Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

Responses to Comments from Government Departments via Planning Department's email on 17.10.2023 on the Planning Application No. Y/SK-HC/6 issued on 31.08.2023 (Comments from the Environmental Protection Department were replied on 30.10.2023)

Comments from the Dist Lands Offr/Sai Kung (District Lands Office, Sai Kung) for Lands Department (LandsD) via Planning Department's email on 17.10.2023; Contact Person: Mr. Raymond LAU (Tel: 2791 7014)

## Comments on Planning Statement

| Item | Comments | Responses |
| :---: | :---: | :---: |
| $\begin{aligned} & \text { LandsD } \\ & \text { - PS1 } \end{aligned}$ | The application site shown edged dotted red at Figure 1.2 in the Planning Statement includes Lot 411 RP in D.D. 210. However, this lot is not included in the Gist of Application and Table 3.6 in the Planning Statement. The applicant should clarify whether Lot 411 RP in D.D. 210 would be included in the application. | Noted. It is clarified that Lot 411 RP in D.D. 210 should be included in the current application (Planning Application No. Y/SK-HC/6). Replacement pages for the application and the Planning Statement have been enclosed this RtoC Table as Attachment 1. |
| $\begin{aligned} & \text { LandsD } \\ & -\mathrm{PS} 2 \end{aligned}$ | Two strips of Government land within the application site abutting the public road might encroach onto (i) the roadside amenity area to be maintained by Leisure and Cultural Services Department; and (ii) the public road to be maintained and managed by Highways Department and Transport Department, comments from relevant department should be sought. | Noted. |
| $\begin{aligned} & \text { LandsD } \\ & \text { - PS3 } \end{aligned}$ | The concerned private lots are old schedule agricultural lots held under Block Government Leases which contain the restriction that no structure is allowed to be erected without the prior approval of the Government. Structure/temporary structures were observed within numerous of the subject private lots. Lease enforcement action had been taken by issuance of warning letter in March 2023 which were forwarded to the Land Registry in early August 2023 as the concerned lot owners failed to purge the breach by deadline. There is also illegal occupation on Government land at the south within the application site. | Noted. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

|  | Land control action would be taken <br> by this office according to case <br> priority. In view of the above, his <br> office has reservation on the <br> planning application. |
| :--- | :--- | :--- | :--- |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| $\begin{array}{\|l\|} \hline \text { TD } \\ \text { TIA5 } \end{array}$ | Section 5 <br> Please provide the internal layout showing the internal transport facilities, including the parking space and loading/unloading area. | Figure 5.1 is updated in the TIA (Ver. B) to show the internal transport facilities, including the parking space and loading/unloading area. This has been enclosed in this RtoC Table as Attachment 2. |
| :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { TD } \\ \text { TIA6 } \\ \hline \end{array}$ | Figure 5.1 <br> Please provide the swept path for vehicular access to the site. | The Swept Path for vehicular access to the site are shown in new Figure 5.2 in the TIA (Ver. B). This has been enclosed in this RtoC Table as Attachment 2. |
| $\begin{array}{\|l\|} \hline \text { TD } \\ \text { TIA7 } \\ \hline \end{array}$ | Figure 5.1 <br> Please indicate the footpath and vehicular access for public use in Luk Mei Tsuen Road | Figure 5.1 is updated in the TIA (Ver. B) to indicate the footpath and vehicular access for public use in Luk Mei Tsuen Road. This has been enclosed in this RtoC Table as Attachment 2. |
| Comments from the Acting Director of Drainage Services for Drainage Services Department (DSD) via Planning Department's email on 17.10.2023; Contact Person: Mr. Henry YEUNG (Tel: 2300 1343) |  |  |
| Comments on Sewerage Impact Appraisal (SIA) |  |  |
| Item | Comments | Responses |
| $\begin{array}{\|l\|} \hline \text { DSD } \\ \text { SIA1 } \\ \hline \end{array}$ | The SIA for the subject planning application needs to meet the full satisfaction of Environmental Protection Department (EPD), the planning authority of sewerage infrastructure. DSD's comments on the captioned SIA submitted by the developer are subject to views and agreement of EPD. | Noted. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| $\begin{aligned} & \hline \text { DSD - } \\ & \text { SIA2 } \end{aligned}$ | Please be reminded that upon connection to the public sewerage network and decommissioning of the proposed on-site sewage treatment plant, the sewerage impact assessment may need to be reviewed and updated or a separate sewerage impact assessment may need to be conducted to assess the potential sewerage impact and/or identify necessary mitigation measures, if required. | Noted. |
| :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { DSD - } \\ & \text { SIA3 } \end{aligned}$ | Please note that the proposed use and design of proposed on-site sewerage treatment plant should be subject to the views and agreement of EPD and any relevant statutory requirements. | Noted. |
| Comments on Drainage Impact Appraisal (DIA) |  |  |
| Item | Comments | Responses |
| $\begin{aligned} & \text { DSD - } \\ & \text { DIA1 } \end{aligned}$ | Section 3.2, 2nd para. second last line <br> Please rectify the typo error "20211" and also, in some other text, "soakway" should read as "soakaway"?) | Noted. It has been amended accordingly in the Sewerage and Drainage Impact Appraisal (Version B) (SDIA (Ver. B)) that have been enclosed in this RtoC Table as Attachment 3. |
| $\begin{aligned} & \hline \text { DSD - } \\ & \text { DIA2 } \end{aligned}$ | Section 3.3 <br> Please supplement and demonstrate with the support of hydraulic calculations to show that the existing perimeter U-channel has sufficient capacity for the surface runoff and the proposed development would not cause any adverse drainage impact to the vicinity. | As there has been no significant development in the area, it can be assumed there is a similar catchment area to the previous SDIA in Application No. A/SK-HC/326. Making reference to the previous SDIA, it is observed that the surface runoff from the Catchment and the Site can be handled with the existing drainage system. Given the above, with the additional perimeter UChannels installed by the Home Affairs Department (HAD) in 2023, there will be even improved the drainage system in the vicinity. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

|  |  | The said SDIA, has been extracted and included in this RtoC Table as Attachment 4. <br> Furthermore, the area surrounding the application site is not an area identified as a Flooding Blackspots by the Drainage Service Department (DSD). <br> It should also be noted, that the Proposed Development would be provided with its own drainage connection and this would not impact the existing perimeter U channel. |
| :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { DSD - } \\ & \text { DIA3 } \end{aligned}$ | Section 3.4 <br> At Parcel B, it seems the proposed new manhole and the associated drainage pipes are in conflict with the soakaway as shown in Figures 2.3 and 3.4. Please clarify. <br> Please also supplement and demonstrate with the support of hydraulic calculations to show that the proposed drainage system will not cause adverse effect to the existing stormwater system in Ho Chung North Road since the proposed drainage systems might create additional flow to the existing drainage system. <br> You may note that the existing drainage system is currently exclusive road drains maintained by Highways Department. | Noted. The figures have been amended to avoid the conflict. This has been updated in the SDIA (Ver. B) that have been enclosed in this RtoC Table as Attachment 3. <br> The supplementary supporting hydraulic calculations have been added to SDIA (Ver. B). It is concluded that there will be no serious adverse drainage impact to the existing drainage system after the implementation of the development. The SDIA (Ver. B) have been enclosed in this RtoC Table as Attachment 3. <br> Noted. |
| $\begin{aligned} & \hline \text { DSD - } \\ & \text { DIA4 } \end{aligned}$ | Please clarify the maintenance responsibility of the connection pipes between the new manholes and the existing manholes. | The connection pipes between the new manholes and the existing manholes will be maintained by the Applicant. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| DSD - | Figure 3.2 |
| :--- | :--- | :--- | :--- |
| DIA5 |  |
| Please clarify what is the "Foul |  |
| Drainage" as shown in the legend. |  |$\quad$| It is assumed "Foul Drainage" is |
| :--- |
| equivalent to "Sewerage Drain". The |
| figure legend has been updated in the |
| SDIA (Ver. B) and enclosed in this |
| RtoC Table as Attachment 3. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| $\begin{aligned} & \hline \text { PlanD - } \\ & \text { VIA2 } \end{aligned}$ | As demonstrated in the photomontages, the proposed development would obstruct part of the open sky view and/or mountain backdrop, reducing the openness for VP1 and VP4. It is therefore not convinced that the visual impact for VP1 and VP4 and the overall visual impact are considered "Enhanced". | In consideration of the obstruct part of the open sky view and/or mountain backdrop which will reduce the openness for VP1 and VP4, the reassessed overall visual impact for VP1 and VP4 will be amended to "Partly Enhanced/Partly Adverse". The replacement pages to the VIA (Ver. A) have been enclosed in this RtoC Table as Attachment 6. |
| :---: | :---: | :---: |
| Comments on Landscape |  |  |
| Item | Comments | Responses |
| $\begin{aligned} & \text { PlanD - } \\ & \text { LA1 } \end{aligned}$ | Having reviewed the submitted documents and the Application Form No.12A Appendix, noting 'Landscape Impact Assessment' as well as 'Tree Survey' were not conducted/submitted in the application. To facilitate TPB's consideration on the application, please note below our comments from landscape planning perspective: <br> Landscape Proposal (Appendix B) <br> a) Para. 4.1.2 - Noting the referred Technical Circulars, i.e. DEVB TC(W) No.3/20012 and No. $4 / 2020$, are promulgated for government projects, the Applicant is reminded to refer to relevant PNs specifically for private development regarding site coverage of greenery as well as tree preservation. | Noted. <br> Noted. The referenced guidelines have been updated to Practice Notes for Professional Persons No. 1/2019 (PNPP No. 1/2019) - Processing and Compliance Checking of Landscape Submissions Related to Planning Applications and Joint Practice Note No. 3 (JPN No. 3) - Landscape and Site Coverage of Greenery. The replacement pages to the Landscape Proposal (Version A) (LP (Ver. A) have been enclosed in this RtoC Table as Attachment 7. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong


## Attachment 1

Replacement Pages for Application and Planning Statement

## Demarcation District 210:

- Lot No. 402 (part)
- Lot No. 403 (part)
- Lot No. 404 (part)
- Lot No. 405
- Section A of Lot No. 406
- Remaining Portion of Lot No. 406
- Section A of Lot No. 407
- Remaining Portion of Section B of Lot No. 407
- Remaining Portion of Lot No. 407
- Section A of Lot No. 409 (Part)
- Remaining Portion of Section B of Lot No. 409
- Remaining Portion of Lot No. 409
- Remaining Portion of Lot No. 410
- Remaining Portion of Lot No. 411
- Lot No. 412
- Lot No. 414
- Remaining Portion of Section A of Lot No. 418 (part)
- Remaining Portion of Lot No. 418
- Adjoining Government Land in Demarcation District 210


## Demarcation District 244:

- Remaining Portion of Lot No. 1860 (part)
- Remaining Portion of Section A of Lot No. 1861
- Adjoining Government Land in Demarcation District 244


|  | Application No． <br> For Official Use Only <br> 請 請編號 |  |
| :---: | :---: | :--- |
|  | 填 寫 此 欄 | Date Received <br> 收到日期 |

1．The completed form and supporting documents（if any）should be sent to the Secretary，Town Planning Board（the Board）， 15／F，North Point Government Offices， 333 Java Road，North Point，Hong Kong．
申請人須把填妥的申請表格及其他支持申請的文件（倘有），送交香港北角渣華道 333 號北角政府合署 15 樓城市規劃委員會（下稱「委員會」）秘書收。

2．Please read the＂Guidance Notes＂carefully before you fill in this form．The document can be downloaded from the Board＇s website at http：／／www．info．gov．hk／tpb／．It can also be obtained from the Secretariat of the Board at 15／F，North Point Government Offices， 333 Java Road，North Point，Hong Kong（Tel： 22314810 or 2231 4835），and the Planning Enquiry Counters of the Planning Department（Hotline： 2231 5000）（17／F，North Point Government Offices， 333 Java Road，North Point，Hong Kong and 14／F，Sha Tin Government Offices， 1 Sheung Wo Che Road，Sha Tin，New Territories）．
請先細閱《申請須知》的資料單張，然後填寫此表格。該份文件可從委員會的網頁下載（網址： http：／／www．info．gov．hk／tpb／），亦可向委員會秘書處（香港北角渣華道 333 號北角政府合署 15 樓－電話：2231 4810或 22314835 ）及規劃署的規劃資料查詢處（熱線：22315000）（香港北角渣華道 333 號北角政府合署 17 樓及新界沙田上禾㟦路 1 號沙田政府合署 14 樓）索取。

3．This form can be downloaded from the Board＇s website，and obtained from the Secretariat of the Board and the Planning Enquiry Counters of the Planning Department．The form should be typed or completed in block letters．The processing of the application may be refused if the required information or the required copies are incomplete．
此表格可從委員會的網頁下載，亦可向委員會秘書處及規劃署的規劃資料查詢處索取。申請人須以打印方式或以正楷填寫表格。如果申請人所提交的資料或文件副本不齊全，委員會可拒絕處理有關申請。

## 1．Name of Applicant 申請人姓名／名稱

（ $\square$ Mr．先生／$\square$ Mrs．夫人／$\square$ Miss 小姐／$\square$ Ms．女士／$\square$ Company 公司／$\square$ Organisation 機構）
Bestime Enterprises Limited

## 2．Name of Authorised Agent（if applicable）獲授權代理人姓名／名稱（如適用）

（ $\square \mathrm{Mr}$ ．先生／$\square \mathrm{Mrs}$ ．夫人／$\square$ Miss 小姐／$\square \mathrm{Ms}$ ．女士／$\square$ Company 公司／$\square$ Organisation 機構）
Prudential Surveyors International Limited

