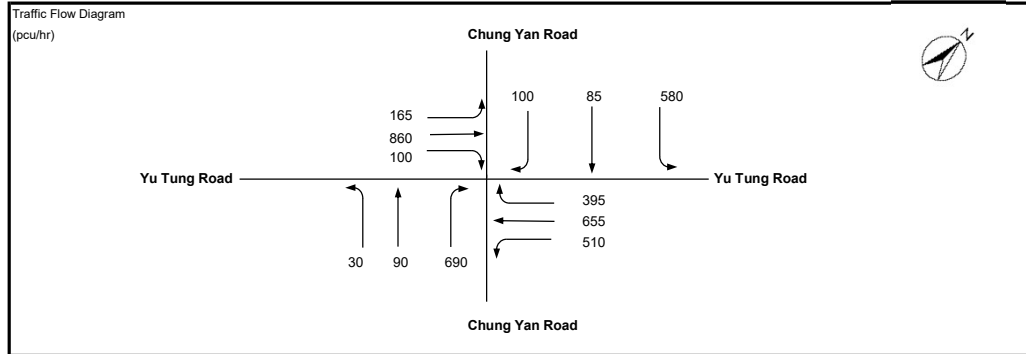
2033 AM Design Traffic Flows (Proposed Scheme) (With Additional Improvements)

DESIGN:

CHECK:

JOB NO:

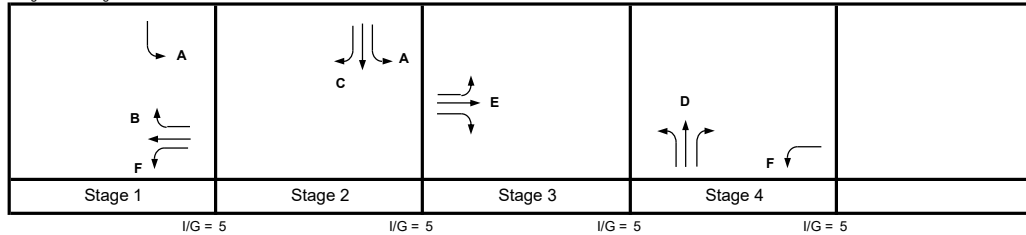
DATE: Nov 24



No. of stages per cycle	N =	4
Cycle time	C =	120 sec
Sum(y)	Y =	0.651
Lost time	L =	16 sec
Total Flow	=	24,700 pcu
Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	83 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	46 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.780
$R.C._{ult}$	$= (Y_{ult} - Y) / Y \times 100\% =$	19.8 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	58 sec
Y_{max}	$= 1 - L/C =$	0.867

J3

Stage/Phase Diagrams



Critical Case : B,C,E,D

$$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 20\%$$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		20				
	B	1	3.500	1								2105		358		358			2105	0.170	
	B	1	3.500	1		28	0					2105			297	57	355	16%	2087	0.170	
	B	1	3.500	1		25	0	0				2105				338	338	100%	1986	0.170	
Chung Yan Road WB	D	4	3.500	1	25	25	0	1				1965	30	90	274	394	8%	70%	1878	0.210	0.210
	D	4	3.500	1		25	0	0				2105			416	416	100%	1986	0.210		
Yu Tung Road NB	E	3	3.500	1	15			1				1965	165	184		349	47%		1876	0.186	0.186
	E	3	3.500	1								2105		391		391			2105	0.186	
	E	3	3.500	1		25	0	0				2105		285	100	385	26%		2073	0.186	
Chung Yan Road EB	A	1,2	3.500	1	16			1				1965	422			422	100%		1797	0.235	0.086
	C	2	3.500	1	18							2105	158	15		173	51%		2019	0.086	
	C	2	3.500	1		25	0	0				2105		70	100	170	100%		1986	0.086	

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

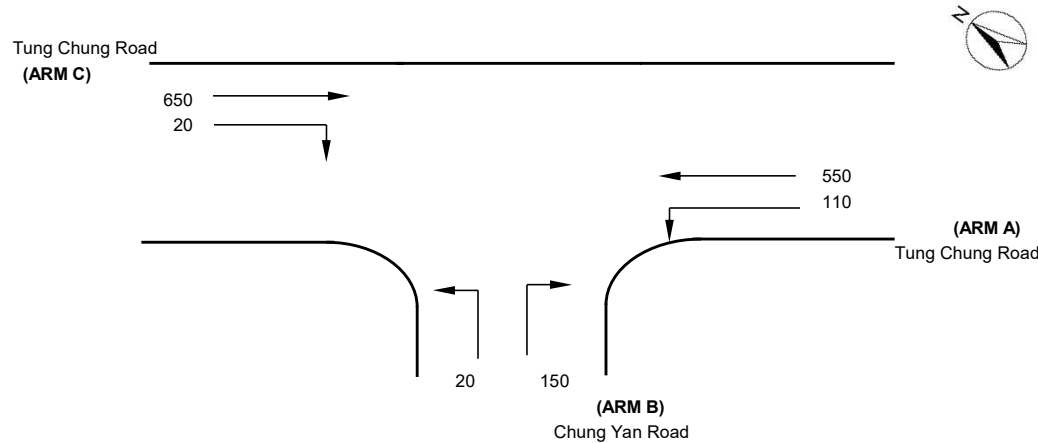
2033 AM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	110 (pcu/hr)
q a-c	=	550 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	650 (pcu/hr)
q c-b	=	20 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	150 (pcu/hr)
q b-c	=	20 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	328
Q b-c	=	570
Q c-b	=	552
Q b-ac	=	345

CRITICAL DFC = 0.49

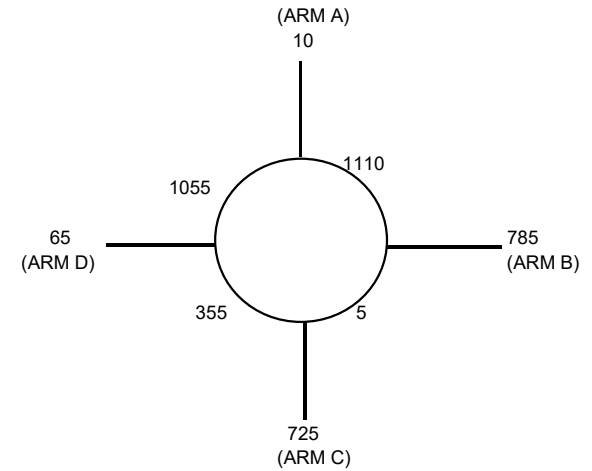
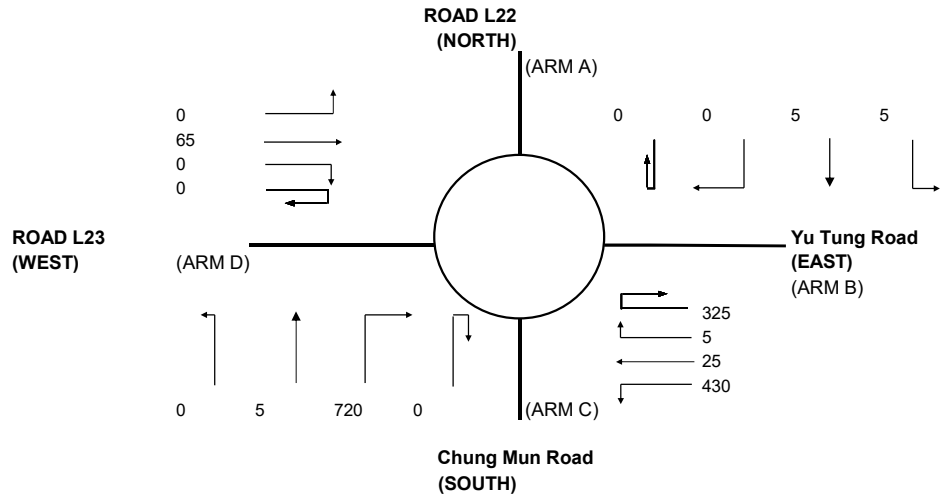
COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.46
DFC b-c	=	0.04
DFC c-b	=	0.04
DFC b-ac	=	0.49

