Proposed Rezoning of the Site from "Other Specified Uses" annotated "Business" to "Other Specified Uses" annotated "Residential Care Home for the Elderly and Hotel" for a Proposed Composite Development with RCHE and Hotel at Nos. 107 – 109 Wai Yip Street, Kwun Tong

(Planning Application No. Y/K14S/4)



Revised Traffic Impact Assessment

Traffic Impact Assessment Final Report March 2025

Prepared by: CKM Asia Limited

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#### 1.0 INTRODUCTION

#### Background

- 1.1 The Subject Site is located at Nos. 107 109 Wai Yip Street in Kwun Tong, which is now vacant. Figure 1.1 shows the location of the Subject Site.
- 1.2 On 29<sup>th</sup> May 2020, the Town Planning Board ("TPB") approved the S16 Planning Application for Office, Shop and Services & Eating Place Uses at 107-109 Wai Yip Street (TPB ref: A/K14/780) ("Approved S16 Scheme"). The Applicant has the intention to rezone the Subject Site and construct residential care home for the elderly ("RCHE") and a hotel (together known as "Proposed Development").
- 1.3 CKM Asia Limited, a traffic and transportation planning consultancy firm, was commissioned by the Applicant, to conduct a traffic impact assessment ("TIA") in support of Proposed Development. This report describes the traffic study undertaken.

#### Scope of the Assessment

- 1.4 The main objectives of this TIA are as follows:
  - To assess the existing traffic issues in the vicinity of the subject site;
  - To quantify the traffic and pedestrians generated by the Proposed Development; and
  - To examine the traffic and pedestrian impact on the local road network in the vicinity of the subject site.

#### Contents of the Report

1.5 After this introduction, the remaining chapters contain the following:

Chapter Two Chapter Three Chapter Four Chapter Five Chapter Six	- - -	describes the existing situation; outlines the development proposal; presents the traffic impact analysis; presents the pedestrian impact analysis presents the traffic and pedestrian sensitivity test; and
Chapter Six Chapter Seven	-	presents the traffic and pedestrian sensitivity test; and summarises the overall conclusion.

#### 2.0 THE EXISTING SITUATION

#### The Subject Site

2.1 The Subject Site fronts onto Wai Yip Street to the south, and is bounded by a service lane to the north. The section of Wai Yip Street fronting the Subject Site is a dual carriageway 3-lane road.

#### Traffic Survey

- 2.2 To quantify the traffic flows at the junctions chosen for the capacity analysis, manual classified counts were conducted on Friday, 15<sup>th</sup> March 2024 during the AM and PM peak periods. The locations of the surveyed junctions are presented in Figure 2.1 and their layout is shown in Figures 2.2 to 2.11.
- 2.3 The surveyed junctions include the following:
  - J1: Hoi Bun Road / Shun Yip Street;
  - J2: Wai Yip Street / Shun Yip Street;
  - J3: Tai Yip Street / Service Lane;
  - J4: Hong Tak Road / Tai Yip Street;
  - J5: Tai Yip Street / Tai Yip Lane;
  - J6: Kwun Tong Road / Hong Tak Road;
  - J7: Wai Yip Street / Lai Yip Street;
  - J8: Kwun Tong Road / Lai Yip Street;
  - J9: Hoi Bun Road / Lai Yip Street and;
  - J10: Lai Yip Street / Hung To Road
- 2.4 The counts were classified by vehicle type to enable traffic flows in passenger car units ("pcu") to be calculated. From the survey, the AM and PM peak hours were found to be between 0845 0945 and 1730 1830 hours respectively.

#### Adjustment of the traffic flows obtained from the traffic survey

- 2.5 The traffic flows obtained from the traffic surveys conducted in March 2024 were reviewed against the traffic flows of the Traffic Impact Assessment of other approved planning applications and found to be of similar order. Nevertheless, adjustment of the traffic flows obtained from the traffic survey is made based on the Annual Average Daily Traffic ("AADT") of Annual Traffic Census ("ATC") station 3020 Wai Yip Street (from Lai Yip Street to Hoi Yuen Road), in order to produce adjusted annual average traffic flows.
- 2.6 AADT is only available up to 2023. However, the AADT for 2019 is not considered due to the impact of the social events, and the AADT for 2020 to 2023 are also not considered due to the impact of the COVID-19 pandemic. Hence, reference is made to 2018 AADT. The 2018 monthly variation in the AADT for ATC station 3020 Wai Yip Street in Kwun Tong, is found in Chart A.

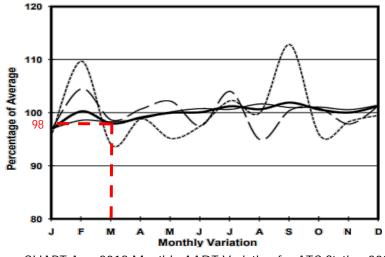


CHART A: 2018 Monthly AADT Variation for ATC Station 3020

2.7 Chart A shows that the AADT for the month of March<sub>7</sub> is around 2% lower than the annual average. In view that the traffic survey for the captioned project was conducted in March, the monthly variation factor of 1.02 (Calculation: 1 / 0.98 = 1.02) is applied to the traffic flows. The adjusted peak hour traffic flows are presented in Figure 2.12.

#### Operational Performance of the Surveyed Junctions

2.8 The existing operational performance of the surveyed junctions is calculated based on the observed traffic counts and the analysis is undertaken using the methods outlined in Volume 2 of Transport Planning and Design Manual ("TPDM"). The existing operational performance of the surveyed junctions are summarised in Table 2.1 and the detailed calculations are found in Appendix 1.

Ref	Junction	Type of Junction	Parameter <sup>(1)</sup>	AM Peak	PM Peak
J1	Hoi Bun Road / Shun Yip Street	Signal	RC	56%	43%
J2	Wai Yip Street / Shun Yip Street	Signal	RC	66%	62%
J3	Tai Yip Street / Service Lane	Priority	RFC	0.016	0.010
J4	Hong Tak Road / Tai Yip Street	Priority	RFC	0.226	0.181
J5	Tai Yip Street / Tai Yip Lane	Priority	RFC	0.058	0.025
J6	Kwun Tong Road / Hong Tak Road	Priority	RFC	0.365	0.454
J7	Wai Yip Street / Lai Yip Street	Signal	RC	69%	87%
J8	Kwun Tong Road / Lai Yip Street	Signal	RC	58%	43%
J9	Hoi Bun Road / Lai Yip Street	Signal	RC	81%	87%
J10	Lai Yip Street / Hung To Road <sup>(2)</sup>	Signal	RC	85%	104%
Notes: (1) DC Deserve Canacity DEC Datie of Flow to Canacity					

TABLE 2.1EXISTING JUNCTION OPERATIONAL PERFORMANCE

Notes: <sup>(1)</sup> RC – Reserve Capacity RFC – Ratio of Flow to Capacity <sup>(2)</sup> Kerbside on-street activities are reflected in the junction performance

2.9 The results in Table 2.1 indicate that the junctions now operate with capacities during the AM and PM peak hours.

#### Pedestrian Facilities

2.10 There are good pedestrian facilities provided in the vicinity of the Subject Site, including footpaths, and at-grade pedestrian crossings are provided at the signalised road junctions.

#### Availability of Public Transport Facilities

- 2.11 The Subject Site is well-served by various types of public transport services, including road-based franchised bus and public light bus. These services operate along Kwun Tong Road and Wai Yip Street within 500m or about 10 minutes' walk away. The Subject Site is located closest to the Ngau Tau Kok MTR Station and the nearest entrance is at Lai Yip Street, which is some 500 metres or 10 minutes' walk away.
- 2.12 Details of the road-based public transport services operating in the vicinity of the Subject Site are shown in Figure 2.13 and Table 2.2.

### TABLE 2.2FRANCHISED BUS AND GMB SERVICES OPERATING CLOSE TO<br/>THE SUBJECT SITE

KMB 11BKwun Tong (Tsui Ping Road) – Kowloon City Ferry10 – 25KMB 11CChuk Yuen Estate – Sau Mau Ping (Upper)15 – 25KMB 11DLok Fu – Kwun Tong Ferry15 – 30KMB 13DPo Tat – Island Harbourview15 – 25KMB 13MKwun Tong (Elegance Road) – Po Tat (Circular)15 – 30KMB 14Lei Yue Mun Estate – China Ferry Terminal12 – 25KMB 14BNgau Tau Kok – Lam Tin (Kwong Tin Estate)15 – 30KMB 14BNgau Tau Kok – Lam Tin (Kwong Tin Estate)15 – 25KMB 14BNgau Tau Kok – Lam Tin (Kwong Tin Estate)12 – 20KMB 15Ping Tin – Hung Hom (Hung Luen Road)12 – 20KMB 15APing Tin – Tsz Wan Shan (North)20 – 30KMB 16ALam Tin (Kwong Tin Estate) – Hung Hom StationAM, PM PeaKMB 16Lam Tin (Kwong Tin Estate) – Hong Kok (Park Avenue)8 – 20KMB 16PKwun Tong Ferry – Mong Kok (Park Avenue)AM, PM PeaKMB 17Kwun Tong (Yue Man Square) – Ho Man Tin (Oi Man Estate)5 – 20KMB 23MLok Wah – Shun Lee (Circular)12 – 20KMB 28BChoi Fook – Kai Tak (Kai Ching Estate)15 – 30KMB 23SKwun Tong (Yue Mun Square) – Lok WahAM PeakKMB 33Tsuen Wan West Station – Yau Tong20 – 25KMB 38Kwai Shing (East) – Ping Tin5 – 20KMB 38Kwai Shing (Central) – Ping Tin5 – 20KMB 400Tsuen Wan (Belvedere Garden) – Laguna City12 – 25KMB 40APing Tin – Kwai Hing StationAM, PM PeaKMB 40B<	Route	Route Routing		
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A = 25	KMB 62X	Tuen Mun Central – Lei Yue Mun Estate	8 – 25	
			AM, PM Peak	
KMB 74CKau Lung Hang – Kwun Tong FerryAM Peak		,		
KMB 74DKau Lung Hang – Kwun Tong Ferry25 – 60				
			AM, PM Peak	
KMB 74FKwun Tong Ferry – Education University of Hong KongAM Peak				
KMB 74PKwun Tong Ferry – Tai Po CentralAM Peak				
KMB 74XTai Po Central – Kwun Tong Ferry3 – 15				
KMB 94XHarro central = Kwah rong reny3 = 13KMB 80Mei Lam – Kwah rong Ferry5 – 20				
KMB 80AMei Lam – Kwun Tung FerryAM Peak				
KMB 80PHin Keng – Kwun Tong FerryAM Peak				

Route	Routing	Frequency (minutes)
KMB 80X	Chun Shek – Kwun Tong Ferry	8 – 25
KMB 83A	Shui Chuen O – Kwun Tong Ferry	AM Peak
KMB 83X	Shui Chuen O – Kwun Tong Ferry	8 - 30
KMB 88X	Fo Tan Chung Yeung Estate – Ping Tin (Circular)	20 - 30
KMB 89	Lek Yuen – Kwun Tong Station	8 - 20
KMB 89B	Shatin Wai – Kwun Tong Station	10 - 25
KMB 89C	Heng On – Kwun Tong (Tsui Ping Road)	12 – 30
KMB 89D	Wu Kai Sha Station – Lam Tin Station	3 – 20
KMB 89P	Ma On Shan Town Centre – Lam Tin Station Bus Terminus	AM Peak
KMB 89X	Shatin Station – Kwun Tong (Tsui Ping Road)	7 – 20
KMB 93K	Po Lam – Mong Kok East Station	15 – 30
KMB 95M	Tsui Lam – Kwun Tong Road (Elegance Road)	20 – 30
KMB 98	Tseung Kwan O Industrial Estate – Ngau Tau Kok Station (Circular)	15 – 20
KMB 98A	Hang Hau (North) (Tseung Kwan O Hospital) – Ngau Tau Kok Station (Circular)	8 – 20
KMB 98B	Hang Hau (North) (Tseung Kwan O Hospital) – Kwun Tong Station	AM Peak
KMB 213B	On Tai – Ting Fu Street (Circular)	AM Peak
KMB 215X	Lam Tin (Kwong Tin Estate) – Kowloon Station	5 – 20
KMB 234C	Sham Tseng – Kwun Tong Station	AM, PM Peak
KMB 234D	Tsing Lung Tau – Kwun Tong Station	AM, PM Peak
KMB 252X	Handsome Court – Lam Tin Station	AM, PM Peak
KMB 258A	Hung Shui Kiu (Hung Fuk Estate) – Lam Tin Station	AM Peak
KMB 258D	Tuen Mun (Po Tin Estate) – Lam Tin Station	5 – 20
KMB 258P	Hung Shui Kiu (Hung Fuk Estate) – Lam Tin Station	AM, PM Peak
KMB 258S	Tuen Mun (Shan King Estate) – Lam Tin Station	AM Peak
KMB 258X	Tuen Mun (Po Tin Estate) – Kwun Tong Ferry	AM, PM Peak
KMB 259D	Tuen Mun (Lung Mun Oasis) – Lei Yue Mun Estate	7 – 25
KMB 259X	Lung Mun Oasis – Kwun Tong Ferry	AM, PM Peak
KMB 267X	Tuen Mun (Siu Hong Court) – Lam Tin Station	AM, PM Peak
KMB 268A	Long Ping Estate – Kwun Tong Ferry	AM, PM Peak
KMB 268C	Long Ping Station – Kwun Tong Ferry	5 – 20
KMB 268P	Ma Wang Road (Shan Shui House) – Kwun Tong Ferry	
	Kwun Tong Ferry – Long Ping Station	AM, PM Peak
KMB 269C	Tin Shui Wai Town Centre – Kwun Tong Ferry	5 – 20
KMB 269S	Tin Shui Wai Town Centre – Kwun Tong Ferry	AM, PM Peak
KMB 274X	Kwun Tong Ferry – Tai Po Central	PM Peak
KMB 277A	Sha Tau Kok – Lam Tin Station	AM, PM Peak
KMB 277E	Lam Tin Station – Sheung Shui (Tin Ping)	15 – 30
KMB 277P	Sheung Shui (Tin Ping) – Lam Tin Station	AM, PM Peak
KMB 277X	Fanling (Luen Wo Hui) – Lam Tin Station	5 – 30
KMB 296A	Sheung Tak – Ngau Tau Kok Station (Circular)	7 – 15
KMB 296C	Sheung Tak – Cheung Sha Wan (Hoi Ying Estate)	15 – 30
KMB N3D	Kwun Tong (Yue Man Square) – Tsz Wan Shan (Central)	Overnight
KMB N293	Sheung Yak – Mong Kok East Station	Overnight
KMB T74	Tai Po (Tai Wo) – Kwun Tong Ferry	AM Peak
KMB T277		
	Sheung Shui – Lam Tin Station	AM, PM Peak
KMB W2	Jordan (West Kowloon Station) – Kwun Tong (Circular)	30 - 60
KMB X42C	Tsing Yi (Cheung Hang Estate) – Yau Tong	7-30
KMB X42P	Tsing Yi (Cheung On Estate) – Lam Tin Station	AM Peak
KMB X89D	Nai Chung – Kwun Tong Ferry	AM, PM Peak
KMB/CTB 101	Kwun Tong (Yue Man Square) – Kennedy Town	3 20
KMB/CTB 101X	Kwun Tong (Yue Man Square) – Kennedy Town	AM, PM Peak
KMB/CTB 606	Siu Sai Wan (Island Resort) – Choi Wan (Fung Shing Street)	20 – 25
KMB/CTB 606A	Shau Kei Wan (Yiu Tung Estate) – Choi Wan (Fung Shing Street)	AM Peak
KMB/CTB 606X	Siu Sai Wan (Island Resort) – Kowloon Bay	AM, PM Peak
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Route	Routing	Frequency			
		(minutes)			
KMB/CTB 619	Shun Lee – Central (Macau Ferry)	4 – 25			
KMB/CTB 619P	Shun Lee – Central (Macau Ferry)	AM Peak			
KMB/CTB 641	Kai Tak (Kai Ching Estate) – Central (Macau Ferry)	AM, PM Peak			
KMB/CTB 671	Diamond Hill Station – Ap Lei Chau Lee Lok Street	15 – 45			
KMB/CTB 671X	Ap Lei Chau Lee Lok Street – Diamond Hill Station	AM Peak			
KMB/CTB N619	Shun Lee – Central (Macau Ferry)	Overnight			
CTB 55	Ching Tin and Wo Tin – Kwun Tong Ferry Pier	AM, PM Peak			
CTB 61R	Lam Tin Station – City One Shatin	12 – 20			
CTB 78C	Queen's Hill Fanling – Kai Tak	AM, PM Peak			
CTB 78P	Queen's Hill Fanling – Kwun Tong	AM Peak			
CTB 78X	Queen's Hill Fanling – Kai Tak	30 - 60			
CTB 796S	Tseung Kwan O Station – Ngau Tau Kok Station (Circular)	Overnight			
CTB 7903	Lohas Park – Kowloon Bay (Circular)	15 – 20			
CTB A22	Lam Tin Station – Airport	15 – 20			
CTB A22 CTB A29	Tseung Kwan O (Po Lam) – Airport / HZMB Hong Kong Port	20 - 60			
CTB E22	Lam Tin (North) – AsiaWorld-Expo	8 - 20			
CTB E22A	Tseung Kwan O (Hong Sing Garden) – AsiaWorld-Expo	25 – 30			
CTB E22C	Tiu Keng Leng Station – Aircraft Maintenance Area	AM, PM Peak			
CTB E22S	Tung Chung (Mun Tung Estate) – Tseung Kwan O (Po Lam)	AM, PM Peak			
CTB E22X	Yau Tong – AsiaWorld-Expo	AM, PM Peak			
CTB N29	Tseung Kwan O (Hong Sing Garden) – Tung Chung Station	Overnight			
CTB NA29	Tseung Kwan O (Po Lam) – Airport / HZMB Hong Kong Port	Overnight			
GMB 22A	Lok Wah Estate – Cheung Yip Street / Kwun Tong Ferry Pier	20			
	(Circular)				
GMB 35	Choi Ha Estate – Hong Lee Court	5 – 7			
GMB 36A	Crocodile Hill (Hong Lee Court) To Yue Man Square Public	4 – 5			
	Transport Interchange (Circular)				
GMB 56	Richland Gardens – Kwun Tong (Shung Yan St)	10 20			
GMB 62S	Kwong Tin Estate – Tsim Sha Tsui (Haiphong Road)	Overnight			
GMB 68	Choi Wan Estate – Kowloon Bay (Enterprise Square)	8 12			
GMB 86	Kai Tak Cruise Terminal – Telford Gardens	8 20			
GMB 90A	Yau Lai Estate – HK Children's Hospital	20 15 – 20			
GMB 90B	Sau Mau Ping Estate Phase 5 – HK Children's Hospital				
GMB 102	2 Hang Hau Station – San Po Kong (Hong Keung Street)				
GMB 102B	Hang Hau (Yuk Ming Court) – Choi Hung	12 – 20			
GMB 102S	Hang Hau Station – San Po Kong (Hong Keung Street)	Overnight			
GMB 104	The HK University of Science and Technology – Ngau Tau	12 – 25			
	Kok Station				
GMB 106	Tseung Kwan O (Po Lam) – Kowloon Bay (Enterprise Square)	7 – 25			
GMB 501S	Sheung Shui Station – Kwun Tong (Yue Man Square)	Overnight			
Note: KMB – Kowloon Motor Bus CTB – City Bus GMB – Green Minibus					

 Note:
 KMB – Kowloon Motor Bus
 CTB – City Bus
 GMB – Green Minibus

#### 3.0 THE PROPOSED DEVELOPMENT

#### Development Parameters

- 3.1 The Proposed Development has a RCHE with: (i) no less than 302, but not more than 557 beds ("RCHE within the Proposed Development"), and (ii) a Hotel with 200 guest rooms ("Hotel within the Proposed Development").
- 3.2 The internal transport facilities and traffic assessment below assume that the RCHE within the Proposed Development has 557 beds, and the Hotel within the Proposed Development has 200 guest rooms.

#### Provision of Internal Transport Facilities

#### (a) RCHE within the Proposed Development

- 3.3 The HKPSG has no recommendation on the provision of internal transport facilities for RCHE, hence, the provision for the RCHE within the Proposed Development, is provided based on the operational needs and also with reference to similar type RCHE in Kwun Tong.
- 3.4 Provision of internal transport facilities for RCHE within the Proposed Development are shown in Table 3.1.

WITHIN THE PROPOSED DEVELOPMENT				
Item	Proposed Provision			
Car Parking	8 nos. car parking spaces provided based on operational needs:			
Space	(i) 5 parking spaces @ 5m (L) x 2.5m (W) x 2.4m (H) for senior			
management staff of RCHE;				
	(ii) 3 parking spaces for RCHE visitors, including			
	- 2 nos. @ 5m (L) x 2.5m (W) x 2.4m (H) ; and			
	- 1 no accessible car parking space @ 5m (L) x 3.5m (W) x 2.4m (H)			
Motorcycle <u>2 nos.</u> motorcycle parking spaces @ 2.4m (L) x 1.0m (W) x 2.4m (H)				
Parking Space	provided			
Loading /	1 no. Heavy Goods Vehicles loading / unloading bay @ 11.0m (L) x 3.5m			
Unloading Bay (W) x 4.7m (H) are provided for shared use, i.e., for RCHE and Hotel				
Ambulance lay-by	1 no. ambulance lay-by @ 9.0m (L) x 3.0m (W) x 3.6m (H) shared use by			
	ambulance and mini-coach is provided based on the operational needs.			

TABLE 3.1PROVISION OF INTERNAL TRANSPORT FACILITIES FOR RCHEWITHIN THE PROPOSED DEVELOPMENT

- 3.5 Table 3.1 shows the provision of 8 car parking spaces, 2 motorcycle parking spaces and 1 ambulance lay-by shared use by ambulance and mini-coach. In addition, 1 HGV loading/unloading bay is also provided which is for shared used with the Hotel within the Proposed Development.
- 3.6 Most RCHEs in Hong Kong are located within buildings where there are other uses, and access to the RCHE is shared with other uses. Therefore, it is not possible to distinguish traffic generated by the RCHE from other uses for these type of RCHEs, i.e., those located within in a multi-use building. Nevertheless, several RCHEs located in a single use building were identified for the conduct of traffic surveys, and the surveyed RCHEs have similar characteristic as the Proposed Development, in terms of: (i) location; (ii) scale; (iii) accessibility to Public Transport Services, and (iv) availability of internal transport facilities.

3.7 The utilisation surveys were conducted from 0800 – 1959 hours on a weekday. Details of the surveyed RCHE are given in Table 3.2, and the survey results are presented in Table 3.3.

Location of Elderly Home	No. of Beds	Accessibility to Public Transport Services	
(A) 8 Kung Lok Road, Kwun Tong	Lok Road, convenient with numerous bus and GMB routes operate		Yes
Lok Road, conve Kwun Tong in the		Access to public transport services from this RCHE is convenient with numerous bus and GMB routes operate in the vicinity. The nearest MTR Ngau Tau Kok Station is located within 500m from this elderly home.	Yes

#### TABLE 3.3 SURVEY RESULTS OF THE 2 SURVEYED RCHES

Time Period	Maximum Number of Vehicles Observed at any time				
(hours)			Medium /	Mini coach	Ambulance
· · ·	and taxi	vehicle <sup>(1)</sup>	heavy goods vehicle		
			<i>y</i> <u>y</u>	1	1
(A) 8 Kung Lok	Road, Kwun To	ng (266 beds)			
0800 – 0859	1	0	0	0	0
0900 – 0959	0	1	0	0	0
1000 – 1059	0	0	0	1	0
1100 – 1159	0	0	0	0	0
1200 – 1259	0	1	0	0	0
1300 – 1359	0	1	0	0	0
1400 – 1459	0	0	0	1	0
1500 – 1559	0	0	0	0	1
1600 – 1659	0	0	0	1	0
1700 – 1759	0	0	0	1	0
1800 – 1859	1	0	0	0	0
1900 – 1959	0	0	0	0	0
Maximum	<u>1</u>	<u>1</u>	<u>0</u>	<u>1</u>	<u>1</u>
(0800 – 1959)					
(B) 88 Kung Lok Road, Kwun Tong (226 beds)					
0800 - 0859	0	0	0	0	0
0900 - 0959	0	1	0	0	0
1000 – 1059	0	0	0	1	0
1100 – 1159	0	0	0	1	0
1200 – 1259	0	1	0	0	0
1300 – 1359	0	0	0	1	0
1400 – 1459	1	0	0	0	0
1500 – 1559	1	0	0	0	0
1600 – 1659	1	0	0	0	0
1700 – 1759	1	0	0	0	0
1800 – 1859	0	0	0	0	0
1900 – 1959	0	0	0	0	0
Maximum (0800 – 1959)	<u>1</u>	<u>1</u>	<u>0</u>	<u>1</u>	<u>0</u>

Note: <sup>(1)</sup> including goods van, light goods vehicle

3.8 Table 3.3 shows the number of vehicles observed within the same hour but these vehicles are not present at the same time. For example, at Location (A) 8

Kung Lok Road, Kwun Tong, during the period 0800 – 0859 hours, the Private car and taxi were not observed at the same time as the Light goods vehicle.

3.9 Table 3.3 also shows the following:

#### (ai) Private car parking spaces

- 3.10 The maximum number of private car and taxi observed at both surveyed RCHEs at the same time was 1. Based on this rate, the RCHE within the Proposed Development is estimated to generate a maximum of 3 vehicles at the same time only. [Calculation: 1 vehicle / 226 beds x 557 beds = 2.46, say, 3]
- 3.11 Taxis stop momentarily to pick-up and drop-off. Therefore, taxis can use the private car parking spaces PC-09 or PC-10 on G/F as shown in Figure 3.1.
- 3.12 Hence, the provision of 8 car parking spaces is more than sufficient to serve the RCHE within the Proposed Development.

#### (aii) Goods Vehicle Loading / Unloading Bay

3.13 As shown in Table 3.3, no more than 1 goods vehicle was observed at any one time. Hence, the provision of 1 HGV loading/unloading bay for shared use by RCHE and Hotel is sufficient to serve the RCHE within the Proposed Development.

#### (aiii) Layby for shared use by ambulance and mini-coach

3.14 As shown in Table 3.3, no ambulance and mini-coach arrived at the same time. Hence, 1 ambulance layby which is for shared use with mini-coach is sufficient to serve the RCHE within the Proposed Development.

#### (b) Hotel within the Proposed Development

3.15 The internal transport facilities for Hotel within the Proposed Development are provided in accordance to the recommendations of the HKPSG, and are presented in Table 3.4.

# TABLE 3.4COMPARISON OF THE HKPSG RECOMMENDATIONS AND<br/>PROPOSED INTERNAL TRANSPORT FACILITIES FOR HOTEL<br/>WITHIN THE PROPOSED DEVELOPMENT

HKPSG Recommendation for a Hotel with 200 guest rooms	Proposed Provision
Car Parking Space	
1 car parking space per 100 rooms. 200/100 = 2 nos.	$\frac{2 \text{ nos. } @ 5m (L) \times 2.5m (W) \times 2.4m}{(H) = HKPSG recommendation}$
Motorcycle Parking Space	
5 to 10% of the total provision for private cars Minimum = $2 \times 5\% = 0.1$ , say 1 no. Maximum = $2 \times 10\% = 0.2$ , say 1 no.	$\frac{1 \text{ no. } @ 2.4 \text{m (L) x 1m (W) x 2.4m (H)}}{= \text{HKPSG recommendation}}$
Taxi and Private Car Layby	
Minimum 2 lay-by for taxi and private cars for $\leq$ 299 rooms $=$ 2 nos.	$\frac{2 \text{ nos. } @ 5m (L) x 2.5m (W) x 2.4m (H)}{= HKPSG recommendation}$
Single-Deck Tour Bus Layby	

HKPSG Recommendation for a Hotel with 200 guest rooms	Proposed Provision
Minimum 1 lay-by for single-deck tour buses for $\leq$ 299	<u>1 no.</u> @ 12m (L) x 3.5m (W) x 3.8m
rooms <u>= 1 no.</u>	(H) <u>= HKPSG recommendation</u>
Goods Vehicle Loading / Unloading Bay	
0.5 - 1 goods vehicle bay per 100 rooms	<u>1 no.</u> @ 7m (L) x 3.5m (W) x 3.6m (H)
Minimum = $200 / 100 \times 0.5 = 1 \text{ no.}$	for Light Goods Vehicles
Maximum = $200 / 100 \times 1 = 2$ nos.	= HKPSG recommendation

3.16 For ease of reference, the internal transport facilities for the Proposed Development presented in Tables 3.1 and 3.4, are summarised in Table 3.5.

### TABLE 3.5SUMMARY OF INTERNAL TRANSPORT FACILITIES PROVIDED<br/>FOR THE PROPOSED DEVELOPMENT

Item	Use	Proposed Provision
Car Parking Space	RCHE	8
	Hotel	2
	Total	<u>10</u>
Ambulance Parking Space	RCHE	1
Motorcycle Parking Space	Hotel	1
	RCHE	2
	Total	3
Taxi and Private Car Layby	Hotel	2
Single-Deck Tour Bus Layby	Hotel	1
LGV Goods Vehicle Loading / Unloading Bay	Hotel	1
HGV Goods Vehicle Loading / Unloading	Shared use by RCHE and	1
Вау	Hotel	
	Total	2

Reasons for Deviation from the HKPSG Maximum Recommendation for Hotel within the Proposed Development

#### (a) Site Constraint

- 3.17 The only internal transport facility for the Hotel within the Proposed Development, which deviates from the HKPSG maximum recommendation is the provision of 1 goods vehicles loading / unloading bay, instead of 2. However, a second goods vehicle loading / unloading bay is provided, which is for shared use with the RCHE within the Proposed Development.
- 3.18 The provision of an additional goods vehicle loading / unloading bay on the ground floor was considered, but not found to be possible due site constraint, and is explained as follows:
  - The Outline Development Plan no. D/K14A/1H require setback along Wai Yip Street of 2.3m, and (ii) 1.5m setback and 1.5m non-building area along the service lane and;
  - (2) With the above setback requirements, the length of the subject site (i.e. measured from Wai Yip Street to the service lane) which is only 21.3m is further reduced to only 17.5m (reduction of length of 17.8%, which is substantial).

3.19 After accommodating the essential facilities such as, structural columns, staircases, escalators, lift lobby and vehicle ramp to the basement car park, etc, the provision of another goods vehicle loading / unloading bay is not possible. The Authorised Person has used his utmost effort to ensure the layout is arranged and utilised in good order.

#### (b) Limited Goods Vehicles Generated

3.20 Goods vehicles generated are mostly related to room cleaning services, and the deliveries of toiletry and beverages. The expected goods vehicle trip generated for the Hotel within the Proposed Development is summarised in Table 3.6.

### TABLE 3.6GOODS VEHICLE TRIP GENERATION FOR HOTEL WITHIN THE<br/>PROPOSED DEVELOPMENT

Item	Activity	Expected goods vehicles generated
Room cleaning service	Replenish cleaning material	4 trips per month
Toiletry	Restock toiletries, eg, shampoo,	1 trip per month
	lotion, etc.	
Beverages	Deliver distilled water	8 trips per month
	Total goods vehicle trips =	13 trips per month

3.21 Table 3.6 shows that the Hotel within the Proposed Development is expected to generate 13 goods vehicle trips per month, or 1 vehicle trip every 2.3 days, which is low. Hence, the provision of 1 LGV goods vehicle loading/unloading bay and 1 HGV loading/unloading bay which is for shared use by RCHE and Hotel, is sufficient to serve the loading / unloading activities of the Hotel.

#### Layout Plans

- 3.22 The carpark layout plans for G/F and B1/F are found in Figures 3.1 3.2. Similar to the Approved S16 Planning Application (TPB ref: A/K14/809), two vehicular access points are provided for the Proposed Development, and these are located at:
  - (i) The service lane at the northern side of the Proposed Development
  - (ii) Wai Yip Street

#### Swept Path Analysis

3.23 The CAD-based swept path analysis program, Autodesk Vehicle Tracking, was used to check the ease of vehicle manoeuvring, and the swept path drawings of vehicle manoeuvring on the parking levels are found in in Appendix 2. Vehicles are found to have no manoeuvring problems and all vehicles could enter and leave the spaces with ease.

#### Traffic Management Plan

- 3.24 Loading / unloading related to goods deliveries will be undertaken during the non-peak hours. The Management Office will ensure good maintenance of the turntable and should there the turntable fail to operate, the Management Office will immediately contact the turntable maintenance company to repair.
- 3.25 If necessary, the Management Office will stagger the delivery of goods so that only 1 goods vehicle will be present at the same time.

#### 4.0 TRAFFIC IMPACT

Design Year

4.1 The Proposed Development is expected to be completed by 2029, and the design year adopted for the capacity analysis is 2032, i.e. 3 years after the completion of the development.

#### Traffic Forecast

4.2 The 2032 traffic flows used for the junction analysis are produced with reference to the (i) 2031 traffic flows from the Base District Traffic Model ("BDTM"); (ii) estimated traffic growth from 2031 to 2032; (iii) the planned developments in the vicinity of the Proposed Development, and (iv) additional traffic generated by the Proposed Development.

#### Estimated Traffic Growth Rate from 2031 to 2032

4.3 Reference is made to the 2019 – based Territorial Population and Employment Data Matrix ("TPEDM") data produced by Planning Department for Kwun Tong District, which are for 2019, 2026 and 2031 and are presented in Table 4.1.

TABLE 4.1	2019-BA	SED TPEDI	M DATA	PRODUCED	ΒY	PLANNING
	DEPARTI	MENT FOR <b>k</b>	WUN TO	NG DISTRICT		

Item	TPEDM Estimation / Projection			
	2019 2026			
Population	693,900	769,400	741,300	
Employment	395,350	410,550	408,250	
Total	<u>1,089,250</u>	<u>1,179,950</u>	1,149,550	
Average Growth%	From 2019 to 2026: +1.15% From 2019 to 2031: +0.45%	From 2026 to 2031: -0.52%	N/A	

4.4 Table 4.1 shows that the highest average annual growth rate is 1.15%. In view that there is no estimation beyond 2031 and to err on the high side, the growth rate of 1.15% per annum is adopted for the traffic growth between 2031 and 2032.