## 3．Application Site 申請地點

|  | Whether the application directly relates to any specific site？ <br> 申請是否直接與某地點有關？ | Yes 是 $\square$ <br> No 否 （Please proceed to Part 6 請繼續填寫第 6 部分） |
| :---: | :---: | :---: |
| （b） | Full address／location／demarcation district and lot number（if applicable） <br> 詳細地址／地點／丈量約份及地段號碼（如適用） | Lots Nos． 402 （part）， 403 （part）， 404 （part），405，Section A of Lot No．406，the Remaining Portion of Lot No．406，Section A of Lot No．407，the Remaining Portion of Section B of Lot No．407，the Remaining Portion of Lot no．407，Section A of Lot No． 409 （part），the Remaining Portion of Section B of Lot No．409，the Remaining Portion of Lot no．409，the Remaining Portion of Lot No．410，the Remaining Portion of Lot No．411，Lot Nos．412，414，the Remaining Portion of Section A of Lot No． 418 （part），the Remaining Portion of Lot No． 418 and adjoining government land in Demarcation District 210；The remaining Portion of Lot No． 1860 （part），the Remaining Portion of Section A of Lot No． 1861 and adjoining government land in Demarcation District 244 at Ho Chung， Sai Kung，New Territories，Hong Kong |
| （c） | Site Area 申請地點面積 | 3，190 $\quad \square$ About 約 |

## Gist of Application 申請摘要

（Please provide details in both English and Chinese as far as possible．This part will be circulated to relevant consultees，uploaded to the Town Planning Board＇s Website for browsing and free downloading by the public and available at the Planning Enquiry Counters of the Planning Department for general information．）
（請盡量以英文及中文填寫。此部分將會發送予相關諮詢人士，上載至城市規劃委員會網頁供公眾免費瀏覽及下載及於規劃署規劃資料查詢處供一般參閱。）

| Application No．申請編號 | （For Official Use Only）（請勿填寫此欄） |
| :---: | :---: |
| Location／address位置／地址 | Lots Nos． 402 （part）， 403 （part）， 404 （part），405，Section A of Lot No．406，the Remaining Portion of Lot No．406，Section A of Lot No．407，the Remaining Portion of Section B of Lot No．407，the Remaining Portion of Lot no．407，Section A of Lot No． 409 （part），the Remaining Portion of Section B of Lot No．409，the Remaining Portion of Lot no．409，the Remaining Portion of Lot No．410，the Remaining Portion of Lot No．411，Lot Nos．412，414，the Remaining Portion of Section A of Lot No． 418 （part），the Remaining Portion of Lot No． 418 and adjoining government land in Demarcation District 210；The remaining Portion of Lot No． 1860 （part），the Remaining Portion of Section A of Lot No． 1861 and adjoining government land in Demarcation District 244 at Ho Chung，Sai Kung，New Territories，Hong Kong |
| Site area地盤面積 | 3,190 sq．m 平方米 $\square$ About 約 <br> （includes Government land of 包括政府土地 606 sq．m 平方米 $\square$ About 約） |
| Plan <br> 圖則 | Approved Ho Chung Outline Zoning Plan No．S／SK－HC／11 |
| Zoning地帶 | Residential（Group D） Residential（Group E） Area shown as＇Road＇ |
| Proposed Amendment（s）擬議修訂 | $\checkmark$ Amend the Covering Notes of the Plan <br> 修訂圖則《註釋》的說明頁 <br> －Amend the Notes of the zone applicable to the site <br> 修訂適用於申請地點土地用途地帶的《註釋》 <br> ＂Residential（Group D）＂，＂R（D）＂）， <br> Rezone the application site from $\qquad$ to＂Residential（Group C）3）＂（＂R（C）3＂）把申請地點由 $\qquad$地帶改劃為 $\qquad$ |

## Development Parameters（for indicative purpose only）發展參數（只作指示用途）

| （i） | Gross floor area and／or plot ratio總樓面面積及／或地積比率 |  | sq．m 平方米 |  | Plot Ratio 地積比率 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Domestic住用 | 2，393 | About 約 Not more than不多於 | 0.75 | －About 約 $\square$ Not more than不多於 |
|  |  | Non－domestic非住用 |  | About 約 Not more than不多於 |  | $\square$ About 約 <br> $\square$ Not more than不多於 |
| （ii） | No．of block幢數 | Domestic住用 | 8 |  |  |  |
|  |  | Non－domestic非住用 |  |  |  |  |
|  |  | Composite綜合用途 |  |  |  |  |

Demarcation District 210 held under Block Government Lease

| Lot No. |
| :--- |
| Lot No. 402 (part) |
| Lot No. 403 (part) |
| Lot No. 404 (part) |
| Lot No. 405 |
| Section A of Lot No. 406 |
| Remaining Portion of Lot No. 406 |
| Section A of Lot No. 407 |
| Remaining Portion of Section B of Lot No. 407 |
| Remaining Portion of Lot No. 407 |
| Section A of Lot No. 409 (part) |
| Remaining Portion of Section B of Lot No. 409 |
| Remaining Portion of Lot No. 409 |
| Remaining Portion of Lot No. 410 |
| Remaining Portion of Lot No. 411 |
| Lot No. 412 |
| Lot No. 414 |
| Remaining Portion of Section A of Lot No. 418 (part) |
| Remaining Portion of Lot No. 418 |
| Adjoining government land in Demarcation District 210 |
| Table 3.6 Lots in Demaration District 210 |

Table 3.6 Lots in Demarcation District 210

## Demarcation District 244 held under Block Government Lease

| Lot No. |
| :--- |
| Remaining Portion of Lot No. 1860 (part) |
| Remaining Portion of Section A of Lot No. 1861 |
| Adjoining government land in Demarcation District 244 |

Table 3.7 Lots in Demarcation District 244
3.5.2 For more efficient land utilisation and better configuration, the Applicant will undertake a land exchange process of 're-acquired and regrant' upon approval of this rezoning. It is proposed to re-acquired an area of about $453 \mathrm{sq} . \mathrm{m}$. that were previous allotted to the Government for road works and to regrant an area of about 153 sq.m. [refer to Figure 3.7]. Thereinto, parts of the private land (highlighted in pink and purple in Figure 3.7) are currently occupied by Luk Mei Tsuen Road, which the Applicant intends to grant right of way and to devote it for public use.

## Attachment 2

Traffic Impact Assessment (Version B)

## Traffic Impact Assessment

For
Amendment of Plan to
Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" (" $\mathrm{R}(\mathrm{E})$ ") and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3")
on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11
at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government Land

Ho Chung, Sai Kung, New Territories, Hong Kong

Prepared by: Prudential Surveyors (Hong Kong) Limited Version: B
Date: November 2023

## TABLE OF CONTENT

1. Introduction ..... 4
1.2 Study Objectives ..... 4
2. Proposed Development ..... 5
3. Existing Traffic Situation. ..... 5
3.1 Existing Road Network ..... 5
3.2 Public Transport ..... 5
3.3 Future Road Network .....  .6
3.4 Traffic Count Surveys .....  .6
3.5 Existing Capacity Assessment ..... 7
4. Future Traffic Situation ..... 8
4.1 2028 Design Year Road Network .....  8
4.2 Traffic Generation .....  8
4.3 Regional Traffic Growth .....  9
4.4 Reference and Design Flows ..... 10
4.5 Capacity Assessment Construction Stage and After Project Completion. ..... 11
5. Transport Provision ..... 14
5.1 Parking and Loading/Unloading Provision ..... 14
5.2 Hong Kong Planning Standards and Guidelines (HKPSG) ..... 15
5.3 Ingress/Egress Points and Internal Manoeuvring ..... 16
6. Conclusions ..... 16

## List of Figures

Figure 1.1 Study Area and Area of Influence
Figure 3.1 Location of Public Transport
Figure 3.2 Location of the Stage 2 of the Hiram's Highway Improvement Project
Figure $3.3 \quad$ Key Junctions
Figure $3.4 \quad 2023$ Observed Peak Hours Traffic Flows
Figure 4.12028 Net Peak Hours Development Traffic Flows
Figure 4.2 2028 Reference Peak Hours Traffic Flows
Figure 4.32028 Design Peak Hours Traffic Flows
Figure $4.4 \quad 2025$ Reference Peak Hours Traffic Flows
Figure $4.5 \quad 2025$ Design Peak Hours Traffic Flows
Figure 4.6 2025 Net Peak Hours Construction Traffic Flows
Figure 5.1 Internal Traffic Layout

## List of Tables

Table 2.1 Proposed GFA of Houses
Table 3.1 Service Provision of Public Transport
Table 3.2 Existing Junction Performance
Table 3.3 Existing Link Performance
Table 4.1 AM/PM Peak Generation and Attraction
Table 4.2 Traffic Data from Annual Traffic Census Reports
Table $4.3 \quad$ Projected Population by TPEDM, 2019-2031
Table $4.4 \quad 2025$ Construction Stage Junction Capacity
Table $4.5 \quad 2025$ Construction Stage Link Capacity
Table $4.6 \quad 2028$ Junction Capacity Assessments
Table $4.7 \quad 2028$ Link Capacity
Table 5.1 Provision of Internal Transport
Table 5.2 HKPSG Requirement and Provision

## List of Appendix

Appendix A Junction Analysis

## 1. Introduction

1.1.1 This Traffic Impact Assessment (TIA) is prepared as part of the Section 12A Application for the amendment of plan to rezone to "Residential (Group C) 3 " ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 (the Approved OZP) at various lots in Demarcation District 210 (D.D.210) and Demarcation District 244 (D.D.244) and adjoining government land, at Ho Chung, Sai Kung, New Territories (the Site) with a Site area about 3,190 sq.m. [Figure 1.1]
1.1.2 The TIA is required as part of the Section 12A planning application for the Proposed Development for rezone the Subject Site from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E)") and an area shown as 'Road' to "Residential (Group C) 3 " (" $\mathrm{R}(\mathrm{C}) 3^{\prime \prime}$ ) zoned with a maximum site coverage of $25 \%$ and a maximum building height of 12 m with 3 storeys over one storey of carport PR of 0.75 on the Approved OZP.
1.1.3 The owner of the Site has the intention to construct six individual houses with six ancillary car parking spaces of 2.5 m X 5 m , six accessible visitor parking space of 3.5 m X 5 m and one light goods vehicles (LGV) loading/unloading bay $3.5 \mathrm{~m} \mathrm{X} \mathrm{7m} \mathrm{in} \mathrm{Parcel} \mathrm{A} \mathrm{\&} \mathrm{B}$ of the Site, and two individual houses with two ancillary car parking spaces of 2.5 m X 5 m , two accessible visitor parking space of 3.5 m X 5 m in Parcel C of the Site.
1.1.4 This traffic impact assessment (TIA) study is to support the proposed development. This report describes the traffic impact assessment undertaken.

### 1.2 Study Objectives

1.2.1 The objectives of this study can be summarised as follows:

- undertake traffic impact assessment to assess the traffic impact to be induced by the proposed development on the nearby road network in the vicinity of the Subject Site;
- design and conduct traffic surveys during peak hours in the vicinity of the Subject Site to supplement available information and traffic data;
- estimate the extra volumes of traffic that will be generated by the proposed development during the peak period (arrivals and departures);
- estimate the likely changes of circulation patterns and traffic flow in the future road network adjacent to the Subject Site;
- review the capacity of the critical links of the road networks adjacent to the Subject Site;
- provide traffic advice on the internal vehicular movements; and
- advise on the provision of internal parking and loading and unloading spaces based on relevant standards and requirements for residential development.


## 2. Proposed Development

2.1.1 The proposed development is to erect six individual houses in Parcel A \& B of the Site and two individual houses in Parcel C of the Site. The proposed gross floor area (GFA) of the houses are summarised in Table 2.1.

| Propose House | Gross Floor Area (GFA) (sqm) (about) |
| :--- | :---: |
| House 1 | 283 |
| House 2 | 283 |
| House 3 | 283 |
| House 4 | 283 |
| House 5 | 283 |
| House 6 | 283 |
| House 7 | 346 |
| House 8 | 346 |
| Total | 2,390 |
| Average Size | 299 |

Table 2.1 Proposed GFA of Houses
2.1.2 The proposed development would adopt a household size of 4 per house. In this connection, a total population of 32 would be used.

## 3. Existing Traffic Situation

### 3.1 Existing Road Network

3.1.1 The Site is located at Ho Chung North Road (former Luk Mei Tsuen Road), which is a Feeder Road with single-two carriageway connecting to Hiram's Highway to the east.
3.1.2 The connecting section of Hiram's Highway was a Rural Road improved in 2021 year, from single-two carriageway to dual-two carriageway.
3.1.3 The critical road links and junctions in this study are, from north to south:

- J1 - Hiram's Highway / Marina Cove North Access
- J2 - Hiram's Highway / Marina Cove South Access
- L1 - Hiram's Highway between Ho Chung North Road (former Luk Mei Tsuen Road) and Ho Chung Road
- J3 - Hiram's Highway / Ho Chung Road
- L2 - Hiram's Highway between Ho Chung Road and Nam Pin Wai Road
- J4 - Hiram's Highway / New Hiram’s Highway / Nam Pin Wai Road (Roundabout)
3.1.4 The Area of Influence (AoI) and Study Area are shown in Figure 1.1.


### 3.2 Public Transport

3.2.1 Public transport services include franchised bus, green minibus (GMB) and public light bus (PLB) in the vicinity are depicted in Figure 3.1 and summarised in Table 3.1.