ROUNDAABOUT CAPACITY CALCULATIO

AECOM

Junction	J5 - Yu Tung Road / Chung Mun Road	Scenario	2033 - AM - Design Traffic Flows (Proposed Scheme)	Project No.	Prepared By	Checked By	Date
				-	-	-	Nov 2024



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	5.75	8.80	7.30	4.85
E = Entry width (m)	6.50	9.00	8.50	6.00
L = Effective length of flare (m)	10.00	7.00	15.00	15.00
R = Entry radius (m)	25.00	25.00	40.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00
A = Entry angle (degree)	20.00	20.00	30.00	20.00
Q = Entry flow (pcu/h)	10	785	725	65
Qc= Circulating flow across entry (pcu/h)	1110	5	355	1055
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.12	0.05	0.13	0.12
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.04	1.04	1.02	1.04
X2= V + ((E-V)/(1+2S))	6.35	8.98	8.26	5.77
M = EXP((D-60)/10)	1.00	1.00	1.00	1.00
F = 303*X2	1926	2722	2501	1749
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.60	0.73	0.70	0.57
Qe= K(F-Fc*Qc)	1320	2839	2309	1204
DFC = Design flow/Capacity = Q/Qe	0.01	0.28	0.31	0.05

TOTAL ENTRY FLOWS = 1585 PCU

CRITICAL DFC = 0.31

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J6 - Tung Chung Road / Shek Mun Kap Road

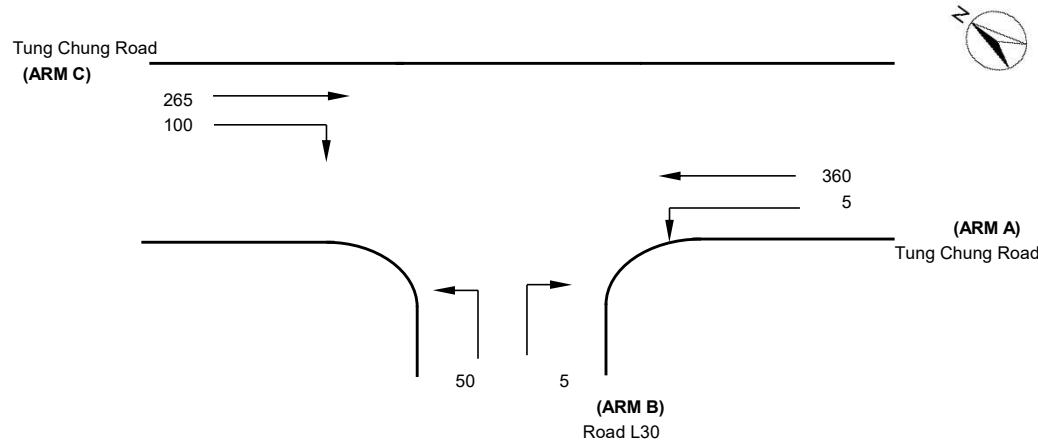
2033 AM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J6

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	5 (pcu/hr)
q a-c	=	360 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	265 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	50 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	488
Q b-c	=	690
Q c-b	=	689
Q b-ac	=	665

CRITICAL DFC = 0.15

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.07
DFC c-b	=	0.15
DFC b-ac	=	0.08

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J7 - Road L29 / Road L30

2033 AM Design Traffic Flows (Proposed Scheme)

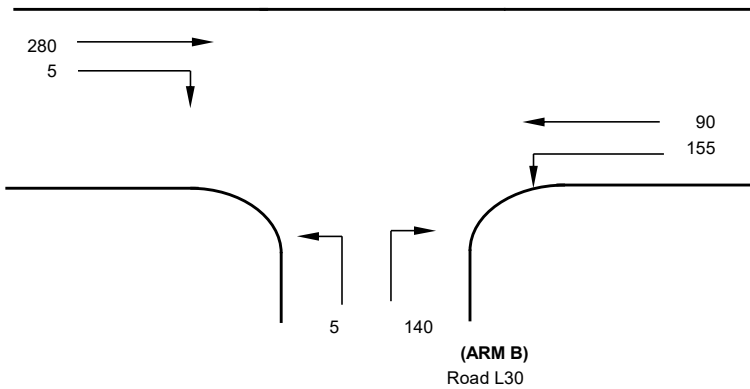
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J7

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	11.7 (metres)
W cr	=	1.5 (metres)
q a-b	=	155 (pcu/hr)
q a-c	=	90 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	280 (pcu/hr)
q c-b	=	5 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	140 (pcu/hr)
q b-c	=	5 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.857384
Y	=	0.596350

THE CAPACITY OF MOVEMENT :

Q b-a	=	479
Q b-c	=	611
Q c-b	=	593
Q b-ac	=	482

CRITICAL DFC = 0.30

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.29
DFC b-c	=	0.01
DFC c-b	=	0.01
DFC b-ac	=	0.30

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J8 - Tung Chung Road / Road L30

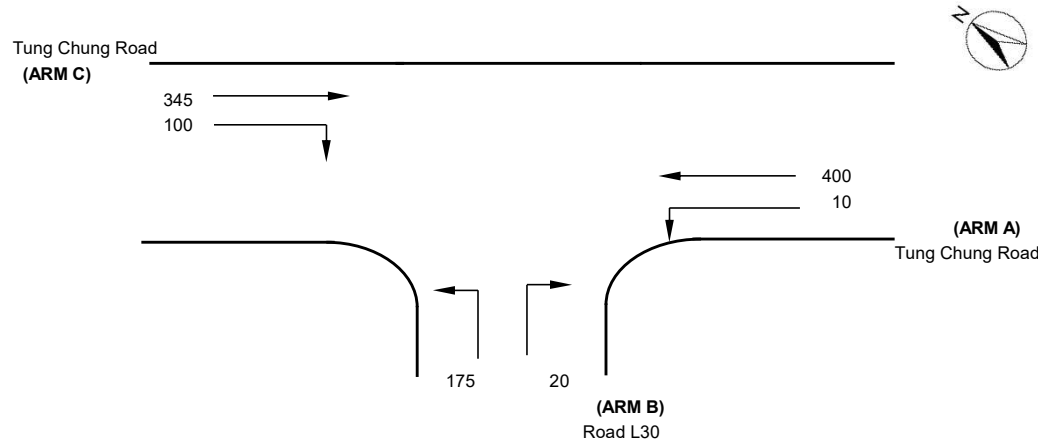
2033 AM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J8