Planned Developments in the Vicinity of the Proposed Development

4.5 The planned developments included in the 2032 reference traffic flows are presented in Table 4.2, and the locations of planned developments are shown in Figure 4.1.

### TABLE 4.2PLANNEDDEVELOPMENTSINTHEVICINITYOFTHEPROPOSEDDEVELOPMENT

Site	Planning Application No. / Plan No.	Address	Use	Development Parameters (Approx.)
1	A/K14/763	350 Kwun Tong Road	Commercial	GFA=25,658m <sup>2</sup>
2	A/K14/766	41 King Yip Street	Commercial	GFA=30,576m <sup>2</sup>
3	A/K14/771	32 Hung To Road	Commercial	GFA=13,122m <sup>2</sup>
4	A/K14/773	82 Hung To Road	Industrial	GFA=13,378m <sup>2</sup>
5	A/K14/774	7 Lai Yip Street	Commercial	GFA=14,775m <sup>2</sup>
6	A/K14/775	132 Wai Yip Street	Commercial	GFA=6,021m <sup>2</sup>
7	A/K14/777	71 How Ming Street	Office	GFA=18,312m <sup>2</sup>
8	A/K14/778	203 Wai Yip Street	Industrial	GFA=13,479m <sup>2</sup>

Site	Planning Application No. / Plan No.	Address	Use	Development Parameters (Approx.)
9	A/K14/782	4 Tai Yip Street	Retail	GFA=8,027m <sup>2</sup>
10	A/K14/787	33 Hung To Road	Industrial	GFA=13,830m <sup>2</sup>
11	A/K14/796	28A Hung To Road	Hotel	No. of rooms=89
12	A/K14/804	334 -336 and 338 Kwun Tong Road	Commercial	GFA=23,211m <sup>2</sup>
13	A/K14/806	11 Lai Yip Street	Office	GFA=15,051m <sup>2</sup>
14	A/K14/807	Kun Tong Inland Lots 1 S.A , 1 RP, 3 and 15	Commercial	GFA=66,890m <sup>2</sup>
15	A/K14/808	201 Wai Yip Street	Commercial	GFA=13,478m <sup>2</sup>
16	A/K14/809	1 Tai Yip Street and 111 Wai Yip Street	Commercial	GFA=13,349m <sup>2</sup>
17	A/K14/810	5 Lai Yip Street	Commercial	GFA=14,788m <sup>2</sup>
18	A/K14/820	73 – 75 Hung To Road	Commercial	GFA=26,757m <sup>2</sup>
19	A/K14/822	25 Tai Yip Street, Kwun Tong	Commercial	$GFA = 5,572m^2$
20	A/K14/819 & S/K14S/URA1/3 Urban Renewal Authority's (URA) latest 'Vertical City' scheme of a mixed use development	Areas 4 and 5 of Kwun Tong Town Centre		GFA = 65,000m <sup>2</sup> , 127,619m <sup>2</sup> and 601m <sup>2</sup>
21	N/A	EKEO Lai Yip Street Development	Commercial	GFA=23,000m <sup>2</sup>
22	N/A	Kwun Tong Action Area	Commercial	GFA=89,350m <sup>2</sup>
23	N/A	Kowloon Bay Action Area	Commercial	GFA=500,000m <sup>2</sup>

- 4.6 The infrastructure and road network included in the BDTM are as follows:
  - Kai Tak Development
  - Tseung Kwan O Lam Tin Tunnel
  - Central Kowloon Route
  - Trunk Road T2 between Central Kowloon Route and Tseung Kwan O Lam Tin Tunnel

#### Traffic Generated by the Proposed Development

- 4.7 In view that the TPDM does not provide trip generation rates for RCHE, reference is made to the traffic generation of similar elderly homes, and the surveyed RCHE are found in Table 3.2.
- 4.8 As for Hotel, reference is also made to surveyed hotels which are of similar class, number of hotel rooms and traffic characteristics, i.e. proximity to the MTR and road-based public transport services. The surveyed hotels are:
  - (i) 254-room Nina Hotel Kowloon East at 38 Chong Yip St, Kwun Tong
  - (ii) 298-room Tuen Mun Pentahotel at 6 Tsun Wen Road, Tuen Mun
- 4.9 The surveyed hotel trip generation rates are found to be lower than the lower limit of rates for Hotel found in the TPDM. Hence, to be conservative, the lower limit of trip generation rates taken from TPDM is adopted to estimate the traffic generation associated to the Hotel within the Proposed Development. The adopted trip generation rates and the calculated traffic generation associated with the Proposed Development are presented in Table 4.3.

#### TABLE 4.3TRAFFIC GENERATION OF THE PROPOSED DEVELOPMENT

Item	Item AM Peak Hour		PM Peak Hour			
	In	Out	2-way	In	Out	2-way
Trip Generation Rates for RCHE (pcu/hour/bed)						
RCHE	0.0155	0.0155	NA	0.0133	0.0133	NA
Trip Generation Rates for hotel (pcu/hour,	5	· /				
Hotel <sup>(1)</sup>	0.0832	0.0843	NA	0.0908	0.0883	NA
Traffic Generation of Proposed Development (pcu/hour)						
RCHE: 557 beds [a]	9	9	18	8	8	16
Hotel: 200 guest rooms [b]	17	17	34	19	18	37
Total [a] + [b]	<u>26</u>	<u>26</u>	<u>52</u>	27	26	53

Note: <sup>(1)</sup> lower limit of rates taken from TPDM

4.10 Table 4.3 shows the Proposed Development generates 52 and 53 more pcu (2way) during the AM and PM peak hours respectively.

Comparison of Traffic Generation between the Approved S16 Scheme (TPB ref: A/K14/780) and the Proposed Development

- 4.11 The traffic generated by the Approved S16 Scheme (TPB ref: A/K14/780) is compared with the Proposed Development and is presented in Table 4.4.
  - TABLE 4.4COMPARISONOFTRAFFICGENERATIONBETWEENTHEAPPROVEDS16SCHEME(TPBREF:A/K14/780)ANDTHEPROPOSEDDEVELOPMENT

	-	T (C)	0	. ,	4	<b>`</b>
Scheme		Iraffic	Generat	tion (po	cu/hou	r)
		AM Peak Hour PM Peak Ho			Hour	
	In	Out	2-way	In	Out	2-way
Approved S16 Scheme (TPB ref: A/K14/780) [A]	42	30	72	21	28	49
Proposed Development [B]	26	26	52	27	26	53
Difference [B] – [A]	<u>-16</u>	-4	-20	+6	-2	+4

4.12 Table 4.4 shows that compared with the Approved S16 Scheme (TPB ref: A/K14/780), the Proposed Development generates 20 pcu (2-way) less and 4 pcu more during the AM and PM peak hours respectively. It can be concluded from traffic generation aspect the Proposed Development is a better-off scheme compared to the Approved S16 Scheme (TPB ref: A/K14/780).

Planned Junction Improvement Schemes

4.13 The planned junction improvement schemes found in the vicinity of the Subject Site are summarized in Table 4.5 and shown in Appendix 3.

TABLE 4.5	PLANNED TRAFFIC IMPROVEMENT SCHEMES IN THE VICINITY
	OF THE PROPOSED DEVELOPMENT

Junction Description of Work			Project Proponent	Estimated Completion Year
J1			Kowloon Bay	Before 2032
	Shun Yip Street	changed at Shun Yip Street	Action Area -	
		Westbound and Eastbound	Feasibility Study	
J7	Wai Yip Street	The road alignment is adjusted	Kowloon Bay	

Junction Description		Description of Work	Project Proponent	Estimated Completion Year
	/ Lai Yip Street	at Lai Yip Street Northbound	Action Area –	
			Feasibility Study	
J8	Kwun Tong		Kwun Tong Action	
	Road / Lai Yip	at Lai Yip Street Northbound	Area – Feasibility	
	Street	reet		
J9	Hoi Bun Road	A new pedestrian crossing	Technical study on	
	/ Lai Yip Street	across Hoi Bun Road	the Lai Yip Street	
		Eastbound is added and	site in Kowloon	
		existing staggered pedestrian	East	
		crossing at Lai Yip Street to be		
		converted to straight crossing		

#### 2032 Traffic Flows

4.14 Year 2032 traffic flows for the following cases are derived:

2032 without the Proposed Development [A]	= 2031 traffic flows derived with reference to BDTM + estimated total growth from 2031 to 2032 + Traffic generated by the planned developments in the vicinity of the Proposed Development
2032 with the Proposed Development [B]	= [A] + traffic generated by the Proposed Development (Table 4.3)

4.15 The 2032 peak hour traffic flows for the cases without and with the Proposed Development, are shown in Figures 4.2 - 4.3, respectively. The ingress/egress vehicular routings to/from the Proposed Development via Wai Yip Street and the service lane at the northern side of the Proposed Development are shown in Figures 4.4 - 4.5.

#### 2032 Junction Operational Performance

4.16 Year 2032 capacity analysis for the cases without and with the Proposed Development are summarized in Table 4.6 and detailed calculations are found in the Appendix 1.

Ref.	Junction	Type of Junction / Parameter <sup>(1)</sup>	Prop	out the osed opment	Prop	n the bosed opment
			AM	PM	AM	PM
			Peak	Peak	Peak	Peak
J1	Hoi Bun Road / Shun Yip Street <sup>(3)</sup>	Signal / RC	22%	17%	22%	17%
J2	Wai Yip Street / Shun Yip Street	Signal / RC	21%	19%	20%	18%
J3	Tai Yip Street / Service Lane	Priority / RFC	0.044	0.036	0.057	0.048
J4	Hong Tak Road / Tai Yip Street	Priority / RFC	0.384	0.294	0.414	0.329
J5	Tai Yip Street / Tai Yip Lane	Priority / RFC	0.135	0.117	0.136	0.117
J6	Kwun Tong Road / Hong Tak Road	Priority / RFC	0.655	0.743	0.678	0.771
J7	Wai Yip Street / Lai Yip Street <sup>(3)</sup>	Signal / RC	26%	35%	26%	35%
J8	Kwun Tong Road / Lai Yip Street <sup>(3)</sup>	Signal / RC	23%	18%	23%	18%
J9	Hoi Bun Road / Lai Yip Street <sup>(3)</sup>	Signal / RC	21%	23%	21%	23%
J10	Lai Yip Street / Hung To Road <sup>(2)</sup>	Signal / RC	33%	41%	33%	41%
Notes:	<sup>(1)</sup> RC – reserve capacity RFC – Rati	o of Flow to Capacit	ty			

TABLE 4.62032 JUNCTION OPERATIONAL PERFORMANCE

<sup>(2)</sup> Kerbside on-street activities are reflected in the junction performance
 <sup>(3)</sup> Junction Improvement Scheme has been incorporated in the assessment

4.17 Table 4.6 shows that the junctions operate with capacities during the AM and PM peak hours for the cases without and with the Proposed Development.

#### 5.0 PEDESTRIAN ASSESSMENT

#### Surveyed Pedestrian Locations

5.1 In order to quantify the existing pedestrian flows, pedestrian counts were conducted at the footpaths and waiting area of the pedestrian crossing shown in Figure 5.1 during the AM and PM peak periods. The survey locations are summarized in Table 5.1.

#### TABLE 5.1 SURVEYED PEDESTRIAN LOCATIONS

Ref.	Location				
	Footpath				
1	Northern footpath of Wai Yip Street between Shun Yip Lane and Tai Yip Street (Eastern side)				
2	Northern footpath of Wai Yip Street between Shun Yip Lane and Tai Yip Street (Western side)				
3	Shun Yip Lane between Wai Yip Street and Service Lane				
	Waiting area of pedestrian crossing				
W1	Western pedestrian crossing of Wai Yip Street / Shun Yip Street				
W2	Eastern pedestrian crossing of Wai Yip Street / Shun Yip Street				

Existing Pedestrian Flows

5.2 The existing peak 15-minute 2-way pedestrian flows are also presented in Figure 5.1.

#### Estimated growth from 2024 to 2032

5.3 The 2032 reference pedestrian flows are estimated with the reference of the existing pedestrian flows and a growth rate of 1.15% per annum, which is derived from the latest TPEDM data.

#### Pedestrian Generated by the Proposed Development

5.4 The pedestrian generations associated with the RCHE and Hotel within the Proposed Development, are estimated based on in-house pedestrian rates. The in-house pedestrian rates are presented in Table 5.2, and the estimated pedestrian generation of Proposed Development is found in Table 5.3.

Use	Pedestrian Generation Rates (pedestrian / 15 min / 100m <sup>2</sup> )					
	AM	Peak	PM Peak			
	In	Out	In	Out		
RCHE <sup>(1)</sup>	0.049	0.004	0.011	0.034		
Hotel <sup>(2)</sup>	0.053	0.173	0.156	0.177		

#### TABLE 5.2IN-HOUSE PEDESTRIAN GENERATION RATES

<sup>(1)</sup> 266-bed RCHE known as Buddhist Sum Ma Shui Ying Care & Attention Home for the Elderly at 8 Kung Lok Road, Kwun Tong

(2) 254-room Nina Hotel Kowloon East at 38 Chong Yip St, Kwun Tong

ABLE 5.3	PEDESTRIA	PEDESTRIAN GENERATED BY THE PROPOSED DEVELOPMENT					
Use	GFA (m²)	Pedestrian Generation (pedestrian / 15 min)					
		AM Peak PM Peak			Peak		
		In	Out	In	Out		
RCHE	557 beds	28	3	7	19		
Hotel	200 rooms	11	35	32	36		
	Total	<u>39</u>	38	39	55		

#### Year 2032 Pedestrian Flows

5.5 The 2032 pedestrian flow with and without the Proposed Development are derived using the following method:

Without the	<ul> <li>= 2024 observed pedestrian flows + growth from 2024</li></ul>
Proposed	to 2032 + pedestrian generated by the planned
Development [a]	developments in the vicinity of the Subject Site
With the Proposed	[a] + pedestrian generated by the Proposed
Development [b]	= Development (Table 5.3)

5.6 The 2032 pedestrian flows without and with the Proposed Development are presented in Figures 5.2 and 5.3.

Level-Of-Service ("LOS") Assessment

5.7 The pedestrian assessment method adopted is referenced to Exhibit 18-3 of Chapter 18 of the Highway Capacity Manual ("HCM") 2000 and the extract of Exhibit 18-3 is summarised in Table 5.4.

LOS	Space (m²/p)	Flow Rate (p/min/m)
A	> 5.6	<u>≤ 16</u>
В	> 3.7-5.6	> 16-23
С	> 2.2-3.7	> 23-33
D	> 1.4-2.2	> 33-49
E	> 0.75-1.4	> 49-75
F	≤ 0.75	variable

#### TABLE 5.4EXTRACT OF EXHIBIT 18-3 OF THE HCM 2000

#### (a) LOS of the Footpaths

5.8 The effective width of the surveyed footpaths and the year 2032 LOS without and with the Proposed Development are presented in Tables 5.5 and 5.6.

#### TABLE 5.5EFFECTIVE WIDTH OF SURVEYED FOOTPATHS

Ref	Footpath width (m)	Effective width (m) <sup>(1)</sup>
1	3.5	2.5
2	2.7	1.7
3	9.8	8.8

Note:<sup>(1)</sup> The effective width does not include 0.5m dead zone on both sides, i.e. 1m

### TABLE 5.6YEAR 2032 LOS OF FOOTPATH WITHOUT AND WITH THE<br/>PROPOSED DEVELOPMENT

Ref.	Peak Period	Year 2032 without the Proposed Development		Year 2032 with the Proposed Development			
	Periou	Flow	Rate <sup>(1)</sup>	LOS	Flow	Rate <sup>(1)</sup>	LOS
		(Ped/15 min)	(Ped/min/m)	L03	(Ped/15 min)	(Ped/ min/m)	LOJ
1	AM	350	9.3	Α	369	9.8	Α
	PM	317	8.5	Α	340	9.1	Α
2	AM	467	18.3	В	516	20.2	В
	PM	336	13.2	Α	395	15.5	Α
3	AM	969	7.3	Α	1008	7.6	Α
	PM	593	4.5	Α	640	4.8	Α

Note: <sup>(1)</sup> pedestrian flow rate = pedestrian flow  $\div$  15 minutes  $\div$  effective width

5.9 Table 5.6 shows that the footpaths achieve LOS A and B during AM and PM peak for the 2032 cases without and with the Proposed Development.

(b) Waiting area of the Pedestrian Crossing

5.10 The year 2032 LOS of pedestrian crossing waiting areas without and with the Proposed Development are presented in Table 5.7.

## TABLE 5.7YEAR 2032 LOS OF PEDESTRIAN CROSSING WAITING AREAS<br/>WITHOUT AND WITH THE PROPOSED DEVELOPMENT

Ref	Area (m²)		of Pedestrians at the ea (ped/signal cycle)	Pedestrian Space (m <sup>2</sup> /ped)		LC	DS		
		AM	PM	AM	PM	AM	PM		
	Without the Proposed Development								
W1	150	47	11	3.2	13.6	С	А		
W2	63	24	24 4		15.8	С	А		
	With the Proposed Development								
W1	150	48	12	3.1	12.5	С	А		
W2	63	25	5	2.5	12.6	С	A		

- 5.11 Table 5.7 shows that the pedestrian crossing waiting areas achieve LOS A and C during AM and PM peak for the 2032 cases without and with the Proposed Development.
- 5.12 It is noted that "In general, LOS C is desirable for most design at streets with dominant 'living' pedestrian activities". Since the LOS in Tables 5.6 and 5.7 are A to C, it can be concluded that the Proposed Development will have no adverse impact to the footpaths and pedestrian crossing waiting areas in the vicinity.

#### 6.0 SENSITIVITY TEST

#### Permitted Maximum Number of Beds for RCHE

- 6.1 Although the proposed maximum number of beds for RCHE is 557, based on the RCHE GFA and the minimum area of floor space per resident as per Code of Practice for Residential Care Homes (Elderly Persons) issued by Social Welfare Department, a total of 644 beds could be provided. Hence, a sensitivity test is undertaken for the RCHE with 644 beds and the Hotel with 200 rooms.
- 6.2 As stated in paragraphs 3.17 3.18, due to site constraints, the Authorised Person has used his utmost effort to ensure the layout is arranged and utilised in good order. Internal transport facilities will remain unchanged as the Proposed Development.

#### Sensitivity Test on Traffic Impact

(a) Comparison of Traffic Generation

6.3 The comparison of traffic generated by the Proposed Development, and the sensitivity test with 644-bed RCHE and 200-room Hotel, is presented in Table 6.1.

Item	А	M Peak Hou	Jr	PM Peak Hour			
	In	Out	2-way	In	Out	2-way	
Proposed Development							
RCHE: 557 beds	9	9	18	8	8	16	
Hotel: 200 guest rooms	17	17	34	19	18	37	
Total [A]	26	26	52	27	26	53	
Sensitivity Test							
RCHE: 644 beds	10	10	20	9	9	18	
Hotel: 200 guest rooms	17	17	34	19	18	37	
Total [B]	27	27	54	28	27	55	
Difference in Traffic Ger	neration (pcu	u/hour)					
[B] – [A]	+1	+1	+2	+1	+1	+2	

TABLE 6.1COMPARISON OF TRAFFIC GENERATION

6.4 Table 6.1 shows that compared with the Proposed Development, the sensitivity test with 644-bed RCHE and 200-room Hotel, generates 2 pcu / hour (2-way) more in both AM and PM peak hours, which is negligible.

#### (b) 2032 Traffic Flows

6.5 The sensitivity test with 644-bed RCHE and 200-room Hotel 2032 peak hour traffic flows are shown in Figure 6.1. The ingress/egress vehicular routings to/from the Proposed Development via Wai Yip Street and the service lane at the northern side of the Proposed Development are shown in Figures 6.2 - 6.3.

#### (c) 2032 Junction Operational Performance

6.6 The comparison of junction capacity analysis for Proposed Development, and the sensitivity test with 644-bed RCHE and 200-room Hotel, is found in Table 6.2 and detailed calculations of the sensitivity test are found in the Appendix 1.

#### TABLE 6.2 OF 2032 JUNCTION **OPERATIONAL** COMPARISON PERFORMANCE

			· L		-			
Ref.	Junction	Type of Junction / Parameter <sup>(1)</sup>	Proposed Development [A]		Sensitivity Test [B]		Difference [B] – [A]	
			AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
J1	Hoi Bun Road / Shun Yip Street <sup>(3)</sup>	Signal / RC	22%	17%	22%	17%	0%	0%
J2	Wai Yip Street / Shun Yip Street	Signal / RC	20%	18%	20%	18%	0%	0%
J3	Tai Yip Street / Service Lane	Priority / RFC	0.057	0.048	0.057	0.048	0.000	0.000
J4	Hong Tak Road / Tai Yip Street	Priority / RFC	0.414	0.329	0.416	0.329	0.002	0.000
J5	Tai Yip Street / Tai Yip Lane	Priority / RFC	0.136	0.117	0.136	0.117	0.000	0.000
J6	Kwun Tong Road / Hong Tak Road	Priority / RFC	0.678	0.771	0.680	0.771	0.002	0.000
J7	Wai Yip Street / Lai Yip Street <sup>(3)</sup>	Signal / RC	26%	35%	26%	35%	0%	0%
J8	Kwun Tong Road / Lai Yip Street <sup>(3)</sup>	Signal / RC	23%	18%	23%	18%	0%	0%
J9	Hoi Bun Road / Lai Yip Street <sup>(3)</sup>	Signal / RC	21%	23%	21%	23%	0%	0%
J10 Notes:	Lai Yip Street / Hung To Road <sup>(2)</sup>	Signal / RC	33%	41% Flow to Ca	33%	41%	0%	0%

<sup>(1)</sup> RC – reserve capacity RFC – Ratio of Flow to Capacity Notes:

<sup>(2)</sup> Kerbside on-street activities are reflected in the junction performance <sup>(3)</sup> Junction Improvement Scheme has been incorporated in the assessment

6.7 Table 6.2 shows there is negligible difference in the junction capacity between the 2 schemes. Hence, the impact of the sensitivity test with a 644-bed RCHE and a 200-room Hotel, is negligible.

#### Sensitivity Test on Pedestrian Impact (a) Comparison of Pedestrian Generation

The comparison of pedestrian generated by the Proposed Development, and the 6.8 sensitivity test with 644-bed RCHE and 200-room Hotel, is presented in Table 6.3.

Item	AM Peak Hour			PM Peak Hour		
	In	Out	2-way	In	Out	2-way
Proposed Development						
RCHE: 557 beds	28	3	31	7	19	26
Hotel: 200 guest rooms	11	35	46	32	36	68
Total [A]	<u>39</u>	<u>38</u>	77	39	<u>55</u>	94
Sensitivity Test						
RCHE: 644 beds	32	3	35	8	22	30
Hotel: 200 guest rooms	11	35	46	32	36	68
Total [B]	43	<u>38</u>	<u>81</u>	40	<u>58</u>	<u>98</u>
Difference in Pedestrian Generation (pcu/hour)						
[B] – [A]	+ 4	+0	+ 4	+1	+3	+4

TABLE 6.3 COMPARISON OF PEDESTRIAN GENERATION

6.9 Table 6.3 shows the pedestrians generated by the sensitivity test, is 4 more (2way) in the AM and PM peak hours, compared to the Proposed Development, which is negligible.

(b) 2032 Pedestrian Flows

6.10 The sensitivity test 2032 pedestrian flows is presented in Figure 6.4.

(c) LOS of the Footpaths

6.11 The sensitivity test year 2032 LOS is presented in Table 6.4.

#### TABLE 6.4SENSITIVITY TEST FOR YEAR 2032 LOS OF FOOTPATH

Ref.	Peak Period	Year 2032 Sensitivity Test			
		Flow	Rate <sup>(1)</sup>	LOS	
		(Ped/15 min)	(Ped/ min/m)		
1	AM	370	9.9	А	
	PM	341	9.1	А	
2	AM	518	20.3	В	
	PM	398	15.6	А	
3	AM	1010	7.7	А	
	PM	642	4.9	А	

Note: <sup>(1)</sup> pedestrian flow rate = pedestrian flow  $\div$  15 minutes  $\div$  effective width

6.12 Table 6.4 shows that the footpaths still achieve LOS A and B during AM and PM peak.

(d) Waiting Area of the Pedestrian Crossing

6.13 Sensitivity test for the year 2032 pedestrian crossing waiting areas is presented in Table 6.5.

### TABLE 6.5SENSITIVITY TEST FOR YEAR 2032 PEDESTRIAN CROSSING<br/>WAITING AREAS

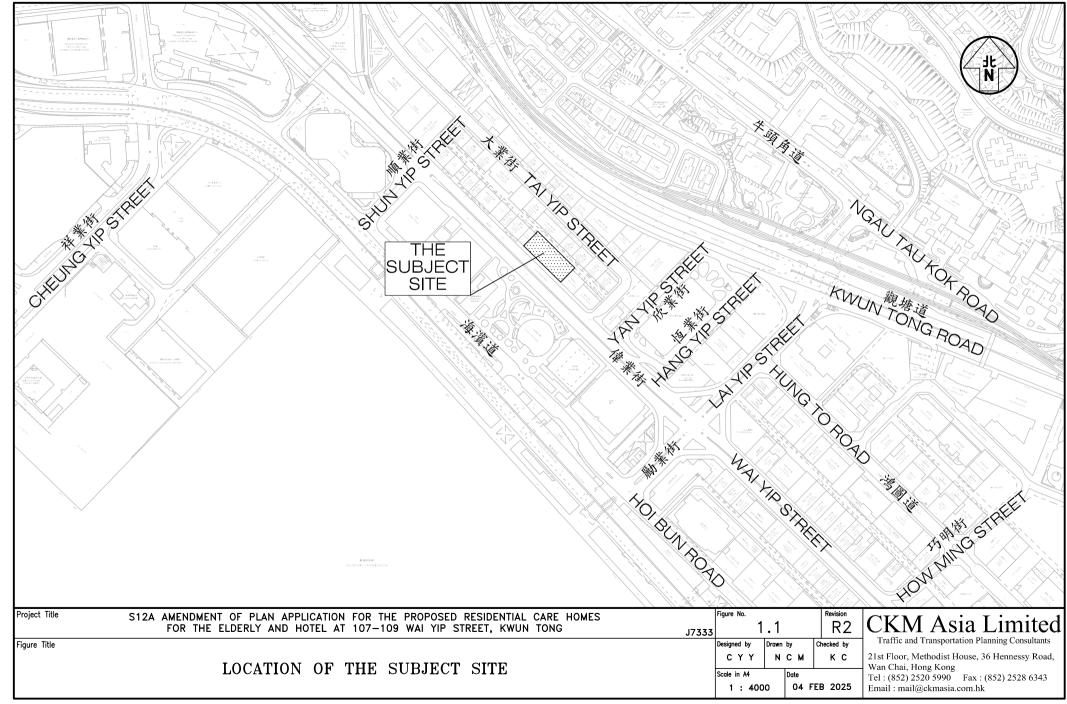
Ref	Area (m²)	0	rage No. of Pedestrians at the hiting area (ped/signal cycle)		Pedestrian Space (m <sup>2</sup> /ped)		
		AM	PM	AM	PM	AM	PM
W1	150	48	12	3.1	12.5	С	А
W2	63	25	5	2.5	12.6	С	А

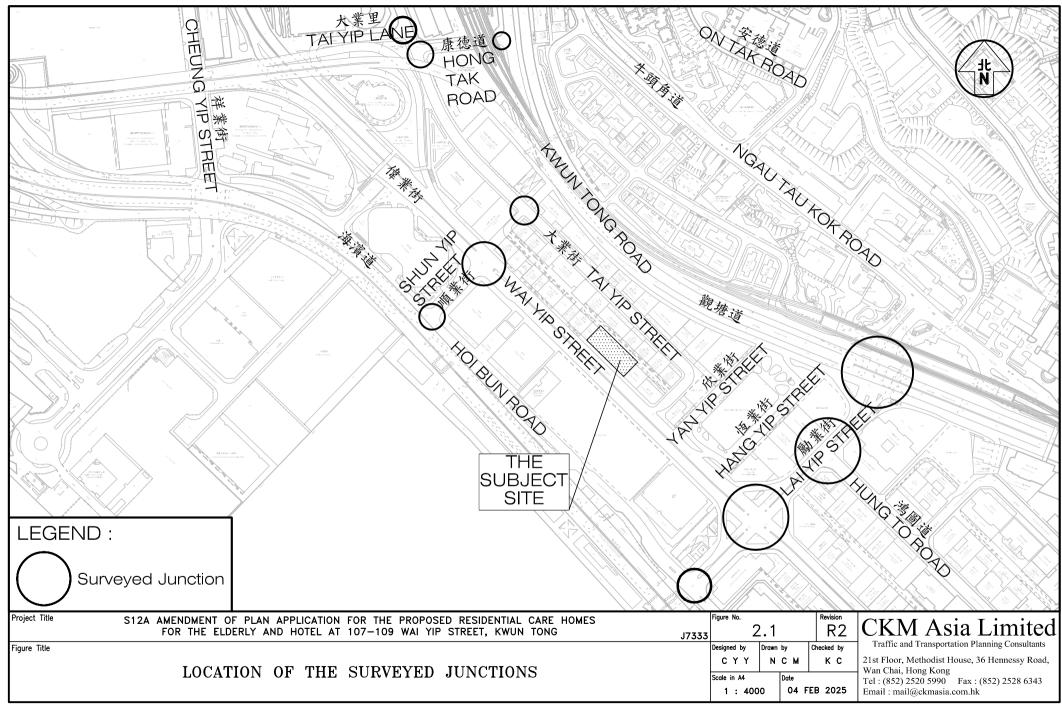
- 6.14 Table 6.5 shows that the pedestrian crossing waiting areas still achieve LOS A and C during AM and PM peak for the sensitivity test.
- 6.15 Since the LOS in Tables 6.4 and 6.5 are A to C, it can be concluded that the sensitivity test found no adverse impact to the footpaths and pedestrian crossing waiting areas in the vicinity.

#### 7.0 CONCLUSION

- 7.1 The Subject Site is located at Nos. 107 109 Wai Yip Street in Kwun Tong. On 29<sup>th</sup> May 2020, the TPB approved the S16 Planning Application (TPB ref: A/K14/780) for Office, Shop and Services & Eating Place Uses at the Subject Site.
- 7.2 Subsequent to the Approved S16 Scheme (TPB ref: A/K14/780), the Applicant has the intention to rezone the Subject Site and construct a building which comprises of a RCHE with (i) no less than 302, but not more than 557 beds, and (ii) hotel with 200 rooms.
- 7.3 Manual classified counts were conducted at the junctions located in the vicinity of the Subject Site in order to establish the peak hour traffic flows. Currently, the surveyed junctions operate with capacities during the AM and PM peak hours.
- 7.4 Similar to the Approved S16 Scheme (TPB ref: A/K14/780), two vehicular access points are provided for the Proposed Development, including, (i) the service lane at the northern side of the Proposed Development, and (ii) Wai Yip Street. Compared to the Approved S16 Scheme (TPB ref: A/K14/780), the Proposed Development is expected to generate less traffic during the AM and PM peak hours.
- 7.5 The internal transport facilities provided for RCHE within the Proposed Development are based on the operational needs and also with reference to similar type RCHE in Kwun Tong. Those for the Hotel within the Proposed Development are provided with reference to the recommendation of the HKPSG. Swept path analysis was conducted to ensure that all vehicles could enter and leave the development and the spaces provided with ease.
- 7.6 The Proposed Development is expected to be completed by 2029, and the junction capacity analysis is undertaken for year 2032. For the design year 2032, the junctions analysed are expected to operate with capacities during the peak hours for the case without and with Proposed Development.
- 7.7 The pedestrian assessment conducted found that the surveyed footpaths and waiting area of the pedestrian crossing would operate with LOS A to C in 2032 for the cases without and with the Proposed Development. Hence, it is concluded that the Proposed Development has <u>no</u> adverse impact to the footpaths and pedestrian crossing in the vicinity.
- 7.8 A sensitivity test for the scheme with 644-bed RCHE and 200-room Hotel is undertaken and found to have no adverse traffic and pedestrian impact.
- 7.9 It is concluded that the Proposed Development will result in <u>no</u> adverse traffic impact to the surrounding road network.