Rezone from "Residential (Group D)" ("R(D)"), ""Residential (Group E)" (" $R(E)$ ") and an area shown as 'Road' to
"Residential (Group C)3" (" $R(C) 3$ ") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11
Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government Land Ho Chung, Sai Kung, New Territories, Hong Kong

| Franchised Bus |  |  |  |
| :--- | :--- | :--- | :---: |
| Route | Destination | Frequency (min) |  |
| 92 | Sai Kung - Diamond Hill Station | $12-20$ |  |
| $92 R$ | Sai Kung - Star Ferry | 20 (Sunday and Holidays only) |  |
| $96 R$ | Wong Shek Pier - Diamond Hill <br> Station | $18-25$ (Sunday and Holidays <br> only) |  |
| 292P | Sai Kung - Kwun Tong | $7: 30$ (Only one departure <br> Monday to Friday) |  |
| 792M | Sai Kung - Tseung Kwan O Station | $15-20$ |  |
| Green Minibus (GMB) Services |  |  |  |
| 1 | Sai Kung - Kowloon Bay | $8-20$ |  |
| 1 A | Sai Kung - San Po Kong | 4 |  |
| 1 S | Sai Kung - San Po Kong | $10-15$ |  |
| 2 | Sai Kung - Ho Chung | $15-30$ |  |
| 12 | Sai Kung - Po Lam | $10-15$ |  |
| 101 M | Sai Kung - Hang Hau Station | $3-5$ |  |
| Public Light Bus (PLB) Services |  |  |  |
| -- | Sai Kung - Kwun Tong | $5-12$ |  |
| -- | Sai Kung - Mong Kok | Depart when fully loaded |  |
| -- | Sai Kung - Causeway Bay | $10-15$ |  |

Table 3.1 Service Provision of Public Transport

### 3.3 Future Road Network

3.3.1 To support the continued development and population growth in Sai Kung Area, Hiram's Highway Improvement is divided into two stages. Stage 1 between Clear Water Bay Road and Marina Cove has been completed in 2021. The works include improvement works that would relieve the traffic congestion on the road section near Marina Cove, enhance the safety of the road section and improve the local access to Ho Chung and Luk Mei Tsuen.
3.3.2 Stage 2 is to improve the section of Hiram's Highway, Po Tung Road and Tai Mong Tsai Road from Marina Cove to the south of Sha Ha. The proposed improvement works will relieve traffic congestion and enhance the safety of the road section at Sai Kung area. The project is currently under review and the commencement date is under review. The location of the improvements for Stage 2 are presented in Figure 3.2.

### 3.4 Traffic Count Surveys

3.4.1 In order to appraise the actual traffic demand for the proposed development, classified turning movement count surveys are carried out during peak hours, 07:00 to 10:00 and 17:00 to 20:00 on both Friday, 3 November 2023 and Sunday, 5 November 2023 at the key junctions of the study area as presented in Figure 3.3.
3.4.2 The traffic count survey data were recorded in a 15 minutes interval, and to be converted into pcu per hour. The highest hourly traffic volume is adopted as the peak hour traffic flow.
3.4.3 The morning and afternoon peak hours during weekday of the road network have been identified as 08:00 to 09:00 and 17:15 to 18:15 respectively. Meanwhile the peak hour of the weekend was observed to be 16:30 to 17:30. The observed traffic flows in the study area is presented in Figure 3.4.

### 3.5 Existing Capacity Assessment

## Junction Capacity

3.5.1 Based on the observed traffic flows, the performance of the key junctions in the vicinity of the subject site during the morning and evening peak hours were assessed. The results area summarised and presented in Table 3.2 and the detailed calculation sheets are attached in Appendix A.
3.5.2 The Design Flow / Capacity (DFC) ratio is measured in evaluating the performance of a roundabout or priority junction. With reference to Ch4, Vol2, TPDM, a DFC ratio of 0.85 can be considered reasonable.
3.5.3 The performance of a traffic signalised junction is indicated by its reserved capacity (RC). A positive RC indicates that the junction is operating with spare capacity. A negative RC indicates that the junction is overloaded; resulting in traffic queues and longer delay.

| Jun <br> No. | Junction Location | Type/ Capacity <br> Index | AM Peak <br> Hour | PM Peak <br> Hour | Weekend <br> Peak Hour |
| :---: | :---: | :---: | :---: | :---: | :---: |
| J1 | Luk Cheung Road <br> /Hiram's Highway <br> / Marina Cove <br> North Access | Priority / DFC | 0.06 | 0.04 | 0.04 |
| J2 | Luk Mei Tsuen <br> Road /Hiram's <br> Highway/ Marina <br> Cove South <br> Access | Signal / RC | $156 \%$ | $168 \%$ | $159 \%$ |
| J3 | Ho Chung Road <br> /Hiram's Highway | Signal / RC | $106 \%$ | $144 \%$ | $109 \%$ |
| J4 | Nam Pin Wai <br> Road / New <br> Hiram's Highway <br> / Hiram's <br> Highway | Roundabout / DFC | 0.60 | 0.52 | 0.55 |
| J5 | Hing Keng Shek <br> Road / Hiram's <br> Highway | Roundabout / DFC | 0.51 | 0.55 | 0.49 |

Notes: RC=reserved capacity; DFC=Design Flow/ Capacity Ratio
Table 3.2 Existing Junction Performance
3.5.4 It can be observed in Table 3.2 that all of the key junctions perform satisfactorily during peak hours with adequate reserved capacities.

## Link Capacity

3.5.5 Considering the routing of development traffic and construction traffic, link capacity of Sai Kung bound of L1 and L2, and Kowloon bound of L2 are assessed.
3.5.6 The result of road link capacity assessment is summarised in Table 3.3. With reference to para 10.6.4.5, Vol6, TPDM, the desirable limit of volume to capacity (V/C) ratio is less than 0.85 for links.

| Link No. | Section of <br> Hiram's <br> Highway | Link <br> Capacity <br> (veh/hr) | Reference Flow |  | Reference V/C Ratio |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Daily <br> Peak | Weekend | Daily <br> Peak | Weekend |  |  |
| L1 <br> (Sai Kung <br> Bound) | Between Ho <br> Chung Road <br> and Luk Mei <br> Tsuen Road | 2600 | 1080 | 940 | 0.42 | 0.36 |
| L2 <br> (Sai Kung <br> Bound) | Between Ho <br> Chung Road <br> and Nam Pin <br> Wai Road | 2600 | 1008 | 1188 | 0.39 | 0.46 |
| L2 <br> (Kowloon <br> Bound) | Between Ho <br> Chung Road <br> and Nam Pin <br> Wai Road | 2600 | 1184 | 1064 | 0.46 | 0.41 |

Notes: Based on TPDM Volume 2 Chapter 2.4 - Design Flow Characteristics, it is assumed 2600 veh/hour for dual two-lane carriageway for one direction of flow.

Table 3.3 Existing Link Performance
3.5.7 It can be seen from Table 3.3 that all of the key links are within design capacities.

## 4. Future Traffic Situation

### 4.1 2028 Design Year Road Network

4.1.1 The anticipated year of completion for the proposed development is 2025. The design year is either 3 years after the completion year or 5 years after the application year, which ever longer. Therefore, Year 2028 is adopted as the design year of this study.

### 4.2 Traffic Generation

4.2.1 The proposed development is intended for eight single-family houses with an average size of 299 sq.m. It is proposed that there will only be 16 parking spaces.
4.2.2 The estimated average traffic generation and traffic attraction rate at peak hours are based on the trip rate based on the Transport Planning and Design Manual published by the Transport Department and are summarised in Table 4.1.

Traffic Impact Assessment for Amendment of Plan
Rezone from "Residential (Group D)" (" $R(D)$ "), ""Residential (Group E)" (" $R(E)$ ") and an area shown as 'Road' to
"Residential (Group C)3" (" $R(C) 3$ ") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11
Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government Land Ho Chung, Sai Kung, New Territories, Hong Kong

| Description | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | Generation | Attraction | Generation | Attraction |
| Trip Rate (pcu/unit/hr) | 0.3252 | 0.2609 | 0.2835 | 0.4074 |
| Private Housing: Low- <br> Density / R(C) (pcu/hr) <br> (8 units) | 2.6 | 2.1 | 2.3 | 3.3 |

Note 1: As the Site is used as a single-family house, the commutes would take place once in the morning and once in the afternoon to/from work/school.
Note 2: The pcu of a private car is taken as 1.
Note 3: Morning peak is defined as 8:00 a.m. to 9:00 a.m. whereas afternoon peak is defined as 6:00 p.m. to 7:00 p.m.

Table $4.1 \mathrm{AM} / \mathrm{PM}$ Peak Generation and Attraction
4.2.3 As shown in Table 4.1, the proposed development would generate $3(2)$ pcus and attract $2(3)$ pcus in the morning (evening) peak hours, which is considered negligible.
4.2.4 The development traffic was re-distributed and assigned onto the existing road network. Figure 4.1 show that resulting assignment of the proposed development traffic.

### 4.3 Regional Traffic Growth

4.3.1 For the estimation of traffic flows in the design year of 2028, it is proposed to adjust the existing traffic flows to take into account of the natural traffic growth which is related to the increase in car usage.

## Annual Traffic Census (ATC)

4.3.2 Reference has been made with uses of 2017 to 2022 (Latest) Annual Traffic Census Reports. The traffic data recorded at counting stations adjacent to the site are shown in Table 4.2.

| Station No./Road <br> Name | $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 9}$ | $\mathbf{2 0 2 0}$ | $\mathbf{2 0 2 1}$ | $\mathbf{2 0 2 2}$ | Growth <br> per <br> Annum |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6055/ Hiram's <br> Highway | 24,050 | 24,450 | 24,280 | 23,360 | 24,460 | 23,480 | $-\mathbf{0 . 4 8 \%}$ |
| 5017/ Clear Water <br> Bay Road | 26,910 | 28,450 | 28,980 | 28,900 | 29,100 | 27,720 | $\mathbf{0 . 5 9 \%}$ |
| 5466 / Clear Water <br> Bay Road | 18,650 | 18,950 | 20,240 | 19,110 | 20,020 | 19,140 | $\mathbf{0 . 5 2 \%}$ |
| 6056/ Sai Sha Road | 10,990 | 11,880 | 11,800 | 11,350 | 11,880 | 11,520 | $\mathbf{0 . 9 5 \%}$ |
| Total Growth per Annum |  |  | $\mathbf{0 . 3 1 \%}$ |  |  |  |  |

Source: Annual Traffic Census, Transport Department
Table 4.2: Traffic Data from Annual Traffic Census Reports
4.3.3 It is noted from Table 4.2 that $+0.31 \%$ annual growth is observed from the traffic flow record over the past five years.

## Territory Population and Employment Data Matrices (TPEDM)

4.3.4 According to the latest 2019-based TPEDM from year 2019 to year 2031 in Southeast New Territories (Other Area) published on the PlanD website. The population growth from the base year 2019 to 2031 is $-1.18 \%$ as shown in Table 4.3.

| Planning Data District | Year 2019 | Year 2026 | Year 2031 | Growth Rate <br> p.a. (\%) |
| :--- | :---: | :---: | :---: | :---: |
| Southeast New <br> Territories (Other Area) | 68,900 | 65,800 | 59,750 | $-1.18 \%$ |

Table 4.3 Projected Population by TPEDM, 2019-2031
4.3.5 After comparing the historical data and the future planning data, for conservative purpose, an annual growth rate of $+1.00 \%$ was adopted.

### 4.4 Reference and Design Flows

4.4.1 The anticipated year of completion and estimated year of population intake of the proposed development is 2025. The design year for assessment is 3 years after the completion year, i.e. Year 2028, is adopted as the design year of this study.
4.4.2 The growth factor derived in Section 4.3 will be applied to of 2023 observed peak hours traffic flows
4.4.3 The traffic generated by 2 planned developments in the study area will also be considered:

| Application | GFA <br> (m2) | Average <br> Flat Size <br> (m2) | No. of <br> Houses | AM <br> Generation | AM <br> Attraction | PM <br> Generation | PM <br> Attraction |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rate (pcu/hr/flat) |  |  |  |  |  |  | 0.2772 |
| 0.1769 | 0.1635 | 0.2394 |  |  |  |  |  |
| A/SK- <br> HC/271 | 2421.6 | 161.4 | 15 | 4.2 | 2.7 | 2.5 | 3.6 |
| A/SK- <br> HC/340 | 9386 | 195.5 | 48 | 13.3 | 8.5 | 7.8 | 11.5 |

4.4.4 The reference and design flows for design year 2028 are calculated from the following formulae:

2028 Reference Flows $=2023$ Observed Flows $x(1+1.00 \%)^{\wedge} 5+$ Traffic Flows
Generated by Adjacent Planned Developments
2028 Design Flows $=2028$ Reference Flows + Proposed Development Traffic
4.4.5 Based on the observed traffic flows and pattern of existing and future road network, the 2028 peak hour Reference Flows at the critical junctions are presented in Figure 4.2. Meanwhile, the design Flows are presented in Figure 4.3.

### 4.5 Capacity Assessment Construction Stage and After Project Completion

## Construction Stage Junction Capacity

4.5.1 Based on similar projects, it is assumed that the development would generate 3(3) and attract 3(3) no. of construction vehicles (i.e. generate 6(6) and attract 6(6) pcus), in the morning (afternoon) peak hours during weekdays. The project is anticipated to be completed 2025. The reference peak hours traffic flows and design peak hours traffic flows are shown in Figures 4.4 and 4.5 respectively. The results are summarised and presented in Table 4.4 and shown in Figure 4.6.

| Jun <br> No. | Junction Location | Type/ Capacity Index | 2025 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Reference |  |  | Design |  |  |
|  |  |  | AM | PM | Week end | AM | PM | Week end |
| J1 | Luk Cheung Road /Hiram's Highway / Marina Cove North Access | Priority / DFC | No Construction Traffic |  |  |  |  |  |
| J2 | Luk Mei Tsuen Road /Hiram's Highway/ Marina Cove South Access | Signal / RC | Construction Traffic Free Flow from Hiram's Highway Northbound Left Turning to Luk Mei Tsuen Road |  |  |  |  |  |
| J3 | Ho Chung <br> Road <br> /Hiram's <br> Highway | Signal / RC | 102\% | 139\% | N/A | 100\% | 137\% | N/A |
| J4 | Nam Pin Wai Road / New Hiram's Highway / Hiram's Highway | Roundabout <br> / DFC | 0.61 | 0.53 | N/A | 0.61 | 0.53 | N/A |
| J5 | Hing Keng <br> Shek Road <br> / Hiram's <br> Highway | Roundabout <br> / DFC | 0.52 | 0.56 | N/A | 0.52 | 0.56 | N/A |

Notes: RC=reserved capacity; DFC=Design Flow/ Capacity Ratio
Table 4.4 2025 Construction Stage Junction Capacity
4.5.2 According to Table 4.4, the capacity of all the keys junctions would be performing satisfactorily during the peak periods for both the Reference and Design Scenarios.