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	10 (pcu/hr)
q a-c	=	400 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	345 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	20 (pcu/hr)
q b-c	=	175 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	460
Q b-c	=	678
Q c-b	=	676
Q b-ac	=	646

CRITICAL DFC = 0.30

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.04
DFC b-c	=	0.26
DFC c-b	=	0.15
DFC b-ac	=	0.30

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J9 - Road L29 / Road L25

2033 AM Design Traffic Flows (Proposed Scheme)

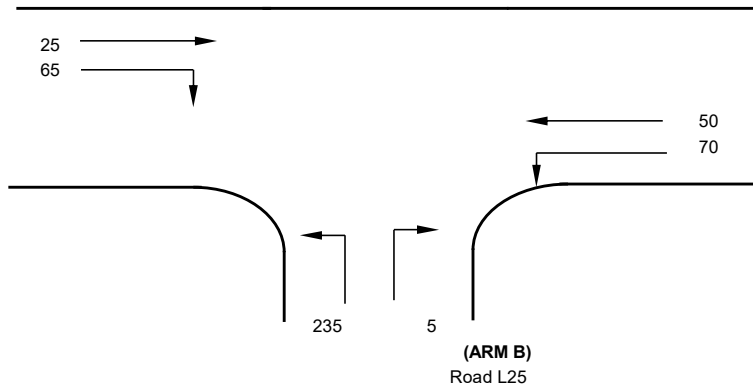
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J9

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	10.3 (metres)
W cr	=	1.5 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	50 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.1 (metres)
Vr c-b	=	100 (metres)
q c-a	=	25 (pcu/hr)
q c-b	=	65 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	235 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.838923
Y	=	0.644650

THE CAPACITY OF MOVEMENT :

Q b-a	=	503
Q b-c	=	623
Q c-b	=	601
Q b-ac	=	620

CRITICAL DFC = 0.39

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.38
DFC c-b	=	0.11
DFC b-ac	=	0.39

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J10 - Road L28 / Road L29 / Shek Mun Kap Road

2033 AM Design Traffic Flows (Proposed Scheme)

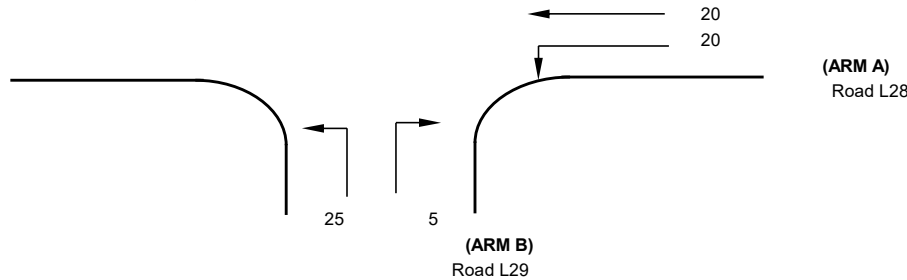
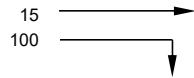
Designed By :

Checked By :

Job No. :

Date : Nov 24

Shek Mun Kap Road
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J10

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	20 (pcu/hr)
q a-c	=	20 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	15 (pcu/hr)
q c-b	=	100 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.7 (metres)
W b-c	=	2.7 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	25 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.867478
E	=	0.894307
F	=	0.857384
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	501
Q b-c	=	659
Q c-b	=	629
Q b-ac	=	627

CRITICAL DFC = 0.16

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.04
DFC c-b	=	0.16
DFC b-ac	=	0.05

JUNCTION CAPACITY CALCULATION

Junction J11 - Chung Mun Road / Road L29

2033 AM Design Traffic Flows (Proposed Scheme)

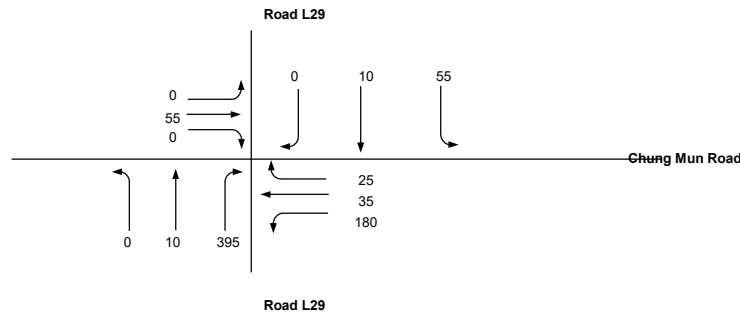
DESIGN:

CHECK:

JOB NO:

DATE: Nov 20

Traffic Flow Diagram (pcu/hr)

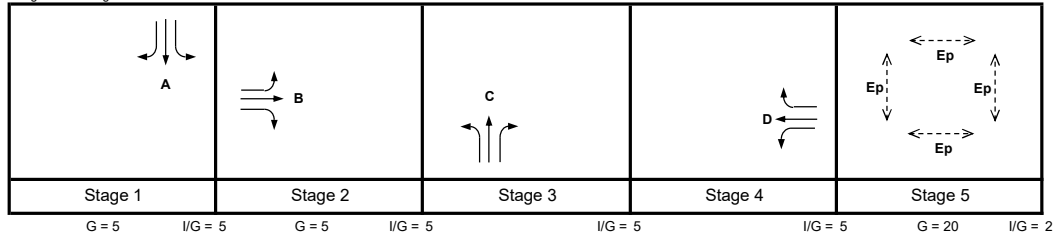


No. of stages per cycle	N =	5
Cycle time	C =	120 sec
Sum(y)	Y =	0.316
Lost time	L =	49 sec
Total Flow	=	9,965 pcu

J11

Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	115 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	72 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.533
$R.C._{ult}$	$= (Y_{ult} - Y) / Y \times 100\% =$	68.3 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	76 sec
Y_{max}	$= 1 - L / C =$	0.592