### Figures

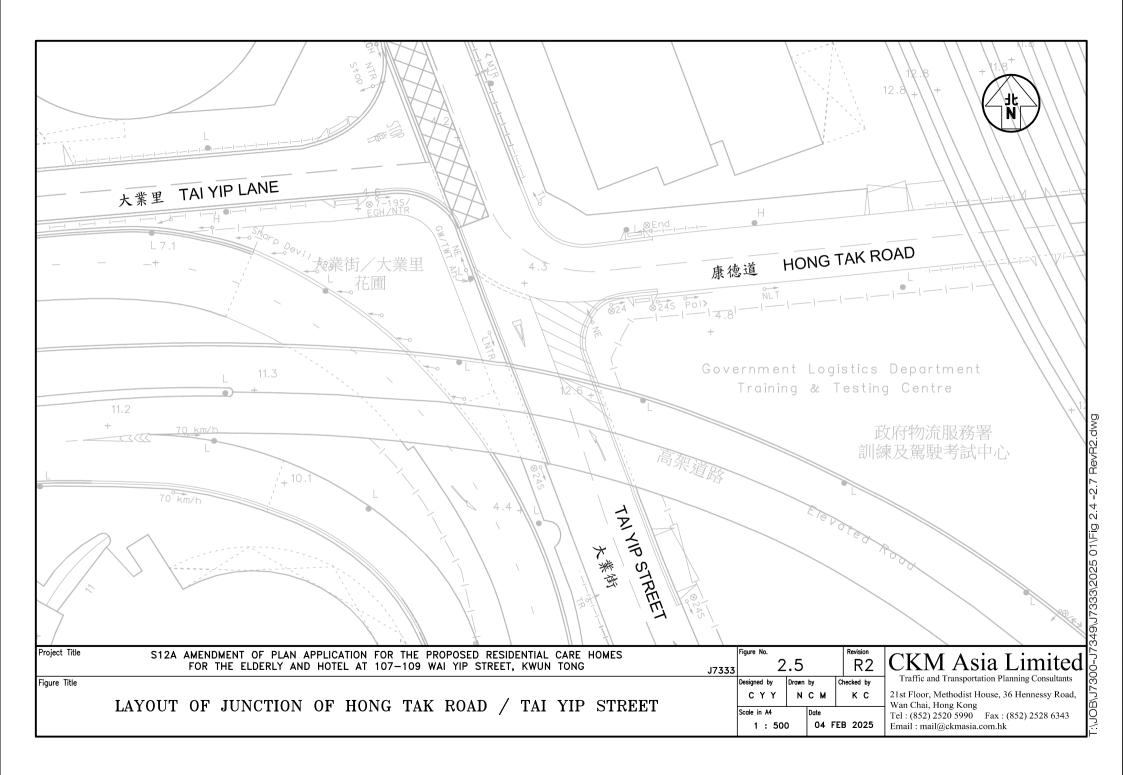


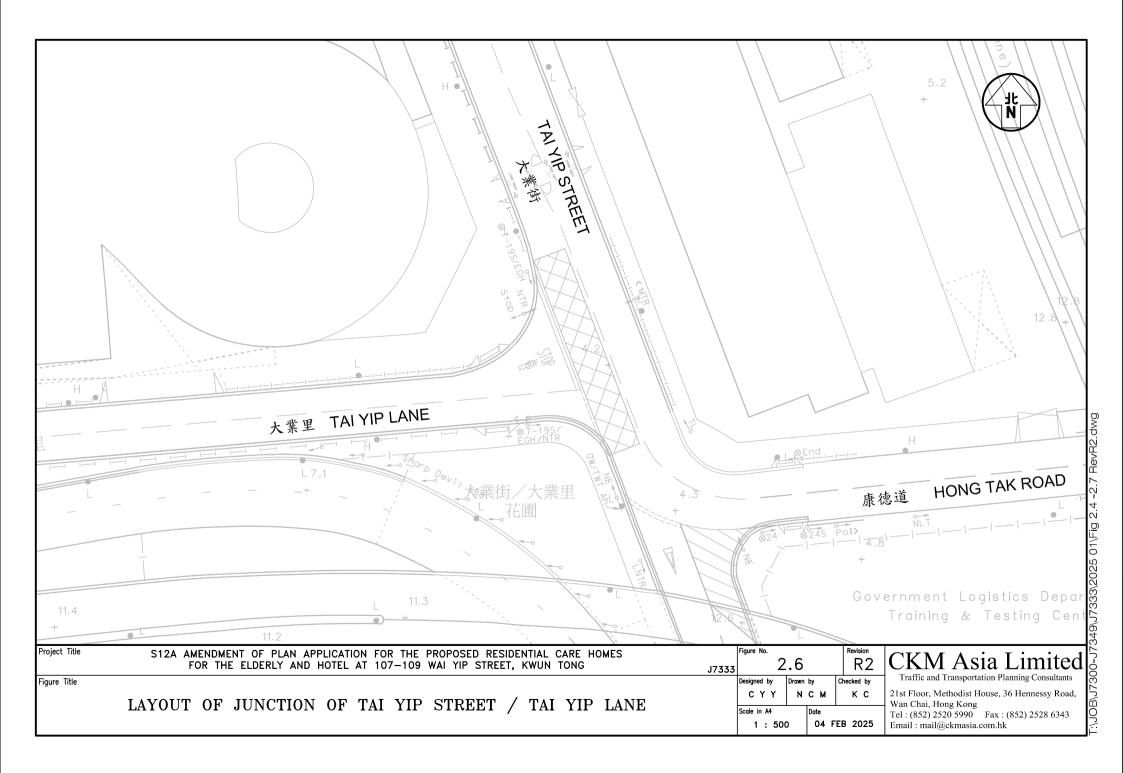


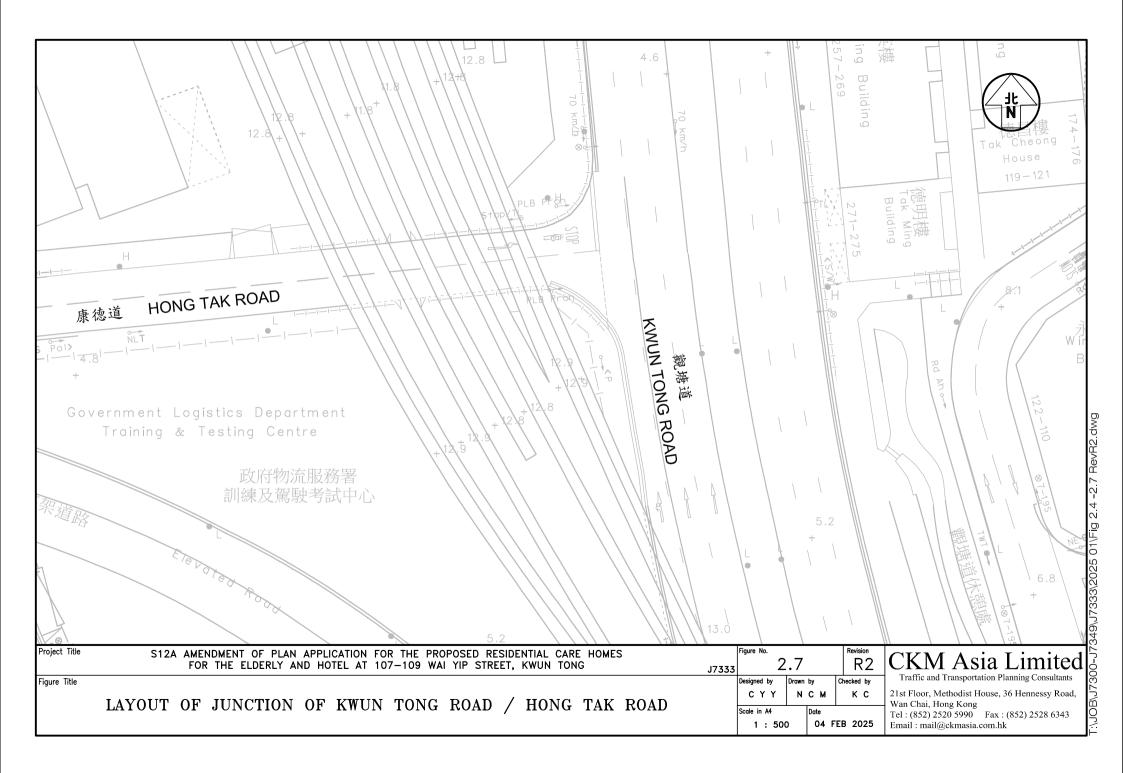
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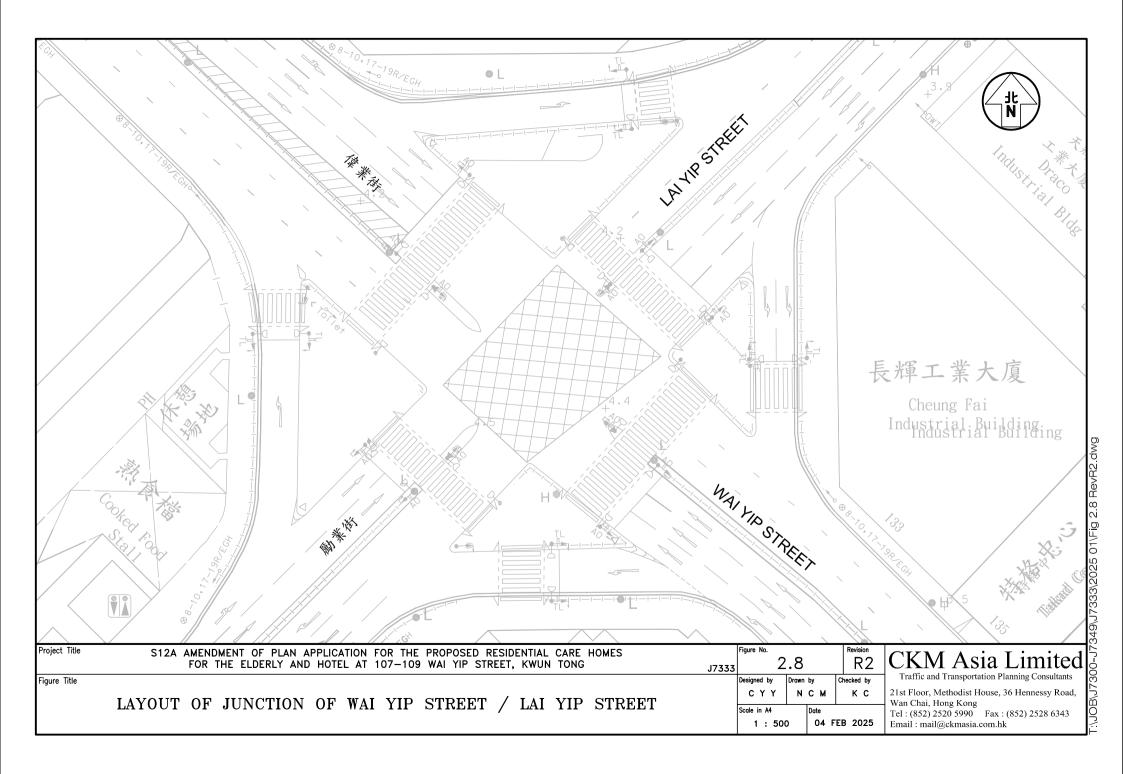
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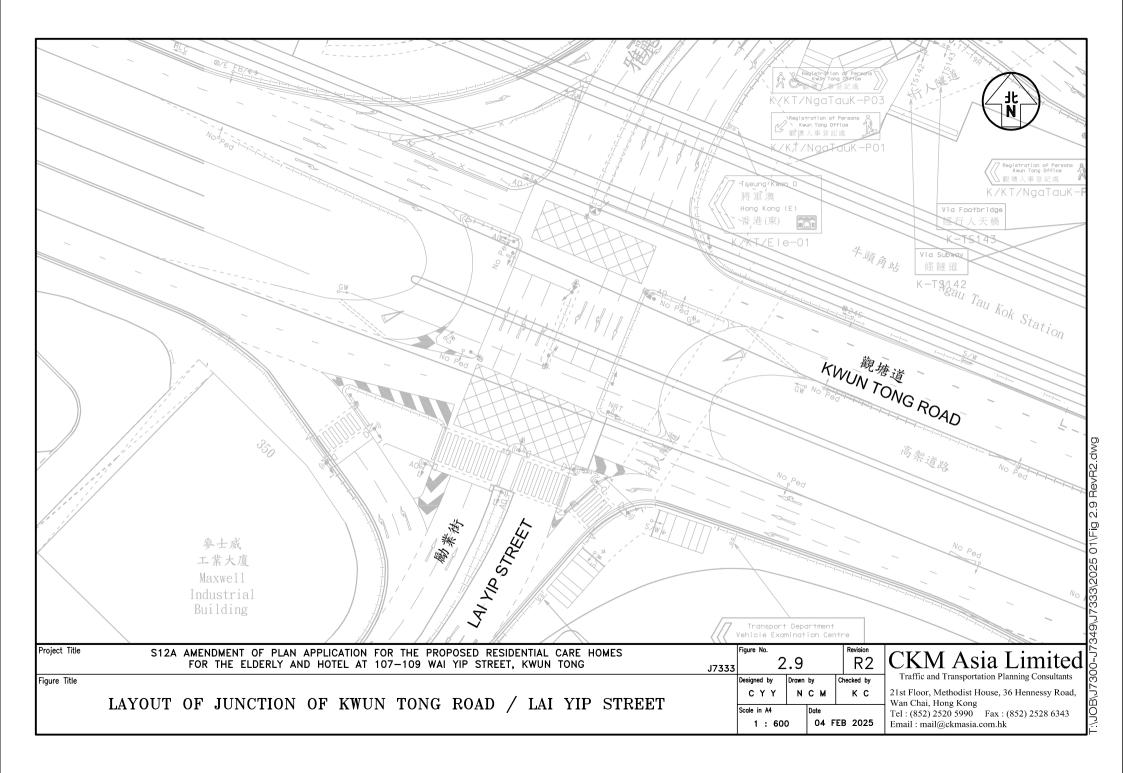
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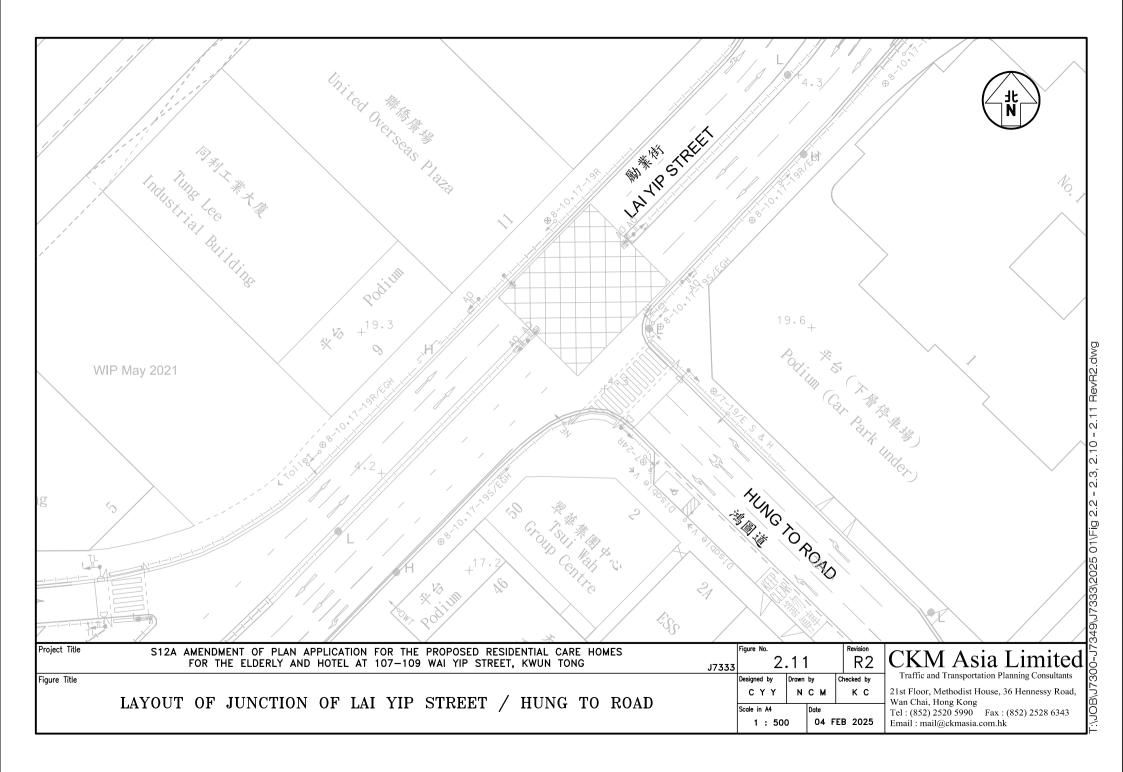


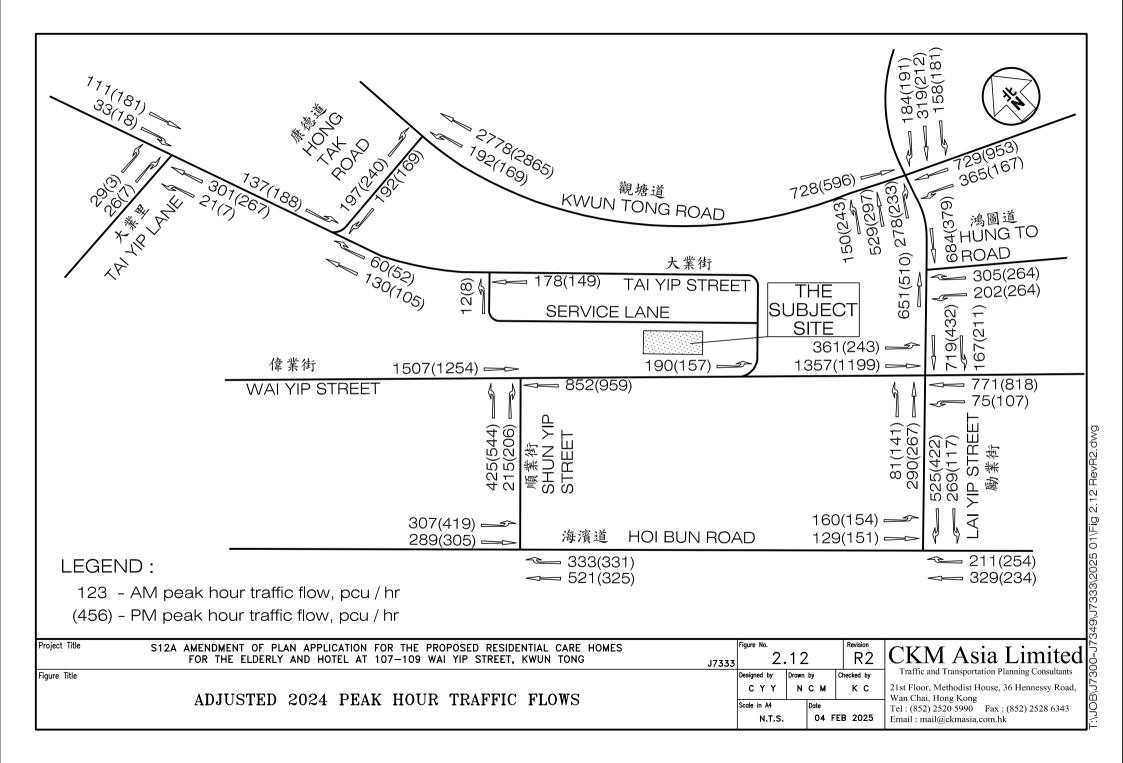


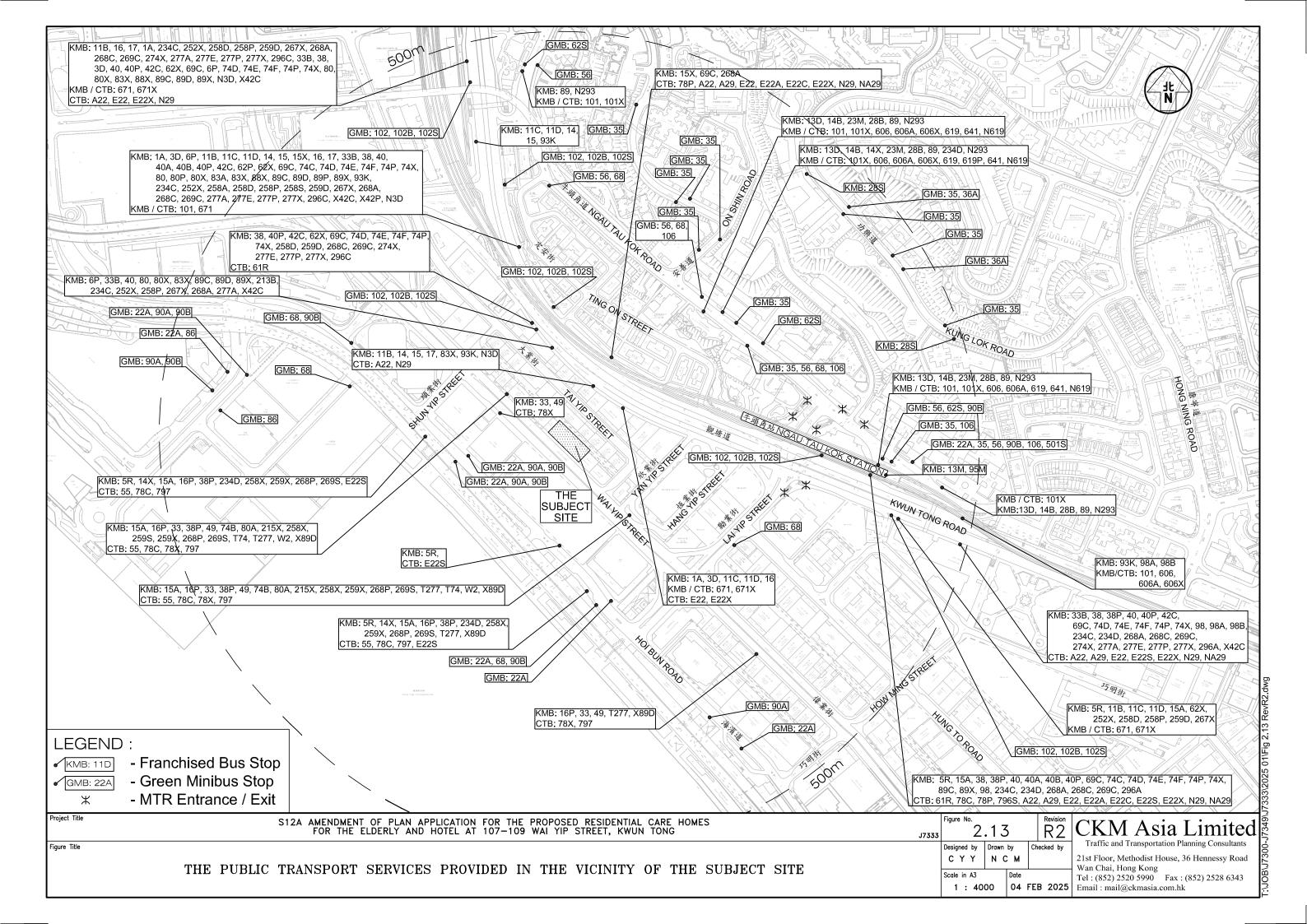


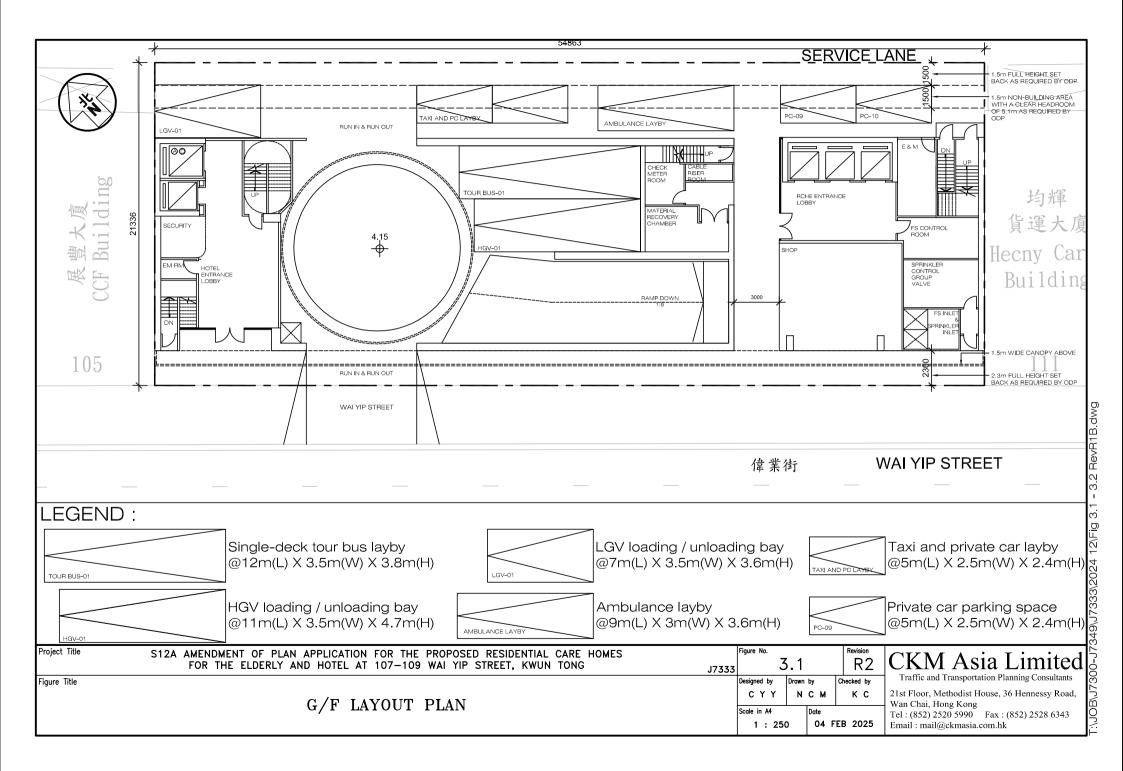


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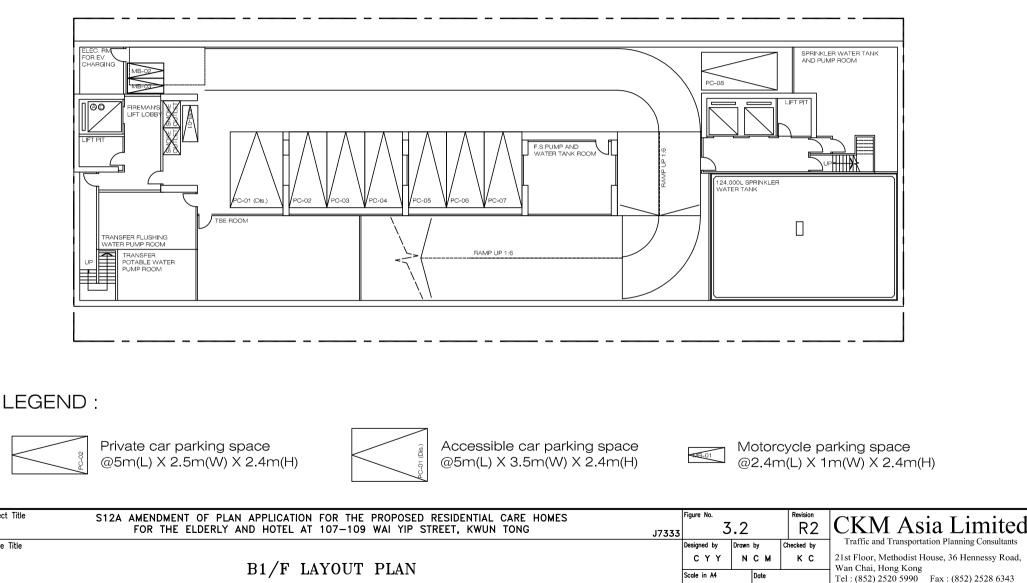






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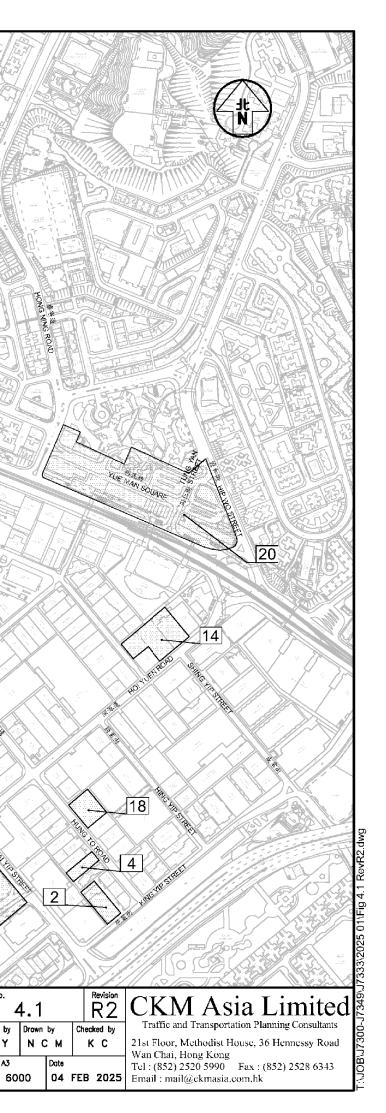
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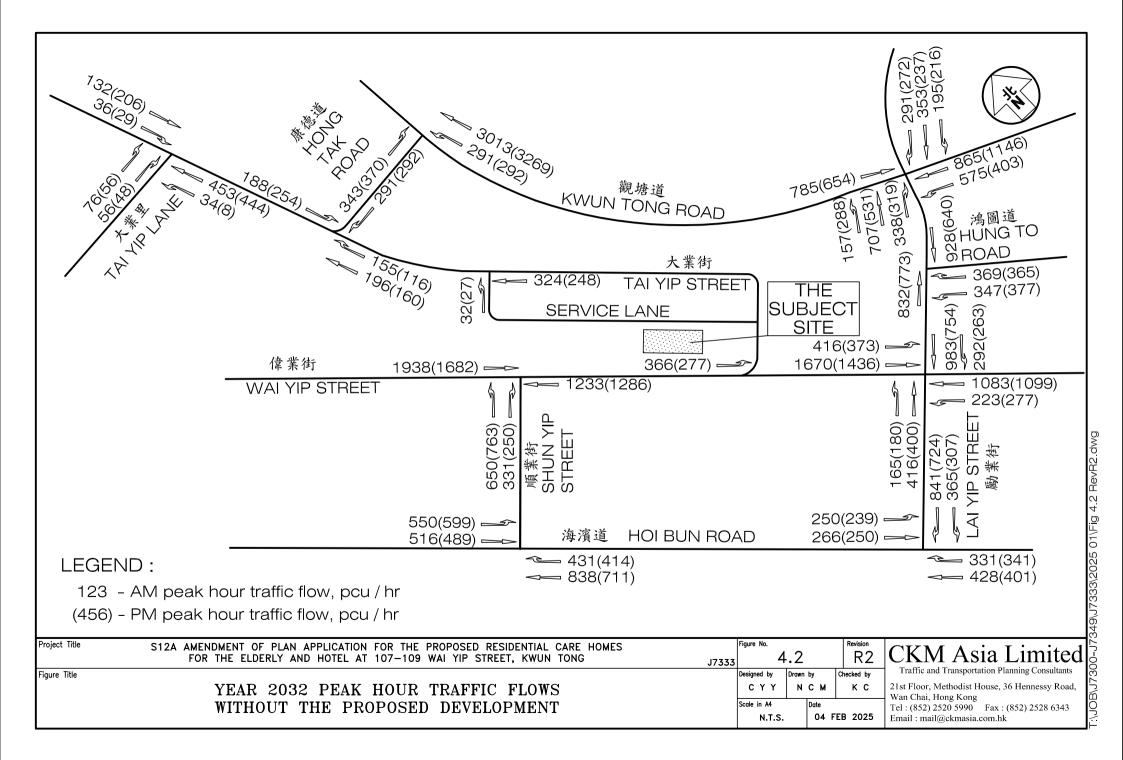
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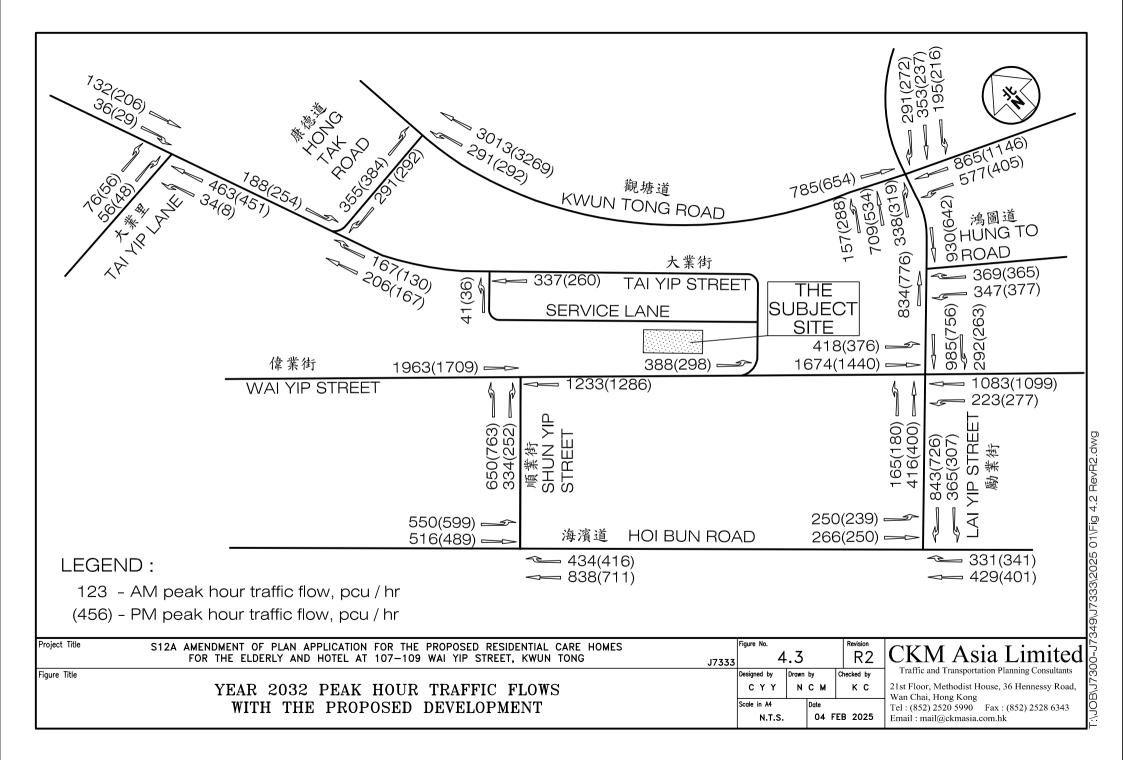
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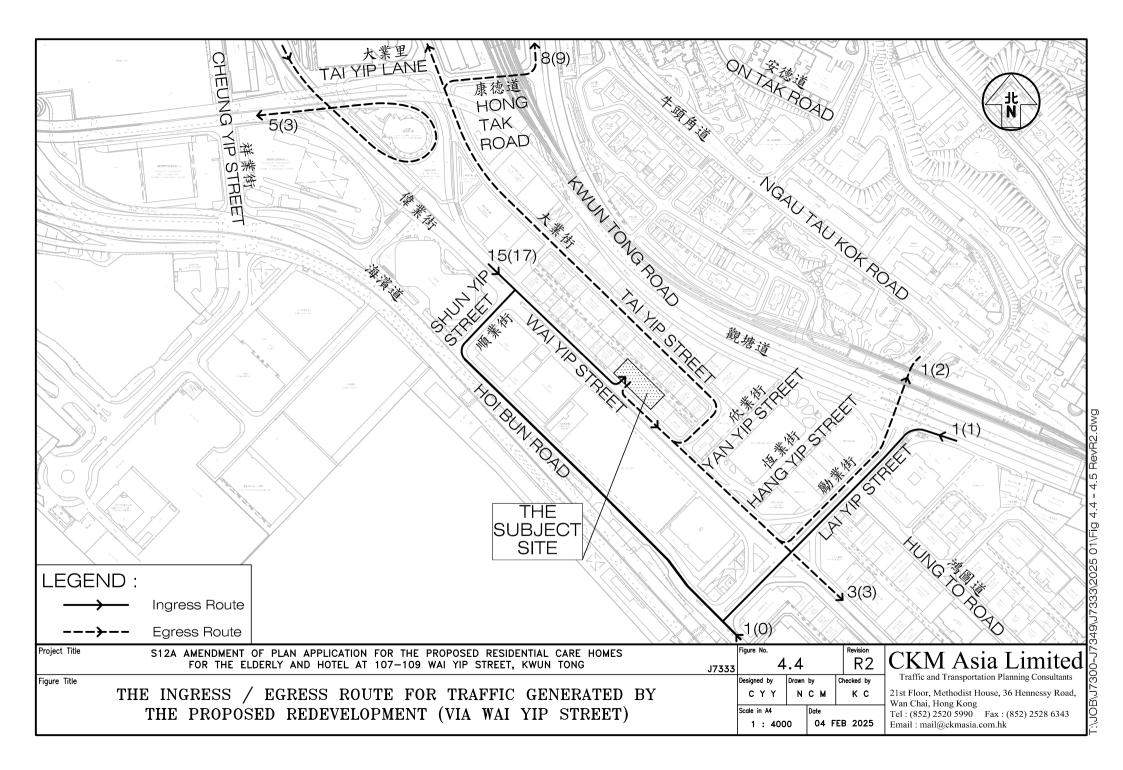
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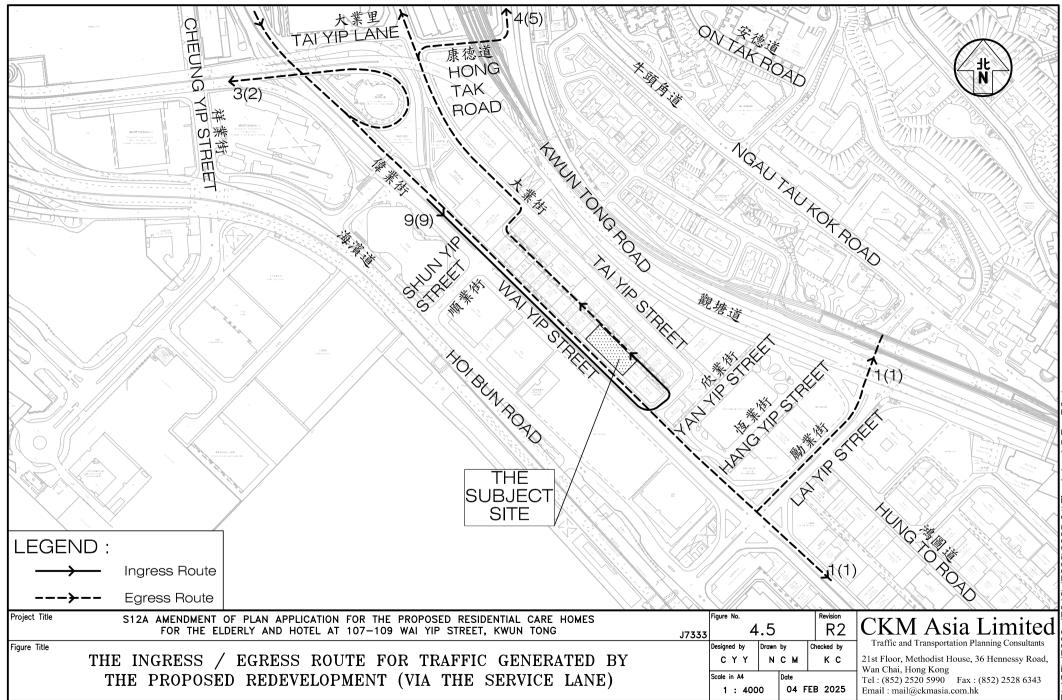
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8	203 Wai Yip Street		S /
9	4 Tai Yip Street		X
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12	334 -336 and 338 Kwun Tong Road		
13	11 Lai Yip Street		
14	Kun Tong Inland Lots 1 S.A , 1 RP, 3 and 15		À,
15	201 Wai Yip Street		:K
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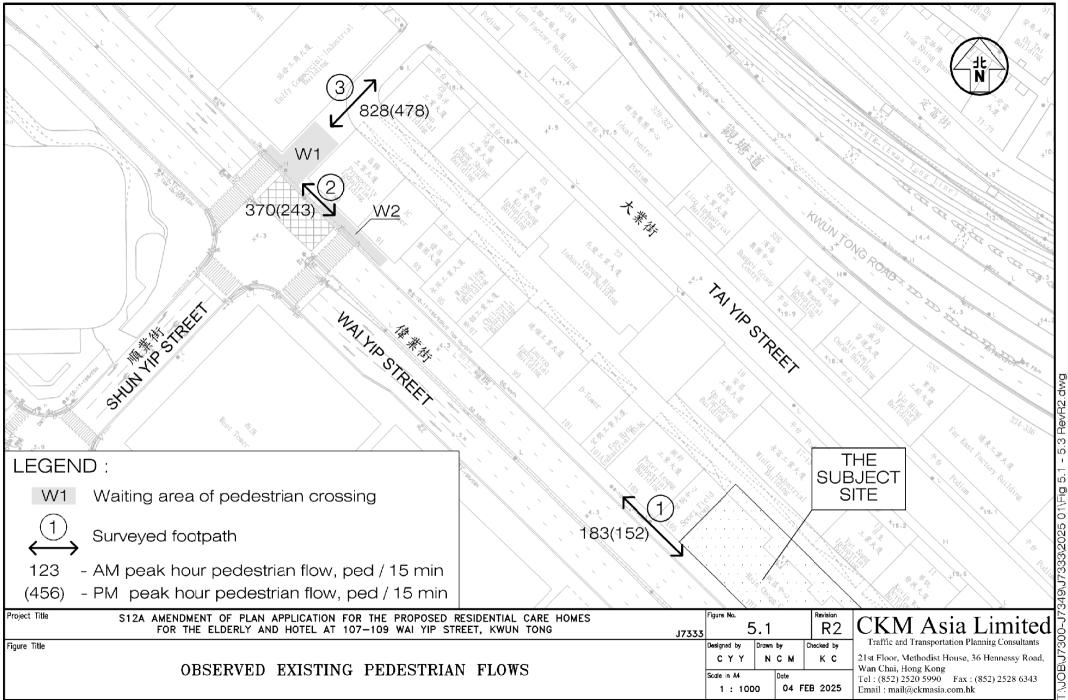