## Construction Stage Link Capability

4.5.3 The link capacity assessment results with reference to the net development are summarised in Table 4.5.

| Link No. | Section of Hiram's Highwa y | $\begin{gathered} \text { Link } \\ \text { Capacit } \\ \text { y } \\ \text { (veh/hr) } \end{gathered}$ | Reference Flow |  | Reference V/C Ratio |  | Design Flow |  | $\begin{gathered} \hline \text { Design V/C } \\ \text { Ratio } \end{gathered}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Daily <br> Peak | Week end | Daily <br> Peak | Week end | Daily <br> Peak | Week end | Daily <br> Peak | Week end |
| L1 (Sai Kung Bound) | Between Ho Chung Road and Luk Mei Tsuen Road | 2600 | 1102 | N/A | 0.42 | N/A | 1108 | N/A | 0.43 | N/A |
| L2 <br> (Sai Kung Bound) | Between Ho Chung Road and Nam Pin Wai Road | 2600 | 1208 | N/A | 0.41 | N/A | 1214 | N/A | 0.47 | N/A |
| L2 <br> (Kowloo <br> n Bound) | Between <br> Ho <br> Chung <br> Road <br> and <br> Nam Pin <br> Wai <br> Road | 2600 | 1313 | N/A | 0.50 | N/A | 1319 | N/A | 0.51 | N/A |

Notes: Based on TPDM Volume 2 Chapter 2.4 - Design Flow Characteristics, it is assumed 2600 veh/hour for dual two-lane carriageway for one direction of flow.

Table 4.5 2025 Construction Stage Link Capacity
4.5.4 It can be seen from Table 4.5 that all of the key links perform satisfactorily during the peak hours with adequate reserve capacities.

## Future Junction Capacity

4.5.5 The widening of Hiram's Highway was completed in 2021, the new signalised junction at Ho Chung Road has been assessed. Capacity assessments were carried out for the major junctions in the local network for both the Reference and Design scenarios. The results are summarised and presented in Table 4.6 with detailed calculations sheets attached in Appendix A.

Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E)") and an area shown as 'Road' to
"Residential (Group C)3" ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11
Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government Land Ho Chung, Sai Kung, New Territories, Hong Kong

| Jun <br> No. | Junction Location | Type/ Capacity Index | 2028 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Reference |  |  | Design |  |  |
|  |  |  | AM | PM | Week end | AM | PM | Week end |
| J1 | Luk Cheung <br> Road <br> /Hiram's <br> Highway / <br> Marina <br> Cove North <br> Access | Priority / DFC | 0.07 | 0.04 | 0.04 | 0.07 | 0.04 | 0.04 |
| J2 | Luk Mei Tsuen Road /Hiram's Highway/ Marina Cove South Access | Signal / RC | 141\% | 153\% | 144\% | 141\% | 153\% | 144\% |
| J3 | Ho Chung <br> Road /Hiram's Highway | Signal / RC | 94\% | 130\% | 97\% | 93\% | 130\% | 96\% |
| J4 | Nam Pin Wai Road / New Hiram's Highway / Hiram's Highway | Roundabout <br> / DFC | 0.64 | 0.55 | 0.58 | 0.64 | 0.55 | 0.59 |
| J5 | Hing Keng Shek Road / Hiram's Highway | Roundabout <br> / DFC | 0.54 | 0.59 | 0.51 | 0.54 | 0.59 | 0.51 |

Notes: RC=reserved capacity; DFC=Design Flow/ Capacity Ratio
Table 4.62028 Junction Capacity Assessments
4.5.6 According to Table 4.6, the capacity of all the key junctions would be preforming satisfactory during the peak periods for bother the Reference and Design Scenarios.

## Future Link Capacity

4.5.7 The road link capacity assessment results with reference to the development traffic are summarised in Table 4.7.

| Link No. | Section of Hiram's Highwa y | Link Capacit y (veh/hr) | Reference Flow |  | Reference V/C Ratio |  | Design Flow |  | Design V/C Ratio |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Daily <br> Peak | Week end | Daily Peak | Week end | Daily <br> Peak | Week end | Daily <br> Peak | Week end |
| L1 <br> (Sai Kung Bound) | Between Ho Chung Road and Luk Mei Tsuen Road | 2600 | 1147 | 999 | 0.44 | 0.38 | 1150 | 1003 | 0.44 | 0.39 |
| L2 <br> (Sai Kung Bound) | Between Ho Chung Road and Nam Pin Wai Road | 2600 | 1256 | 1129 | 0.41 | 0.48 | 1259 | 1132 | 0.48 | 0.44 |
| L2 <br> (Kowloo <br> n Bound) | Between Ho Chung Road and Nam Pin Wai Road | 2600 | 1370 | 1273 | 0.53 | 0.49 | 1373 | 1275 | 0.53 | 0.49 |

Notes: Based on TPDM Volume 2 Chapter 2.4 - Design Flow Characteristics, it is assumed 2600 veh/hour for dual two-lane carriageway for one direction of flow.

Table 4.7 2028 Link Capacity
4.5.8 Table 4.7 demonstrates that all of the key links perform satisfactorily during peak hours with adequate reserve capacities after completion of the improvement works.

## 5. Transport Provision

### 5.1 Parking and Loading/Unloading Provision

5.1.1 With reference to the proposed plan, 12 car parking spaces ( 6 ancillary carparking spaces and 6 accessible/visitor parking space) and one LGV loading/unloading bay for the residential development are proposed to serve the needs occupants in Parcel A \& B and 4 car parking spaces ( 2 ancillary carparking spaces and 2 accessible/visitor parking space) are proposed to serve the needs occupants in Parcel C. This is summarised in Table 5.1.

| Type of Parking Space/Bay | Provision |
| :--- | :---: |
| Parcel A \& B for 6 Houses |  |
| Private Car (2.5m X 5m) | 6 |
| Accessible Visitor (3.5X 5m) | 6 |
| Loading/Unloading Bay (3.5 X 7m) | 1 |
| Parcel C for 2 Houses |  |
| Private Car (2.5m X 5m) | 2 |
| Accessible Visitor (3.5X 5m) | 2 |

Table 5.1 Provision of Internal Transport

### 5.2 Hong Kong Planning Standards and Guidelines (HKPSG)

5.2.1 The car parking requirements and loading/unloading provisions for the proposed development in accordance with the HKPSG are listed in Table 5.2.

| Development | Facility | HKPSG Standard | Required | Provision |
| :---: | :---: | :---: | :---: | :---: |
| Residential (8 units with avg. <br> size of 299 <br> sqm) | Car Parking | Global Parking Standard (GPS) = 1 <br> Car space per 4-7 <br> flats <br> R1 = 7.0 for avg. flat size over 160 sqm R2 = 1 (outside a 500 m radius of rail station) R3 $=1.3$ of domestic plot ratio 0.00-1.00 | 11-19 | 16 |
|  | Loading/Unloading Bay | Minimum of 1 Loading/Unloading Bay for goods vehicles within the site for every 800 flats or part thereof, subject to a minimum of 1 bay for each housing block or as determined by the Authority. | 1 | 1 |

Table 5.2 HKPSG Requirement and Provision

### 5.3 Ingress/Egress Points and Internal Manoeuvring

5.3.1 The proposed ingress and egress point to all Parcels of the Site will be from Ho Chung North Road. In all Parcels of the Site, adequate maneuvering space is proposed for the maneuvering within the Site for the vehicles such that no vehicle queuing outside the Site would occur as a result of the proposed developments. In addition, there will be no reverse onto/from Ho Chung North Road to the Site. [Figure 5.1]

## 6. Conclusions

6.1.1 The traffic generation from the proposed development (including the construction period) is minimal in nature and will have will have minimal traffic impact to the surrounding network.
6.1.2 The proposed development would provide a total of 16 carparking spaces and 1 loading/unloading bay which fulfills the requirements of HKPSG.
6.1.3 The proposed development will provide adequate maneuvering space within all Parcels of the Site. Therefore, no queuing or reversing motion will occur at the street level.
6.1.4 As a result, it is concluded that the proposed development would not generate any significant adverse impact to the traffic of the surrounding vicinity of the Site.


| PRUDENTIAL <br>  $\begin{array}{ll}\text { TE: } & 25078333 \\ \text { FAX: } & 25988578\end{array}$245 DES VOEUX ROND CENTRN HONO KONG <br>  <br> 103 |  | JOB titte: <br> Amendment of Plan to Rezone from "Residential (Group D)" (R(D)"), "Residential (Group EE) "R(E)") and an area shown as 'Road to "Residential (Group E)1) ("R(E) $\left.(1)^{*}\right)$ No. S/SK-HC/11 at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjioining Gcvernment land, Ho Chung, Sal Kıng, New Teritiories, Hong Kong | STUDY AREA AND AREA OF INFLUENCE |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
|  |  | Rev |  | Dascriplion | Date |
















| PRUDENTIAL |  | JOB titLe: <br> Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E") "R(EE)" and an area shown as 'Road' to "Residential (Group E)11) "R(E)") or "Residential (Group (c)1 (RCC)" on the Approved Ho chung Dutine Zoring Pan No. $\mathrm{S} / \mathrm{SK}$-HC/11 at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land, Ho Chung, Sai Kung, New Teritiories, Hong Kong | Drawing Title <br> SWEPT PATH ANALYSIS <br> (PARCEL C) |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
|  |  | Rev |  | Descripion | Date |

Appendix A
Junction Calculations









































| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width ( m ) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
|  | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
|  | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 851 | 103 | 1287 | 40 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 82 | 973 | 154 | 1231 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
| S | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S}))$ | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2297 | 1454 | 2151 | 848 | Total In Sum = | 1429 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.37 | 0.07 | 0.60 | 0.05 | DFC of Critical Approach = | 0.60 |  |











| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 1045 | 57 | 1273 | 39 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 104 | 1165 | 105 | 1146 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2285 | 1356 | 2178 | 884 | Total $\ln$ Sum $=$ | 1369 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.46 | 0.04 | 0.58 | 0.04 | DFC of Critical Approach = | 0.58 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 907 | 108 | 1373 | 42 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 86 | 1024 | 162 | 1313 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2295 | 1428 | 2146 | 813 | Total In Sum = | 1522 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.40 | 0.08 | 0.64 | 0.05 | DFC of Critical Approach = | 0.64 |  |





| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 1048 | 57 | 1275 | 39 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 104 | 1168 | 105 | 1149 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2285 | 1355 | 2178 | 883 | Total In Sum = | 1371 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.46 | 0.04 | 0.59 | 0.04 | DFC of Critical Approach = | 0.59 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
| E | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 868 | 105 | 1313 | 40 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 83 | 992 | 157 | 1255 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | = | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | $0.21 * T d(1+0.2 *$ X2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2296 | 1444 | 2149 | 837 | Total In Sum = | 1458 | PCU |
| DFC | = | Design flow/Capacity = Q/Qe | 0.38 | 0.07 | 0.61 | 0.05 | DFC of Critical Approach = | 0.61 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 1171 | 65 | 1136 | 46 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 106 | 1318 | 159 | 1012 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | = | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $\left.1+0.2^{*} \mathrm{X} 2\right)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2284 | 1279 | 2148 | 941 | Total $\ln$ Sum $=$ | 1247 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.51 | 0.05 | 0.53 | 0.05 | DFC of Critical Approach = | 0.53 |  |



| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width ( m ) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 874 | 105 | 1319 | 40 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 83 | 998 | 163 | 1261 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2296 | 1441 | 2146 | 835 | Total $\ln$ Sum $=$ | 1464 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.38 | 0.07 | 0.61 | 0.05 | DFC of Critical Approach = | 0.61 |  |



| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 1177 | 65 | 1142 | 46 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 106 | 1324 | 165 | 1018 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
| S | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2284 | 1276 | 2145 | 939 | Total In Sum = | 1253 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.52 | 0.05 | 0.53 | 0.05 | DFC of Critical Approach = | 0.53 |  |



| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
|  | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
|  | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 927 | 55 | 1122 | 33 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 46 | 942 | 61 | 1170 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
| S | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S}))$ | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21^{*} \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2317 | 1469 | 2202 | 874 | Total In Sum = | 0 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.40 | 0.04 | 0.51 | 0.04 | DFC of Critical Approach = | 0.51 |  |





| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 897 | 30 | 1083 | 8 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 35 | 900 | 28 | 1100 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
| S | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * T d(1+0.2 *$ X2 $)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2323 | 1490 | 2220 | 904 | Total In Sum = | 0 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.39 | 0.02 | 0.49 | 0.01 | DFC of Critical Approach = | 0.49 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 992 | 58 | 1179 | 34 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 48 | 1007 | 81 | 1247 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2316 | 1436 | 2191 | 841 | Total In Sum = | 0 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.43 | 0.04 | 0.54 | 0.04 | DFC of Critical Approach = | 0.54 |  |





| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width ( m ) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 953 | 31 | 1138 | 8 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 37 | 956 | 39 | 1166 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S}))$ | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2322 | 1462 | 2214 | 876 | Total In Sum = | 0 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.41 | 0.02 | 0.51 | 0.01 | DFC of Critical Approach = | 0.51 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
|  | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
|  | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 992 | 58 | 1179 | 34 |  |  |  |
|  | $=$ | Circulating flow across entry (pcu/h) | 48 | 1007 | 81 | 1247 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
|  | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
|  | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | $0.21 * \mathrm{Td}\left(1+0.2^{*} \mathrm{X} 2\right)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | $\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2316 | 1436 | 2191 | 841 | Total $\ln$ Sum $=$ | 0 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.43 | 0.04 | 0.54 | 0.04 | DFC of Critical Approach = | 0.54 |  |