Stage/Phase Diagrams



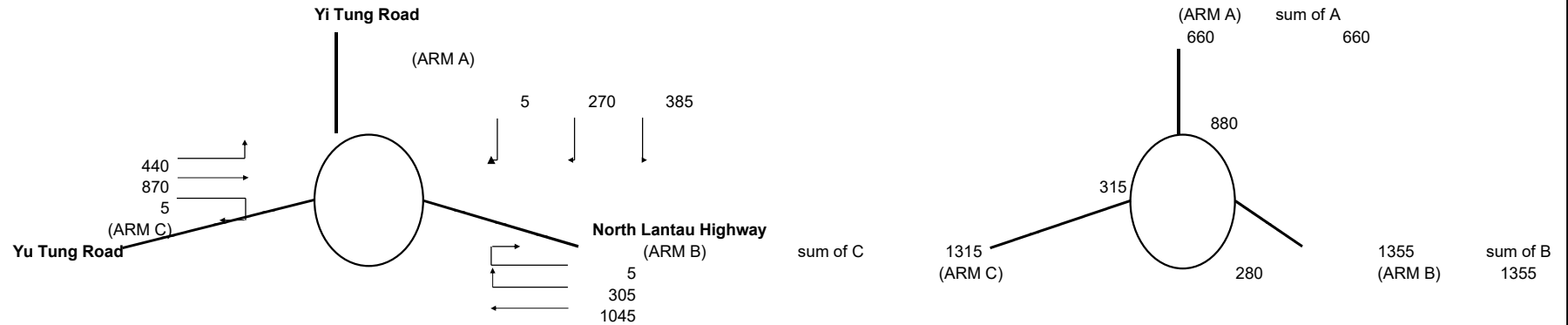
Critical Case : A,B,C,D,Ep

$R.C.(C) = (0.9 \times Y_{max} - Y) / Y \times 100\% = 68\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					↔	A							1	3.500	1		30	25			
↔	B	2	3.500	1	30	25	0	1	0		1965	0	55	0	55	0%	0%	1965	0.028		
↔	C	3	3.500	1	30	25	0	1	0		1965	0	10	395	405	0%	98%	1856	0.218	0.218	
↑	D	4	3.500	1	15	25	0	1	0		1965	114	35	25	126	100%	20%	1786	0.064	0.064	
↔	D	4	3.500	1	15	25	0	0	0		2105	66	35	25	126	52%	20%	1978	0.064		
Pedestrian Crossing				Ep	5	min.	GM	15	+	FGM	5	=	20	sec							

ROUNDBABOUT CAPACITY CALCULATION

Junction	Tung Chung East Interchange (J1)	Scenario	2033 - PM - Design Traffic Flows (Proposed Scheme)	Project No.	Prepared By	Checked By	Date
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ARM	A	B	C
INPUT PARAMETERS:			
V = Approach half width (m)	8.00	7.10	8.00
E = Entry width (m)	12.00	12.00	12.00
L = Effective length of flare (m)	10.00	12.00	12.00
R = Entry radius (m)	55.00	60.00	40.00
D = Inscribed circle diameter (m)	107.00	107.00	107.00
A = Entry angle (degree)	40.00	45.00	45.00
Q = Entry flow (pcu/h)	660	1355	1315
Qc = Circulating flow across entry (pcu/h)	880	280	315
OUTPUT PARAMETERS:			
S = Sharpness of flare = 1.6(E-V)/L	0.64	0.65	0.53
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.00	0.98	0.97
X2 = V + ((E-V)/(1+2S))	9.75	9.22	9.94
M = EXP((D-60)/10)	109.95	109.95	109.95
F = 303*X2	2956	2795	3010
Td = 1+(0.5/(1+M))	1.00	1.00	1.00
Fc = 0.21*Td(1+0.2*X2)	0.62	0.60	0.63
Qe = K(F-Fc*Qc)	2399	2576	2734
DFC = Design flow/Capacity = Q/Qe	0.28	0.53	0.48

TOTAL ENTRY FLOWS = 3330 PCU
CRITICAL DFC = 0.53

JUNCTION CAPACITY CALCULATION

Junction J2 - Yu Tung Road / Shun Tung Road - Improved Scheme

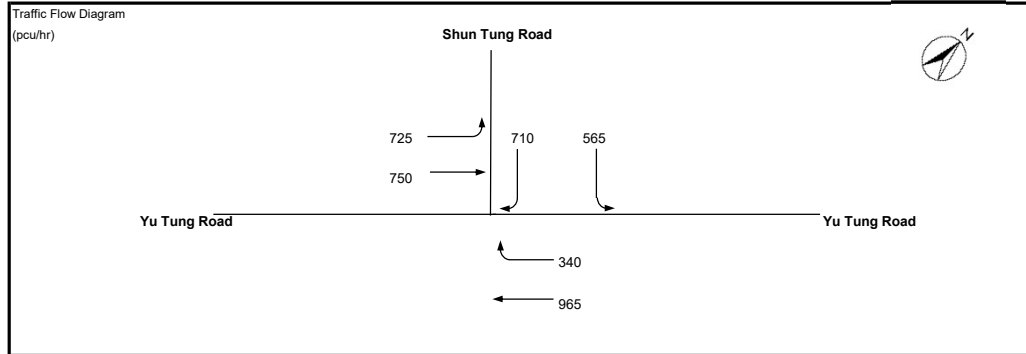
2033 PM Design Traffic Flows (Proposed Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

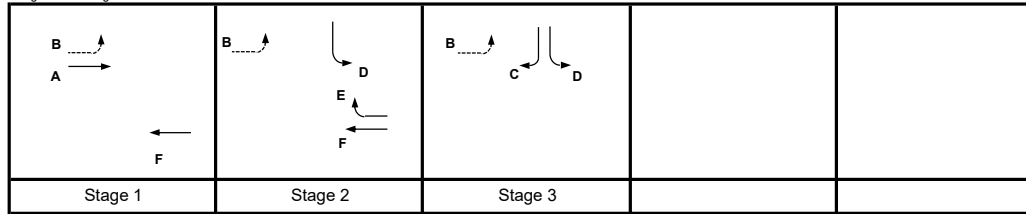
DATE: Nov 20



No. of stages per cycle	N =	3
Cycle time	C =	120 sec
Sum(y)	Y =	0.519
Lost time	L =	12 sec
Total Flow	=	18,780 pcu
Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	48 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	25 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.810
$R.C._{ult}$	$= (Y_{ult} - Y) / Y \times 100\% =$	55.9 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	28 sec
Y_{max}	$= 1 - L / C =$	0.900

J2

Stage/Phase Diagrams



I/G = 5

I/G = 5

I/G = 5

Critical Case : A,E,C

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 56\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road NB	A							1	4.000	2		50				
Shun Tung Road EB	D	2,3	3.700	1	25		0	0			1985	565		565	100%		1873	0.302	0.175		
	C	3	3.700	2		30	0	0			4250		710	710		100%	4048	0.175	0.175		
Yu Tung Road SB	F	1,2	3.650	2			0	0			4100		965	965		100%	4100	0.235	0.170		
	E	2	3.650	1		25	0	0			2120		340	340		100%	2000	0.170	0.170		

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

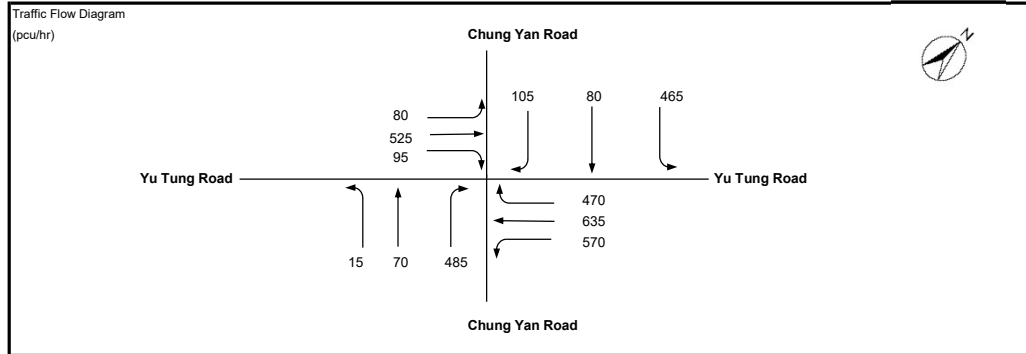
2033 PM Design Traffic Flows (Proposed Scheme) (With CEDD Improvements)

DESIGN:

CHECK:

JOB NO:

DATE: Nov 20

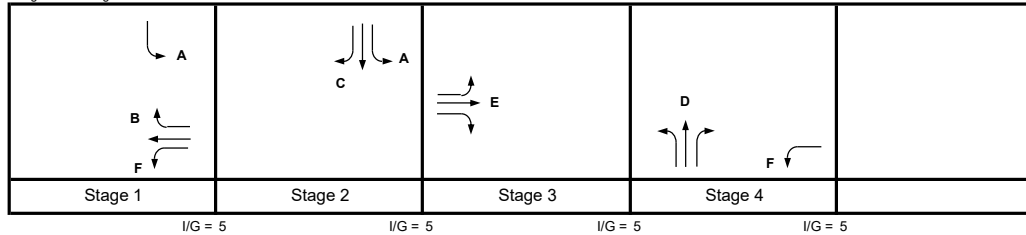


No. of stages per cycle	N = 4
Cycle time	C = 120 sec
Sum(y)	Y = 0.610
Lost time	L = 16 sec
Total Flow	= 22,595 pcu

J3

Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	74 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	41 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.780
R.C. _{ult}	$= (Y_{ult} - Y) / Y \times 100\% =$	27.9 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	50 sec
Y_{max}	$= 1 - L / C =$	0.867

Stage/Phase Diagrams



Critical Case : B,C,E,D

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 28\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					Yu Tung Road SB	F							1,4	3.500	1		20				
	B	1	3.500	1								2105		565		565		2105	0.268		
	B	1	3.500	1		28	0	0				2105		70	470	540	87%	2011	0.268		
Chung Yan Road WB	D	4	3.500	1	25	25	0	1				1965	15	70	192	277	5%	69%	1881	0.147	0.147
	D	4	3.500	1		25	0	0				2105		293	293	293	100%	1986	0.147		
Yu Tung Road NB	E	3	3.500	1	15			1				1965	80			80	100%		1786	0.045	0.149
	E	3	3.500	1				0				2105		313	313	313		2105	0.149		
	E	3	3.500	1		25	0	0				2105		212	95	307	31%	2067	0.149		
Chung Yan Road EB	A	1,2	3.500	1	16			1				1965	465			465	100%		1797	0.259	0.046
	C	2	3.500	1		18		0				2105		80	15	95	15%	2078	0.046		
	C	2	3.500	1		25	0	0				2105		90	90	90	100%	1986	0.046		

JUNCTION CAPACITY CALCULATION

Junction J3 - Yu Tung Road / Chung Yan Road - Improved Scheme

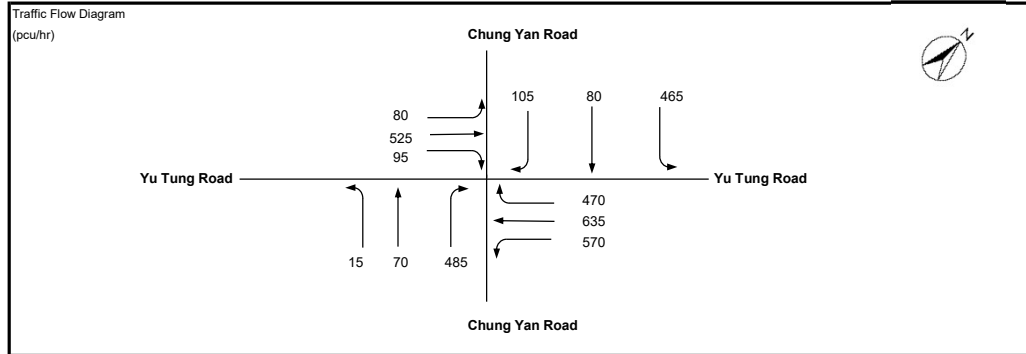
2033 PM Design Traffic Flows (Proposed Scheme)

DESIGN:

CHECK:

JOB NO:

DATE: Nov 20

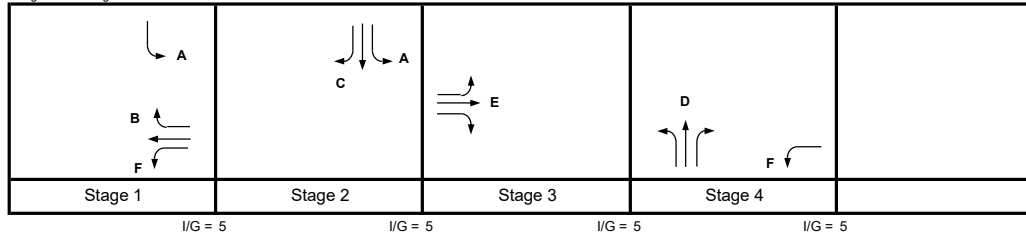


No. of stages per cycle	N = 4
Cycle time	C = 120 sec
Sum(y)	Y = 0.505
Lost time	L = 16 sec
Total Flow	= 24,700 pcu

J3

Optimum Cycle C_o	$= (1.5 \times L + 5) / (1 - Y) =$	59 sec
Min. Cycle Time C_m	$= L / (1 - Y) =$	32 sec
Y_{ult}	$= 0.9 - 0.0075 \times L =$	0.780
R.C. _{ult}	$= (Y_{ult} - Y) / Y \times 100\% =$	54.5 %
Practical Cycle Time C_p	$= 0.9 \times L / (0.9 - Y) =$	36 sec
Y_{max}	$= 1 - L/C =$	0.867

Stage/Phase Diagrams



Critical Case : B,C,E,D

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 55\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
Yu Tung Road SB																					
↑	F	1,4	3.500	1	20			1		0		1965	570			570	100%		1828	0.312	
↑	B	1	3.500	1				0		0		2105		377		377			2105	0.179	
↑	B	1	3.500	1			28	0		0		2105		258	114	371	31%		2071	0.179	
↑	B	1	3.500	1			25	0	0	0		2105		356	356	356	100%		1986	0.179	0.179
Chung Yan Road WB																					
←	D	4	3.500	1	25	25	0	1		0		1965	15	70	192	277	5%	69%	1881	0.147	0.147
←	D	4	3.500	1		25	0	0		0		2105		293	293	293	100%		1986	0.147	
Yu Tung Road NB																					
↑	E	3	3.500	1	15			1		0		1965	80	139		219	37%		1896	0.116	0.116
↑	E	3	3.500	1				0		0		2105		243		243			2105	0.116	
↑	E	3	3.500	1		25	0	0		0		2105		143	95	238	40%		2056	0.116	
Chung Yan Road EB																					
←	A	1,2	3.500	1	16			1		0		1965	400	61		400	100%		1797	0.223	
←	C	2	3.500	1	18			0		0		2105	65	61		126	52%		2018	0.062	0.062
←	C	2	3.500	1		25	0	0		0		2105		19	105	124	85%		2003	0.062	

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J4 - Tung Chung Road / Chung Yan Road