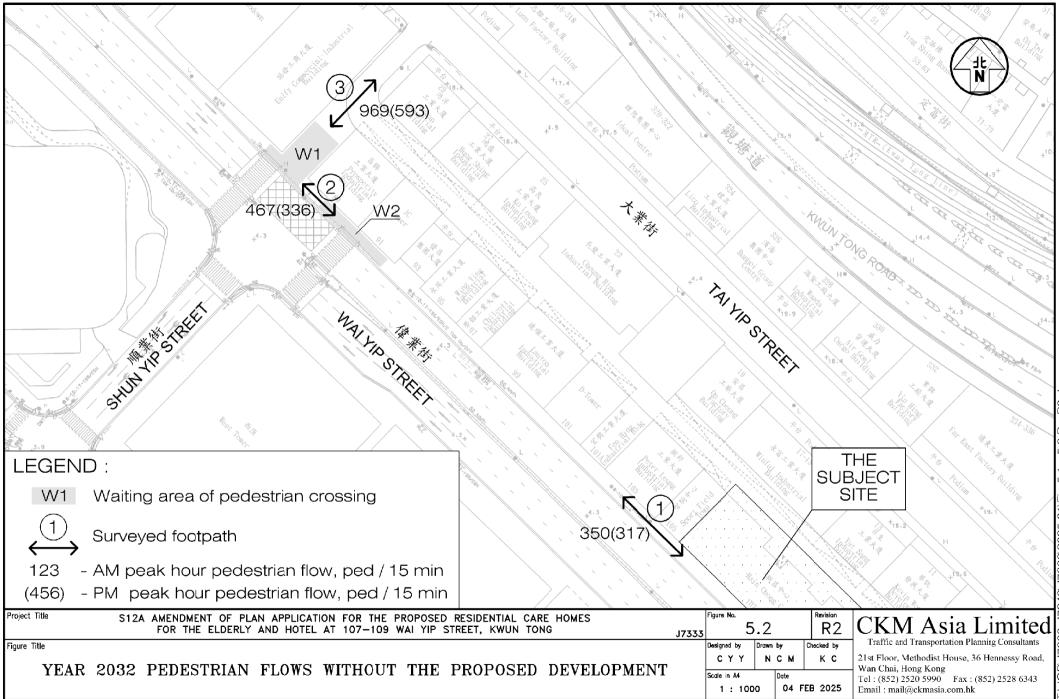


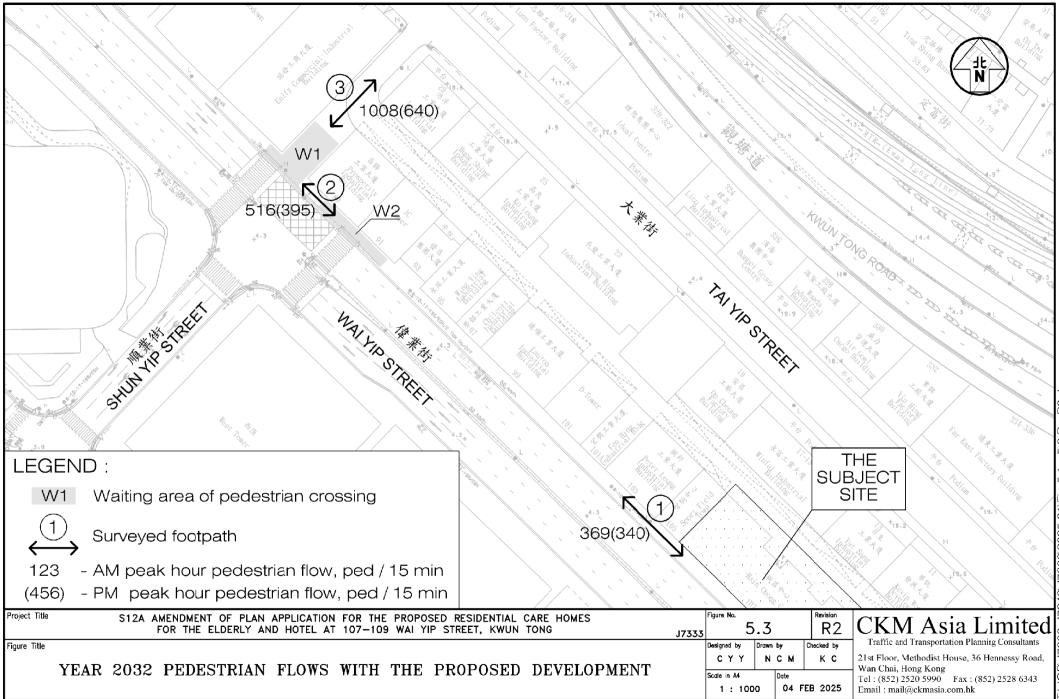




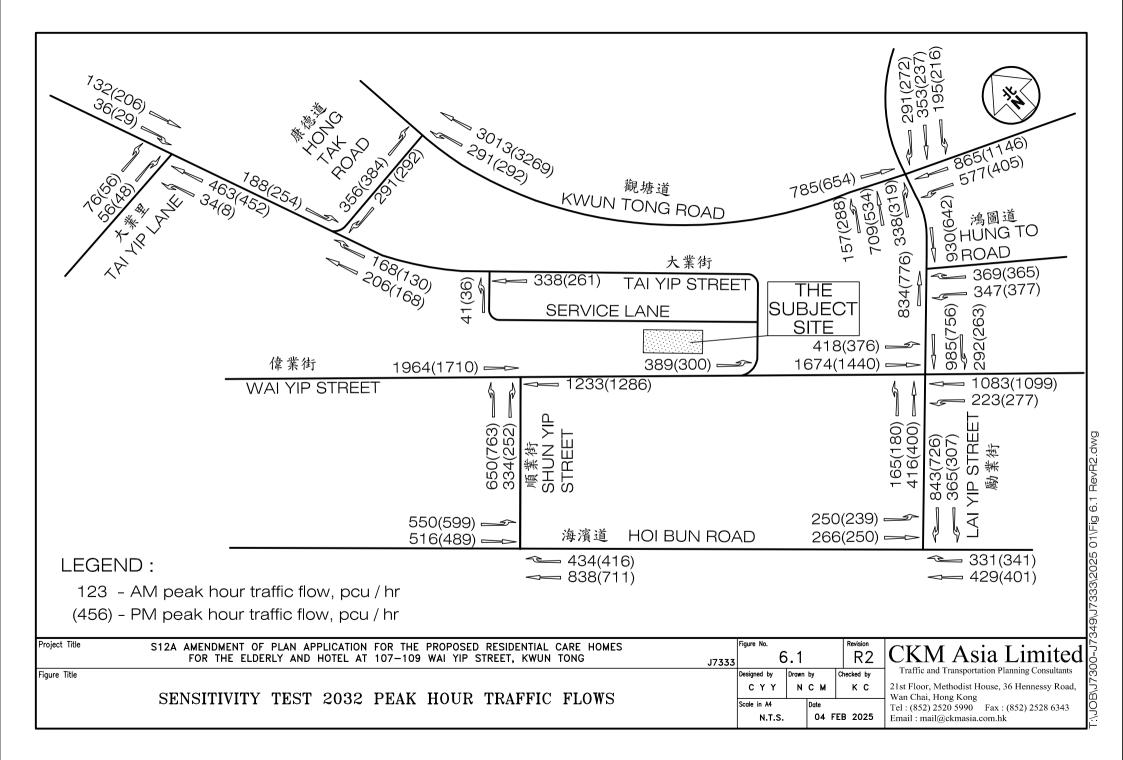


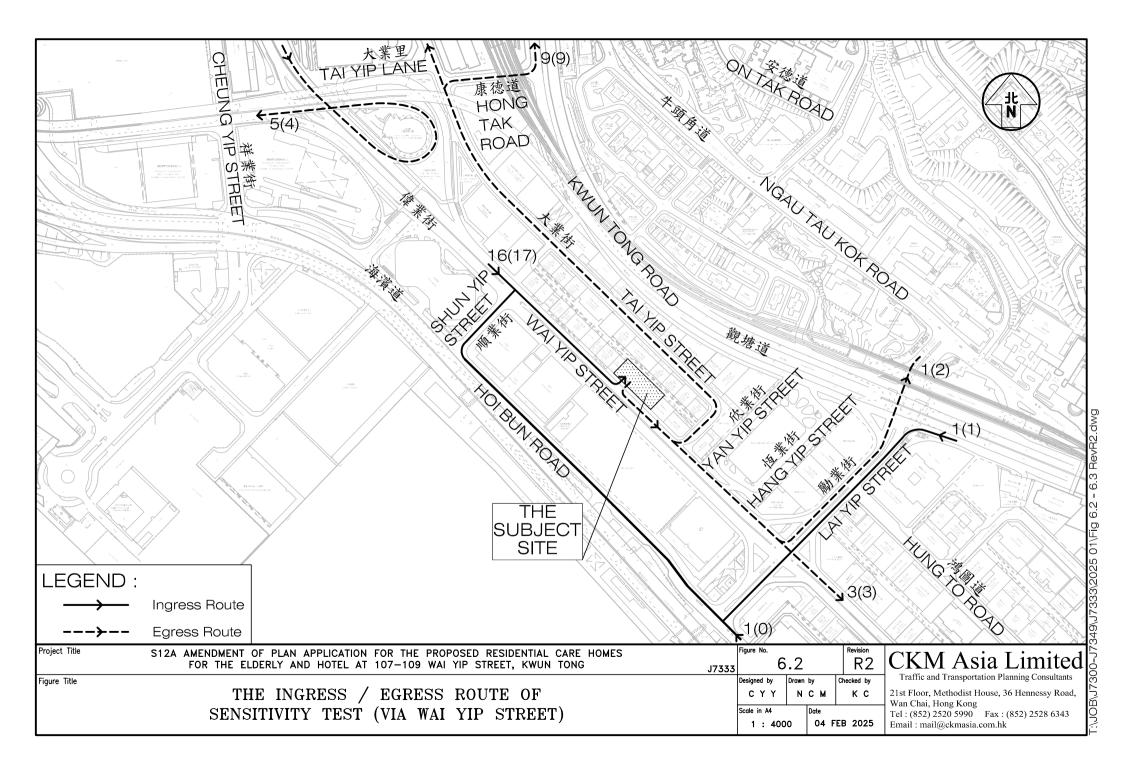
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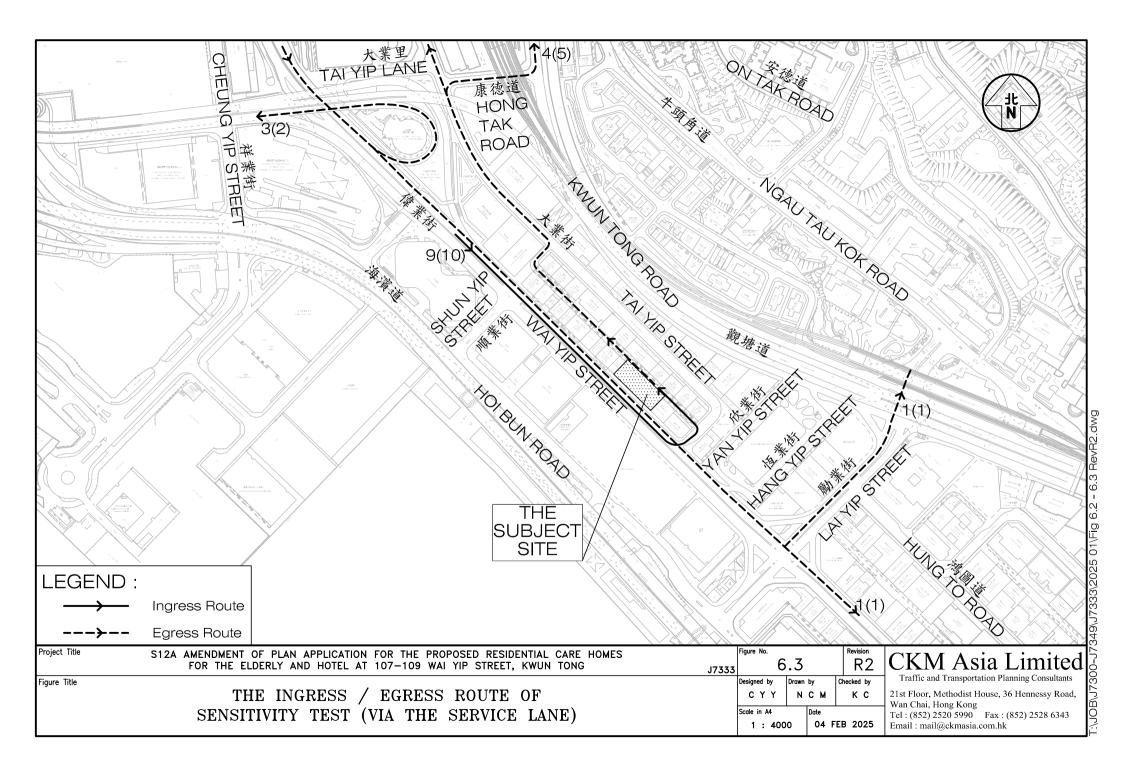


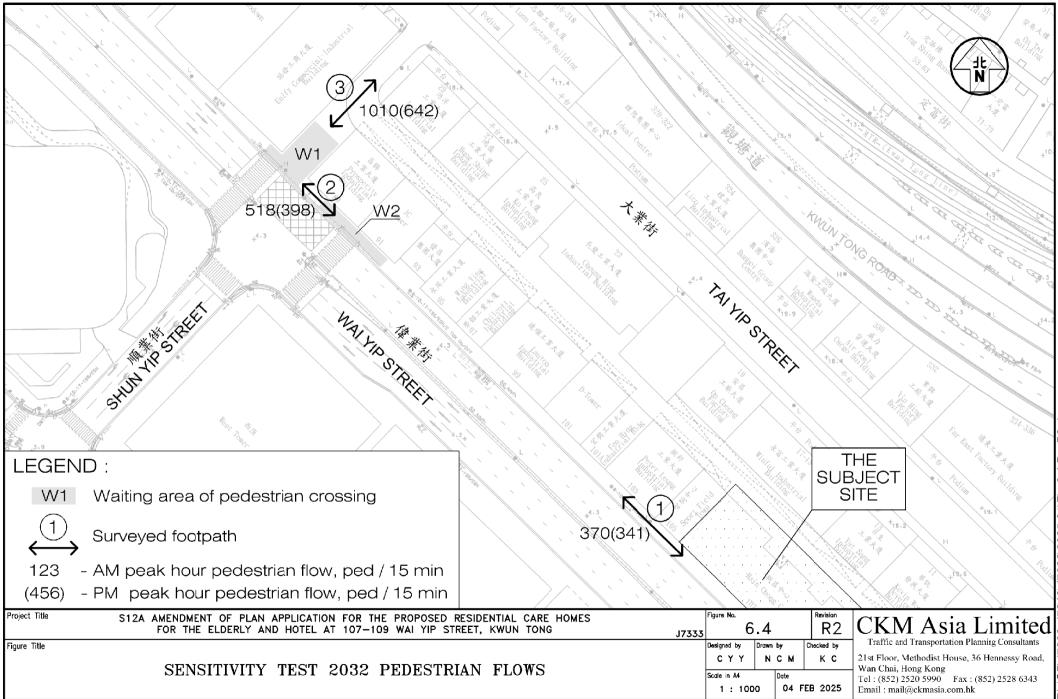


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Appendix 1 – Calculation

Junction: Scenario:	Hoi Bun F	Road / Shun Yi Condition	p Stree	t										-	Job Nu	mber: P.	J7333 1
Design Year:	2024	Designe	ed By:					Checke	ed By:				-	Date:	5 Fe	ebruary	2025
				~				<b>T</b> 1 00	0.1.51	AM Peak		0.14 L	<b>T</b> 1 00	0.1.5	PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Hoi Bun Road	WB	SA	A1	1	3.50				1965	422	0.215	0.215		1965	326	0.166	0.166
		SA+RT	A2	1	3.50	25.0		77	2012	432	0.215		100	1986	330	0.166	
Hoi Bun Road	EB	LT	B1	2	3.50	15.0		100	1786	307	0.172	0.172	100	1786	419	0.235	0.235
		SA	B2	2	3.50				2105	289	0.137			2105	305	0.145	
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n a da atrian in ha			0	4.0		uniu a		4:	7		<u></u>	7		СМ –	4.4		
pedestrian pha	ise		Ср	1,3			rossing		7		GM +	7		GM =	14	sec	
			Dp	2,3			rossing		6	sec		6		GM =	12	sec	
			Ep	3		min c	rossing	time =	11	sec	GM +	12	sec F	GM =	23	sec	
			Fp	3		min c	rossing	time =	8	sec	GM +	6	sec F	GM =	14	sec	
AM Traffic Flow (pcu/h	r)			PM Traffic	Flow (pcu/hr	1									Note:		
	,		N		u ·				N		00(W–3.25		S=2080+10				
307					419				'\	S <sub>M</sub> =S÷(1+	1.5f/r)	S	6 <sub>M</sub> =(S–230)	÷(1+1.5f/r)			
Î Î.					Î.						AM	Peak	PM	Peak			
	289					305					1+2		1+2				
		333 †						331 1		Sum y	0.387		0.401				
		521 -					325	▲—		L (s)	39		39				
										C (s)	118		108				
										practical y	0.603		0.575				
										R.C. (%)	56%		43%				
						0				11.0. (70)	0070		1070				
1		2				3		Ep									
Cp		в1 —					Ср	1									
*		B2	<b>→</b>				÷ •		Fp								
	t	Dp					Dp										
	∔_ہ	A2	4				•	•									
			<u> </u>		1/2	۱ <u>ــــــــــــــــــــــــــــــــــــ</u>		22	1/2	ـــــــــــــــــــــــــــــــــــــ	~		1/2	I			
AM G =		I/G = 8	G =		I/G =	8	G =	23	I/G =	2	G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	:	I/G = 8	G =		I/G =	8	G =	23	I/G =	2	G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:		Road / Shun Yi												-	Job Nu	imber:	
Scenario: Design Year:		the Proposed D Designe						Checke	ed By:					Date:	5 F(	P. ebruary 2	
		,			<del></del>					AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	) % Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)		y value	Critical y
Hoi Bun Road	W/R	SA	A1	1,2	3.50		Glauton		1965	838	0.426			1965	711	0.362	
	110	RT	A2	1,2	3.50	25.0		100	1986	431		0.217	100	1986	414		0.208
			H2	<u>  '</u>	3.00	20.0		100	1900	431	0.211	0.211	100	1900	414	0.200	0.200
				$\vdash$			──′					- 075	100	1700	500		
Hoi Bun Road	EB	LT	B1	2	3.50	15.0	───╯	100	1786	491	0.275			1786	503	0.282	
		SA+LT	B2	2	3.50	20.0	───′	10	2089	575	0.275	<b> </b>	16	2080	585	0.281	──
			⊢−−−−		──	──	<u> '</u>	<b> </b> '	──	<b> </b>	<u> </u>	<b> </b>		──	──	──	──
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pedestrian pha	ase		Ср	1,3	+		rossing t		7		GM +	7		GM =	14	sec	+
			Dp	3	──		crossing t		6	sec (		6		FGM =	12	sec	──
			Ep	3	──		crossing t		11	sec (	GM +	12	sec F	=GM =	23	sec	
			Fp	3	<u> </u>	min c	crossing t	time =	8	sec (	GM +	6	sec F	=GM =	14	sec	<u> </u>
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AM Traffic Flow (pcu/h	r)		Ν	PM Trame	Flow (pcu/hr	)			Ν	S=1940+1	00(W–3.25	i) :	S=2080+10	0(W-3.25)			
550				1	599					S <sub>M</sub> =S÷(1+	1.5f/r)	5	S <sub>M</sub> =(S–230)	÷(1+1.5f/r)		n Improver by Other	
l ↑			$\sim$	1	t				$\sim$		AM	Peak	PM	Peak	Guionic	by Outo.	Fiojool
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		431	I	1				414	I	2	0.492		0.490	'	1		
		838	I	1			711	Ļ		Sum y				$\vdash$	1		
		000	I	1					I	L (s)	39		39	<b> </b> '	1		
			I	1					I	C (s)	118	──	108	<b> </b> '	4		
			I	1					I	practical y	0.603	<b> </b>	0.575	<b>↓</b> '	4		
				Ĺ						R.C. (%)	22%		17%				
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										2							
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =		I/G = 8	G =		I/G =		G =	23	I/G =	2	G =		I/G =		G =		
G =	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction: Scenario:		Road / Shun Y Proposed Dev			bed RC	HE and	200-roo	m Hotel	)					-	Job Nu	mber: P.	
	2032	Designe											-	Date:	5 Fe	ebruary 2	
					1					AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow	y value	Critical y
Lisi Dum Daard		C.A.		4.0	2.50		Gradient		(pcu/hr)		0.400				(pcu/hr)	0.000	
Hoi Bun Road	WB	SA	A1	1,2	3.50				1965	838	0.426			1965	711	0.362	
		RT	A2	1	3.50	25.0		100	1986	434	0.219	0.219	100	1986	416	0.209	0.209
Hoi Bun Road	FB	LT	B1	2	3.50	15.0		100	1786	491	0 275	0.275	100	1786	503	0.282	0.282
TIOI Dull Road												0.215					0.202
		SA+LT	B2	2	3.50	20.0		10	2089	575	0.275		16	2080	585	0.281	
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pedestrian pha	ase		Ср	1,3		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
			Dp	3		min c	rossing	time =	6	sec	GM +	6	sec F	GM =	12	sec	
			Ep	3		min c	rossing	time =	11	sec	GM +	12	sec F	GM =	23	sec	
			Fp	3		min c	rossing	time =	8	sec	GM +	6	sec F	GM =	14	sec	
AM Traffic Flow (pcu/h	<i>*</i> 1			DM Troffie	Flow (pcu/hr					1					Matai		
Aim Tranic Flow (pcu/ii	1)		Ν	PWITAIIIC	riow (pcu/iii				Ν	S=1940+1	00(W-3.25	i) :	S=2080+10	00(W-3.25)	Note:		
550					599					S <sub>M</sub> =S÷(1+	1.5f/r)	s	6 <sub>M</sub> =(S-230)	÷(1+1.5f/r)		Improver by Other	
†			$\backslash$		1				$\backslash$		AM	Peak	PM	Peak		,	,
	516				<b>&gt;</b>	489					1+2		1+2				
		434 †						416		Sum y	0.494		0.491				
		838 ◀					711	•		L (s)	39		39				
										C (s)	118		108		1		
										practical y	0.603		0.575				
										R.C. (%)	22%		17%				
-		-				-				R.C. (%)	2270		1770				
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Cp		B1	Ĵ				Cp	·····•									
¥		<sub>В2</sub>	→				∳ Ą		Fp								
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		— A2 — A1			—— A1		٦	•									
	-			-	——- A1												
AM G =		I/G = 8	G =		I/G =	8	G =	23	I/G =	2	G =		I/G =	1	G =		
G =		I/G = 8	G =		I/G =	5	G =		I/G =	-	G =				G =		
						0		22		2			I/G =				
PM G =		I/G = 8	G =		I/G =	Ø	G =	23	I/G =		G =		I/G =		G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:	Hoi Bun Roa				200 room										Job Nu	mber:	
Scenario: Design Year:	Sensitivity To 2032	Designe						Checke	ed By:					Date:	5 Fe	P. ebruary 2	
		,								AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)		Turning %	Sat. Flow	Flow	y value	Critical y	Turning %	Sat. Flow	Flow	y value	Critical y
Le: Due Dood		<u> </u>	A 1	10	2 50		Gradient		(pcu/hr)	(pcu/hr)	0 406			(pcu/hr)	(pcu/hr)	0.262	
Hoi Bun Road	WВ	SA	A1	1,2	3.50				1965	838	0.426	2.040	100	1965	711	0.362	
		RT	A2	1	3.50	25.0		100	1986	434	0.219	0.219	100	1986	416	0.209	0.209
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Hoi Bun Road	EB	LT	B1	2	3.50	15.0		100	1786	491	0.275	0.275	100	1786	503	0.282	0.282
		SA+LT	B2	2	3.50	20.0		10	2089	575	0.275		16	2080	585	0.281	
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pedestrian pha	ase		Ср	1,3	[	min c	rossing	tim <u>e =</u>	7	sec (	GM +	7	sec F	GM =	14	sec	Ī
			Dp	3			rossing		6	sec (		6		GM =	12	sec	
			Ep	3	1		rossing		11	sec (		12	sec F		23	sec	
			<u> </u>	3			rossing		8	sec (		6		GM =	14		
			<u>- гр</u>	3		THE G	rossing	line –	0	350 1		U	5601	Givi –	14	sec	
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AM Traffic Flow (pcu/h	r)			PM Traffic I	Flow (pcu/hr)					s=1040+1	-0/M-3 25		·	0/1AI_2 25)	Note:		
	r)		N	PM Traffic F		) 			N		100(W-3.25		5=2080+10			Improven	rent
AM Traffic Flow (pcu/h	r)		× ×	PM Traffic F	Flow (pcu/hr)	) 			N N	S=1940+1 S <sub>M</sub> =S÷(1+			S=2080+10 <sub>M</sub> =(S–230)		Junction	Improven by Other	
			-	PM Traffic F					-		1.5f/r)		<sub>M</sub> =(S–230)		Junction		
	<sup>nr)</sup> 516		-	PM Traffic I		489			-		1.5f/r)	S	<sub>M</sub> =(S–230)	÷(1+1.5f/r)	Junction		
		434	-	PM Traffic I				416	-		1.5f/r)	S	M=(S-230)	÷(1+1.5f/r)	Junction		
	516	434 338	-	PM Traffic I			711	1 1	-	S <sub>M</sub> =S÷(1+	1.5f/r) AM 1+2	S	<b>м=(S-230)</b> РМ 1 1+2	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I			711	1 1	-	Sum y L (s)	1.5f/r) AM 1+2 0.494 39	S	PM 1+2 0.491 39	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I			711	1 1	-	Sum y L (s) C (s)	1.5f/r) AM 1+2 0.494 39 118	S	PMI 1+2 0.491 39 108	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I			711	1 1	-	Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I			711	1 1	-	Sum y L (s) C (s)	1.5f/r) AM 1+2 0.494 39 118	S	PMI 1+2 0.491 39 108	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I				<u> </u>	-	Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	516	t	-	PM Traffic I		489		1 1	-	Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
	516	t	-	PM Traffic I		489		Ep.		Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	516	t	-	PM Traffic I		489	Cp ¢	Ep.	-	Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	516	2 338	-	PM Traffic I	<sup>599</sup> <b>↑</b> ,	489		Ep.		Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	516	2 B1 B2	-	PM Traffic I		489	Cp ¢	Ep.		Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	516	2 338	-	PM Traffic I	<sup>599</sup> <b>↑</b> ,	489	Cp ¢	Ep.		Sum y L (s) C (s) practical y	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction		
550 ,	<sup>►</sup> 516 8	2 338	-	PM Traffic I	<sup>599</sup> <b>↑</b> ,	489	Cp ¢	Ep •		S <sub>M</sub> =S+(1+ Sum y L (s) C (s) practical y R.C. (%)	1.5f/r) AMI 1+2 0.494 39 118 0.603	S	м=(S-230) РМ і 1+2 0.491 39 108 0.575	÷(1+1.5f/r)	Junction	by Other	
550 , 1 	<ul> <li>516</li> <li>8</li> <li>↓</li> <li>↓<td>2 B1 B2 - A1</td><td></td><td>PM Traffic :</td><td>599</td><td>489 3 8</td><td>Cp Dp</td><td>Ep •</td><td>Fp</td><td>S<sub>M</sub>=S+(1+ Sum y L (s) C (s) practical y R.C. (%)</td><td>1.51/r) AM 1+2 0.494 39 118 0.603 22%</td><td>S</td><td>m=(S-230) PM 1+2 0.491 39 108 0.575 17%</td><td>÷(1+1.5f/r)</td><td>Junction Scheme</td><td>by Other</td><td></td></li></ul>	2 B1 B2 - A1		PM Traffic :	599	489 3 8	Cp Dp	Ep •	Fp	S <sub>M</sub> =S+(1+ Sum y L (s) C (s) practical y R.C. (%)	1.51/r) AM 1+2 0.494 39 118 0.603 22%	S	m=(S-230) PM 1+2 0.491 39 108 0.575 17%	÷(1+1.5f/r)	Junction Scheme	by Other	
550 , 1  AM G =	516 8 • • • • • • • • • • • • • • • • • • •	$338 \bullet$	G =	PM Traffic	599 ↑ → A1 I/G =	489 3 8	Cp Dp	Ep	Fp I/G =	S <sub>M</sub> =S+(1+ Sum y L (s) C (s) practical y R.C. (%)	1.51/r) AM 1+2 0.494 39 118 0.603 22% G =	S	m=(S−230) PM 1+2 0.491 39 108 0.575 17%	÷(1+1.5f/r)	G =	by Other	