| ARM |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Approach half width ( m ) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=\quad$ Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ Entry flow (pcu/h) | 953 | 31 | 1138 | 8 |  |  |  |
| Qc | $=$ Circulating flow across entry (pcu/h) | 37 | 956 | 39 | 1166 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |
|  | $=$ Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=1-0.00347(\mathrm{~A}-30)-0.978(1 / \mathrm{R}-0.05)$ | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
|  | $=\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S}))$ | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=303 * \times 2$ | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=1+(0.5 /(1+\mathrm{M})$ ) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=0.21 * \mathrm{Td}(1+0.2 * \mathrm{X} 2)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=\mathrm{K}\left(\mathrm{F}-\mathrm{Fc}{ }^{*} \mathrm{Qc}\right)$ | 2322 | 1462 | 2214 | 876 | Total In Sum = | 0 | PCU |
| DFC | $=$ Design flow/Capacity $=$ Q/Qe | 0.41 | 0.02 | 0.51 | 0.01 | DFC of Critical Approach = | 0.51 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 946 | 56 | 1145 | 33 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 46 | 960 | 62 | 1193 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2317 | 1460 | 2201 | 864 | Total $\ln$ Sum $=$ | 0 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.41 | 0.04 | 0.52 | 0.04 | DFC of Critical Approach = | 0.52 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 1307 | 47 | 985 | 14 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 42 | 1296 | 37 | 999 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $\left.1+0.2^{*} \mathrm{X} 2\right)$ | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2319 | 1290 | 2215 | 947 | Total In Sum = | 0 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.56 | 0.04 | 0.44 | 0.01 | DFC of Critical Approach = | 0.56 |  |



| ARM |  |  | A | B | C | D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| INPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
| V | $=$ | Approach half width (m) | 7.5 | 6.0 | 7.6 | 3.5 |  |  |  |
|  | $=$ | Entry width (m) | 8.5 | 7.0 | 7.6 | 6.0 |  |  |  |
| L | $=$ | Effective length of flare (m) | 13.5 | 6.0 | 0.0 | 6.0 |  |  |  |
| R | $=$ | Entry radius (m) | 20.0 | 20.0 | 22.5 | 17.0 |  |  |  |
| D | $=$ | Inscribed circle diameter (m) | 78.0 | 78.0 | 78.0 | 78.0 |  |  |  |
| A | $=$ | Entry angle (degree) | 50.0 | 40.0 | 40.0 | 30.0 |  |  |  |
| Q | $=$ | Entry flow (pcu/h) | 946 | 56 | 1145 | 33 |  |  |  |
| Qc | $=$ | Circulating flow across entry (pcu/h) | 46 | 960 | 62 | 1193 |  |  |  |
| OUTPUT PARAMETERS: |  |  |  |  |  |  |  |  |  |
|  | $=$ | Sharpness of flare $=1.6(\mathrm{E}-\mathrm{V}) / \mathrm{L}$ | 0.12 | 0.27 | 0.00 | 0.67 |  |  |  |
| K | $=$ | 1-0.00347(A-30)-0.978(1/R-0.05) | 0.93 | 0.97 | 0.97 | 0.99 |  |  |  |
| X2 | $=$ | $\mathrm{V}+((\mathrm{E}-\mathrm{V}) /(1+2 \mathrm{~S})$ ) | 8.31 | 6.65 | 7.60 | 4.57 |  |  |  |
| M | $=$ | $\operatorname{EXP}((\mathrm{D}-60) / 10)$ | 6 | 6 | 6 | 6 |  |  |  |
| F | $=$ | 303*X2 | 2517 | 2016 | 2303 | 1385 |  |  |  |
| Td | $=$ | 1+(0.5/(1+M)) | 1.07 | 1.07 | 1.07 | 1.07 |  |  |  |
| Fc | $=$ | 0.21*Td( $1+0.2 *$ K2) | 0.60 | 0.52 | 0.57 | 0.43 |  |  |  |
| Qe | $=$ | K(F-Fc*Qc) | 2317 | 1460 | 2201 | 864 | Total $\ln$ Sum $=$ | 0 | PCU |
| DFC | $=$ | Design flow/Capacity = Q/Qe | 0.41 | 0.04 | 0.52 | 0.04 | DFC of Critical Approach = | 0.52 |  |




## Attachment 3

Sewerage and Drainage Impact Appraisal (Version B)

## Sewerage and Drainage Impact Appraisal

For<br>Amendment of Plan to<br>Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E)") and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3" on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land<br>Ho Chung, Sai Kung, New Territories, Hong Kong

| Prepared by: | Prudential Surveyors International Limited |
| :--- | :--- |
| Version: | A B |
| Date: | August November 2023 |

## TABLE OF CONTENTS

TABLE OF CONTENTS ..... 2

1. Introduction .....  4
1.1 Background .....  4
1.2 Site and its Surroundings .....  .4
1.3 Proposed Development. .....  4
2. Sewerage Impact Appraisal .....  5
2.1 Scope of Works .....  .5
2.2 Existing Sewerage Facilities .....  .5
2.3 Proposed Sewerage Treatment .....  .5
2.4 Assessment Criteria, Methodology and Assumptions .....  6
2.5 Estimation of Sewerage Flow .....  .6
2.6 Discussion .....  7
2.7 Conclusion .....  8
3. Drainage Impact Appraisal .....  8
3.1 Scope of Works. .....  8
3.2 Existing and Planned Drainage Facilities .....  8
3.3 Drainage Catchment Area .....  8
3.4 Drainage Calculations for the Proposed Provision of Drainage Facilities .....  9
3.5 Proposed Drainage System ..... 10
3.6 Discussion ..... 10
3.7 Conclusion ..... 10

## List of Figures

Figure 1.1 Location Plan
Figure 1.2 The Site and Its Surroundings
Figure 2.1 Existing DSD Utility Record Plan
Figure 2.2 Reference Septic Tank
Figure 2.3 Proposed Sewerage Layout Plan
Figure 3.1A Proposed Drainage Layout from the Hiram's Highway Improvement Stage 1 Project
Figure 3.1B Proposed Drainage Layout from the Hiram's Highway Improvement Stage 1 Project
Figure 3.2 Drainage Layout Plan
Figure 3.3 Catchment Area Plan
Figure 3.4 Proposed Drainage Connection
Figure 3.5 Design Calculation of Proposed Drainage System

## List of Tables

Table 1.1 Proposed GFA of Houses
Table $2.1 \quad$ Calculation of Septic Tank
Table 2.2 Estimated Sewerage Flow from the Site

## 1．Introduction

## 1．1 Background

1．1．1 This Sewerage and Drainage Appraisal is to support a planning permission from the Town Planning Board（TPB）under Section 12A of the Town Planning Ordinance（CAP． 131）for a proposed rezone of the Subject Site from＂Residential（Group D）＂（＂R（D）＂）， ＂Residential（Group E）＂（＂R（E）＂）and an area shown as＇Road＇to＂Residential（Group C）3）＂（＂R（C）3＂）within various lots within DD210 and DD244 and adjoining government land in Ho Chung，Sai Kung，New Territories．The application Site（the Site）is composed of 3 parcels，namely Parcel A，B and C．［refer to Figure 1．1］

1．1．2 The owner of the application Site has the intention to construct six individual houses with twelve car parking spaces in Parcels A and B of the Site and two individual house with four car parking spaces in Parcel C of the Site．

## 1．2 Site and its Surroundings

1．1．3 A site visit was carried out on 6 July 2023．Per the observations from the site visit，it was observed that the Site is situated in rural environs with a mixture of residential， industrial and storage uses with dwellings．［refer to Figure 1．2］The details of the surrounding are that：
－to the north of the Site is some 2 and 3 －storey rural housing；
－to the east of the Site are some car repair workshops and to the further east are residential blocks of Marina Cove；
－to the south of the Site is the former Production Centre of Asia Television Limited （abandoned）；and
－to the west of the Site is Luk Mei Village with a mixture of traditional single－storey village－type developments and modern 3 －storey New Territories Exempted Houses （NTEHs）．

1．1．4 Apart from residential buildings，there are scattered structures in the vicinity of the Site intended primarily for industrial uses including an unnamed warehouse，a motor repair workshop（Bayview Motors Company），a food factory under Koon Yick Food Manufacturing Company（冠益華記食品廠）（＂Koon Yick＂）．

## 1．3 Proposed Development

1．1．5 The Proposed Development is to erect six individual houses in Parcel A and B of the Site and two individual houses in Parcel C of the Site．The proposed gross floor area（GFA）of the houses are summarised in Table 1.1

| Propose House | Gross Floor Area（GFA）（sqm）（about） |
| :--- | :---: |
| House 1 | 283.35 |
| House 2 | 283.35 |
| House 3 | 283.35 |
| House 4 | 283.35 |
| House 5 | 283.35 |
| House 6 | 283.35 |
| House 7 | 345.75 |
| House 8 | 345.75 |
| Total | $2,391.6$ |
| Average | 298.95 |

Table 1．1 Proposed GFA of Houses

## 2. Sewerage Impact Appraisal

### 2.1 Scope of Works

The objective of this Sewerage Impact Appraisal (SIA) is to assess whether the capacity of the sewerage networking is sufficient to cope with the peak sewerage flow arising from the proposed comprehensive residential development.

Existing drainage record plan from the Drainage Services Department (DSD) is shown in Figure 2.1.

### 2.2 Existing Sewerage Facilities

According to the drainage record plan, there is no existing public sewerage network serving the Site. [refer to Figure 2.1]. Hence, the Site is an unsewered area at present.

### 2.3 Proposed Sewerage Treatment

In consideration that the Site is unsewered area, it is necessary to consider the provision of an on-site underground Sewerage Treatment Plant, which will be used for treatment of sewerage generated from the Proposed Development.

The applicant will be responsible for the construction, operation and maintenance of the on-site underground Sewerage Treatment Plant and all inter-connecting sewerage pipework (polyethylene pipes) within the Site. The sewerage collected from each house will be discharged to septic tank and soil soakway soakaway pit.

The design, operation and maintenance of the proposed underground Sewerage Treatment Plant are in compliance with EPD's Practice Note for Professional Person (ProPECC) PN 5/93. It is proposed to construct four eight entire underground Sewerage Treatment Plant (involve inlet trap, septic tank, outlet trap, inter-connecting pipes and soil soakway soakaway pit) for proposed houses. The proposed capacity of the each septic tank is 15.98 cu.m and it is greater than the estimated daily water consumption of each proposed house. A reference septic tank is illustrated in Figure 2.2 and the calculation of septic tank are shown in Table 2. For the proposed soil soakway soakaway pit, its size should be determined basing on soil absorption rate and therefore it should be determined in detail design stage.

|  |  | $(\mathrm{mm})$ |
| :--- | :--- | :--- |
| Proposed Septic Tank <br> Capacity | (L-t)x BxD | $(5700-150) 1600 \times 1800=$ <br> $15.98 \mathrm{cu} . \mathrm{m}$ |
| The proposed septic tank <br> System aims to serve one <br> house with 4 Nos. of <br> Person. |  |  |
| Estimate Ultimate per <br> capita daily water <br> consumption | Design Flow Rate <br> x Peak Factor | $0.37 \times 6=2.22$ <br> cu.m/person/day |
| Required Septic Tank <br> Capacity | Nos of Person Per House x <br> estimated daily water | $4 \times 2.22=8.88$ is less <br> Septic Tank Capacity |


|  | consumption | (15.98 cu.m) |
| :--- | :--- | :--- |
| Tank to be desludged <br> every 6 months |  |  |
| The soil soakway soakaway pit to <br> be designed in accordance with <br> PROPECC PN5/93 and its size <br> shall be determined base on <br> absorption capacity of soil and <br> ultimate consumption rate. |  |  |

Table 2.1 - Calculation of Septic Tank
In addition, the proposed septic tank would be inspected at least once every 6 months by the applicant. If there is any flooding / overflow from the Septic Tank or foul smell become noticeable, immediate inspection would be carried out. Desluging the Septic Tank when thickness of sludge exceeds 30 cm or $1 / 4$ of overall water depth or clogging of the septic tank outlet pipe or the soakaway pit or soil is suspected. Last, disposing the sludge would be carried out properly. Sludge removed would be transported by specialist contractors to sewerage treatment works for disposal.

The location of the proposed underground Sewerage Treatment Plant for the Site is illustrated in Figure 2.3.

Once the concerned public sewerage system is available in the vicinity, the Septic Tank System will be abandoned and replaced with a pump pit and a connection terminal manhole. All sewerage generated from the Proposed Development will be conveyed to the public sewerage system.

### 2.4 Assessment Criteria, Methodology and Assumptions

The adopted unit flow factor and global peaking factors will adopt the figures stipulated in the Guidelines for Estimating Sewerage Flows for Sewerage Infrastructure Planning (GESF) (Version 1.0) issued by the Environmental Protection Department (EPD) in March 2005 to estimate the sewerage flow generated from the Proposed Development.

With reference to Table T-1: Unit Flow Factors for Domestic Flows in the GESF (Version 1.0), the unit flow factors for private housing R4 domestic flow is $0.37 \mathrm{cu} . \mathrm{m} /$ person/day.

### 2.5 Estimation of Sewerage Flow

The primary source of contaminants arising from the Site will be from bathrooms, toilets and kitchens from residential houses.