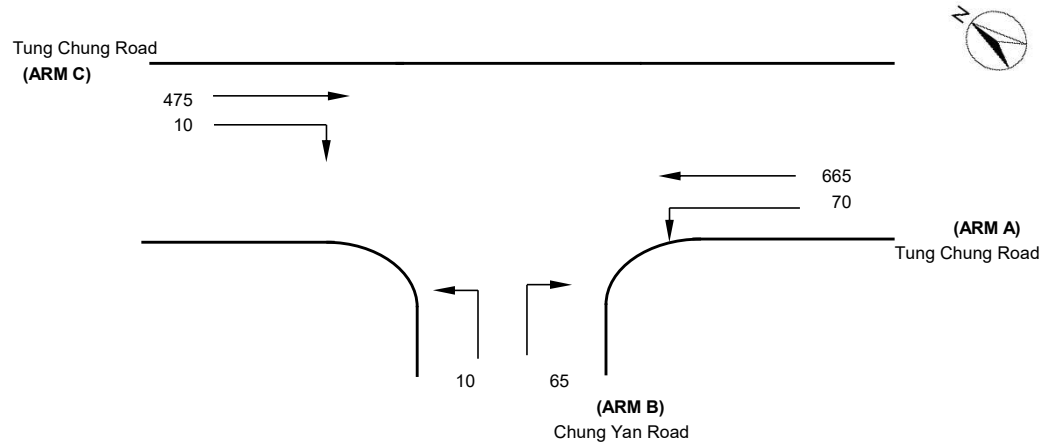
2033 PM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J4

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	665 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	100 (metres)
q c-a	=	475 (pcu/hr)
q c-b	=	10 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	65 (pcu/hr)
q b-c	=	10 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.948063
E	=	0.977385
F	=	0.977385
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	335
Q b-c	=	544
Q c-b	=	533
Q b-ac	=	353

CRITICAL DFC = 0.21

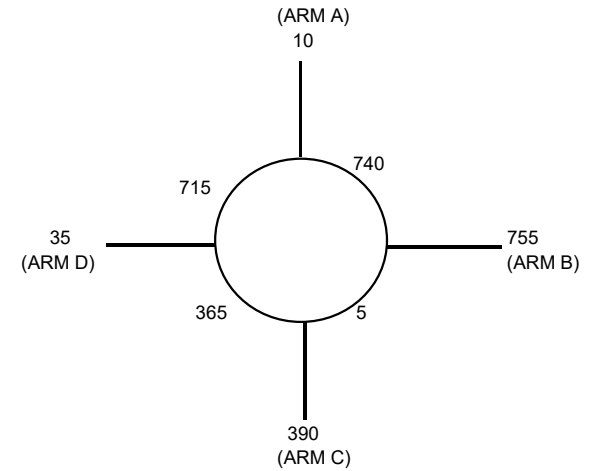
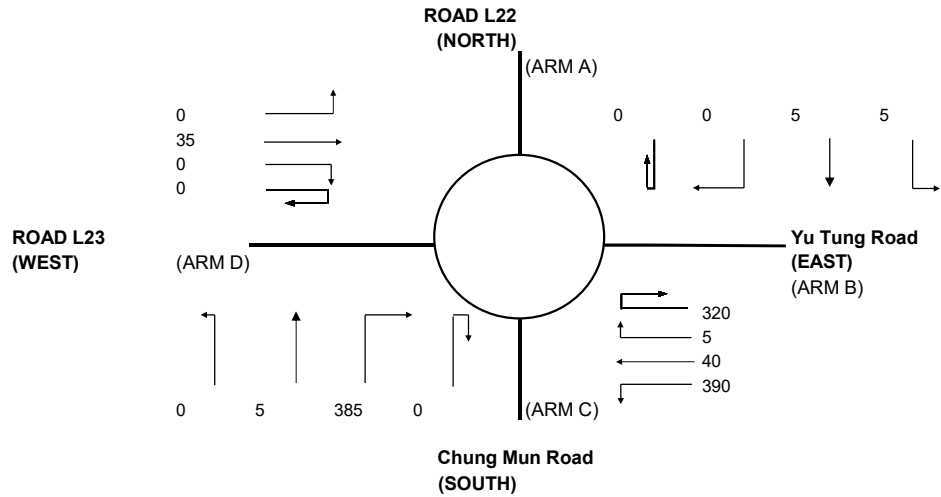
COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.19
DFC b-c	=	0.02
DFC c-b	=	0.02
DFC b-ac	=	0.21

ROUNDBABOUT CAPACITY CALCULATIO

AECOM

Junction	J5 - Yu Tung Road / Chung Mun Road	Scenario	2033 - PM - Design Traffic Flows (Proposed Scheme)	Project No.	Prepared By	Checked By	Date
				-	-	-	Nov 2024



ARM	A	B	C	D
INPUT PARAMETERS:				
V = Approach half width (m)	5.75	8.80	7.30	4.85
E = Entry width (m)	6.50	9.00	8.50	6.00
L = Effective length of flare (m)	10.00	7.00	15.00	15.00
R = Entry radius (m)	25.00	25.00	40.00	25.00
D = Inscribed circle diameter (m)	60.00	60.00	60.00	60.00
A = Entry angle (degree)	20.00	20.00	30.00	20.00
Q = Entry flow (pcu/h)	10	755	390	35
Qc= Circulating flow across entry (pcu/h)	740	5	365	715
OUTPUT PARAMETERS:				
S = Sharpness of flare = 1.6(E-V)/L	0.12	0.05	0.13	0.12
K = 1-0.00347(A-30)-0.978(1/R-0.05)	1.04	1.04	1.02	1.04
X2= V + ((E-V)/(1+2S))	6.35	8.98	8.26	5.77
M = EXP((D-60)/10)	1.00	1.00	1.00	1.00
F = 303*X2	1926	2722	2501	1749
Td= 1+(0.5/(1+M))	1.25	1.25	1.25	1.25
Fc= 0.21*Td(1+0.2*X2)	0.60	0.73	0.70	0.57
Qe= K(F-Fc*Qc)	1550	2839	2302	1405
DFC = Design flow/Capacity = Q/Qe	0.01	0.27	0.17	0.02

TOTAL ENTRY FLOWS = 1190 PCU

CRITICAL DFC = 0.27

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J6 - Tung Chung Road / Shek Mun Kap Road

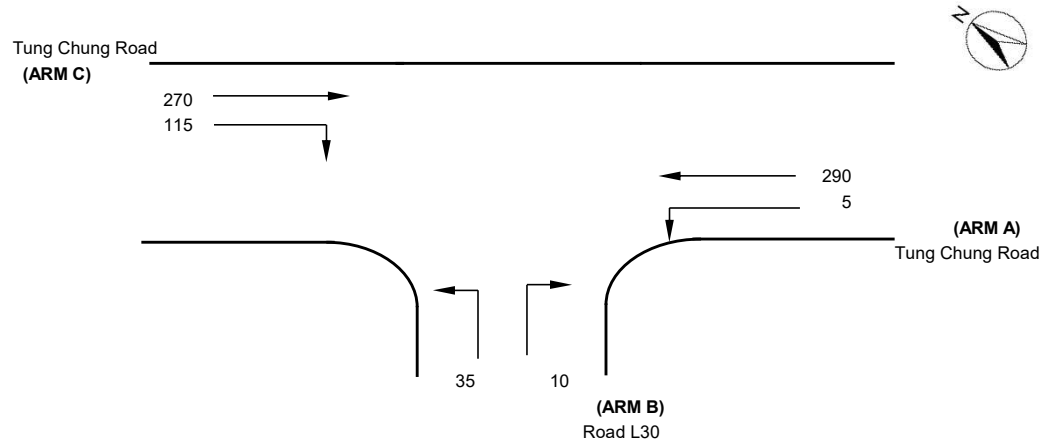
2033 PM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J6