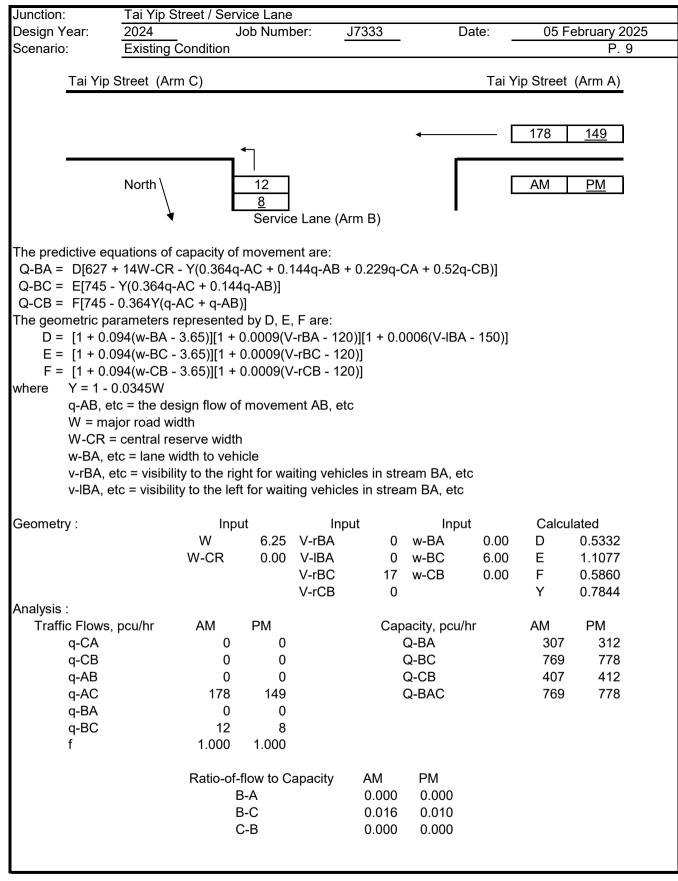
Junction:	Wai Yip Stre		ip Stree	t											Job Nu	mber:	-
Scenario: Design Year:	Existing Cor		ed By:					Checke	d By:					Date:	5 Fe	P. ebruary 2	
	A		Dhara	01		Dealine (m)	0( LI- 1-11	Turning %	Oct. Flam	AM Peak		O-Hissley	Turnin e 0/	Orth Flow	PM Peak		Oritication
	Approach		Phase	Stage		Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Wai Yip Street	t EB	SA	A1	1	3.50				1965	480	0.244	0.244		1965	399	0.203	
		SA	A2	1	3.50				2105	514	0.244			2105	427	0.203	0.203
		SA	A3	1	3.50				2105	513	0.244			2105	428	0.203	
Wai Yip Street		SA	B1	1	3.50				1965	271	0.138			1965	305	0.155	
	. 110	SA	B2	1	3.50				2105	290	0.138			2105	327	0.155	
		SA	B3	1	3.50				2105	291	0.138			2105	327	0.155	
Shun Yip Stree	et NB	LT	C1	3	3.50	15.0		100	1786	205	0.115	0.115	100	1786	261	0.146	0.146
		LT+RT	C2	3	3.50	18.0		100	1943	222	0.114		100	1943	283	0.146	
		RT	C3	3	3.50	25.0		100	1854	213	0.115		100	1854	206	0.111	
pedestrian pha	ase		Dp	1,2		min c	rossing	time =	8	sec	GM +	11	sec F	GM =	19	sec	
			Ep	2		min c	rossing	time =	12		GM +	9	sec F		21	sec	
			Fp	2		min c	rossing	time =	13	sec	GM +	12	sec F	GM =	25	sec	
AM Traffic Flow (pcu/hr	r)		Z €	PM Traffic	Flow (pcu/hr)	)			Z	S=1940+1	00(W–3.25	) :	S=2080+10	0(W–3.25)	Note:		
AM Traffic Flow (pcu/h	ir)		z≮	PM Traffic	Flow (pcu/hr)	)			× ₹	S=1940+1 S <sub>M</sub> =S÷(1+			S=2080+10 <sub>M</sub> =(S–230)·	```	Note:	I	
	<sup>sr)</sup> ★ 1507		z	PM Traffic		1254			× €		1.5f/r)		<sub>M</sub> =(S−230)·	```	Note:	I	
			z	PM Traffic					×√		1.5f/r) AM 1+3	S	M=(S-230) PM I 1+3	÷(1+1.5f/r)	Note:	I	
	+ 1507	352 ←	Z	PM Traffic			959		z	S <sub>M</sub> =S÷(1+ Sum y	1.5f/r) AM 1+3 0.359	S	PM I 1+3 0.349	÷(1+1.5f/r)	Note:	I	
	► 1507		z	PM Traffic		<sup>.</sup> 1254			Z	S <sub>M</sub> =S÷(1+ Sum y L (s)	AM 1+3 0.359 40	S	PM 1 1+3 0.349 40	÷(1+1.5f/r)	Note:	L	
	★ 1507		z	PM Traffic		<sup>.</sup> 1254	959 206		Z	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s)	1.5f/r) AM 1+3 0.359 40 118	S	м=(S-230) РМ 1 1+3 0.349 40 108	÷(1+1.5f/r)	Note:		
,	► 1507		Z	PM Traffic		<sup>.</sup> 1254		•	Z	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
	► 1507		N AND AND AND AND AND AND AND AND AND AN	PM Traffic		<sup>.</sup> 1254		4	Z	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s)	1.5f/r) AM 1+3 0.359 40 118	S	м=(S-230) РМ 1 1+3 0.349 40 108	÷(1+1.5f/r)	Note:		
	► 1507		z	PM Traffic		<sup>.</sup> 1254			Z	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
	► 1507		Z	PM Traffic	544	<sup>.</sup> 1254			z <u>(</u>	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
	► 1507	2	Fp Fp	PM Traffic		<sup>.</sup> 1254			z (	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
425 -	► 1507	2 B3 B2		PM Traffic	544	<sup>.</sup> 1254			Z (	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
425 -	<ul> <li>▶ 1507</li> <li>8</li> <li>▶ 215</li> <li>↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓</li></ul>	2 B3			544	· 1254	206		z <	S <sub>M</sub> =S÷(1+ Sum y L (s) C (s) practical y	1.5f/r) AM 1+3 0.359 40 118 0.595	S	м=(S-230) <sup>-</sup> РМ 1 1+3 0.349 40 108 0.567	÷(1+1.5f/r)	Note:		
425 -	+ 1507     8     4     215     7	B3 B2 B1	Fp Dp		544 •	3 C1				Sum y L (s) practical y R.C. (%)	1.5f/r) AM 1+3 0.359 40 118 0.595 66%	S	M=(S-230) PM I 1+3 0.349 40 108 0.5667 62%	÷(1+1.5f/r)			
425 ·	+ 1507     8     4     215     7     215     7	B3 B2 B1 G = 7	Fp		544 < Ep	3 C1	206		I/G =	Sum y L (s) practical y R.C. (%)	1.5f/r) AM 1+3 0.359 40 118 0.595	S	m=(S-230) PM 1 1+3 0.349 40 108 0.567 62% I/G =	÷(1+1.5f/r)	G = G =		
425 - 1 A1 A2 A3 AM G =		B3 B2 B1	Fp G =	25	544 •	1254	$206$ $\begin{array}{c} \bullet\\ $	· · · · · · · · · · · · · · · · · · ·		Sum y L (s) C (s) practical y R.C. (%)	1.5f/r) AM 1+3 0.359 40 118 0.595 66% G =	S	M=(S-230) PM I 1+3 0.349 40 108 0.5667 62%	÷(1+1.5f/r)	G =		

						0			narys								
Junction:	Wai Yip Stre														Job Nu	mber:	J7333
Scenario:	Without the															P.	
Design Year:	2032	Designe	ed By:				-	Checke	d By:					Date:	5 Fe	ebruary 2	2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
						ridaido (iii)	Gradient	, dirining , o	(pcu/hr)	(pcu/hr)		ontiour y	rannig /o	(pcu/hr)	(pcu/hr)		on liber y
Wai Yip Street	t EB	SA	A1	1	3.50				1965	617	0.314			1965	535	0.272	
		SA	A2	1	3.50				2105	661	0.314	0.314		2105	573	0.272	0.070
		SA	A3	1	3.50				2105	660	0.314			2105	574	0.273	0.273
Wai Yip Street	t WB	SA	B1	1	3.50				1965	392	0.199			1965	409	0.208	
		SA	B2	1	3.50				2105	420	0.200			2105	438	0.208	
		SA	B3	1	3.50				2105	421	0.200			2105	439	0.209	
Shun Yip Stree	et NB	LT	C1	3	3.50	15.0		100	1786	314	0.176	0.176	100	1786	365	0.204	
		LT+RT	C2	3	3.50	18.0		100	1943	341	0.176		100	1943	398	0.205	0.205
		RT	C3	3	3.50	25.0		100	1854	326	0.176		100	1854	250	0.135	
u - l - tri- u - l			Du	4.0					0						40		
pedestrian pha	ase		Dp En	1,2 2			rossing t		8 12		GM +	11 9	sec F		19 21	sec	
			Ep Fp	2			rossing t		12		GM + GM +	9 12	sec F sec F		25	sec sec	
			гр	2			IUSSING		13	Sec		12	Sec P		20	Sec	
AM Traffic Flow (pcu/h	nr)			PM Traffic	Flow (pcu/hr)	1				8-1040+4	00(W–3.25	\	6=2080+10	0/14/ 2 25)	Note:		
			N K						N K	S_1940+1 S <sub>M</sub> =S÷(1+			M=(S-230)				
	<b>N</b> 4000					4000				5 <sub>M</sub> =5∓(1∓							
	+ 1938		`			1682			`		AM 1+3	Peak	PM 1+3	Peak			
										Sum y	0.490		0.478				
	12	33 🔶					1286	•		L (s)	40		40				
	. –				763 ◄	⊢ <b>_</b>	250			C (s)	118		108				
650					100						-						
650					703					practical y	0.595		0.567				
650					703					practical y R.C. (%)	0.595 21%		0.567 19%				
650		2			765	3											
650		2			765	3											
650		2			•	3											
$\begin{array}{c} 650 \\ \hline 1 \\ A1 \longrightarrow \\ A2 \longrightarrow \\ A3 \longrightarrow \end{array}$			Fp		Ep	3											
$\begin{array}{c} 650 \\ \hline 1 \\ A1 \longrightarrow \\ A2 \longrightarrow \\ A3 \longrightarrow \end{array}$		B3 B2	Fp		•	3	<u>∗</u> ↓ [+										
1 A1 → → A2 → → A3 → →	331	В3	Fp • • • •		•	•	<b>◆</b> →   →										
1 A1 → → A3 → → D <sub>1</sub>	331 331	B3 B2 B1	<b>∢</b> Dp	-	Ep	t C1	← → → <u>C2 C3</u> G =		I/G =	R.C. (%)	21%		19%		6=		
$\begin{array}{c} 1 \\ A1 \longrightarrow \\ A2 \longrightarrow \\ A3 \longrightarrow \end{array}$	331 331 	B3 B2	<b>4</b>	-	•	C1	← → → → → → → → → → → → → → → → → → → →		I/G = I/G =	R.C. (%)					G = G =		
1 A1 A2 A3 A3 AM G =	331	B3 B2 B1 G= 7	<b>∢</b> Dp G =	25	Ep I/G =	• C1 8	G =			R.C. (%)	21% G =		19% I/G =				

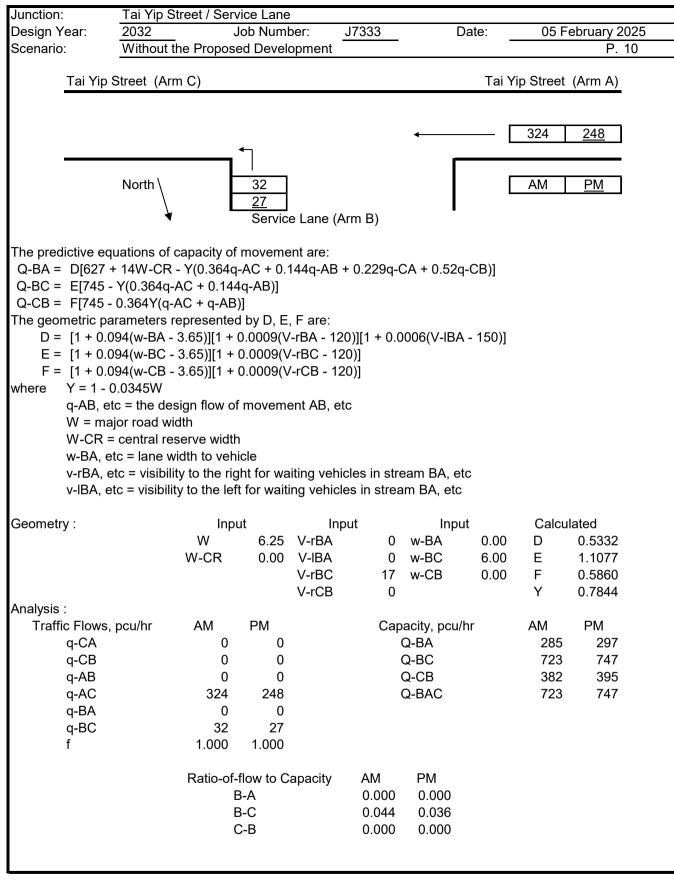
hum at the			(in 0)			-			-						1.0		17000
Junction: Scenario:		reet / Shun Y roposed Dev			-bed RC	HE and	200-roo	m Hotel	)						Jod Nu	mber: P.	
Design Year:	2032							Checke					-	Date:	5 Fe	ebruary 2	
	Approach		Phase	Stage	Width (m)	Radius (m)		Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Wai Yip Street	t EB	SA	A1	1	3.50		Gradient		(pcu/hr) 1965	(pcu/hr) 625	0.318	0.318		(pcu/hr) 1965	(pcu/hr) 544	0.277	
wai np otreet		SA	A2	1	3.50				2105	669	0.318	0.010		2105	583	0.277	0.277
		SA	A3	1	3.50				2105	669	0.318			2105	582	0.276	
Wai Yip Street	t WB	SA	B1	1	3.50				1965	392	0.199			1965	409	0.208	
		SA	B2	1	3.50				2105	420	0.200			2105	438	0.208	
		SA	B3	1	3.50				2105	421	0.200			2105	439	0.209	
Shun Yip Stree	ot NB	LT	C1	3	3.50	15.0		100	1786	315	0.176		100	1786	365	0.204	
	ernd	LT+RT	C2	3	3.50	18.0		100	1943	342	0.176	0.176	100	1943	398	0.204	0.205
		RT	C3	3	3.50	25.0		100	1854	327	0.176	0.110	100	1854	252	0.136	0.200
pedestrian pha	ase		Dp	1,2		min c	rossing	time =	8	sec	GM +	11	sec F	GM =	19	sec	
			Ep	2		min c	rossing	time =	12		GM +	9	sec F		21	sec	
			Fp	2		min c	rossing	time =	13	sec	GM +	12	sec F	GM =	25	sec	
AM Traffic Flow (pcu/h	nr)			PM Traffic	Flow (pcu/hr)										Note:		
			N K						N K		00(W-3.25		S=2080+10	· · ·			
						4700				S <sub>M</sub> =S÷(1+	1		6 <sub>M</sub> =(S−230) <sup>.</sup>				
	► 1963		``			1709			``		AM 1+3	Peak	PM 1+3	Peak			
										Sum y	0.494		0.482				
	1	233 🔶					1286	•		L (s)	40		40				
650	← → 334	4			763 ◄	<b>⊢</b>	252			C (s)	118		108				
										practical y	0.595		0.567				
										R.C. (%)	20%		18%				
1		2				3											
A1>		<b>+</b>															
A2 A3			Fp		Ep												
	←	_ вз				•	<b>┽</b> ┍┝										
<b>4</b>	·····•	- <sup>B2</sup> B1	4	<b>.</b>													
D			Dp			C1	C2 C3										
AM G =	=	I/G = 7	G =	25	I/G =	8	G =		I/G =	2	G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G=		I/G = 7	G =	25	I/G =	8	G =		I/G =	2	G =		I/G =		G =		
G =	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

						_			narys								
Junction:	Wai Yip Stree													-	Job Nu	mber:	-
Scenario: Design Year:	Sensitivity Te				:00-room			Checke	d By:				<u> </u>	Date:	5 Fe	P. bruary 2	
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
	Арргоасті		Pliase	Slage	width (m)	Radius (III)	Gradient	running %	(pcu/hr)	(pcu/hr)	y value	Chucary	Turning %	(pcu/hr)	(pcu/hr)	y value	Chucary
Wai Yip Street	EB	SA	A1	1	3.50				1965	625	0.318	0.318		1965	544	0.277	
		SA	A2	1	3.50				2105	670	0.318			2105	583	0.277	0.277
		SA	A3	1	3.50				2105	669	0.318			2105	583	0.277	
Wai Yip Street	WB	SA	B1	1	3.50				1965	392	0.199			1965	409	0.208	
I		SA	B2	1	3.50				2105	420	0.200			2105	438	0.208	
		SA	B3	1	3.50				2105	421	0.200			2105	439	0.209	
Shun Yip Stree	et NB	LT	C1	3	3.50	15.0		100	1786	315	0.176		100	1786	365	0.204	
		LT+RT	C2	3	3.50	18.0		100	1943	342	0.176	0.176	100	1943	398	0.205	0.205
		RT	C3	3	3.50	25.0		100	1854	327	0.176		100	1854	252	0.136	
					<u> </u>												
pedestrian pha	ase		Dp	1,2		min c	rossing	time =	8	sec	GM +	11	sec F	GM =	19	sec	
			Ep	2		min c	rossing	time =	12	sec	GM +	9	sec F	GM =	21	sec	
			Fp	2		min c	rossing	time =	13	sec	GM +	12	sec F	GM =	25	sec	
	4-			DM T#	<b></b>										b1-4		
AM Traffic Flow (pcu/h	r)		N	PM Trainc	Flow (pcu/hr)	)			N		00(W-3.25		S=2080+10		Note:		
			,						ĺ.	S <sub>M</sub> =S÷(1+	1.5f/r)	S	S <sub>M</sub> =(S–230)	÷(1+1.5f/r)			
	▶ 1964		١			1710			١			Peak		Peak			
											1+3		1+3				
	123	33 🔶					1286	←		Sum y	0.495		0.482				
					763 ◄	<b>⊢</b> ,→	252			L (s)	40 118		40 108				
650					705		252			C (s)	0.595		0.567				
650	334												0.007				
650	334									practical y R.C. (%)	20%		18%				
1	334	2				3							18%				
650	334	2				3							18%				
1 A1 ───► A2 ───►	334	ŧ				3							18%				
$\begin{array}{c} 650 \\ \hline 1 \\ A1 \longrightarrow \\ A2 \longrightarrow \\ A3 \longrightarrow \end{array}$			Fp		Ep	3							18%				
1 A1 ───► A2 ───►		B3 B2	Fp	4	Ep	3	*ๅ* [*						18%				
1 A1 ───► A2 ───►		вз	Fp 4Dp	•	Ep		← ← ←						18%				
1 A1 → A2 A3 → D <sub>1</sub>		B3 B2 B1	<b>∢</b> Dp		,	C1	← → → <u>C2 C3</u> G =		I/G =	R.C. (%)	20%				G =		
$\begin{array}{c} 1 \\ A1 \longrightarrow \\ A2 \longrightarrow \\ A3 \longrightarrow \end{array}$		B3 B2 B1 = 7	<b>4</b>		Ep , //G = //G =	C1	G = G =		1/G = 1/G =	R.C. (%)			18% I/G = I/G =		G = G =		
1 A1 A2 A3 A3 AM G =		B3 B2 B1 = 7	<b>∢</b> Dp G =	25	, I/G =	C1	G =			R.C. (%)	20% G =		I/G =				