Table 2.2 shows the estimated peak sewerage flow for the Proposed Development.

| Calculation for Sewerage Flow Generation Rate of the Site |  |  |  |
| :---: | :---: | :---: | :---: |
| 1a. Total number of units | = | 8 | units |
| 1 b . Total number of residents | $=$ | 32 | people |
| 1c. Design flow | = | 0.37 | cu.m/person/day - refer to Private R4 in Table T-1 ofGESF |
| 1d. Sewerage generation rate | = | 9.25 | cu.m/day |
| 1e. Peak factor | = | 6 | refer to Section 3.3 from EPD's Guidelines for Design of Small Sewerage Treatment Plant |
| 1f. Estimated total peak flow | $=$ | $\begin{gathered} 6 \times 9.25 \\ =55.5 \end{gathered}$ | cu.m/day |
| Sewerage to be discharge to Septic Tank |  |  |  |
| 2a. Number of septic tank proposed for the development | $=$ | 8 | units |
| 2b. Number of persons served by each septic tank | = | $32 / 8=4$ | people |
| 2c. Required capacity of each septic tank | = | $\begin{gathered} 4 \mathrm{x} 0.37 \times 6 \\ =8.88 \end{gathered}$ | cu.m/day |
| 2d. Design capacity of each septic tank | = | $\begin{gathered} 15.984 \\ >8.88 \end{gathered}$ | cu.m/day - refer to Table 2.1 |

Table 2.2 - Estimated Sewerage Flow from the Site

As shown in Table 2.2 above, the estimated total peak flow for the Proposed Development is 55.5 cu.m/day and the capacity of each proposed septic tank (15.984 cu.m/day) is greater than required capacity ( 8.88 cu.m/day).

### 2.6 Discussion

According to the drainage record plans obtained from DSD, there is no existing public sewerage network serving the Site. Sewerage from the Site is proposed to be discharged to the proposed underground Sewerage Treatment Plant.

The applicant shall take the maintenance responsibility of the septic tank and soil
soakway soakaway pit in order to maintain the operation of the proposed underground Sewerage Treatment Plant.

According to the design of the septic tank for the Proposed Development presented in Table 2 and estimated sewerage generation, it is anticipated that the proposed underground Sewerage Treatment Plants shown in Figure 2.3 will have sufficient capacity to cater for sewerage generated from the proposed residential development.

### 2.7 Conclusion

Based on the sewerage generated and the capacity of the septic tank, it is anticipated that there will be no serious adverse sewerage impact to the area after the implementation of the development.

## 3. Drainage Impact Appraisal

### 3.1 Scope of Works

The objective of this Drainage Impact Appraisal (DIA) is to assess whether the Proposed Development may cause adverse impacts on drainage and flooding. These impacts will be identified and mitigation measures will be proposed in order to demonstrate that the Proposed Development will not cause an unacceptable increase in the risk of flooding in areas upstream of, adjacent to or downstream of the development.

Existing drainage record plan from the Drainage Services Department (DSD) was attached in Figure 2.1.

### 3.2 Existing and Planned Drainage Facilities

According to the drainage record plan, there is no existing public drainage network serving the Site [refer to Figure 2.1].

Upon a site investigation carried out on July 6 2023, a series of unnamed stormwater manholes were located along Ho Chung North Road and Luk Mei Tsuen Road (main road) and a series of U-channels were identified along Luk Mei Tsuen Road of the Parcel B and Parcel C of the Site. According to the information provided by the Contractor of Highways Department's Hiram's Highway Improvement Stage 1 Project [refer to Figure 3.1A and 3.1B], there is an existing nominal diameter (DN) 300 storm drain located under Ho Chung North Road and 450-525 storm drains located under Luk Mei Tsuen Road in the vicinity of the Site. The storm drains were completed in February $2021{ }^{1}$. The U-channels identified along Luk Mei Tsuen Road were recently built in 2023.

A drainage layout plan comprising the mentioned drainage information is presented in Figure 3.2.

### 3.3 Drainage Catchment Area

The drainage catchment areas included upstream catchment area and the Site. Figure 3.3 illustrates the estimated overall upstream catchment area. The catchment area within the Site includes the open area and the roof of the buildings.

[^0]The surface runoff discharged from the upstream catchment area would be collected by the existing perimeter U-channel surrounding the Site along Luk Mei Tsuen Road.

### 3.4 Drainage Calculations for the Proposed Provision of Drainage Facilities

The Rational Method has been adopted for hydraulic analysis and the peak runoff is given by the following expression:

$$
\mathrm{Q}=0.278 \mathrm{C} \mathrm{i} \mathrm{~A}
$$

where
$Q=$ peak runoff in $\mathrm{m}^{3} / \mathrm{s}$
C = runoff coefficient
$\mathrm{i}=$ rainfall intensity in $\mathrm{mm} / \mathrm{hr}$
$\mathrm{A}=$ catchment area in $\mathrm{km}^{2}$
The average rainfall intensity (i) is estimated on the basis of the design rainfall duration and 50 years return period according to Chapter 4 and Table 3a of the Stormwater Drainage Manual (fifth edition, Jan The design rainfall duration is taken as the time of concentration $\left(\mathrm{t}_{\mathrm{c}}\right)$ :

$$
\mathrm{t}_{\mathrm{c}}=0.14465 \mathrm{~L} /\left(\mathrm{A}^{0.1} \mathrm{H}^{0.2}\right)
$$

where
A = catchment area ( $\mathrm{m}^{2}$ )
$H=$ average catchment slope $(\mathrm{m} / 100 \mathrm{~m})$
$\mathrm{L}=$ catchment Length ( m )
The Site is divided into 3 catchment areas for drainage calculation, in which Parcel A and B are redefined as catchment A1 and A2, while Parcel C is redefined as catchment A3. The catchment area refers to Figure 3.4.

Assuming that:
i. The area of Catchment:

$$
\begin{aligned}
& \text { A1 }=678.22 \mathrm{~m}^{2}\left(0.0006 \mathrm{~km}^{2}\right) ; \\
& \text { A2 }=1265.38 \mathrm{~m}^{2}\left(0.0012 \mathrm{~km}^{2}\right) ; \text { and } \\
& \text { A3 }=922.58 \mathrm{~m}^{2}\left(0.0009 \mathrm{~km}^{2}\right) .
\end{aligned}
$$

ii. The catchment is almost paved and therefore the value of runoff coefficient (C) is taken as 0.95 .

The time of concentration of catchment A1, A2 and A3 are: 7.8609 mins, 15.5561 mins, and 15.2357 mins respectively. The average rainfall intensity for catchment A1, A2 and A3 would then be $205.5142 \mathrm{~mm} / \mathrm{hr}, 170.3364 \mathrm{~mm} / \mathrm{hr}$ and $171.3696 \mathrm{~mm} / \mathrm{hr}$ respectively. Therefore, the total peak runoff from Parcel A and B is $0.0865 \mathrm{~m}^{3} / \mathrm{s}$, while the total peak runoff from Parcel C is $0.0407 \mathrm{~m}^{3} / \mathrm{s}$.

The detailed design calculations of proposed drainage system are provided in Figure 3.5. In accordance with the Chart for the Rapid Design of Channels in "Geotechnical Manual for Slopes", 300 mm surface U-channel in 1:100 gradient is considered adequate to dissipate all the stormwater accrued by the Site. The intercepted stormwater will then be discharged to the proposed 300 mm surface U-channel and connect to the existing storm drain outside the Site along Ho Chung North Road.

### 3.5 Proposed Drainage System

For Parcel A and B of the Site, the surface runoff discharged from the Site will gravitate to lower grounds and be collected by the proposed perimeter 300 mm U-channel surrounding the Site and the proposed 300 mm U-channel located across the Site. The storm water collected from the U-channel would flow into the 300 mm precast concrete pipes to a proposed new manhole. The new manhole will be connected to the existing unnamed stormwater manhole along Ho Chung North Road.

For Parcel C of the Site, the surface runoff discharged from the Site will be collected by the proposed perimeter 300 mm U-channel surrounding the Site. The storm water collected from the U-channel would flow into the 300 mm precast concrete pipes to a proposed new manhole. The new manhole will be connected to the existing unnamed stormwater manhole along Ho Chung North Road.

The indicative drainage connection is shown in Figure 3.4.

### 3.6 Discussion

According to the drainage record plans obtained from DSD, there is no existing public drainage network serving the Site. A series of unnamed drainage pipes on Ho Chung North Road are have been built for the Highways Department's Hiram's Highway Improvement Stage 1 Project. These drainage pipes are capable to collect the surface runoff from the Site.

The surface runoff from the Site will be collected by the proposed perimeter U-channel and discharged to the unnamed storm water manholes along Ho Chung Road/Luk Mei Tsuen Road.

The estimated flow rate of surface runoff discharge from the Site to public 300 dia. drainage pipe on Ho Chung North Road is about $0.13 \mathrm{~m}^{3} / \mathrm{s}$ and the public pipe is capable to collect the runoff.

Therefore, the proposed drainage connection is feasible for the Proposed Development.

### 3.7 Conclusion

Based on the proposed drainage system, It is anticipated that there will be no serious adverse drainage impact to the existing drainage system after the implementation of the development.





1. ALL dimensions in millimetres unless otherwise stated.
2. SIZE
a. $4 B>L>3 B$
b. $1800 \mathrm{~mm} \geqslant \mathrm{D}>1200 \mathrm{~mm}$
c. RATIO OF VOLUMES OF FIRST AND SECOND CHAMBERS $=2: 1$
3. CAPACITY (SUBJECT TO NOTE 2)
a. CAPACITY $C=(L-t) \times B \times D$
b. NOT LESS THAN $2.3 \mathrm{~m}^{2}$ BUT NOT MORE THAN $41 \mathrm{~m}^{3}$
c. NOT LESS THAN an Where N IS THE NUMBER OF PERSONS SERVED AND O IS THE ESTIMATED ULTIMATE PER CAPITA DAILY WATER CONSUMPTION
d. SURFACE WATER MUST NOT BE CONNECTED TO THE TANK
4. TANK TO BE DESLUDGED EVERY 6 MONTHS
5. NO OVERFLOW OR BYPASS PIPE IS ALLOWED
6. PLEASE REFER TO THE BOOKLET "GUIDANCE NOTES ON DISCHARGES FROM VILLAGE HOUSES" PUBLISHED BY EPD FOR FURTHER GUIDELINES ON OPERATION AND MAINTENANCE OF SEPTIC TANK SYSTEM.

| PRUDENTIAL翌 <br> SURVEYING - LAND ADVISORY•VALUATION 行 | JOB TITLE <br> Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residentia Group E)" ("R(E)") and an area shown as 'Road' to "Residential' (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 at Various Government land, Ho Chung, Sai Kung, New Territories, Hong Kong | Drawing Title <br> REFERENCE SEPTIC TANK |  |  |  | DrawnCNCheckedRT | Date <br> 07/08/23 <br> Approved <br> RT | Drawing No. <br> Fig. 2.2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  | Scale |  | Rev. |
|  |  |  | Rev | Description | Date |  | . s . |  |



| - PRUDENTIAL ${ }^{\text {P1 }}$ |  | JOB TITLE: <br> Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" "R(E)") and an area shown as "Road' to "Residential (Group C)3) ("R(C)3"), on the Approved Ho Chung Outine Zoning Plan No. S/SK-HC/11 at Various Lots in on the Approved Ho Chung Outine Zoning Plan No. S/SK-HC/11 at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land, Ho Chung, Sai Kung, New Territories, Hong Kong | Drawing Title <br> PROPOSED SEWERAGE LAYOUT PLAN |  |  | Severage layout Uodate |  | Drawn Date |  |  |  | Drawing No <br> Figure 2.3 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Senerage layout Uodale |  | Checked | Rt | ${ }^{\text {App }}$ | Rt |  |  |
|  |  |  |  | Re |  | Descripion | Date | sca |  | ¢о@ ${ }^{\text {¢ }}$ |  |  | 1 |









## Formula Used

Time of Concentration
$\mathrm{t}_{\mathrm{c}}=0.14465 \mathrm{~L} /\left(\mathrm{A}^{0.1} \mathrm{H}^{0.2}\right)$

Intensity
$I=\frac{a}{(t c+b)^{c}}$

Runoff
$\mathrm{Q}=0.278 \mathrm{C} \mathrm{i} \mathrm{A}$

## Runoff Calculation after Proposed Development

| Runoff Coeff. for Paved Area [C] | Intensity Coeff. (taken from Table 3a of Stormwater Design Manual, 1 in 50 return) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.95 | a | 451.3 | b | 2.46 | c | 0.337 |
| Catchment | Area [A] <br> ( $\mathrm{km}^{2}$ ) | $\begin{aligned} & \text { Average Slope }[\mathrm{H}] \\ & \text { (m per } 100 \mathrm{~m}) \end{aligned}$ | Longest Path [L] (m) | Time of Conc. [tc] (min.) | Intensity [i] (mm/hr) | Designed <br> Runoff [Q] $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ |
| A1 | 0.0006 | 1 | 25.8804 | 7.8609 | 205.5142 | 0.0326 |
| A2 | 0.0012 | 1 | 54.8909 | 15.5561 | 170.3364 | 0.0540 |
| Total Peak Runoff from Parcel A and Parcel B (m/s) |  |  |  |  |  | 0.0865 |


| Catchment | $\begin{gathered} \text { Area [A] } \\ \left(\mathrm{km}^{2}\right) \end{gathered}$ | $\begin{aligned} & \text { Average Slope }[\mathrm{H}] \\ & \text { (m per } 100 \mathrm{~m}) \end{aligned}$ | Longest Path [L] (m) | Time of Conc. [tc] (min.) | Intensity [i] (mm/hr) | $\begin{gathered} \hline \text { Designed } \\ \text { Runoff [Q] } \\ \left(\mathrm{m}^{3} / \mathrm{s}\right) \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A3 | 0.0009 | 1 | 52.2359 | 15.2357 | 171.3696 | 0.0407 |
| Total Peak Runoff from Parcel C (m3/s) |  |  |  |  |  | 0.0407 |
| Total Peak Runoff from the Site ( $\mathrm{m}^{3} / \mathrm{s}$ ) |  |  |  |  |  | 0.1273 |