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	5 (pcu/hr)
q a-c	=	290 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	270 (pcu/hr)
q c-b	=	115 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	10 (pcu/hr)
q b-c	=	35 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	502
Q b-c	=	710
Q c-b	=	709
Q b-ac	=	650

CRITICAL DFC = 0.16

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.02
DFC b-c	=	0.05
DFC c-b	=	0.16
DFC b-ac	=	0.07

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J7 - Road L29 / Road L30

2033 PM Design Traffic Flows (Proposed Scheme)

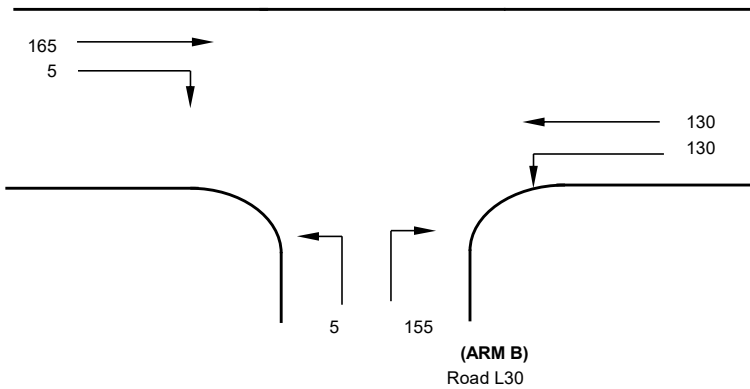
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J7

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	11.7 (metres)
W cr	=	1.5 (metres)
q a-b	=	130 (pcu/hr)
q a-c	=	130 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	165 (pcu/hr)
q c-b	=	5 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	155 (pcu/hr)
q b-c	=	5 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.857384
Y	=	0.596350

THE CAPACITY OF MOVEMENT :

Q b-a	=	486
Q b-c	=	605
Q c-b	=	590
Q b-ac	=	489

CRITICAL DFC = 0.33

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.32
DFC b-c	=	0.01
DFC c-b	=	0.01
DFC b-ac	=	0.33

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J8 - Tung Chung Road / Road L30

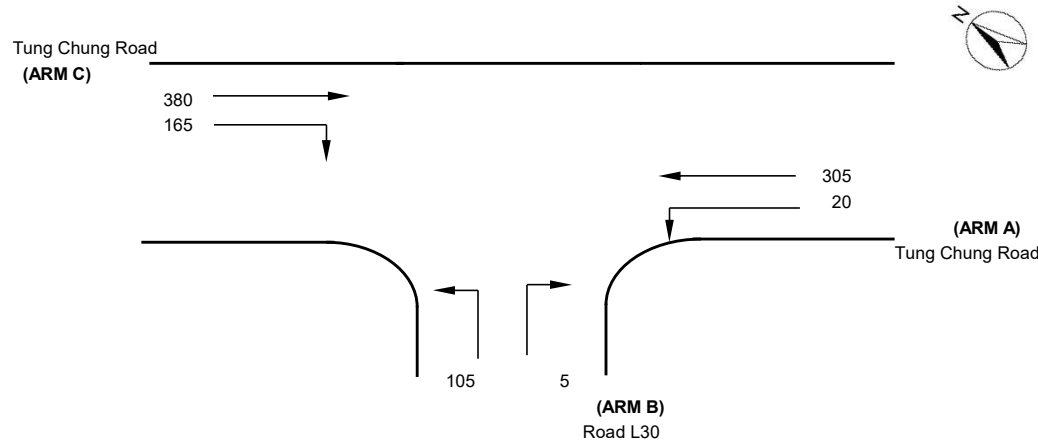
2033 PM Design Traffic Flows (Proposed Scheme)

Designed By :

Checked By :

Job No. :

Date : Nov 24



NOTES : (GEOMETRIC INPUT DATA)

J8

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	20 (pcu/hr)
q a-c	=	305 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	3.6 (metres)
Vr c-b	=	200 (metres)
q c-a	=	380 (pcu/hr)
q c-b	=	165 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	3.6 (metres)
W b-c	=	3.6 (metres)
VI b-a	=	200 (metres)
Vr b-a	=	200 (metres)
Vr b-c	=	200 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	105 (pcu/hr)

GEOMETRIC FACTORS :

D	=	1.098970
E	=	1.066962
F	=	1.066962
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	453
Q b-c	=	704
Q c-b	=	700
Q b-ac	=	687

CRITICAL DFC = 0.24

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.15
DFC c-b	=	0.24
DFC b-ac	=	0.16

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J9 - Road L29 / Road L25

2033 PM Design Traffic Flows (Proposed Scheme)

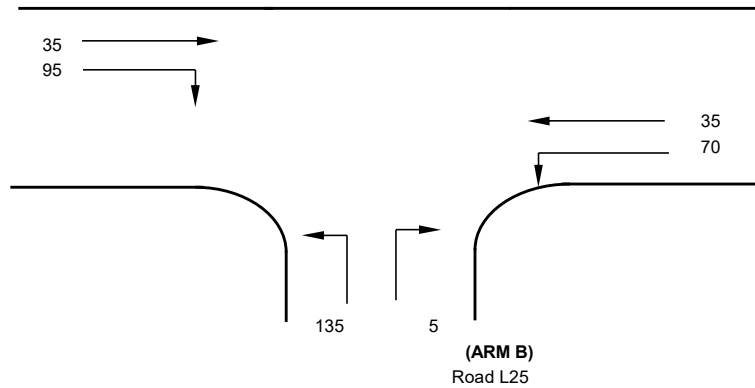
Designed By :

Checked By :

Job No. :

Date : Nov 24

Road L29
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J9

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	10.3 (metres)
W cr	=	1.5 (metres)
q a-b	=	70 (pcu/hr)
q a-c	=	35 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.1 (metres)
Vr c-b	=	100 (metres)
q c-a	=	35 (pcu/hr)
q c-b	=	95 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.3 (metres)
W b-c	=	2.3 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	135 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.831663
E	=	0.857384
F	=	0.838923
Y	=	0.644650

THE CAPACITY OF MOVEMENT :

Q b-a	=	496
Q b-c	=	626
Q c-b	=	604
Q b-ac	=	620

CRITICAL DFC = 0.23

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.22
DFC c-b	=	0.16
DFC b-ac	=	0.23

PRIORITY JUNCTION CAPACITY CALCULATION

Junction J10 - Road L28 / Road L29 / Shek Mun Kap Road

2033 PM Design Traffic Flows (Proposed Scheme)

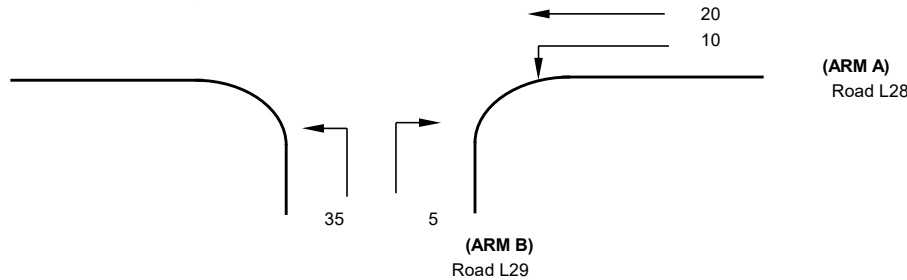
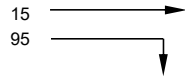
Designed By :