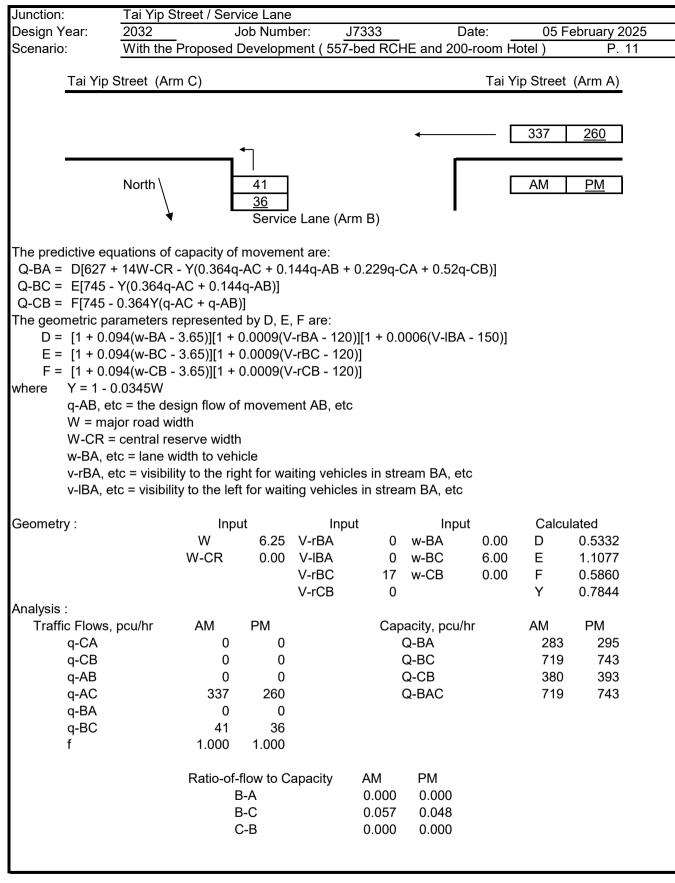
## **Priority Junction Analysis**

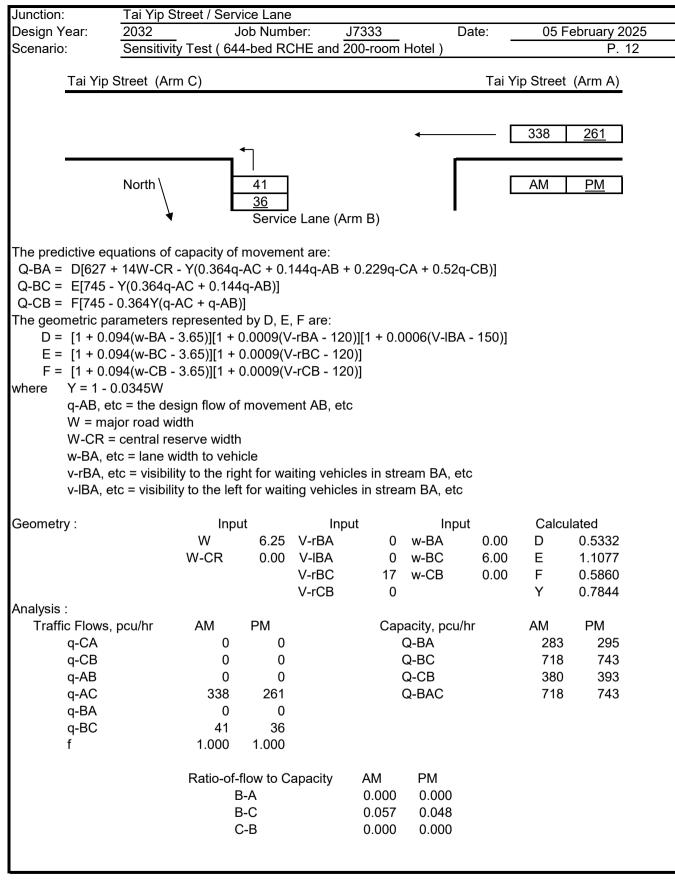


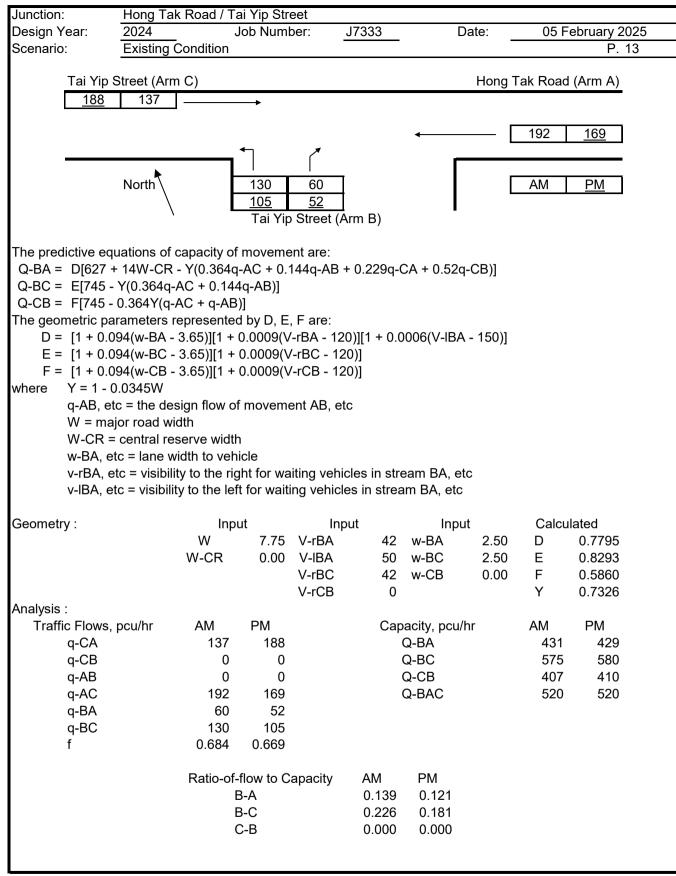
## **Priority Junction Analysis**

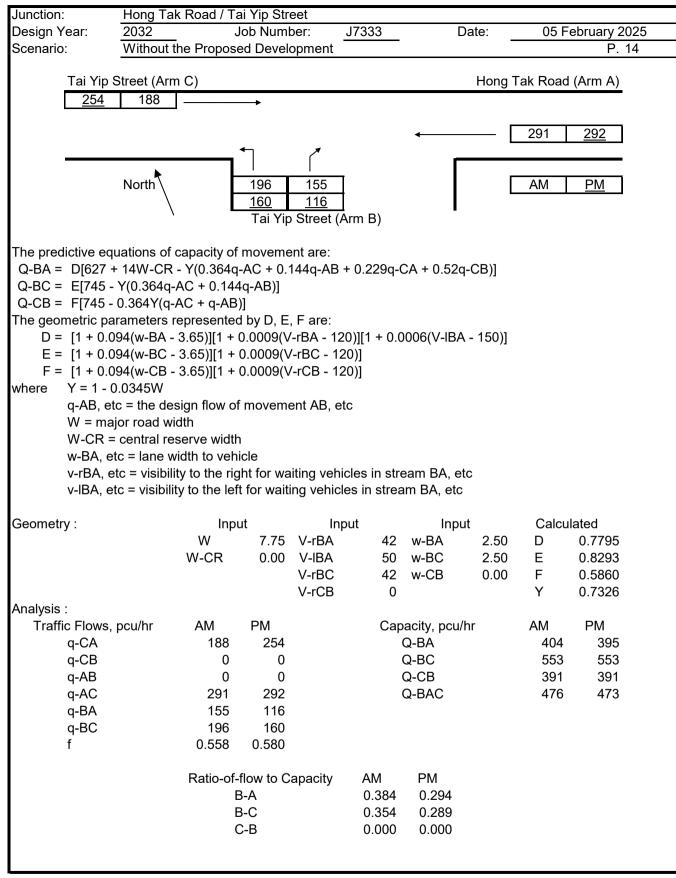


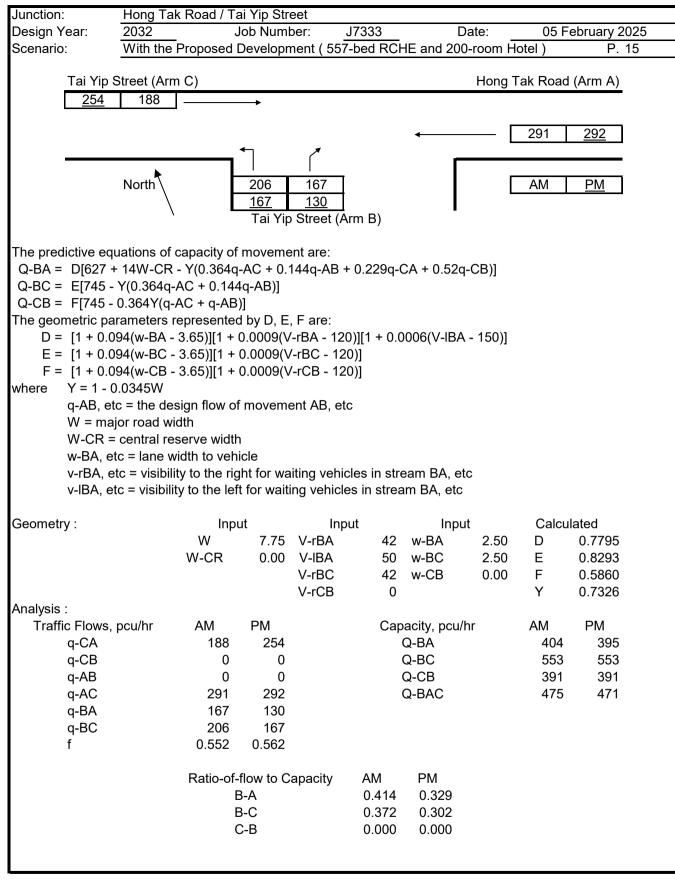
## **Priority Junction Analysis**

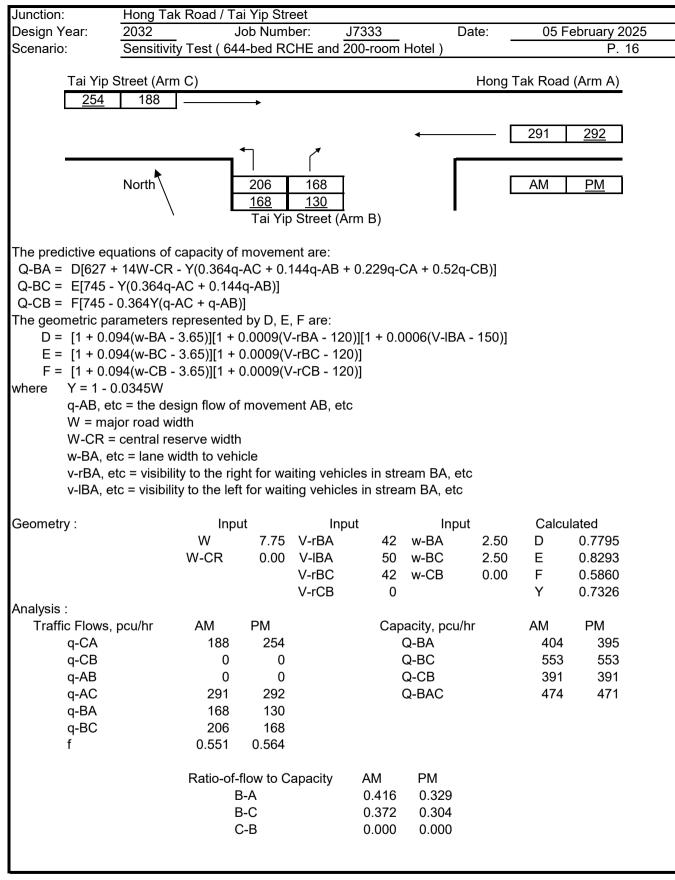


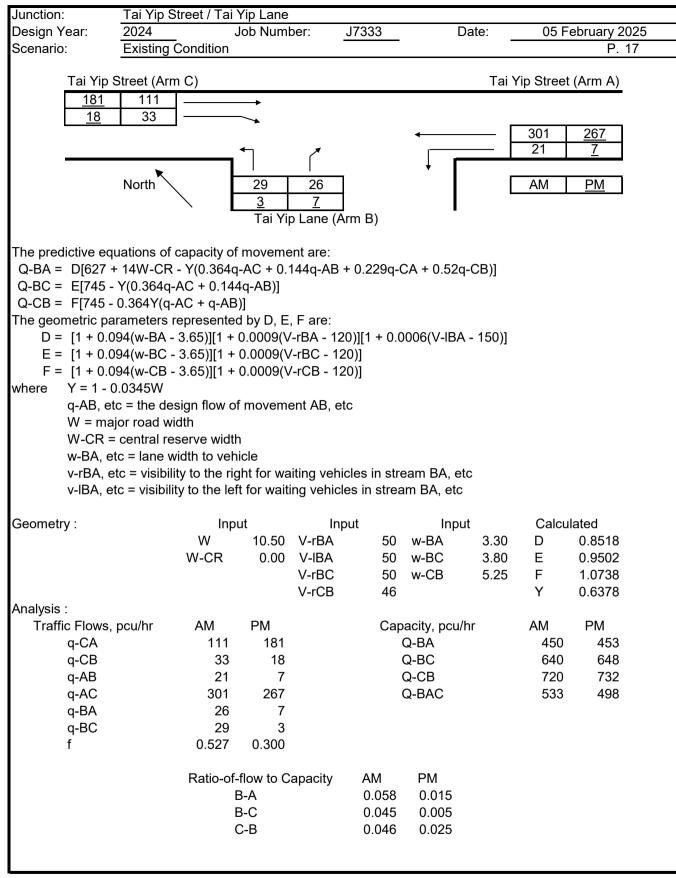


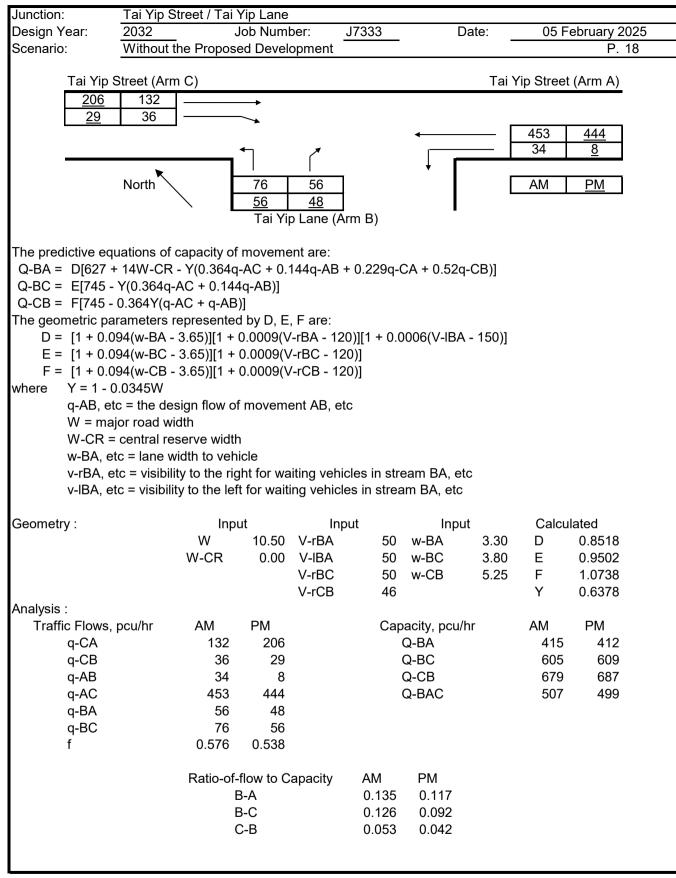


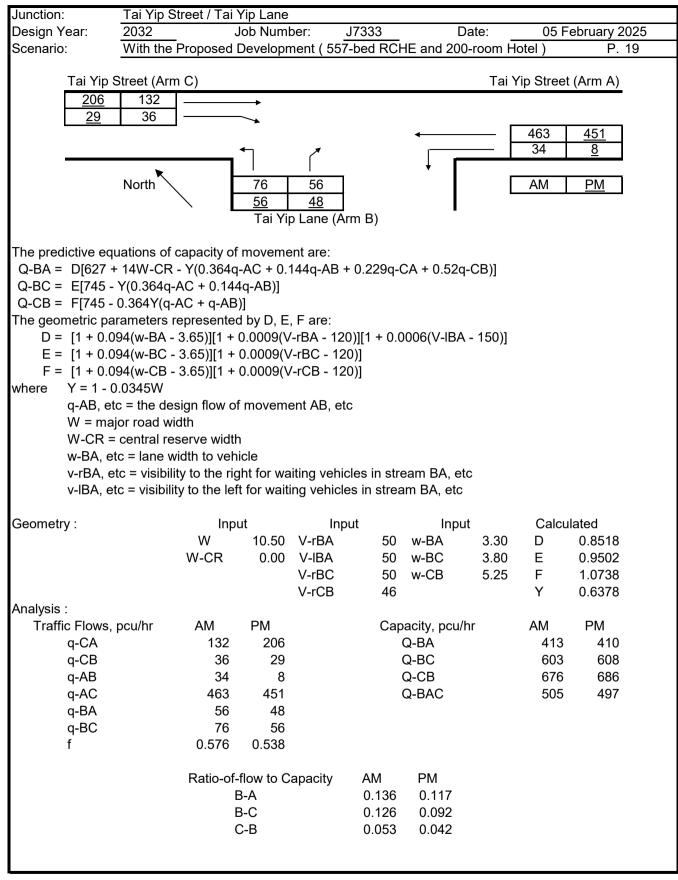


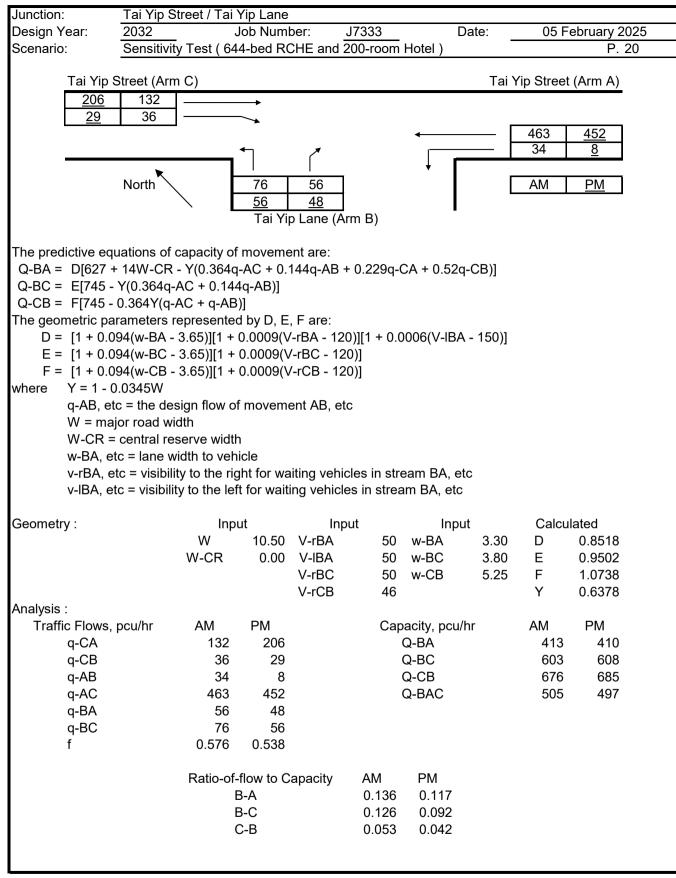


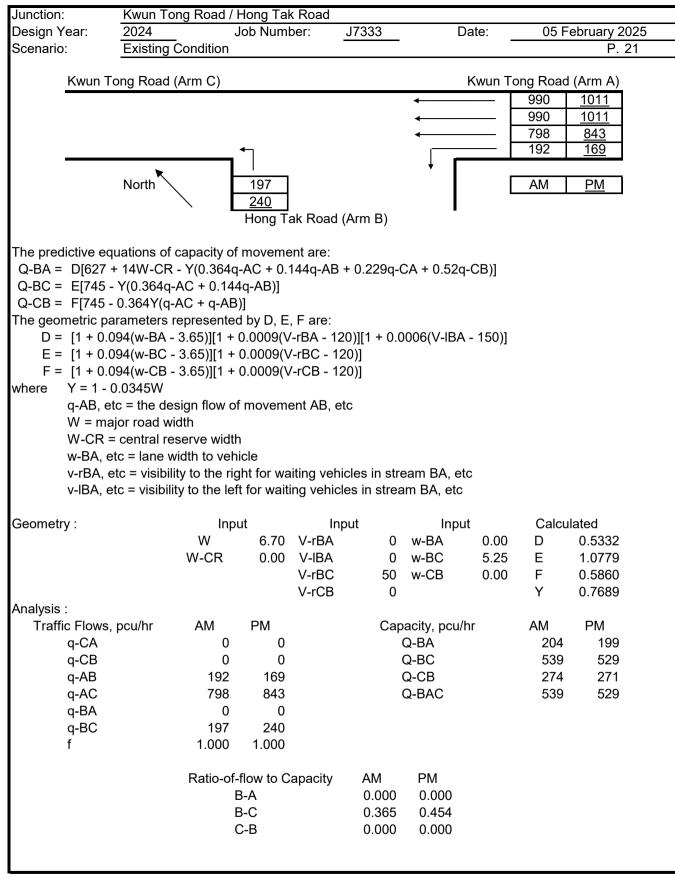


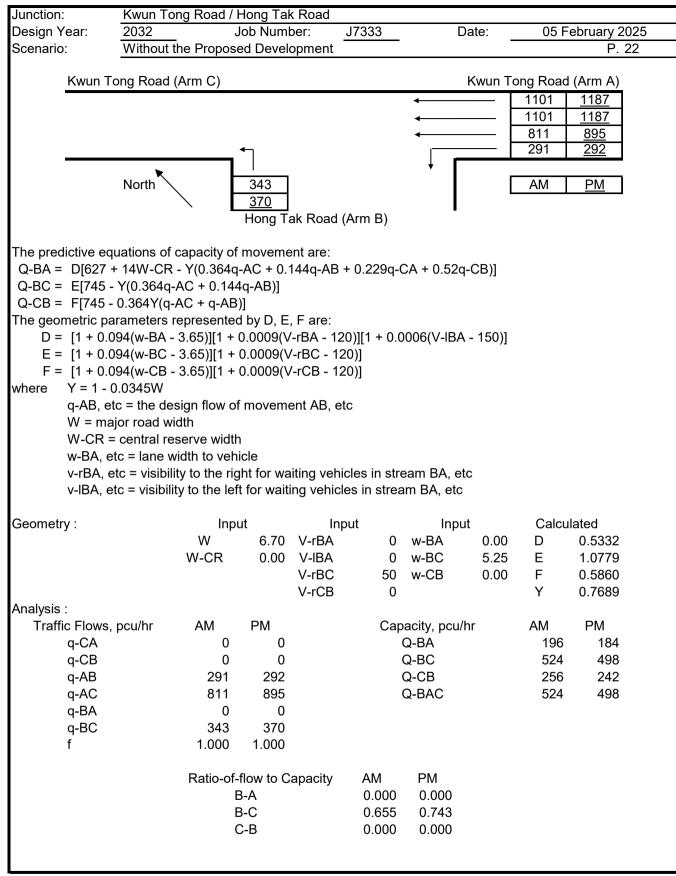


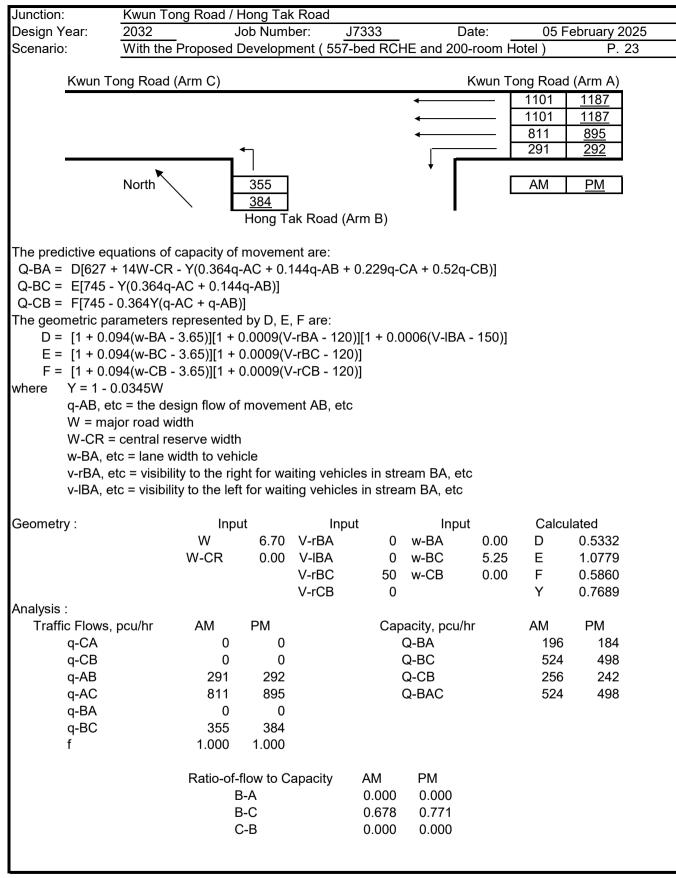


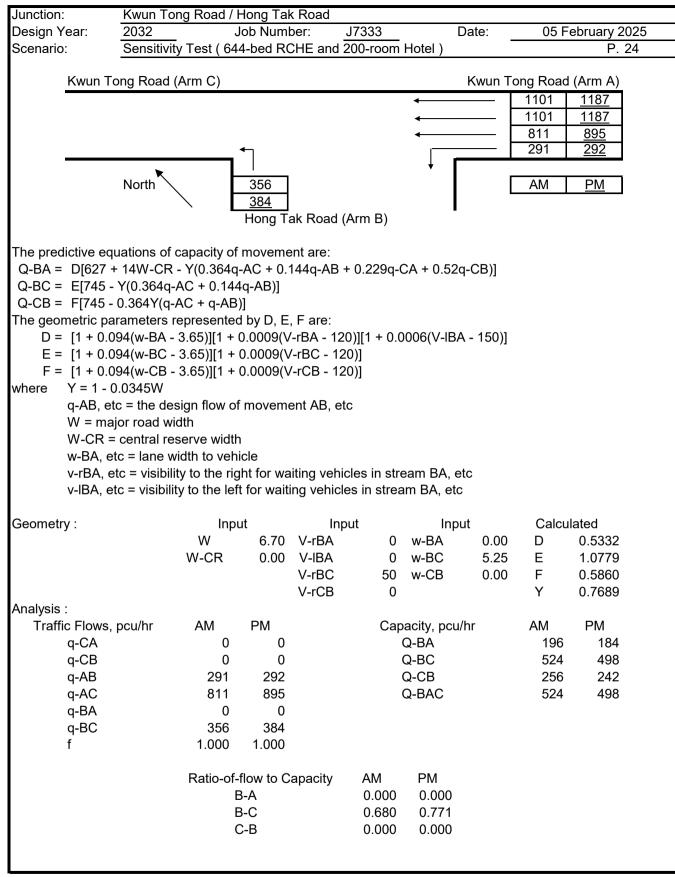












						-			-								
Junction:	Wai Yip S	treet / Lai Yip	Street											-	Job Nu	mber:	J7333
Scenario:	Existing C															P.	25
Design Year:	2024	Designe	ed By:					Checke	d By:					Date:	5 Fe	ebruary	2025
					1					AM Peak				-	PM Peak	_	-
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Wai Yip Street	t WB*	LT	A1	3	2.80	20.0		100	1763	75	0.043		100	1763	107	0.061	
		SA	A2	3	2.80				2035	386	0.190			2035	409	0.201	
		SA	A3	3	2.80				2035	385	0.189			2035	409	0.201	
Lai Yip Street	SB	LT	B1	2	3.10	20.0		100	1971	167	0.085		100	1991	211	0 106	0.106
	00	SA	B2	1,2	3.10	20.0		100	2185	370	0.169	0.169	100	2198	223	0.100	0.100
		SA	B3	1,2	3.10				2065	349	0.169	0.100		2065	209	0.101	
				,													
Wai Yip Street	t EB	SA+LT	C1	3	3.30	20.0		63	2097	575	0.274		50	2142	489	0.228	0.229
		SA	C2	3	3.30				2085	572	0.274	0.274		2085	476	0.228	
		SA	C3	3	3.30				2085	571	0.274			2085	477	0.229	
Lai Yip Street	NB	SA+LT	D1	1,2	3.80	20.0		44	2111	184	0.087		70	2095	202	0.096	
		SA	D2	1,2	3.80				2135	187	0.088			2135	206	0.096	
pedestrian pha	ase		Ep	3			rossing		11		GM +	10		GM =	21	sec	
			Fp	1,2			rossing		7		GM +	11	sec F		18	sec	
			Gp	1,2			rossing		5		GM +	10		GM =	15	sec	
			Hp Ip	1,3 3			rossing rossing		5 5		GM + GM +	7 7		GM = GM =	12 12	sec sec	
			ιp	5		minu	lossing	ume –	5	360		1	3601	GIVI -	12	360	
AM Traffic Flow (pcu/h	r)		N	PM Traffic	Flow (pcu/hr)	1	1		N	S=1940+1	00(W-3.25	) 5	S=2080+10	0(W-3.25)	Note:		
	Ļ	→ 167	N K				<b>_</b>	211	Ν Λ	S <sub>M</sub> =S÷(1+			<sub>M</sub> =(S-230)			orary Tr	
361	Ļ		$\setminus$		243		Ļ		$\setminus$		AM	Peak	PM	Peak	•	ement is s at the	
Ť	71	19			†		432				2+3	1,2+3	2+3	1,2+3			
$\longrightarrow$	1357				_ <b>_</b>	1199				Sum y	0.359	0.444	0.335	0.330			
	290	771	-			267	818	<b>↓</b>	-	L (s)	33	20	33	20			
81	1 +	75			141	•		107		C (s)	120	120	108	108			
										practical y	0.653	0.750	0.625	0.733			
										R.C. (%)	82%	69%	87%	122%			
1	B3 B2	2		B3 B2 B	1	3											
Gp 4		Hp Gp	*				↑	Ep	Нр								
Fp	** [	Fp	1	+ + + +	<b>→</b>	C1 C2	<b>→</b>	_	4.2								
<b>`</b> ††	Fp	<b>N</b> .	<b>†</b> †	Fp 🔻	****	С3	-		A3 A2 A1								
	Gp		▲		Gp <sup>™</sup>	lp. ▲	. <b>* 4</b> Ep	▶ ↓	—— A1								
D1 D2			D1 D2			_											
	- 11	I/G = 2	G =		I/G =		G =		I/G =		G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G = G =	- 11	I/G = 2 I/G =	G = G =		I/G = I/G =		G = G =		I/G = I/G =		G = G =		I/G = I/G =		G = G =		
G	-		62		1/G =	15	62		1/6 =	J	6 =		1/G =		6=		

-					-	J	Junci										
Junction:	Wai Yip St	reet / Lai Yip	Street											-	Job Nu	mber:	J7333
Scenario:		e Proposed D															26
Design Year:	2032	Designe	ed By:					Checke	d By:					Date:	5 Fe	ebruary 2	2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
							Gradient		(pcu/hr)	(pcu/hr)		Ontioar y		(pcu/hr)	(pcu/hr)		Ontioar y
Wai Yip Street	t WB	SA+LT	A1	3	2.80	20.0		53	1943	422	0.217		62	1944	445	0.229	
		SA SA	A2 A3	3	2.80				2035 2035	442 442	0.217			2035 2035	466 465	0.229	
		34	AS	3	2.80				2035	442	0.217			2035	405	0.229	
Lai Yip Street	SB	LT	B1	2	3.10	20.0		100	1971	292	0.148		100	1991	263	0.132	0.138
		SA	B2	1,2	3.10				2185	505	0.231			2198	389	0.177	
		SA	B3	1,2	3.10				2065	478	0.231			2065	365	0.177	
Wai Yip Street	t EB	SA+LT	C1	3	3.30	20.0		60	2101	699	0.333	0.333	61	2127	611	0.287	0.287
		SA	C2	3	3.30				2085	694	0.333			2085	599	0.287	
		SA	C3	3	3.30				2085	693	0.332			2085	599	0.287	
		<u></u>	<b>D</b> (		0.00	45.0			0000	000	0.400	0.4.46		0077	000	0.400	
Lai Yip Street	NR	SA+LT	D1	2	3.80	15.0		58	2066	286	0.138	0.148	63	2077	286	0.138	
		SA	D2	2	3.80				2135	295	0.138			2135	294	0.138	
pedestrian pha	ase		Ep	3		min c	rossing	time =	11	sec	GM +	10	sec F	GM =	21	sec	
			Fp	1			rossing		7		GM +	11		GM =	18	sec	
			Gp	1,2			rossing		5		GM +	10		GM =	15	sec	
			Нр	1,3		min c	rossing	time =	5	sec	GM +	7	sec F	GM =	12	sec	
AM Traffic Flow (pcu/h	ir)			PM Traffic	Flow (pcu/hr)		1			S=1940+1	00(W-3.25	) 9	6=2080+10	0/W-3 25)	Note:		
		→ 292	N K					263	N K	S <sub>M</sub> =S÷(1+			M=(S-230)			Improver	
416	Ļ				373		Ļ		$\setminus$			Peak		Peak	Scheme	by Other	Project
+10 ↑	983	3			†		754				2+3	1,2+3	2+3	1,2+3			
	1670					1436				Sum y	0.481	0.564	0.425	0.464			
	416 1	083 -	-			400	1099	•	-	L (s)	39	10	39	10			
165	5 ←	223			180	⊷		277		C (s)	120	120	108	108			
										practical y	0.608	0.825	0.575	0.817			
										R.C. (%)	26%	46%	35%	76%			
1	B3 B2	2		B3 B2 B	1	3											
Gpi▲		Hp Gp	4				<b>↑</b> .	Ep	▲								
Fp	+ + 1			+ + + +	→	C1 C2	→ →										
•	Fp		11	Fp 🕇	*****	C3	<b>→</b>	+	A3 A2								
	Gp	•	•		Gp▲		<b>_</b> Ep	·····•	↓ — A1								
			D1 D2														
		I/G = 5	G =		I/G =		G =		I/G =		G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
		I/G = 5	G =		I/G =		G =		I/G =		G =		I/G =		G =		
G =	-	I/G =	G =		I/G =	υ	G =		I/G =	υ	G =		I/G =		G =		

						9											
Junction:	Wai Yip Str	reet / Lai Yip	Street												Job Nu	mber:	J7333
Scenario:		oposed Dev							<i>,</i>								27
Design Year:	2032	Designe	ed By:					Checke	d By:				-	Date:	5 Fe	ebruary 2	2025
	Approach		Phase	Stage	Midth (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
	Approach		Phase	Slage	width (m)	Radius (III)	% Op-fill Gradient	running %	(pcu/hr)	(pcu/hr)	y value	Chucary	Turning %	(pcu/hr)	(pcu/hr)	y value	Chucary
Wai Yip Stree	t WB	SA+LT	A1	3	2.80	20.0		53	1943	422	0.217		62	1944	445	0.229	
		SA	A2	3	2.80				2035	442	0.217			2035	466	0.229	
		SA	A3	3	2.80				2035	442	0.217			2035	465	0.229	
Lai Yip Street	SB	LT	B1	2	3.10	20.0		100	1971	292	0.148		100	1991	263	0 132	0.138
Lai rip otreet	00	SA	B2	1,2	3.10	20.0		100	2185	506	0.232		100	2198	390	0.132	0.100
		SA	B3	1,2	3.10				2065	479	0.232			2065	366	0.177	
Wai Yip Stree	t EB	SA+LT	C1	3	3.30	20.0		60	2101	701	0.334		61	2127	613	0.288	0.289
		SA	C2	3	3.30				2085	696	0.334	0.334		2085	601	0.288	
		SA	C3	3	3.30				2085	695	0.333			2085	602	0.289	
Lai Yip Street	NB	SA+LT	D1	2	3.80	15.0		58	2066	286	0.138	0.148	63	2077	286	0.138	
		SA	D2	2	3.80				2135	295	0.138			2135	294	0.138	
nodoctrian phy	200		En	3		min o	roccing	timo -	11		GM +	10	000 F	GM =	21		
pedestrian pha	456		Ep Fp	1			rossing rossing		<u>11</u> 7		GM +	10 11		GM =	18	sec sec	
			Gp	1,2			rossing		5		GM +	10	sec F		15	sec	
			Hp	1,3			rossing		5		GM +	7		GM =	12	sec	
														-			
AM Traffic Flow (pcu/h	nr)		N	PM Traffic	Flow (pcu/hr)	1	1		N	S=1940+1	00(W-3.25	) :	S=2080+10	0(W-3.25)	Note:		
	_	→ 292	5					263	7	S <sub>M</sub> =S÷(1+	1.5f/r)	s	S <sub>M</sub> =(S−230)	÷(1+1.5f/r)		Improver by Other	
418	Ļ		$\setminus$		376		ţ		$\setminus$		AM	Peak	PM	Peak	Scheme	by Other	Project
t i	985	5			†		756				2+3	1,2+3	2+3	1,2+3			
	1674				_ <b>_</b>	1440				Sum y	0.482	0.566	0.426	0.466			
	416 1	083	-			400	1099	<b>←</b>	-	L (s)	39	10	39	10			
16	5 🛶	223			180	•		277		C (s)	120	120	108	108			
										practical y	0.608	0.825	0.575	0.817			
	•									R.C. (%)	26%	46%	35%	75%			
1	B3 B2	2		B3 B2 B	1	3											
Gp 🍕	A	Hp Gp	4				<b>↑</b>	Ep	▲								
Fp	+ + 🛉			+ + +	<b>→</b>	C1 C2	<u>→</u>										
*	Fp .		<u>†</u> †	Fp 🕇	••••	C3	<b>→</b>	+ +	A3 A2								
	Gp	•			Gp▲		<b>_</b> Ep	·····•	A1								
			I I D1 D2														
AM G =	= 18	I/G = 5	G =		I/G =	12	G =		I/G =	6	G =		I/G =		G =		
G =	=	I/G =	G =		I/G =	6	G =		I/G =	6	G =		I/G =		G =		
PM G =	= 18	I/G = 5	G =		I/G =	12	G =		I/G =	6	G =		I/G =		G =		
	=	I/G =	G =		I/G =	6	G =		I/G =	6	G =		I/G =		G =		

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Junction:	Wai Yip Str	reet / Lai Yip	Street												Job Nu	mber:	J7333
Scenario:		Test ( 644-be															28
Design Year:	2032	Designe	ed By:				-	Checke	d By:				-	Date:	5 Fe	ebruary 2	2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
	Арргоаст		FlidSe	Stage	wider (iii)	Radius (III)	Gradient	runnig %	(pcu/hr)	(pcu/hr)	y value	Chucary	running 70	(pcu/hr)	(pcu/hr)	y value	Critical y
Wai Yip Stree	t WB	SA+LT	A1	3	2.80	20.0		53	1943	422	0.217		62	1944	445	0.229	
		SA	A2	3	2.80				2035	442	0.217			2035	466	0.229	
		SA	A3	3	2.80				2035	442	0.217			2035	465	0.229	
Lai Yip Street	SB	LT	B1	2	3.10	20.0		100	1971	292	0.148		100	1991	263	0 132	0.138
	00	SA	B2	1,2	3.10	20.0		100	2185	506	0.232		100	2198	390	0.177	0.100
		SA	B3	1,2	3.10				2065	479	0.232			2065	366	0.177	
Wai Yip Stree	t EB	SA+LT	C1	3	3.30	20.0		60	2101	701	0.334		61	2127	613	0.288	0.289
		SA	C2	3	3.30				2085	696	0.334	0.334		2085	601	0.288	
		SA	C3	3	3.30				2085	695	0.333			2085	602	0.289	
Lai Yip Street	NB	SA+LT	D1	2	3.80	15.0		58	2066	286	0.138	0.148	63	2077	286	0.138	
		SA	D2	2	3.80				2135	295	0.138			2135	294	0.138	
nodoctrion phy	200		En	3		mino	roccing	timo -	11		GM +	10	000 F	GM =	21		
pedestrian pha	456		Ep Fp	1			rossing rossing		<u>11</u> 7		GM +	10 11		GM =	18	sec sec	
			Gp	1,2			rossing		5		GM +	10	sec F		15	sec	
			Hp	1,3			rossing		5		GM +	7		GM =	12	sec	
				· · ·													
AM Traffic Flow (pcu/h	nr)		N	PM Traffic	Flow (pcu/hr)	)	1		N	S=1940+1	00(W-3.25	) :	S=2080+10	0(W-3.25)	Note:		
	_	→ 292	7				-	263	7	S <sub>M</sub> =S÷(1+	1.5f/r)	s	6 <sub>M</sub> =(S−230)	÷(1+1.5f/r)		Improver	
418	Ļ				376		Ļ		$\setminus$		AM	Peak	PM	Peak	Scheme	by Other	Project
t i	985	5			†		756				2+3	1,2+3	2+3	1,2+3			
	1674				_ <b>_</b>	1440				Sum y	0.482	0.566	0.426	0.466			
	416 1	083	-			400	1099	<b>←</b>	-	L (s)	39	10	39	10			
165	5 🛶	223			180	•		277		C (s)	120	120	108	108			
										practical y	0.608	0.825	0.575	0.817			
	•									R.C. (%)	26%	46%	35%	75%			
1	B3 B2	2		B3 B2 B	1	3											
Gp ⁴	A	.▼ ►. Hp Gp	4				<b>↑</b>	Ep	▲								
Fp	+ + †			+ ↓ ▲ Ι	<b>→</b>	C1 C2	<b>→</b>		·								
*	Fp .		<u>†</u> †	Fp 🕇	••••	C3	<b>→</b>	<b>↓</b>	A3								
	Gp	•			Gp▲		<b></b> ∎ Ep	·····•	A1								
			I I D1 D2														
AM G =	= 18	I/G = 5	G =		I/G =	12	G =		I/G =	6	G =		I/G =		G =		
G =	=	I/G =	G =		I/G =	6	G =		I/G =	6	G =		I/G =		G =		
PM G=	= 18	I/G = 5	G =		I/G =	12	G =		I/G =	6	G =		I/G =		G =		

$\begin{array}{c c c c c c c c c c c c c c c c c c c $							-											
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Junction:	· · · · ·		Yip Stre	et										-	Job Nu		J7333
$\begin{array}{c c c c c c c c c c c c c c c c c c c $				ed By:					Checke	d By:				<u>-</u>	Date:	5 Fe		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $									<b>T</b> 1 0/	0.1.5				<b>T</b> 1 00	0.15			
$\begin{array}{c c c c c c c c c c c c c c c c c c c $		Approach		Phase	Stage	Width (m)	Radius (m)		Turning %			y value	Critical y	Turning %			y value	Critica
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Kwun Tong Ro	ad EB				3.20					351							
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			SA	A2	1,2	3.20				2075	377	0.182	0.182		2075	308	0.148	
$\begin{split} \begin{array}{c c c c c c c c c c c c c c c c c c c $	_ai Yip Street N	NB	LT+SA	B1	5	3.50	30.0		31	2118	480	0.227		63	2105	387	0.184	0.18
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			SA	B2	5	3.50				2105	477	0.227	0.227		2105	386	0.183	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Elegance Road	I NB	SA	B3	5	3.50				2105	278	0.132			2105	184	0.087	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			SA+RT	B4	5	3.50	18.0		9	2089	276	0.132		37	2042	179	0.088	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			RT	B5	5	3.50	15.0		100	1914	253	0.132		100	1914	167	0.087	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Kwun Tong Ro	ad WB	LT	C1	1.5	3.30	15.0		100	1768	365	0.206		100	1768	167	0.094	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	0																	0.22
$\begin{array}{c c c c c c c c c c c c c c c c c c c $																		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $			1.7	D4	2.4	2.50	45.0		100	4700	450	0.000	0.000	400	4700	404	0.404	0.40
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Elegance Road	158					15.0		100				0.088	100				0.10
$\begin{array}{c c c c c c c c c c c c c c c c c c c $							10.0		15					47				
$\frac{1}{10000000000000000000000000000000000$																		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $				04	3,4	5.50	15.0		100	1314	150	0.003		100	1914	120	0.007	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $																		
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	pedestrian pha	se		Fp	12		min c	rossina	time =	12	sec	GM +	10	sec F	GM =	22	sec	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $																		
$\begin{array}{c} 184 \\ + 158 \\ 319 \\ - 728 \\ - 728 \\ - 729 \\ - 529 \\ 150 \\ - 278 \end{array} \begin{array}{c} 191 \\ + 181 \\ 212 \\ - 596 \\ - 506 \\ - 50$				Gp			min c	rossing	time =	5	sec	GM +	5	sec F	GM =	10	sec	
$\begin{array}{c} 184 \\ + 158 \\ 319 \\ - 728 \\ - 728 \\ - 729 \\ - 529 \\ 150 \\ - 278 \end{array} \begin{array}{c} 191 \\ + 181 \\ 212 \\ - 596 \\ - 506 \\ - 50$																		
$\begin{array}{c} 184 \\ + 158 \\ 319 \\ - 728 \\ - 728 \\ - 729 \\ - 529 \\ 150 \\ - 278 \end{array} \begin{array}{c} 191 \\ + 181 \\ 212 \\ - 596 \\ - 506 \\ - 50$																		
$\begin{array}{c} 184 \\ + 158 \\ 319 \\ - 728 \\ - 728 \\ - 729 \\ - 529 \\ 150 \\ - 278 \end{array} \begin{array}{c} 191 \\ + 181 \\ 212 \\ - 596 \\ - 506 \\ - 50$																		
$\begin{array}{c} 184 \\ + 158 \\ 319 \\ - 728 \\ - 728 \\ - 729 \\ - 529 \\ 150 \\ - 278 \end{array} \begin{array}{c} 191 \\ + 181 \\ 212 \\ - 596 \\ - 506 \\ - 50$																		
$ \begin{array}{c} \begin{array}{c} \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} $	AM Traffic Flow (pcu/hr)	)		N	PM Traffic I	low (pcu/hr)				N	S=1940+1	00(W–3.25	) :	S=2080+10	0(W–3.25)			
$ \begin{array}{c} & \longrightarrow & 728 \\ & & & & & & & & & & & & & & & & & & $		184 🕂	158	1			191	$\leftrightarrow$	181	1	S <sub>M</sub> =S÷(1+	1.5f/r)	s	S <sub>M</sub> =(S–230)	÷(1+1.5f/r)	1) In AM Sequenc	Peak, Sta e : 2>4>5	age >2
$\begin{array}{c} 10^{\circ} \\ 729 \\ 150 \\ \hline \\ 529 \\ 150 \\ \hline \\ 278 \end{array} \qquad \begin{array}{c} 729 \\ 953 \\ 297 \\ 243 \\ \hline \\ 297 \\ 167 \\ \hline \\ (s) \\ 18 \\ 108 \\ \hline \\ practical y \\ 0.786 \\ 0.733 \\ \hline \\ (c. (\%) \\ 58\% \\ 43\% \\ \hline \\ B3 B4 B5 \\ \hline \\ B1 B2 \\ \hline \\ \\ \\ B1 B2 \\ \hline \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $			9	١				212		\		AM	Peak	PM	Peak			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		728			-		596					2+4+5		2+3+5		Sequenc	e : 2>3>5	>2
$\begin{array}{c c c c c c c c c c c c c c c c c c c $											Sum y	0.497		0.512				
$150 + 278$ $243 + 233$ $R.C.(%) \overline{58\%} \overline{43\%}$ $R.C.(%) \overline{58\%} \overline{56\%}$ $R.C.(%) \overline{58\%} \overline{56\%}$ $R.C.(%) \overline{56\%}$ $R.$			ŧ	_				953	ŧ	-	L (s)							
$A^{1} \longrightarrow C^{3} \\ A^{2} \longrightarrow C^{3} \\ F_{P} \longrightarrow C^{1} \\ C^{2} \\ F_{P} \longrightarrow C^{1} \\ C^{2} \\ C^{2$			365				▲		167		C (s)							
$A_{2} \xrightarrow{A_{1}} \underbrace{\longrightarrow}_{C_{2}} \underbrace{C_{3}}_{C_{2}} \xrightarrow{A_{1}} \underbrace{\longrightarrow}_{C_{2}} \underbrace{C_{3}}_{F_{p}} \underbrace{\bigoplus}_{F_{p}} \underbrace{C_{3}}_{G_{2}} \xrightarrow{C_{3}} \underbrace{\bigoplus}_{F_{p}} \underbrace{\bigoplus}_{F_{p}} \underbrace{\bigoplus}_{G_{p}} \underbrace{\bigoplus}_{F_{p}} \underbrace{\bigoplus}_{F_{p$	150	← → 278				243	←   →	233			practical y							
$A1 \longrightarrow A2 \longrightarrow C3 \longrightarrow C2 \\ F_{P} \longrightarrow C1 \\ M \\ $		I									R.C. (%)	58%		43%				
$A^{2} \longrightarrow G^{2} \oplus G^{2$	1	<b>}</b>					3	D4 D3	D2 D1		4	D4 D3	D2 D1		5			
$F_{P} \leftarrow C_{1} \leftarrow C_{2} \leftarrow F_{P} \leftarrow F_{P$															↑ <b> </b> •[	<b>→</b>		
$F_{P} \leftarrow E_{P} \leftarrow C^{1} \qquad F_{P} \leftarrow E_{P} \leftarrow G_{P} \qquad F_{P} \leftarrow G_{P} \qquad F_{P} \leftarrow C^{1} \qquad f_{B1} = D \qquad f$				$\leftarrow$				₹	Ļ			-	Ļ		B3 B4 B5			
Image: Contract of the second secon	Fp. <b>€</b> p	Ep C1				02	Fp		Gp ▼.		Fp <sub>.</sub>				<b>⊷</b> †↑		Ļ	— C1
	<u> </u>				··· <b>·A</b>		<b>*</b> ****		·****		<b>*</b> *							
PM 1/G = 1/G = 7 1/G = 7 1/G = 6 1/G = 3	AM	1/0	G =			I/G =				I/G =	5			I/G =	10		I/G =	3
	PM	1/0	G =			I/G =	7			I/G =	7			I/G =	6		I/G =	3