## Drainage Capacity Check after Proposed Development

| Section | Catchment | Open Circular Channel Size [D] (mm) | Roughtness Factor [n] | Length <br> [L] <br> (m) | $\begin{gathered} \hline \text { I.L. } \\ \text { (mPD) } \end{gathered}$ |  | Gradient <br> [S] | Wetted CrossSectional Area <br> [A] <br> $\left(\mathrm{m}^{2}\right)$ | Wetted Perimeter [P] (m) | Hydraulic Radius R=A/P (m) | $\begin{aligned} & \text { Velocity } \\ & \begin{array}{l} \mathrm{V}=\mathrm{R}^{2 / 3} \mathrm{~s}^{1 / 2} / \mathrm{n} \\ (\mathrm{~m} / \mathrm{s}) \end{array} \end{aligned}$ | $\begin{aligned} & \text { Capacity } \\ & \mathrm{Q}=\mathrm{AV} \\ & \left(\mathrm{~m}^{3} / \mathrm{s}\right) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Upstream | Downstream |  |  |  |  |  |  |
| SMH1 to SMH2 | A1+A2 | 300 | 0.015 | 22 | 14.18 | 13.35 | 0.04 | 0.07 | 0.94 | 0.08 | 2.30 | 0.16 |
| SMH4 to SMH5 | A3 | 300 | 0.015 | 12.5 | 11.02 | 10.02 | 0.08 | 0.07 | 0.94 | 0.08 | 3.35 | 0.24 |



## Attachment 4

Extracted Pages of Sewerage and Drainage Impact Assessment from Town Planning Application No. A/SK-HC/326

For channel and pipe capacity check, Manning's formula is adopted:

$$
V=\frac{1}{n} R^{0.67} S^{0.5}
$$

```
where \(\quad V=\) velocity \((\mathrm{m} / \mathrm{s})\)
    \(\mathrm{n}=\) roughness factor,
    \(\mathrm{R}=\) hydraulic mean depth based on a fully wetted cross-section \(=\mathrm{A} / \mathrm{P}\)
    \(\mathrm{A}=\) wetted cross-sectional area \(\left(\mathrm{m}^{2}\right)\),
    \(P=\) wetted perimeter \((\mathrm{m})\), and
    \(\mathrm{S}=\) gradient of channel
```


### 3.4 Drainage Catchment Area

The drainage catchment areas included upstream catchment area and the Site. Appendix G illustrates the estimated overall upstream catchment area. The catchment area within the subject site includes the open area and the roof of the buildings.

### 3.5 Proposed Drainage System

For the Upper portion, the surface runoff discharged from the site will gravitate to lower grounds and be collected by the proposed perimeter U-channel surrounding the Site and the proposed U-channel located across the Site. The storm water collected from the U-channel would flow into the precast concrete pipes to a proposed new manhole. The new manhole will be connected to the existing unnamed stormwater manhole along Ho Chung North Road.

For the runoff discharged from the northern portion of the site (out of the proposed development) will gravitate to the proposed rock trap. The storm water collected then would flow into the public drainage system via proposed 525 mm dia. pipe. Last, the pipe will be connected to an unnamed stormwater manhole along Luk Mei Tsuen Road.

For the Lower Portion, the surface runoff discharged from the site will be collected by the proposed perimeter U-channel surrounding the Site. The storm water collected from the U-channel would flow into the precast concrete pipes to a proposed new manhole. The new manhole will be connected to the existing unnamed stormwater manhole along Ho Chung North Road.

The indicative drainage layout plan and detailed drainage impact assessment calculation refers to Appendix H and Appendix I.

### 3.6 Discussion

According to the drainage record plans obtained from DSD, there is no existing public drainage network serving the project site. A series of unnamed drainage pipes on Ho Chung North Road are under construction by Highways Department's Hiram's Highway Improvement Stage 1 Project, which is capable to collect all surface runoff from our project site and catchment area nearby.

According to the aerial photo and site inspection, the site is paved area and therefore no changes to the catchment areas as well as existing and planned drainage system.

The drainage system constructed by Highways Department's Hiram's Highway Improvement Stage 1 Project is checked and found adequate to collect all surface runoff from the proposed development. The estimated flow rate of surface runoff discharge from the proposed development to public 300 dia. drainage pipe on Ho Chung North Road is about $0.19 \mathrm{~m}^{3} / \mathrm{s}$ and the public pipe is capable to collect the runoff.

Therefore, the proposed drainage connection is feasible for the proposed development.

## Appendix G

## Catchment Plan



## Appendix H

Indicative Drainage Layout Plan


## Appendix I

## Drainage Impact Assessment Calculation

Drainage Assessment for Proposed Residential Development at Various Lots in DD210, Sai Kung, N.T.
Public Drainage System Check


Catchment Area



| $\begin{array}{c}\text { Intensity coeff. (taken from table 3a of stormwater } \\ \text { design manual, } 1 \text { in } 50 \text { return) }\end{array}$ |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| a | 451.3 | b | 2.46 | c | 0.337 |


| Catchment | Area | $\begin{aligned} & \text { Unpaved Area } \\ & (\mathrm{m} 2) \end{aligned}$ | Paved Area (m2) | Average Slope H (m per 100m) | Longest Path L (m) | Inlet time t0 (min) | Time of Conc. tc (min) | Intensity i (mm/hr) | Runoff from Unpaved Area (m3/s) | Runoff from paved Area (m3/s) | $\begin{aligned} & \text { Peak Runoff Qr } \\ & (\mathrm{m} 3 / \mathrm{s}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A1 | 55082 | 55082 | 0 | 25 | 1500 | 38.26 | 38.26 | 129.41 | 0.694 | 0.000 | 0.694 |
| A2 | 10325 | 10325 | 0 | 20 | 250 | 7.88 | 7.88 | 205.37 | 0.206 | 0.000 | 0.206 |
| A3 | 18066 | 4500 | 13566 | 10 | 500 | 17.12 | 17.12 | 165.61 | 0.073 | 0.562 | 0.635 |
| A4 | 5303 | 0 | 5303 | 10 | 325 | 12.58 | 12.58 | 181.01 | 0.000 | 0.240 | 0.240 |
| A2 $2+\mathrm{A} 3$ | 28391 | 14825 | 13566 | 13 | 750 | 23.30 | 23.30 | 151.01 | 0.218 | 0.513 | 0.730 |
| A1+A3 | 73148 | 59582 | 13566 | 17 | 2000 | 53.56 | 53.56 | 116.22 | 0.674 | 0.394 | 1.068 |
| $1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$ | 88776 | 74407 | 14369 | 14 | 2400 | 65.54 | 65.54 | 108.87 | 0.788 | 0.391 | 1.180 |
| R1 | 800 | 800 | 0 | 9 | 35 | 1.67 | 1.67 | 279.78 | 0.022 | 0.000 | 0.022 |
| R2 | 413 | 0 | 413 | 3 | 25 | 1.59 | 1.59 | 281.69 | 0.000 | 0.029 | 0.029 |



| 108.57 | 0.786 | 0.390 | 1.176 |
| :---: | :---: | :---: | :---: |


| 108.26 | 0.784 | 0.389 | 1.173 |
| :--- | :--- | :--- | :--- |



| 107.28 | 0.785 | 0.397 | 1.182 |
| :--- | :--- | :--- | :--- |

$V=\frac{1}{n} R^{0.67} S^{0.5}$
Manning Formula (pipe or channel velocity)
$\mathrm{Q}_{\mathrm{p}}=0.278 \mathrm{i} \sum_{j=1}^{m} C j A j$
$\mathrm{t}_{0}=0.14465 \mathrm{~L} /\left(\mathrm{A}^{0.1} \mathrm{H}^{0.2}\right)$

## $I=\overline{(t c+b)^{c}}$

Runoff Calulation (Upstream)

| Catchment |
| :---: |
| A 1 |
| A 2 |
| A 3 |
| A 4 |
| $\mathrm{~A} 2+\mathrm{A} 3$ |
| $\mathrm{~A} 1+\mathrm{A} 3$ |
| $\mathrm{~A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$ |
| R 1 |
| R 2 |


|  | Catchment |
| :---: | :---: |
| $\begin{array}{c}\text { Rock trap to } \\ \text { MH1 }\end{array}$ | $\mathrm{A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$ |


| $\begin{array}{c}\text { Rock trap to } \\ \text { MH1 }\end{array}$ | A1 +A2 2 A3 3 +A | 88776 |
| :---: | :---: | :---: |
| MH1 to MH2 | A1+A2 $2+\mathrm{A} 3+\mathrm{A} 4$ | 8877 |


| MH1 |  | 88776 | 74407 | 14369 |
| :---: | :---: | :---: | :---: | :--- |
| MH1 to MH2 | $\mathrm{A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$ | 88776 | 74407 | 14369 |

74407
800
SMH6 to SMH7 $\begin{gathered}\begin{array}{c}\mathrm{A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4+ \\ \mathrm{R} 1+\mathrm{R} 2\end{array}\end{gathered}$ 89989
Driange Capacity Check

| Driange Capacity Check |  |  |  |
| :---: | :---: | :---: | :---: |


| Section | Catchment |
| :---: | :---: |
| $\begin{array}{c}\text { Rock trap to } \\ \text { MH1 }\end{array}$ | $\mathrm{A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$ |

MH1 to MH2 $\quad \mathrm{A} 1+\mathrm{A} 2+\mathrm{A} 3+\mathrm{A} 4$

| SMH5 to SMH6 | $\mathrm{R} 1+\mathrm{R} 2$ |
| :--- | :--- |


| SMH6 to SMH7 | A1 +A2 $2+\mathrm{A} 3+\mathrm{A} 4+$ <br> $\mathrm{R} 1+\mathrm{R} 2$ |
| :--- | :---: |

Therefore the upstream runoff can be discharged to government public drainage through proposed 525 pipe.
Drainage Assessment for Proposed Residential Development at Various Lots in DD210, Sai Kung, N.T.
Design Calculation of Proposed Drainage System

| Catchment Area |  | Surface Condition before proposed development | Surface Condition after proposed development |
| :---: | :---: | :---: | :---: |
| Catchment | Area (m2) |  |  |
| A5 | 2113 | paved | paved |
|  |  |  |  |
|  |  |  |  |
|  |  |  |  |

Intensity Runoff

## $\mathrm{Q}_{\mathrm{p}}=0.278 \mathrm{i} \mathrm{I}_{j=1}^{m} \mathrm{Cj}_{\mathrm{j}} \mathrm{Aj}$

| 60 | ¢ $¢ 0$ |
| :---: | :---: |
|  | 万 earv pasedun |
| 10: \#әoう Houny |  |

Manning Formula (pipe or channel velocity)
$V=\frac{1}{n} R^{0.67} S^{0.5}$

| Runoff Calulation <br> Section | Catchment | ment Area | Unpaved Area (m2) | Paved Area (m2) | Average Slope H (m per 100m) | Longest Path L (m) | Inlet time t0 (min) | Pipe Length Lj <br> (m) | Flow Velocity $\mathrm{V}(\mathrm{m} / \mathrm{s})$ | Flow Time tf (min) | Time of Conc. tc (min) | Intensity i (mm/hr) | Runoff from Unpaved Area (m3/s) | Runoff from paved Area (m3/s) | $\begin{aligned} & \text { Peak Runoff Qr } \\ & \quad(\mathrm{m} 3 / \mathrm{s}) \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| CP7 to ST1 | A5 | 2113 | 0 | 2113 | 1 | 75 | 5.05 | 41 | 1.70 | 0.401 | 5.45 | 224.82 | 0 | 0.119 | 0.119 |
| ST1 to MH3 | A5 | 2113 | 0 | 2113 | - | - | 5.45 | 1.8 | 2.47 | 0.012 | 5.46 | 224.71 | 0 | 0.119 | 0.119 |
| MH3 to SMH1 | A5 | 2113 | 0 | 2113 | - | - | 5.46 | 11 | 2.00 | 0.092 | 5.55 | 223.84 | 0 | 0.118 | 0.118 |
| SMH1 to SMH2 | A5 | 2113 | 0 | 2113 | - | - | 5.55 | 18 | 3.83 | 0.078 | 5.63 | 223.10 | 0 | 0.118 | 0.118 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Driange Capacity Check after proposed development |  |  |  |  |  |  |  |  |  |  |  |  |  |  | OK |
| Section | Catchment | Drainage | Diameter D (mm) | US IL (mPd) | DS IL (mPd) | Length L (m) | Gradient (1 in) | $\begin{gathered} \hline \text { Roughness coeff } \\ \mathrm{n}(\mathrm{~mm}) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { Cross Sectional } \\ \text { Area } \mathrm{A}(\mathrm{~m} 2) \\ \hline \end{gathered}$ | Wetted Perimeter $\mathrm{P}(\mathrm{m})$ | Hydraulic Radius $\mathrm{R}(\mathrm{mm})$ | Velocity v (m/s) | $\begin{gathered} \hline \text { Design Capacity } \\ Q c(m 3 / \mathrm{s}) \\ \hline \end{gathered}$ | Utilization (\%) |  |
| to CP1 | A5 | 300 C | 300 | - | - | 41 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 86.88 |  |
| ST1 to MH3 | A5 | 300 pipe | 300 | 12.85 | 12.8 | 1.8 | 36 | 0.012 | 0.071 | 0.942 | 0.0750 | 2.47 | 0.175 | 68.07 |  |
| MH3 to SMH1 | A5 | 300 pipe | 300 | 12.70 | 12.5 | 11 | 55 | 0.012 | 0.071 | 0.942 | 0.0750 | 2.00 | 0.141 | 83.82 |  |
| SMH1 to SMH2 | A5 | 300 pipe | 300 | - | - | 18 | 15 | 0.012 | 0.071 | 0.942 | 0.0750 | 3.83 | 0.270 | 43.63 |  |