Checked By :

Job No. :

Date : Nov 24

Shek Mun Kap Road
(ARM C)



NOTES : (GEOMETRIC INPUT DATA)

J10

- W = Major Road Width (6.4 - 20.0)
- W cr = Central Reserve width (1.2 - 9.0, kerbed central reserve only)
- W b-a = Lane width available to vehicle waiting in stream b-a (2.05 - 4.07)
- W b-c = Lane width available to vehicle waiting in stream b-c (2.05 - 4.07)
- W c-b = Lane width available to vehicle waiting in stream c-b (2.05 - 4.07)
- VI b-a = Visibility to the left for vehicles waiting in stream b-a (22.0 - 250.0)
- Vr b-a = Visibility to the right for vehicles waiting in stream b-a (17.0 - 250.0)
- Vr b-c = Visibility to the right for vehicles waiting in stream b-c (17.0 - 250.0)
- Vr c-b = Visibility to the right for vehicles waiting in stream c-b (17.0 - 250.0)

- D = Stream-specific B-A
- E = Stream-specific B-C
- F = Stream-specific C-B
- Y = (1-0.0345W)

GEOMETRIC DETAILS:

MAJOR ROAD (ARM A)

W	=	7.3 (metres)
W cr	=	0 (metres)
q a-b	=	10 (pcu/hr)
q a-c	=	20 (pcu/hr)

MAJOR ROAD (ARM C)

W c-b	=	2.3 (metres)
Vr c-b	=	100 (metres)
q c-a	=	15 (pcu/hr)
q c-b	=	95 (pcu/hr)

MINOR ROAD (ARM B)

W b-a	=	2.7 (metres)
W b-c	=	2.7 (metres)
VI b-a	=	100 (metres)
Vr b-a	=	100 (metres)
Vr b-c	=	100 (metres)
q b-a	=	5 (pcu/hr)
q b-c	=	35 (pcu/hr)

GEOMETRIC FACTORS :

D	=	0.867478
E	=	0.894307
F	=	0.857384
Y	=	0.748150

THE CAPACITY OF MOVEMENT :

Q b-a	=	504
Q b-c	=	660
Q c-b	=	632
Q b-ac	=	636

CRITICAL DFC = 0.15

COMPARISON OF DESIGN FLOW TO CAPACITY :

DFC b-a	=	0.01
DFC b-c	=	0.05
DFC c-b	=	0.15
DFC b-ac	=	0.06

JUNCTION CAPACITY CALCULATION

Junction J11 - Chung Mun Road / Road L29

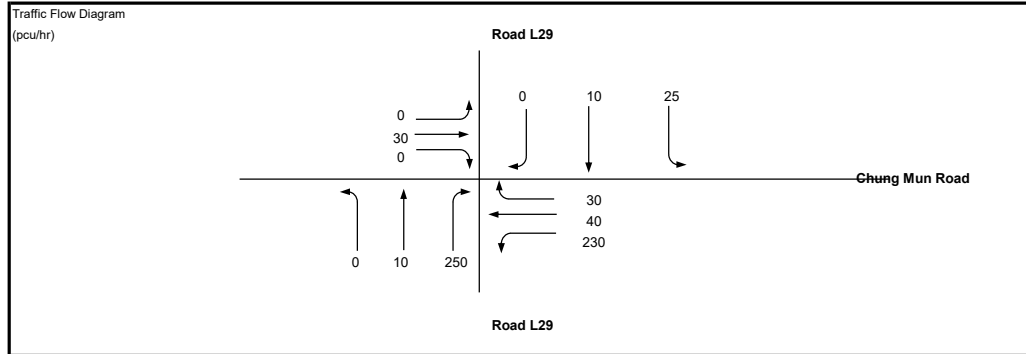
2033 PM Design Traffic Flows (Proposed Scheme)

DESIGN:

CHECK:

JOB NO:

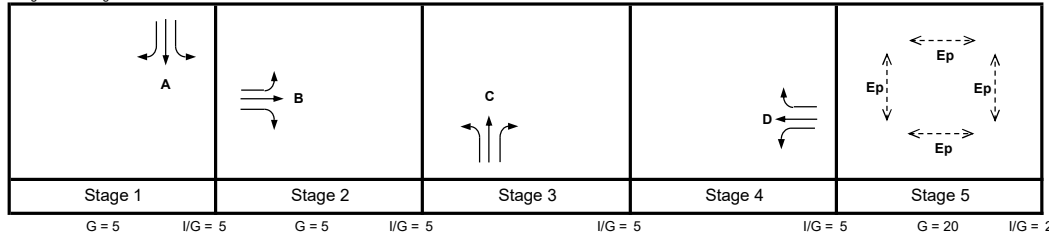
DATE: Nov 20



No. of stages per cycle	N =	5
Cycle time	C =	120 sec
Sum(y)	Y =	0.220
Lost time	L =	50 sec
Total Flow	=	9,965 pcu
Optimum Cycle C_o	= $(1.5 \times L + 5) / (1 - Y)$	103 sec
Min. Cycle Time C_m	= $L / (1 - Y)$	64 sec
Y_{ult}	= $0.9 - 0.0075 \times L$	0.525
R.C. _{ult}	= $(Y_{ult} - Y) / Y \times 100\%$	138.9 %
Practical Cycle Time C_p	= $0.9 \times L / (0.9 - Y)$	66 sec
Y_{max}	= $1 - L/C$	0.583

J11

Stage/Phase Diagrams



Critical Case : A,B,C,D,Ep

R.C.(C) = $(0.9 \times Y_{max} - Y) / Y \times 100\% = 139\%$

MOVEMENT	PHASE	STAGE	LANE WIDTH (m)	NO. OF LANES	RADIUS (m)		OPPOSING TRAFFIC	NEAR SIDE LANE	UPHILL GRADIENT (%)	GRADIENT EFFECT (pcu/hr)	ADDITIONAL CAPACITY (pcu/hr)	STRAIGHT-AHEAD SAT. FLOW (pcu/hr)	FLOW (pcu/hr)			TOTAL FLOW (pcu/hr)	PROPORTION OF TURNING VEHICLES (%)		REVISED SAT. FLOW (pcu/hr)	FLOW FACTOR y	CRITICAL y
					LEFT	RIGHT							LEFT	STRAIGHT AHEAD	RIGHT		LEFT	RIGHT			
					↔	A							1	3.500	1		30	25			
↔	B	2	3.500	1	30	25	0	1	0		1965	0	30	0	30	0%	0%	1965	0.015		
↔	C	3	3.500	1	30	25	0	1	0		1965	0	10	250	260	0%	96%	1858	0.140	0.140	
↑	D	4	3.500	1	15	25	0	1	0		1965	143	40	30	143	100%	19%	1786	0.080	0.080	
↔	D	4	3.500	1	15	25	0	0	0		2105	87	40	30	157	56%	19%	1973	0.080		
Pedestrian Crossing				Ep	5	min.	GM	15	+	FGM	5	=	20	sec							