						•			,								
Junction:	Kwun Tong	Road / Lai `	Yip Stre	et										-	Job Nu		J7333
Scenario:	Without the																30
Design Year:	2032	Designe	ed By:					Checke	ed By:				-	Date:	5 Fe	ebruary 2	2025
										AM Peak				1	PM Peak	1	
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Kwun Tong Ro	ad EB	SA	A1	1,2	3.20				1935	379	0.196			1935	316	0.163	
		SA	A2	1,2	3.20				2075	406	0.196			2075	338	0.163	
Lai Yip Street I	NB	LT	B1	5	3.30	30.0		100	2035	157	0.077		100	2052	288	0.140	
		SA	B2	5	3.30				2085	523	0.251			2085	425	0.204	
		SA	B3	5	3.30				2085	522	0.250			2085	425	0.204	
Elegence Dee		64	D4	F	2.50				2105	250	0 171			2105	204	0.140	
Elegance Road	מאו נ	SA SA+RT	B4 B5	5 5	3.50 3.50	18.0		3	2105 2100	359 359	0.171 0.171		18	2105 2074	294 289	0.140	
		RT	во В6	5	3.50	15.0		3 100	1914	327	0.171		100	1914	269	0.139	
		NI	БО	5	5.50	15.0		100	1914	521	0.171		100	1914	207	0.139	
Kwun Tong Ro	ad WR	LT	C1	1,5	3.30	15.0		100	1768	575	0.325	0.325	100	1768	403	0.228	0.228
rtwair rong rte		SA	C2	1,0	3.50	10.0		100	2105	433	0.206		100	2105	573	0.272	0.272
		SA	C3	1,2	3.50	1	1	İ	2105	432	0.205			2105	573	0.272	
				- ,													
Elegance Road	d SB	LT	D1	3,4	3.50	15.0		100	1786	195	0.109	0.109	100	1786	216	0.121	0.121
		SA	D2	3,4	3.50				2105	224	0.106			2105	178	0.085	
		SA+RT	D3	3,4	3.50	18.0		40	2037	217	0.107		65	1997	169	0.085	
		RT	D4	3,4	3.50	15.0		100	1914	203	0.106		100	1914	162	0.085	
pedestrian pha	ISE		Ep	1,2		min c	rossing	time =	12	sec	GM +	10	sec F	GM =	22	sec	
			Fp	1,2,3,4		min c	rossing	time =	5	sec	GM +	7	sec F	GM =	12	sec	
			Gp	2,3		min c	rossing	time =	5	sec	GM +	5	sec F	GM =	10	sec	
AM Traffic Flow (pcu/hr			Ν	PM Traffic I	Flow (pcu/hr)				Ν	S=1940+1	00(W–3.25	) :	S=2080+10	· ,	Note:		
	291	195	1			272	ŧ	216	1	S <sub>M</sub> =S÷(1+	1.5f/r)	s	6 <sub>M</sub> =(S–230)	÷(1+1.5f/r)		ion Improv by Other	
	35	3	١				237		1		AM	Peak	PM	Peak	2) In AM	Peak, Sta	age
	785			_		654					2+4+5		2+3+5		Sequend	ce : 2>4>5	5>2
		NOF 4	_				4440		_	Sum y	0.640		0.621			Peak, Sta ce : 2>3>5	
		365					1146	¥	_	L (s)	15		20		ocquerie	. 2- 0- 0	<i>F</i> Z
(57	707	575				531 1	0.40	403		C (s)	118		108				
157	338				288		319			practical y	0.786		0.733				
		la								R.C. (%)	23%		18%	-			
1		2				3	D4 D3	D2 D1		4	D4 D3	D2 D1		5			
A1 A2		A1 — A2 —												141	<b>→</b>		
-	ca	3	•		— <sub>C3</sub>		┥				┥			B3 B4 B5			
<u>-</u>		2	<u>-</u>		— C2		*	•			*	•		4 ↑ ↑ ↑			— C1
Fp,	► †	Fp	Ep	Gp		Fp▼		Gp		Fp▼						ŧ	0.
*				•				•						B1 B2 B	3		
АМ	1/	G =			I/G =				I/G =	5			I/G =	10		I/G =	3
РМ	1/	G =			I/G =	7			I/G =	7			I/G =	6		I/G =	3

						0											
Junction:	Kwun Tong F														Job Nu	mber:	
Scenario:	With the Pro																31
Design Year:	2032	Designe	ed By:					Checke	d By:				-	Date:	5 Fe	ebruary	2025
	Approach		Phase	Stage	Midth (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
				-		Radius (III)	Gradient	Turning %	(pcu/hr)	(pcu/hr)	y value	Chucary	Turning %	(pcu/hr)	(pcu/hr)	y value	Critical y
Kwun Tong Ro	ad EB	SA	A1	1,2	3.20				1935	379	0.196			1935	316	0.163	
		SA	A2	1,2	3.20				2075	406	0.196			2075	338	0.163	
		1.7	D4	-	2.20	20.0		100	2025	457	0.077		100	2052	200	0.440	
Lai Yip Street I	NB	LT SA	B1 B2	5 5	3.30 3.30	30.0		100	2035 2085	157 524	0.077		100	2052 2085	288 427	0.140	
		SA	B3	5	3.30				2085	523	0.251			2085	426	0.203	
		0,1	20		0.00				2000	020	0.201			2000	.20	0.201	
Elegance Road	d NB	SA	B4	5	3.50				2105	360	0.171			2105	295	0.140	
		SA+RT	B5	5	3.50	18.0		3	2100	359	0.171		18	2074	290	0.140	
		RT	B6	5	3.50	15.0		100	1914	328	0.171		100	1914	268	0.140	
Kwun Tong Ro	oad WB	LT	C1	1,5	3.30	15.0		100	1768	577		0.326	100	1768	405	0.229	
		SA	C2	1,2	3.50				2105	433	0.206	0.206		2105	573	0.272	0.272
		SA	C3	1,2	3.50				2105	432	0.205			2105	573	0.272	
Elegance Road	1 SB	LT	D1	3,4	3.50	15.0		100	1786	195	0 100	0.109	100	1786	216	0.121	0.121
	100	SA	D2	3,4	3.50	13.0		100	2105	224	0.109	0.109	100	2105	178	0.085	0.121
		SA+RT	D3	3,4	3.50	18.0		40	2037	217	0.107		65	1997	169	0.085	
		RT	D4	3,4	3.50	15.0		100	1914	203	0.106		100	1914	162	0.085	
pedestrian pha	ise		Ep	1,2		min c	rossing	time =	12	sec	GM +	10	sec F	GM =	22	sec	
			Fp	1,2,3,4			rossing		5	sec	GM +	7	sec F	GM =	12	sec	
			Gp	2,3		min c	rossing t	time =	5	sec	GM +	5	sec F	GM =	10	sec	
AM Traffic Flow (pcu/hr	r) I			PM Traffic	Flow (pcu/hr	)	1			S=1940+1	00(W-3.25	) 9	S=2080+10	0(W_3 25)	Note:		
	291 +	→ 195	N K			272	← →	216	N K	S <sub>M</sub> =S÷(1+			S <sub>M</sub> =(S-230)	. ,		on Improv	
	+ 353	3					↓ 237				-	Peak		Peak		by Other	
	785			-		654					2+4+5	Car	2+3+5	Car		Peak, St e : 2>4>{	
										Sum y	0.641		0.622		3) In PM	Peak, St	age
	8	65 🗕	-				1146	<b>↓</b>	-	L (s)	15		20			e : 2>3>8	
	709	577				534		405		C (s)	118		108				
157	′← → 338				288		319			practical y	0.786		0.733				
										R.C. (%)	23%		18%				
1		2				3	D4 D3	D2 D1		4	D4 D3	D2 D1		5			
A1 A2	$\rightarrow$	A1 A2												1 ∱	<b>→</b>		
←	Сз		•		— <sub>C3</sub>		⊷↓	1			⊷↓			B3 B4 B5			
-	C3 C2 C1		<u>-</u>		— C2		+	•			+	*		▲ 1 ↑ ↑		r	— C1
Fp,	▶ ↓ 0.	Fp, ▼	Ep	Gp		Fp▼		Gp		Fp▼						Ŧ	0.
•		*		•				•		-				B1 B2 B	3		
AM	I/G	6 =			I/G =				I/G =	5			I/G =	10		I/G =	3
PM	I/G	; =			I/G =	7			I/G =	7			I/G =	6		I/G =	3
	1/6				"G =				<i></i>	•			"G -	5		1/G =	5

					_	J	June										
Junction:	Kwun Tong														Job Nu	mber:	
Scenario: Design Year:	Sensitivity T 2032	est ( 644-b Designe						Checke	d By:					Date:	5 Fe	P. ebruary:	32 2025
										AM Peak					PM Peak		1
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Kwun Tong Ro	oad EB	SA	A1	1,2	3.20				1935	379	0.196			1935	316	0.163	
		SA	A2	1,2	3.20				2075	406	0.196			2075	338	0.163	
Lai Yip Street	NB	LT	B1	5	3.30	30.0		100	2035	157	0.077		100	2052	288	0.140	
		SA	B2	5	3.30				2085	524	0.251			2085	427	0.205	
		SA	B3	5	3.30				2085	523	0.251			2085	426	0.204	
Elegance Roa	d NP	SA	B4	5	3.50				2105	360	0.171			2105	295	0.140	
Elegance Roa		SA+RT	В5	5	3.50	18.0		3	2105	359	0.171		18	2105	295	0.140	
		RT	B6	5	3.50	15.0		100	1914	328	0.171		100	1914	268	0.140	
			00		0.00	10.0		100	1314	520	0.171		100	1314	200	0.140	
Kwun Tong Ro	oad WB	LT	C1	1,5	3.30	15.0		100	1768	577	0.326	0.326	100	1768	405	0.229	0.229
		SA	C2	1,2	3.50				2105	433	0.206			2105	573	0.272	
		SA	C3	1,2	3.50				2105	432	0.205			2105	573	0.272	
													100				
Elegance Roa	d SB	LT	D1	3,4	3.50	15.0		100	1786	195		0.109	100	1786	216	0.121	0.121
		SA	D2	3,4	3.50	10.0		40	2105	224	0.106		6F	2105	178	0.085	
		SA+RT RT	D3 D4	3,4 3,4	3.50 3.50	18.0 15.0		40 100	2037 1914	217 203	0.107		65 100	1997 1914	169 162	0.085	
		NI	D4	3,4	3.50	15.0		100	1914	203	0.100		100	1914	102	0.005	
			_						10								
pedestrian pha	ase		Ep	1,2			rossing		12		GM +	10	sec F		22	sec	
			Fp Gp	1,2,3,4 2,3			rossing t rossing t		5 5		<u>GM +</u> GM +	7 5	sec F	GM =	12 10	sec sec	
AM Traffic Flow (pcu/h				DM Troffic	Flow (pcu/hr						l				Note:		
Aw traffic flow (peari	" <sup>"</sup> 291 <del>• • •</del>	→ 195	N	r wi franci	now (pea/m	272	$ \rightarrow $	216	N		00(W–3.25	•	S=2080+10	. ,		on Improv	/ement
	ŧ		$\langle \rangle$			212	ŧ	210	$\langle \rangle$	S <sub>M</sub> =S÷(1+	1.5f/r)	s	6 <sub>M</sub> =(S−230) <sup>·</sup>	÷(1+1.5f/r)		by Other	
	35 • 785	3	`	_	<b>,</b>	654	237		1			Peak		Peak		Peak, St	
	100					004					2+4+5		2+3+5			e : 2>4>§	
	8	65 +	_				1146	<b>↓</b>	_	Sum y	0.641 15		0.622 20			Peak, St e : 2>3>{	
	709	577				534		↓ 405		L (s) C (s)	118		108				
157	+	011			288	<b>↑</b>	319	100		practical y	0.786		0.733				
101				200		010			R.C. (%)	23%		18%					
1		2				3				4				5			
A1		A1 —		;				D2 D1			D4 D3			† † r	→		
n4	-	A2 -		-													
<b>↓</b>	C3	3	$\leftarrow$		C3 C2		4-]	ţ			+↓	ţ		B3 B4 B5			
Fp. <del>,</del> ∢Ep	Fpr Ep C1 Fpr Ep							Gp		Fp▼						ţ	C1
AM	1/0	G =		-	I/G =	<u> </u>		-	I/G =	5			I/G =	B1 B2 B	3	I/G =	3
DM		<u> </u>			1/2	7			1/0	7			1/0	6		10	2
PM	1/0	G =			I/G =	1			I/G =	1			I/G =	б		I/G =	3

						•											
Junction:		Road / Lai Yip	Street											-	Job Nu	mber:	
Scenario: Design Year:		Condition Design	ed By:					Checke	d By:					Date:	5 Fe	P. ebruary 2	33 2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Liei Dum Deed		1.7		4			Gradient	100	(pcu/hr)	(pcu/hr)		0.000	100	(pcu/hr)	(pcu/hr)		0.007
Hoi Bun Road	EB	LT SA	A1 A2	1 1	3.30 3.30	15.0		100	1768 2085	160 129	0.090	0.090	100	1768 2085	154 151	0.087	0.087
					0.00				2000	.20	0.002			2000		0.012	
Hoi Bun Road	WB	SA	B1	1,2	3.30				1945	329	0.169			1945	234	0.120	
		RT	B2	2	3.30	20.0		100	1940	211	0.109	0.109	100	1940	254	0.131	0.131
Lai Yip Street	SB	LT RT	C1 C2	3 3	3.30 3.30	18.0 25.0		100 100	1795 1967	269 264	0.150 0.134	0.150	100 100	1795 1967	117 212	0.065	
		RT	C2	3	3.30	22.0		100	1967	264	0.134		100	1967	212	0.108	0.108
			00		0.00	22.0		100	1302	201	0.104		100	1992	210	0.100	0.100
pedestrian pha	ise		Dp	1,2,4		min c	rossing	time =	12	sec	GM +	9	sec F	GM =	21	sec	
			Ep	3,4		min c	rossing	time =	7	sec	GM +	6	sec F	GM =	13	sec	
			Fp	4		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
AM Traffic Flow (pcu/hr	r)		N	PM Traffic	Flow (pcu/hr	)			N	S=1940+1	00(W-3.25	) :	S=2080+10	0(W-3.25)	Note:		
		1	5						5	S <sub>M</sub> =S÷(1+			<sub>M</sub> =(S-230)	÷(1+1.5f/r)			
			$\setminus$						$\backslash$		AM	Peak	PM	Peak			
	525	269				422		117			1+2+3		1+2+3				
100					454					Sum y	0.349		0.326				
160 1	100	211			154 1			254		L (s)	35		35				
<b>_</b>	129	329		-	>	151	234 🗲	1		C (s)	118 0.633		108 0.608				
		529					204			practical y R.C. (%)	81%		87%				
1		2				3				4							
	<b>↓▶</b> Dp			∎ Dp		۹	<b>&gt;</b> Ep ←	C3 C2 C1	*	<b>4</b> …	Ep	den no de la constante de la					
	B1 <b></b> €	_		B2 B1◀	_							Fp					
AM G =	:	I/G = 8	G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	:	I/G = 8	G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G =		
	:																

						9											
Junction:		oad / Lai Yip												-	Job Nu		J7333
Scenario:		e Proposed I															34
Design Year:	2032	Design	ed By:					Checke	d By:				-	Date:	5 F	ebruary 2	2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
			Phase	Slage		Radius (III)	% Op-fill Gradient	running %	(pcu/hr)	(pcu/hr)			ruming %	(pcu/hr)	(pcu/hr)		
Hoi Bun Road	EB	LT*	A1	1	3.65	15.0		100	1800	250	0.139	0.139	100	1800	239	0.133	0.133
		SA*	A2	1	3.65				2120	266	0.125			2120	250	0.118	
Hoi Bun Road	WB	SA	B1	1,2	3.30				1945	428	0.220			1945	401	0.206	
		RT	B2	2	3.30	20.0		100	1940	331	0.171	0.171	100	1940	341	0.176	0.176
Lai Yip Street	SB	LT	C1	3	3.30	18.0		100	1795	365	0.203		100	1795	307	0.171	
		RT	C2	3	3.30	25.0		100	1967	422	0.215	0.215	100	1967	363	0.185	0.185
		RT	C3	3	3.30	22.0		100	1952	419	0.215		100	1952	361	0.185	
									_								
pedestrian pha	ase*		Fp	4			rossing		7		GM +	7		GM =	14	sec	
			Gp Hp	4			rossing rossing		8 10		GM + GM +	8 9		GM = GM =	16 19	sec	
			пр	4			USSING	ume –	10	Sec		9	Secr	- Givi –	19	sec	
AM Traffic Flow (pcu/h	r)		N	PM Traffic	Flow (pcu/hr)	1			N	S=1940+1	00(W–3.25	) :	S=2080+10	0(W-3.25)	Note:		
		1	5						5	S <sub>M</sub> =S÷(1+	1.5f/r)	s	6 <sub>M</sub> =(S−230)	÷(1+1.5f/r)			
			$\mathbf{X}$						$\setminus$		AM	Peak	PM	Peak		ement S er Projec	
	841	365				724	↔	307			1+2+3		1+2+3		-	-	
										Sum y	0.524		0.493				
250 ↑		224			239 †			244		L (s)	35		35				
	266	331 1		-	→	250		341 1		C (s)	118		108				
	428	3 ← └──					401 🗲			practical y	0.633		0.608				
										R.C. (%)	21%		23%				
1		2				3				4							
<b>1</b>							+	L L C3 C2 C1	*		Hp	• •					
A1								03 02 01									
				в2 Ĺ							Gp	Fp					
	B1 <b>←</b>			B1 <b>∢</b>						¥		¥					
															-		
AM G =		I/G = 8	G = G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G = G =		
G = PM G =		I/G = 1/G = 8	G =		I/G =	5	G = G =		I/G =	8	G = G =	14	I/G =	3	G =		
PM G = G =		I/G = 8	G =		I/G =	5	G =		I/G =	5	G =		I/G =	5	G =		
6 =	-		6 -		i/G =		62		1/G =		6=		1/G =		G =		

						0	June		,								
Junction:	Hoi Bun Roa	d / Lai Yip	Street											-	Job Nu	mber:	
Scenario:	With the Pro																35
Design Year:	2032	Designe	ed By:					Checke	d By:				-	Date:	5 Fe	ebruary 2	2025
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
				-			Gradient		(pcu/hr)	(pcu/hr)		-		(pcu/hr)	(pcu/hr)		
Hoi Bun Road	EB	LT*	A1	1	3.65	15.0		100	1800	250	0.139	0.139	100	1800	239		0.133
		SA*	A2	1	3.65				2120	266	0.125			2120	250	0.118	
Hoi Bun Road	WB	SA	B1	1,2	3.30				1945	429	0.221			1945	401	0.206	
		RT	B2	2	3.30	20.0		100	1940	331	0.171	0.171	100	1940	341	0.176	0.176
Lai Yip Street	SB	LT	C1	3	3.30	18.0		100	1795	365	0.203		100	1795	307	0.171	
		RT	C2	3	3.30	25.0		100	1967	423	0.215	0.215	100	1967	364	0.185	0.185
		RT	C3	3	3.30	22.0		100	1952	420	0.215		100	1952	362	0.185	
pedestrian pha	ase*		Fp	4		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
<u>r</u>			Gp	4			rossing		8		GM +	8		GM =	16	sec	
			Нр	4		min c	rossing	time =	10	sec	GM +	9	sec F	GM =	19	sec	
AM Traffic Flow (pcu/h	r)		N	PM Traffic I	Flow (pcu/hr)				N		00(W–3.25		S=2080+10		Note:		
	1		`\						$\mathbf{X}$	S <sub>M</sub> =S÷(1+	1.5f/r)	S	6 <sub>M</sub> =(S−230)	÷(1+1.5f/r)	*Junction	on ement S	cheme
	843	→ 365	`			726	₅⊥₊	307	`			Peak		Peak	by Othe	er Projec	ct
	0.0					. 20					1+2+3		1+2+3				
250					239					Sum y	0.525 35		0.494 35				
	266	331		-	1	250		341		L (s) C (s)	118		108				
	429	<b>↓</b>					401 🗕	<u> </u>		practical y	0.633		0.608				
	-						-			R.C. (%)	21%		23%				
1		2				3				4							
							•	J↓L	•	۹		••••					
A1								C3 C2 C1		Ť	Нр	1					
,				+							Gp	Fp					
	B1 <b>←</b>			B2 └── B1◀───	_					Ļ	·	,					
										•		Ť					
AM G =	· I/G	G= 8	G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G =		
G =	· //0	G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =		G = 8	G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G =		
G =	· I/G	G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

					_	5	Junci	-	,								
Junction:	Hoi Bun Ro													-	Job Nu	mber:	
Scenario: Design Year:	Sensitivity 7 2032				00-room			Checke	d By:				<u> </u>	Date:	5 Fe	P. ebruary	36 2025
	Approach		Phase	Stage	Width (m)	Radius (m)		Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %		PM Peak Flow	y value	Critical y
Hoi Bun Road	ED	LT*	A1	1	3.65	15.0	Gradient	100	(pcu/hr) 1800	(pcu/hr) 250	0.139	0.139	100	(pcu/hr) 1800	(pcu/hr) 239	0 122	0.133
HOI BUIL ROAU	ED	SA*	A1 A2	1	3.65	15.0		100	2120	266	0.139	0.139	100	2120	250	0.133	0.133
Hoi Bun Road	WB	SA	B1	1,2	3.30				1945	429	0.221			1945	401	0.206	
	110	RT	B2	2	3.30	20.0		100	1940	331	0.171	0.171	100	1940	341		0.176
Lai Yip Street	SB	LT	C1	3	3.30	18.0		100	1795	365	0.203		100	1795	307	0.171	
		RT	C2	3	3.30	25.0		100	1967	423	0.215	0.215	100	1967	364		0.185
		RT	C3	3	3.30	22.0		100	1952	420	0.215		100	1952	362	0.185	
pedestrian pha	ase*		Fp	4		min c	rossing	time =	7	Sec	GM +	7	sec F	GM =	14	sec	
podooundin pric			Gp	4			rossing		8		GM +	8		GM =	16	sec	
			Нр	4		min c	rossing	time =	10	sec	GM +	9	sec F	GM =	19	sec	
AM Traffic Flow (pcu/h	r)		N N	PM Traffic	Flow (pcu/hr)				N N		00(W–3.25		S=2080+10		Note: *Junctio	00	
			$\setminus$						$\setminus$	S <sub>M</sub> =S÷(1+		<b>o</b> Peak		÷(1+1.5f/r) Peak	Improv	ement S er Projec	
	843 🕇	365				726	<b>←</b> ⊥→	307			1+2+3		1+2+3		-,	,	
250					239					Sum y	0.525		0.494				
230	266	331			t	250		341		L (s)	35 118		35 108				
	429					200	401 🗲	<u> </u>		C (s) practical y	0.633		0.608				
	120						101			R.C. (%)	21%		23%				
1		2				3				4							
<b>1</b> A1							+	L C3 C2 C1	*		Hp	<b>†</b>					
▶ A2				+							Gp	Fp					
	B1 <b>←</b>			B2 └ B1 <b>∢</b>						¥		ţ					
AM G =	: 1	/G = 8	G =		I/G =	5	G =		I/G =	8	G =	14	I/G =	3	G =		
G =		/G = /G = 8	G = G =		I/G =	5	G = G =		I/G =	8	G = G =	14	I/G =	3	G = G =		
G =		/G = 0	G =		I/G =		G =		I/G =		G =		I/G =		G =		

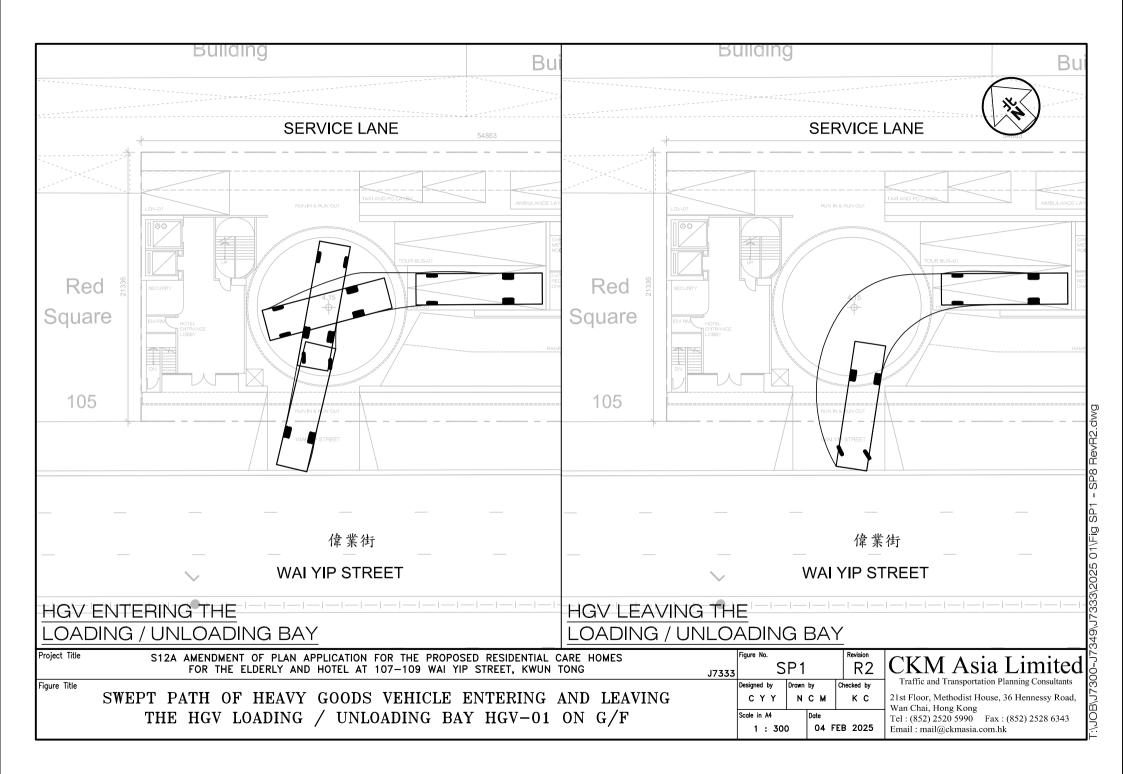
						•											
Junction:	Lai Yip S	Street / Hung To	Road												Job Nu	mber:	J7333
Scenario:	Existing	Condition														Ρ.	37
Design Year:	2024	Designe	ed By:				. –	Checke	d By:					Date:	5 Fe	ebruary 2	2025
										AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Lai Yip Street S	SB	SA	A1	1	3.50				1965	330	0.168	0.168		1965	183	0.093	
		SA	A2	1	3.50				2105	354	0.168			2105	196	0.093	
Lai Yip Street I	NB	SA	B1	1	3.50				1965	314	0.160			1965	246	0.125	0.125
		SA	B2	1	3.50				2105	337	0.160			2105	264	0.125	
Hung To Road	IWB	LT	C1	2	3.50	15.0											
		LT+RT	C2*	2	3.50	18.0		100	1943	507	0.261	0.261	100	1943	528	0.272	0.272
		RT	C3	2	3.50	25.0											
pedestrian pha	ase		Dp	1		min c	rossing	time =	7	sec	GM +	16	sec F	GM =	23	sec	
			•														
AM Traffic Flow (pcu/hr	r)		N	PM Traffic I	Flow (pcu/hr)				Ν	S=1940+1	00(W–3.25	) :	5=2080+10	0(W–3.25)	Note:		
			7						7	S <sub>M</sub> =S÷(1+	1.5f/r)	s	<sub>M</sub> =(S−230)·	÷(1+1.5f/r)		that phas are blocke	
	↓ 684					↓ 379					AM	Peak	PMI	Peak	on-street	parking a	activities
		305						264			1+2		1+2		along Hu	ing To Ro	ad
		t						t		Sum y	0.429		0.397				
651		ţ			510			t		L (s)	14		11				
t t		202			Î			264		C (s)	120		108				
										practical y	0.795		0.808				
										R.C. (%)	85%		104%				
1		2															
				*													
★ A2	2 A1	Dp		↓ +	— C3 _ C2												
B1 B2	¥	5P			— 02 — C1												
1 1				+													
AM G=	:	I/G = 10	G =		I/G =	6	G =		I/G =		G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G=		I/G = 6	G =		I/G =	7	G =		I/G =		G =		I/G =		G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
-			-				-				-				-		

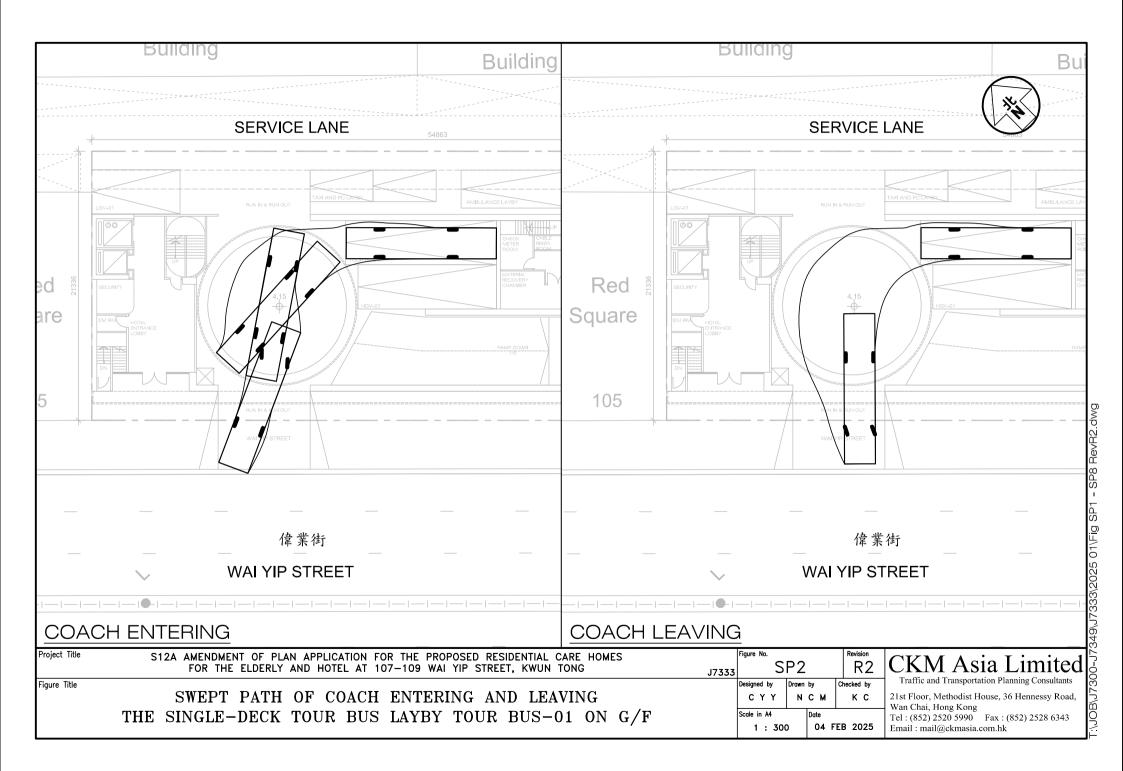
						gilai (											
Junction:		Street / Hung To												-	Job Nu	mber:	
Scenario: Design Year:		the Proposed D Designe						Checke	d By:					Date:	5 Fe	P. bruary	38 2025
		_	-						-								
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	AM Peak Flow	y value	Critical y	Turning %		PM Peak Flow	y value	Critical y
Lai Yip Street	SB	SA	A1	1	3.50		Gradient		(pcu/nr) 1965	(pcu/hr) 448	0 228	0.228		(pcu/hr) 1965	(pcu/hr) 309	0.157	
20. 1. p 0 001		SA	A2	1	3.50				2105	480	0.228	0.220		2105	331	0.157	
Lai Yip Street	NB	SA	B1	1	3.50				1965	402	0.205			1965	373	0.190	0.190
		SA	B2	1	3.50				2105	430	0.204			2105	400	0.190	
Hung To Road	WB	LT	C1	2	3.50	15.0											
		LT+RT	C2*	2	3.50	18.0		100	1943	716	0.369	0.369	100	1943	742	0.382	0.382
		RT	C3	2	3.50	25.0											
pedestrian pha	ase		Dp	1		min c	rossing	time =	7	sec	GM +	16	sec F	GM =	23	sec	
AM Traffic Flow (pcu/h	ır)			PM Traffic	Flow (pcu/hr)										Note:		
			N A						N K	S=1940+1 S <sub>M</sub> =S÷(1+	00(W-3.25		S=2080+10	÷(1+1.5f/r)	Assume	that phas	es C1
	Ļ					Ļ				3 <sub>M</sub> =3÷(1+					anu Co a	are blocke t parking	a une io
	928	369	``			640		365	``		AM 1+2	Peak	PM 1+2	Peak		ing To Ro	
		309 <b>↑</b>						305 ↑		Cum 1	0.597		0.572				
832		↓			773			↓		Sum y L (s)	14		11				
1		347			1			377		C (s)	120		108				
										practical y	0.795		0.808				
-					I					R.C. (%)	33%		41%				
1		2															
A	2 A1 ∳	)n			C3 C2												
B1 B2	∎ T	φ.			— C1												
Î Î				+													
AM G=		I/G = 10	G =		I/G =	6	G =		I/G =		G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G=	-	I/G = 6	G =		I/G =	7	G =		I/G =		G =		I/G =		G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

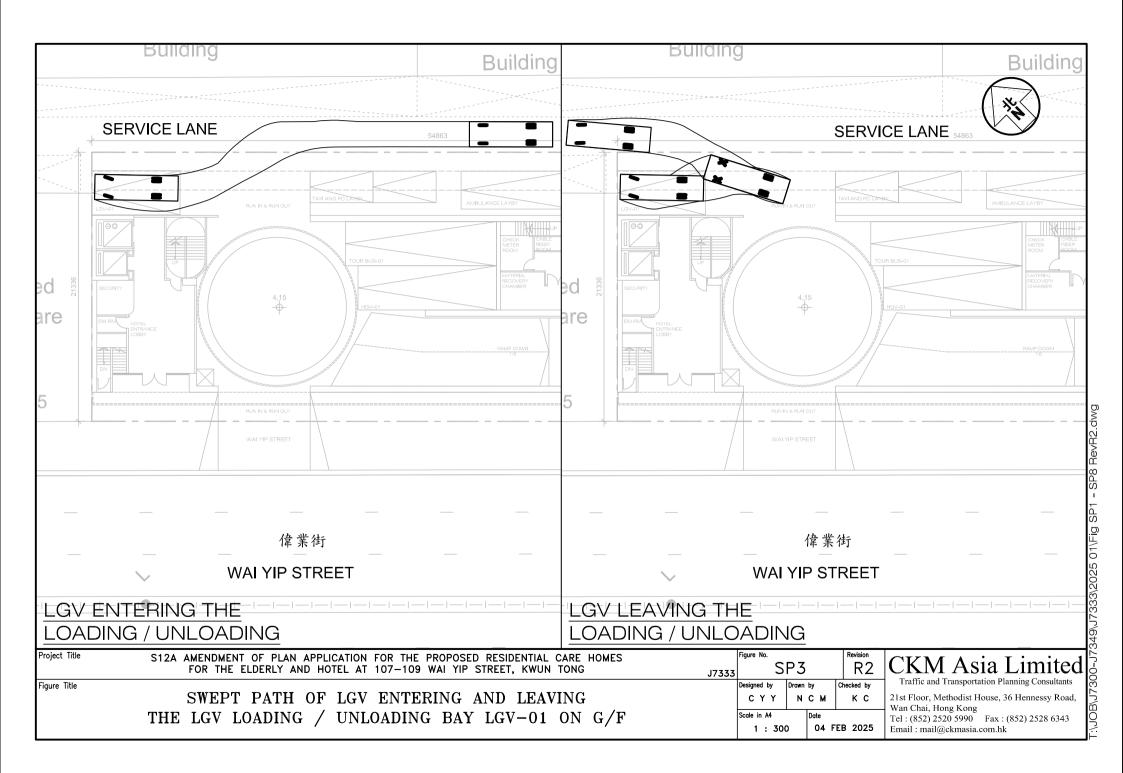
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Junction:		reet / Hung To												-	Job Nu	mber:	J7333
Scenario:		Proposed Dev															39
Design Year:	2032	Designe	ed By:					Checke	d By:				-	Date:	5 Fe	ebruary	2025
										AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Lai Yip Street :	SB	SA	A1	1	3.50				1965	449	0.228	0.229		1965	310	0.158	
		SA	A2	1	3.50				2105	481	0.229			2105	332	0.158	
Lai Yip Street I	NB	SA	B1	1	3.50				1965	403	0.205			1965	375	0.191	0.191
		SA	B2	1	3.50				2105	431	0.205			2105	401	0.190	
Hung To Road	WB	LT	C1	2	3.50	15.0											
0		LT+RT	C2*	2	3.50	18.0		100	1943	716	0.369	0.369	100	1943	742	0.382	0.382
		RT	C3	2	3.50	25.0											
pedestrian pha			Dp	1		min o	rossing	timo -	7		GM +	16	000 F	GM =	23		
pedesthan pha	450		υр	1			lossing	ume –	1	Sec		10	Sec F	Givi –	23	sec	
AM Traffic Flow (pcu/h	r)			PM Traffic	Flow (pcu/hr)										Note:		
			N A						N K	S=1940+1 S <sub>M</sub> =S÷(1+	00(W-3.25		S=2080+10 S <sub>M</sub> =(S-230)			that phas	es C1
	↓ 930					↓ 642			$\setminus$			Peak				are blocke t parking a	u uue it
	000	369				012		365			1+2	Call	1+2	Call	along Hu	ing To Ro	ad
		t						t		Sum y	0.597		0.573				
834		ŧ			776			ŧ		L (s)	14		11				
Î		347			Î			377		C (s)	120		108				
										practical y	0.795		0.808				
										R.C. (%)	33%		41%				
1		2															
	2 A1			t	— C3												
~	Dp			ţ—	C2												
B1 B2 ↑ ↑	·			Ļ	— C1												
AM G =	:	I/G = 10	G =		I/G =	6	G =		I/G =		G =		I/G =	1	G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	-	I/G = 6	G =		I/G =	7	G =		I/G =		G =		I/G =		G =		
G =	:	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

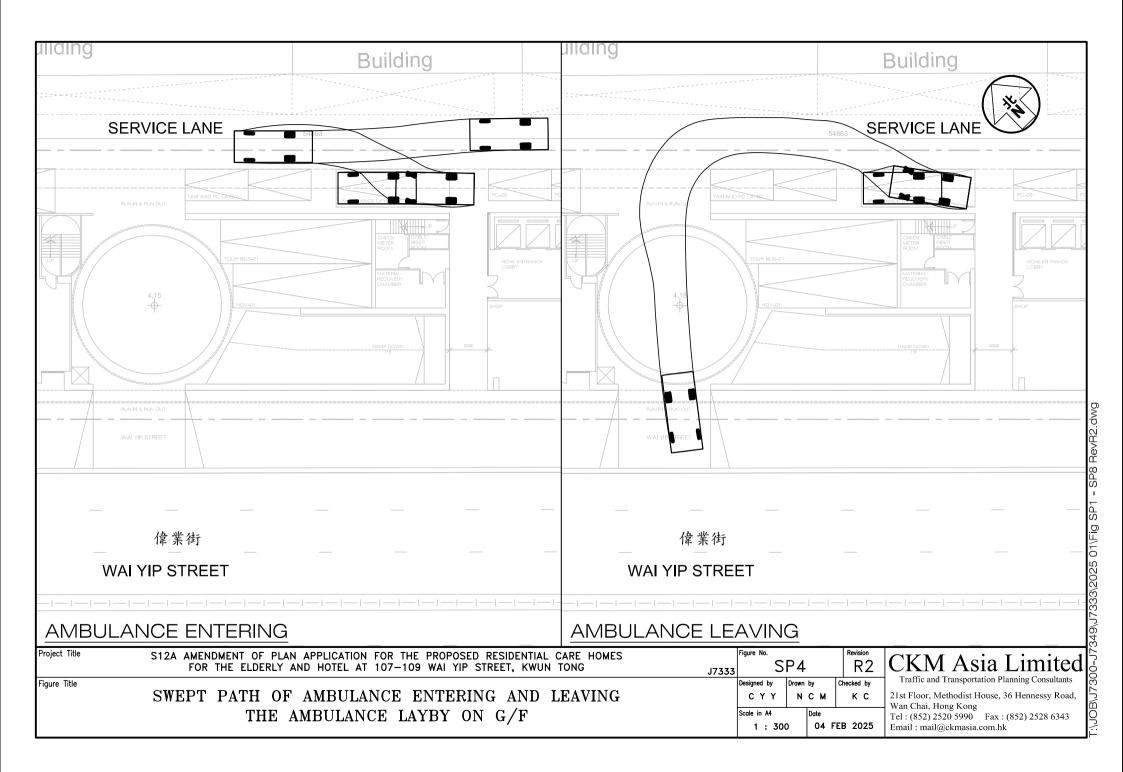
					0.	ignal .	ounot		naryc								
Junction: Scenario:		reet / Hung To / Test ( 644-bo		E and 2	00 room									•	Job Nu	mber:	<u>J7333</u> 40
Design Year:		Designe						Checke	d By:					Date:	5 F	ebruary 2	
	Approach		Phase	Stage	Width (m)	Radius (m)		Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical
Lai Yip Street	SB	SA	A1	1	3.50		Gradient		(pcu/hr) 1965	(pcu/hr) 449	0.228	0.229		(pcu/hr) 1965	(pcu/hr) 310	0.158	
		SA	A2	1	3.50				2105	481	0.229	0.220		2105	332	0.158	
Lai Yip Street	NB	SA	B1	1	3.50				1965	403	0.205			1965	375	0.191	0.191
		SA	B2	1	3.50				2105	431	0.205			2105	401	0.190	
Hung To Road	WB	LT	C1	2	3.50	15.0											
		LT+RT		2	3.50	18.0		100	1943	716	0.369	0.369	100	1943	742	0.382	0.382
		RT	C3	2	3.50	25.0											
pedestrian pha	ase		Dp	1		min c	rossing	time =	7	Sec	GM +	16	sec F	GM =	23	sec	
				•								10	0001		20		
AM Traffic Flow (pcu/h	ır)		Аz	PM Traffic	Flow (pcu/hr)				N K	S=1940+1 S <sub>M</sub> =S÷(1+	00(W–3.25 1.5f/r)		S=2080+10 m=(S-230)			that phas	
	↓ 930		$\setminus$			↓ 642			$\setminus$			Peak		-	and C3 a on-stree	are blocke t parking a	activities
		369						365			1+2		1+2		along Hi	ung To Ro	bad
		<u>†</u>						<b>İ</b>		Sum y	0.597		0.573				
834 †		<b>*</b> 347			776 †			377		L (s)	14		11				
										C (s) practical y	120 0.795		108 0.808				
ļ										R.C. (%)	33%		41%				
1		2															
A	2 A1	,		t	— C3 — C2												
B1 B2	÷			ţ	— C1												
AM G =		I/G = 10	G =		I/G =	6	G =		I/G =	<u> </u>	G =		I/G =		G =		
G =		I/G =	G =		I/G =	-	G =		I/G =		G =		I/G =		G =		
PM G=	-	I/G = 6	G =		I/G =	1	G =		I/G =		G =		I/G =		G =		

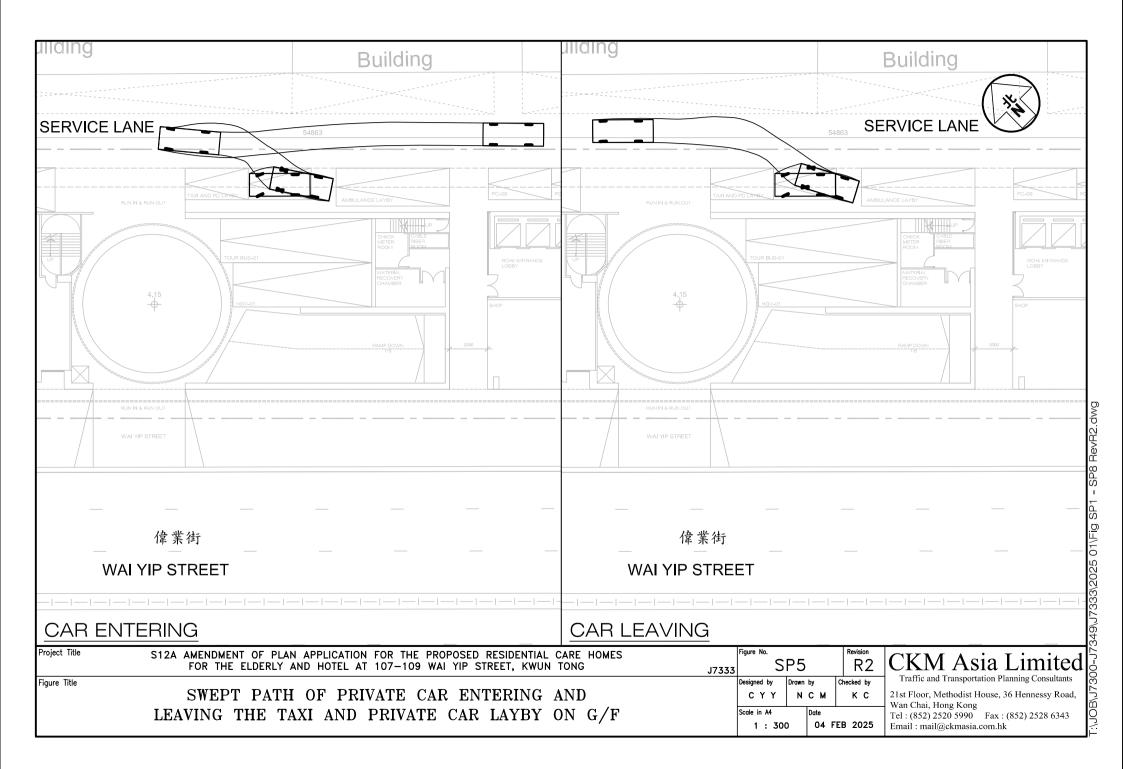
Appendix 2 – Swept Path Analysis

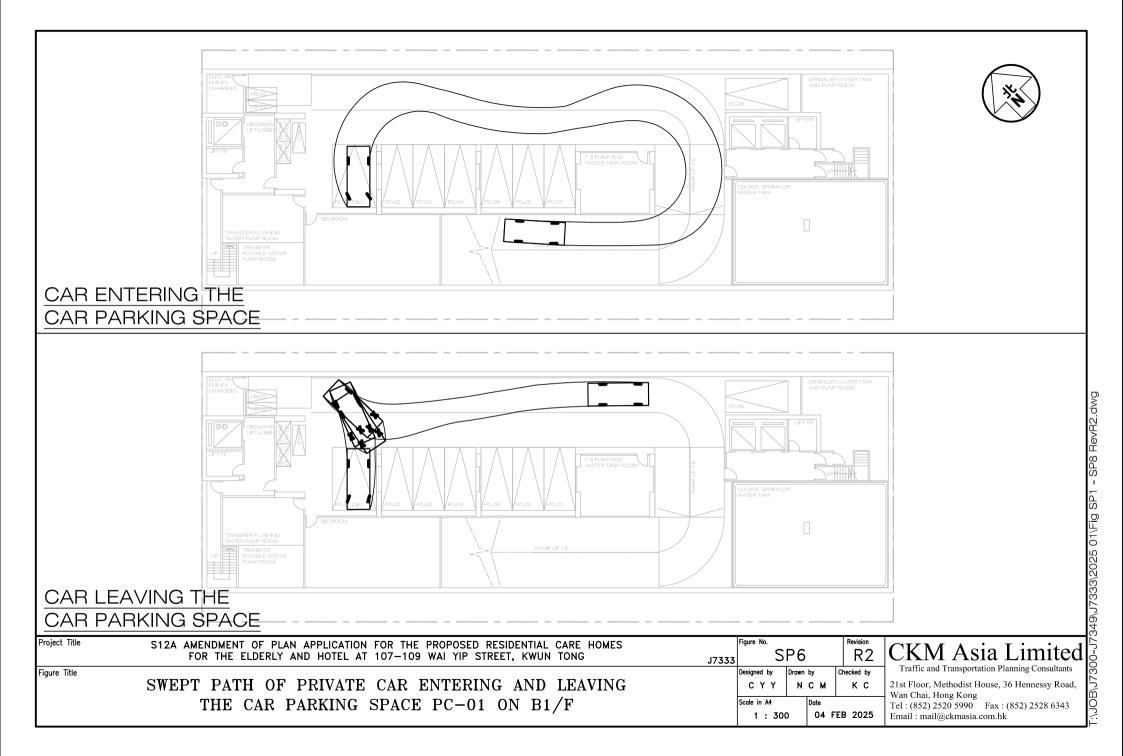


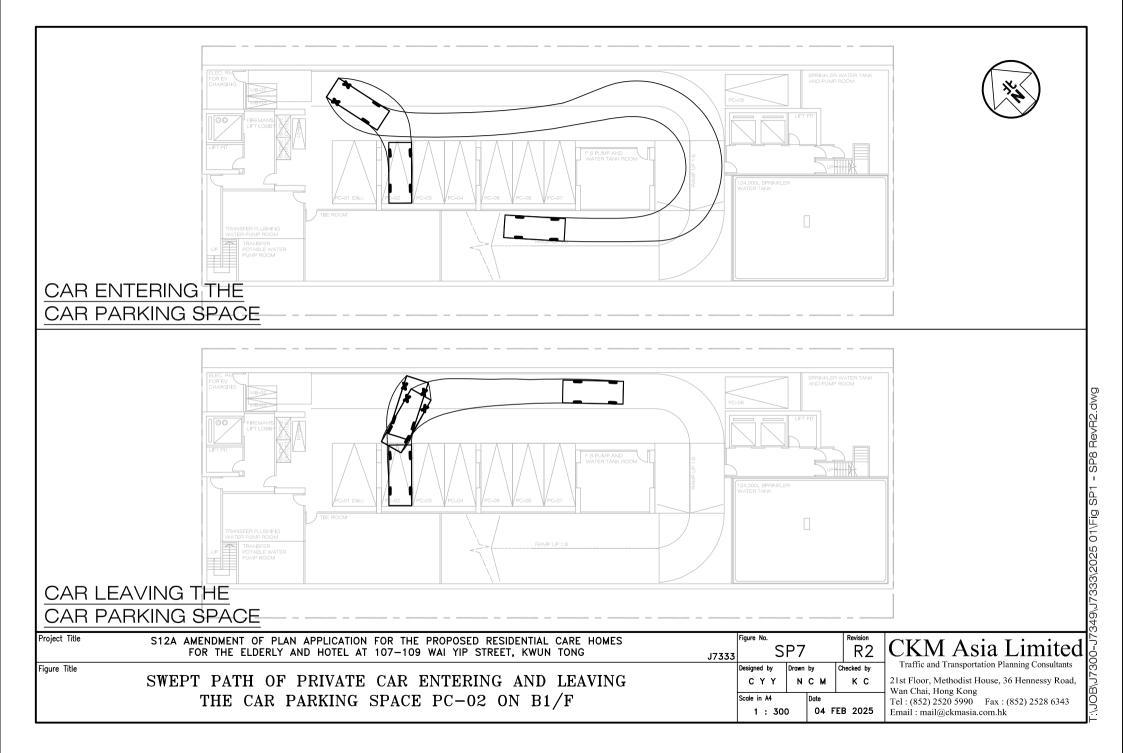


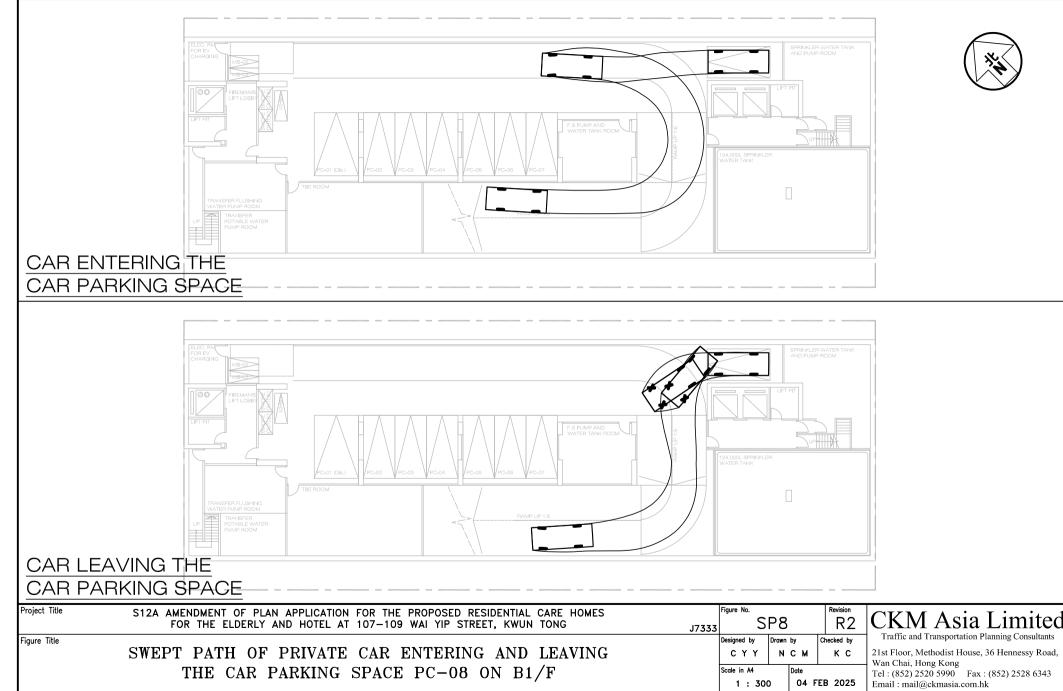






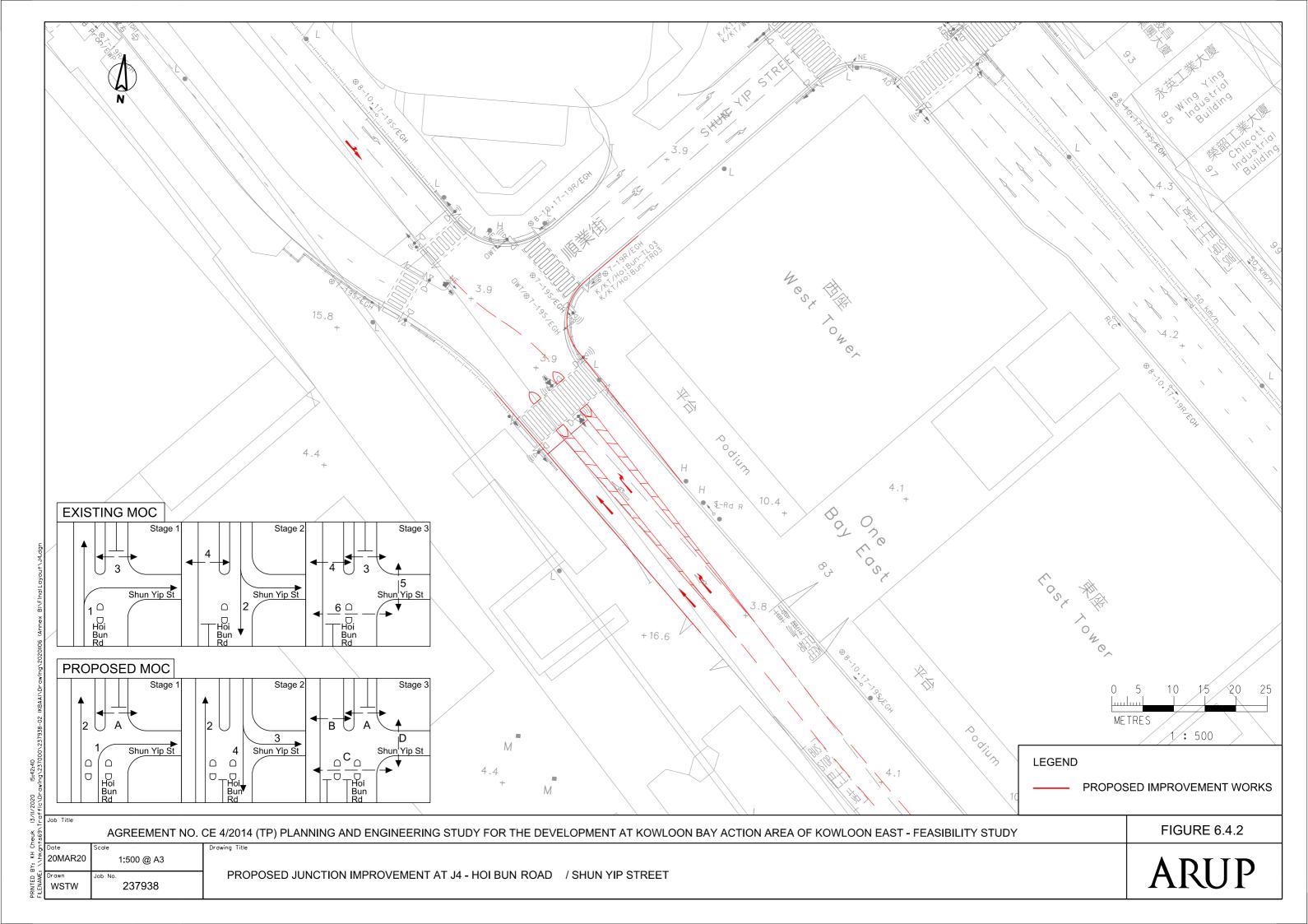


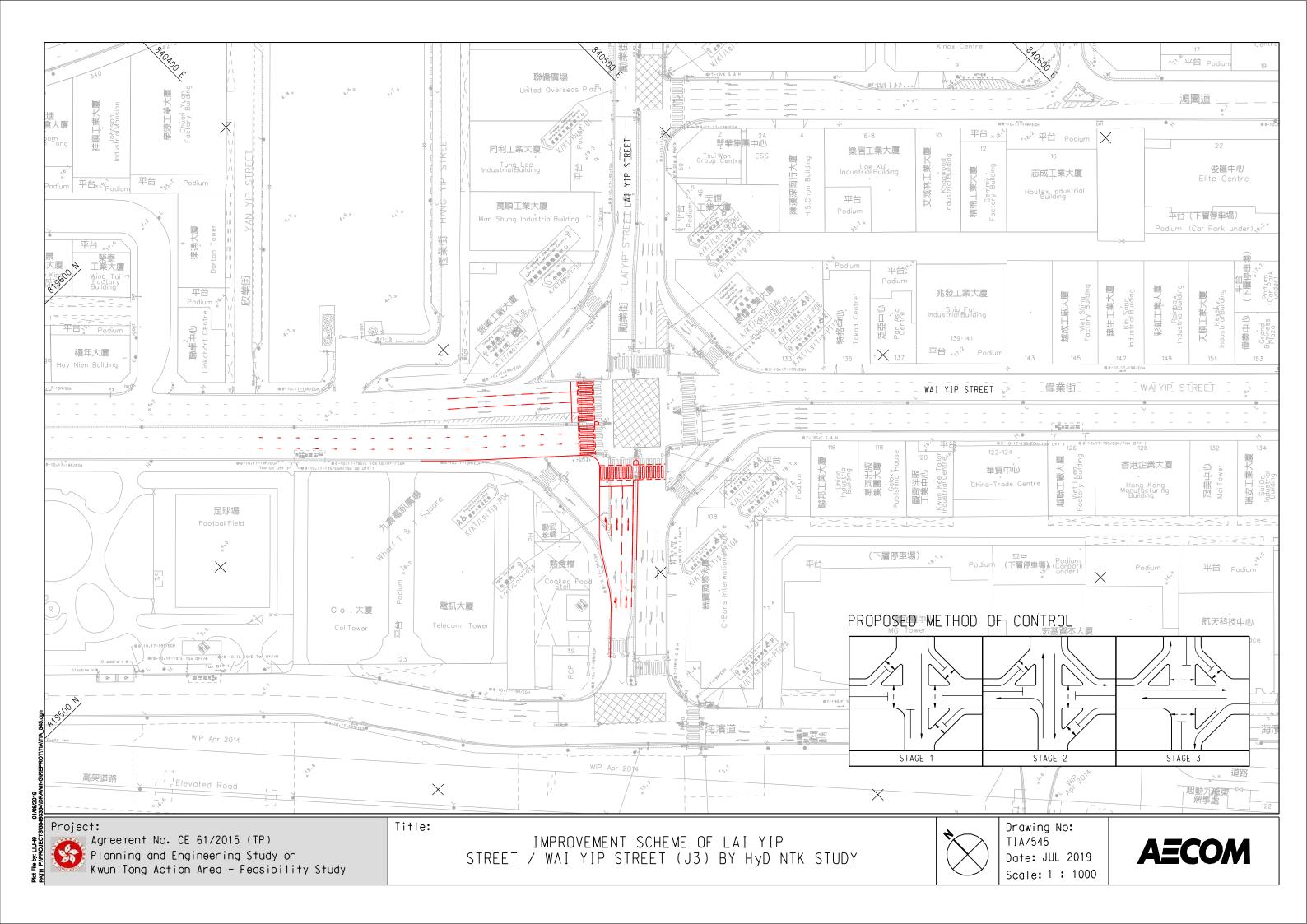


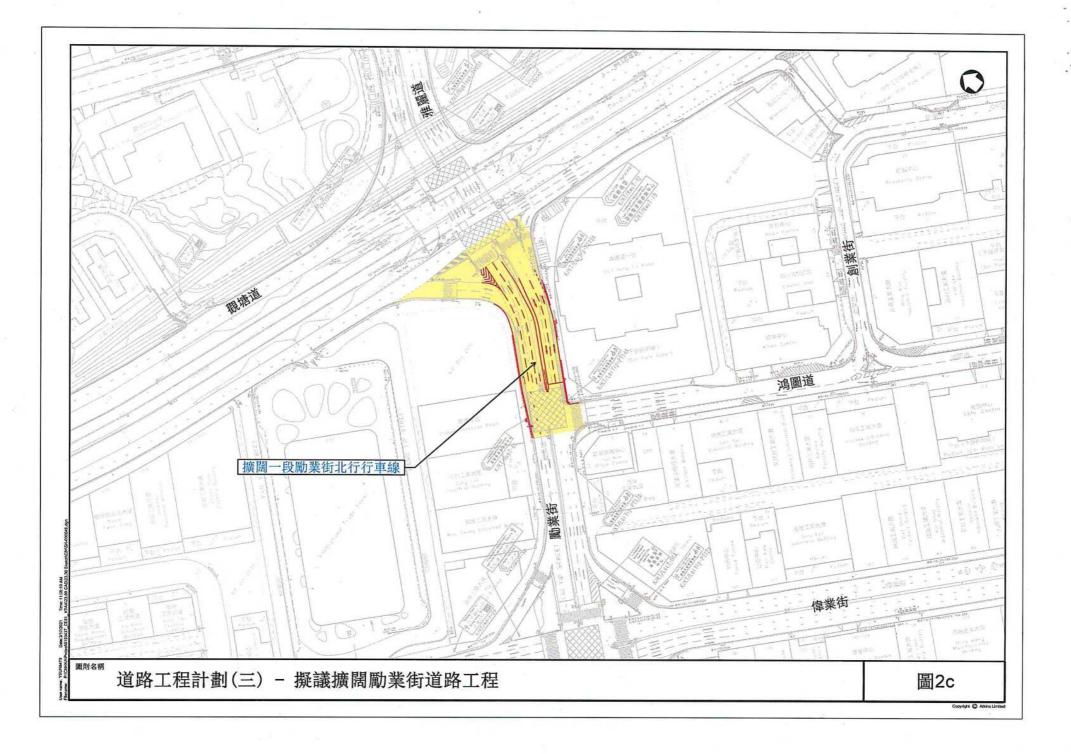


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Appendix 3 – Planned Developments in the Vicinity of the Proposed Redevelopment







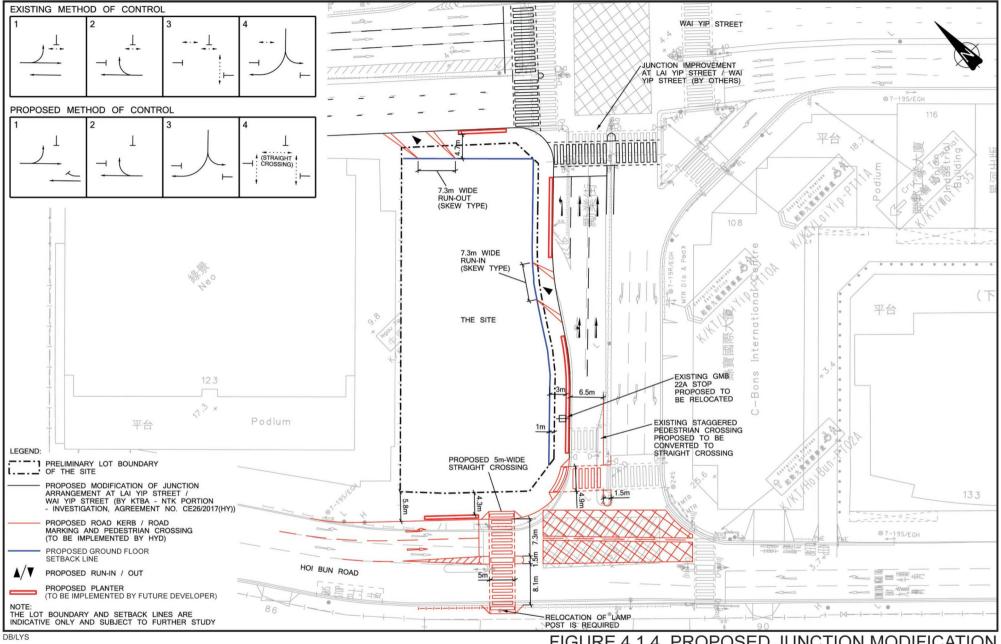


FIGURE 4.1.4 PROPOSED JUNCTION MODIFICATION

參考編號	繪圖
REFERENCE №.	DRAWING
M/K14S/23/35	5b
101/11/14/07/20/00	50