Effect to Public Drainage Pipe due to proposed development
Since the catchment area is paved area before and after proposed development, therefore the runoff increased to public drainage is 0
Drainage Assessment for Proposed Residential Development at Various Lots in DD210, Sai Kung, N.T.
Design Calculation of Proposed Drainage System

| Catchment Area | Surface Condition before proposed | Surface Condition after proposed |
| :--- | :--- | :--- |


| Intensity coefi. (taken from table 3 o of stormwater <br> design manual 1 in 5 r return) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| a | 451.3 | b | 2.46 | c | 0.337 |



Manning Formula (pipe or channel velocity)
$\mathrm{V}=\frac{1}{\mathrm{n}} \mathrm{R}^{0.67} \mathrm{~S}^{0.5}$

| Section | Catchment | Area | $\underset{(\mathrm{m} 2)}{\text { Unpaved Area }}$ | Paved Area (m2) | Average Slope H (m per 100m) | Longest Path L (m) | Inlet time t0 (min) | Pipe Length Lj $(\mathrm{m})$ | Flow Velocity $V(\mathrm{~m} / \mathrm{s})$ | $\underset{(\mathrm{min})}{\text { Flow Time tf }}$ | Time of Conc. tc (min) | $\underset{(\mathrm{mm} / \mathrm{hr})}{\text { Intensity } \mathrm{i}}$ | Runoff from Unpaved Area (m3/s) | Runoff from paved Area (m3/s) | Peak Runoff Qr (m3/s) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| to BCP1 | B1 | 650 | 650 | 0 | 9 | 35 | 1.71 | 16 | 1.70 | 0.157 | 1.86 | 275.54 | 0.017 | 0.000 | 0.017 |
| BCP1 to BST1 | B3 | 540 | 0 | 540 | 1 | 25 | 1.93 | 16 | 1.70 | 0.157 | 2.08 | 270.95 | 0 | 0.037 | 0.037 |
| BCP1 to BST1 | B3 + $\mathrm{B}_{1}$ | 1190 | 1421 | 540 | - | - | 1.86 | 16 | 1.70 | 0.157 | 2.02 | 272.25 | 0.038 | 0.037 | 0.074 |
| to BCP2 | B2 | 530 | 530 | 0 | 9 | 52 | 2.59 | 26 | 1.70 | 0.255 | 2.84 | 257.21 | 0.013 | 0.000 | 0.013 |
| BCP2 to BCP3 | B2 | 530 | 530 | 0 | - | - | 2.84 | 9 | 1.70 | 0.088 | 2.93 | 255.79 | 0.013 | 0.000 | 0.013 |
| BCP3 to BST1 | B2 | 530 | 530 | 0 | - | - | 2.93 | 11 | 2.00 | 0.092 | 3.02 | 254.34 | 0.013 | 0.000 | 0.013 |
| BST1 to BMH1 | B1+B2+B3 | 1720 | 1180 | 540 | - | - | 2.02 | 6 | 1.91 | 0.052 | 2.07 | 271.19 | 0.031 | 0.037 | 0.068 |
| BMH1 to SMH3 | B1+B2+B3 | 1720 | 1180 | 540 | - | - | 2.07 | 11 | 2.00 | 0.092 | 2.16 | 269.36 | 0.03 | 0.036 | 0.067 |
| SMH3 to SMH4 | B1+B2+B3+A5 | 3833 | 1180 | 2653 | - | - | 5.63 | 13 | 3.83 | 0.057 | 5.69 | 222.58 | 0.03 | 0.148 | 0.173 |
| Driange Capacity Check after proposed development (taken from previous sheet) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Section | Catchment | Drainage | Diameter D (mm) | US IL (mPd) | DS IL (mPd) | Length L (m) | Gradient (1 in) | $\begin{array}{\|c} \text { Roughness coeff } \mathrm{n} \\ (\mathrm{~mm}) \end{array}$ | $\begin{gathered} \text { Cross Sectional } \\ \text { Area } \mathrm{A}(\mathrm{~m} 2) \\ \hline \end{gathered}$ | $\begin{array}{\|c} \hline \text { Wetted Perimeter } \\ \mathrm{P}(\mathrm{~m}) \\ \hline \end{array}$ | $\begin{array}{\|c} \hline \text { Hydraulic Radius } \\ \mathrm{R}(\mathrm{~mm}) \\ \hline \end{array}$ | Velocity v (m/s) | $\begin{gathered} \text { Design Capacity } \\ \text { Qc }(\mathrm{m} 3 / \mathrm{s}) \\ \hline \end{gathered}$ | Utilization (\%) |  |
| to BCP1 | B1 | 300uc | 300 | - | - | 16 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 12.74 |  |
| BCP1 to BST1 | B3 | 300uc | 300 | - | - | 16 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 26.76 |  |
| BCP1 to BST1 | B3 + B1 | 300uc | 300 | - | - | 16 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 54.40 |  |
| to BCP2 | B2 | 300uc | 300 | - | - | 26 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 9.70 |  |
| BCP2 to BCP3 | B2 | 300uc | 300 | - | - | 9 | 100 | 0.013 | 0.080 | 0.771 | 0.1042 | 1.70 | 0.137 | 9.64 |  |
| BCP3 to BST1 | B2 | 300DIP | 300 | 13.75 | 13.55 | 11 | 55 | 0.012 | 0.071 | 0.942 | 0.0750 | 2.00 | 0.141 | 9.29 |  |
| BST1 to BMH1 | $\mathrm{B} 1+\mathrm{B} 2+\mathrm{B} 3$ | 300DIP | 300 | 13.00 | 12.9 | 6 | 60 | 0.012 | 0.071 | 0.942 | 0.0750 | 1.91 | 0.135 | 50.14 |  |
| BMH1 to SMH3 | $\mathrm{B} 1+\mathrm{B} 2+\mathrm{B} 3$ | 300DIP | 300 | 12.80 | 12.6 | 11 | 55 | 0.012 | 0.071 | 0.942 | 0.0750 | 2.00 | 0.141 | 47.68 |  |
| SMH3 to SMH4 | B1+B2+B4+A5 | 300DIP | 300 | - | - | - | 15 | 0.012 | 0.071 | 0.942 | 0.0750 | 3.83 | 0.270 | 64.10 | OK |

Effect to Public Drainage Pipe due to proposed development
Since no catchment area changed before and after proposed development, therefore the runoff increased to public drainage is 0

## Attachment 5

Water Supply Appraisal (Version B)

## Water Supply Appraisal

For
Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E)" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3" on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 at Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land

Ho Chung, Sai Kung, New Territories, Hong Kong

Prepared by: Prudential Surveyors International Limited

Version Date:

A B
August November 2023

## TABLE OF CONTENT

1. Introduction ..... 3
2. Proposed Development ..... 3
3. Water Supply Appraisal ..... 3
4. Conclusion ..... 5

## List of Figures

Figure 1.1 Location Plan
Figure 2.1 Utility Plan
Figure 3.1 Copy of the Fresh Water Mains Record Plan
Figure 3.2 Proposed Water Supply Connection

## List of Table

Table 1.1 Proposed GFA of Houses
Table 3.1 Estimated Fresh Water and Salt Water Demand from the Proposed Development
Table 3.2 Water Supply Estimation

## 1. Introduction

1.1.1 This Water Supply Appraisal is to support a planning permission from the Town Planning Board (TPB) under Section 12A of the Town Planning Ordinance (CAP. 131) for a proposed rezone of the Subject Site from "Residential (Group D)" ("R(D)"), "Residential (Group E )" (" $\mathrm{R}(\mathrm{E})$ " and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3" within various lots within DD210 and DD244 and adjoining government land in Ho Chung, Sai Kung, New Territories. The application Site (the Site) is composed of 3 parcels, namely Parcel A, B and C. [refer to Figure 1.1]
1.1.2 The owner of the application site has the intention to construct six individual houses with twelve car parking spaces in Parcels A and B of the Site and two individual houses with four car parking spaces in Parcel C of the Site.

## 2. Proposed Development

2.1.1 The proposed development (the Proposed Development) is to erect six individual houses in Parcel A and B of the Site and two individual houses in Parcel C of the Site. The proposed gross floor area (GFA) of the houses are summarised in Table 1.1.

| Propose House | Gross Floor Area (GFA) (sqm) (about) |
| :--- | :---: |
| House 1 | 283.35 |
| House 2 | 283.35 |
| House 3 | 283.35 |
| House 4 | 283.35 |
| House 5 | 283.35 |
| House 6 | 283.35 |
| House 7 | 345.75 |
| House 8 | 345.75 |
| Total | $2,391.6$ |
| Average | 298.95 |

Table 1.1 Proposed GFA of Houses

## 3. Water Supply Appraisal

### 3.1 Methodology

3.1.1 The waterworks impacts arising from the proposed development are assessed with reference to the following information:

- WSD Departmental Instruction (DI) No. 1309
- EPD Guidelines for Estimating Sewage Flows (GESF) for Sewage Infrastructure Planning No.: EPD/TP 1/05.
3.1.2 The fresh water system is modelled under the following condition:
- Design peak flow of fresh water distribution main $=3 \times$ MDD (Mean Daily Demand)
3.1.3 The salt water system is modelled under the following condition:
- Design peak flow of salt water distribution main (sub-main) $=2 \times$ MDD (Mean Daily Demand)


### 3.2 Existing Water Supply

3.2.1 According to the utility plan provided by the Highways Department Contractor for the Hiram's Highway Improvement Stage 1 Project, there is an existing nominal diameter (DN) 400 fresh water main located under Luk Mei Tsuen Road/ Ho Chung North Road in the vicinity of the Site. The existing water system is shown in Figure 2.1. The fresh water main was completed in February $2021{ }^{1}$.
3.2.2 According to the existing water mains record plan provided by Water Supplies Department, there are existing water mains within the Site and would be affected by the proposed development [refer to Figure 3.1].
3.2.3 According to the existing water mains record plan extracted from the Water Services Department (WSD) and Figure 2.1, there are no existing salt water mains in the vicinity of the Site [refer to Figure 3.1].
3.2.4 Based on the existing water mains record extracted from Water Supplies Department (WSD), the site is not within WSD gathering grounds. [refer to Figure 3.1]

### 3.3 Proposed Water Supply Connection

3.3.1 Estimation for peak fresh water and salt water consumption for the Proposed Development is presented in Table 3.1. As discussed in paragraph 3.2.3, there is no existing salt water mains in the vicinity of the Site and no available flushing water supplies near the Site. Thus, fresh water shall be used for flushing purpose.

| Description | Daily Water Demand of <br> Proposed Development <br> $\left(\mathbf{m}^{3} /\right.$ day $)$ | Peaking Factor | Peak Demand <br> $\left(\mathbf{m}^{3} /\right.$ day $)$ |
| :--- | :--- | :---: | :---: |
| Fresh Water + <br> Flushing Water | 11.84 | 3 | 35.52 |
| Total Fresh Water Demand |  |  | $\mathbf{3 5 . 5 2}$ |

Table 3.1 Estimated Fresh Water and Salt Water Demand from the Proposed Development
3.3.2 The water supply to Parcel A and B of the Site could be supplied with a connection to the existing DN400 fresh water main (water supply) that is located along Ho Chung North Road. The water supply to Parcel C of the site could also be connected to this fresh water main. The proposed connections are shown in Figure 3.2.
3.3.3 The existing water mains affected by the proposed development would be diverted.
3.3.4 As discussed in paragraph 3.2.2, there are existing fresh water main along Ho Chung North Road. Assuming the fresh water and flushing water for the Site will be sourced

[^1]from that existing fresh water main -400 mm nominal diameter ductile iron pipe (DI400) and velocity is ranging $1-3 \mathrm{~m} / \mathrm{s}$, the capacity and utilization ratio of each is estimated in Table 3.2:

| Description | Peak <br> Demand <br> ( $\mathrm{m}^{3} /$ day) | Total <br> Peak <br> Demand <br> ( $\mathrm{m}^{3} / \mathrm{s}$ ) | Fresh <br> Water <br> Supply <br> Main <br> Nominal <br> Diameter <br> (mm) | Internal <br> Diameter <br> for Fresh <br> Water <br> Main <br> Pipes <br> (mm) | Assume Velocity ( $\mathrm{m} / \mathrm{s}$ ) | Pipe <br> Capacity $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Utilisation Ratio |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Total Fresh <br> Water <br> Demand | 35.52 | 0.0004 | 400 | 382 | $\begin{gathered} 3 \text { (upper } \\ \text { limit) } \end{gathered}$ | 0.3438 | 0.12\% |
|  |  |  |  |  | $\begin{gathered} 1 \text { (lower } \\ \text { limit) } \end{gathered}$ | 0.1146 | 0.35\% |

Table 3.2 Water Supply Estimation
3.3.5 As indicated in Table 3.2, the estimated total peak fresh water demand would be about $0.12-0.35 \%$ of the fresh water main capacity ${ }^{2}$. This means the Proposed Development would take up less than $0.35 \%$ of the fresh water capacity which is an insignificant of the total capacity. Therefore, no strong adverse impact on the water supply is anticipated due to the Proposed Development.

## 4. Conclusion

4.1.1 In general, fresh water supply could be provided to the Site. This could be achieved by connecting the existing fresh water mains located on Ho Chung North Road for the Proposed Development.
4.1.2 The peak estimated fresh water and flushing water demand from the Proposed Development are about $35.52 \mathrm{~m}^{3} /$ day. Since there are no existing salt water mains in the vicinity of the Site, fresh water shall be used for flushing purpose. The total estimated peak fresh water demand is about $0.12-0.35 \%$ of the fresh water main capacity. The results indicate that the Proposed Development would take up less than $0.35 \%$ of the fresh water capacity which is an insignificant of the total capacity. Therefore, no strong adverse impact on the existing water supply system due to the Proposed Development.

[^2]




PRUDENTIAL翌


|  |  |  | ${ }^{\text {Drawn }}$ CN | Date | Drawing No. |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | 07/08/23 |  |
|  |  |  | Checked | Approved | FIGURE 3.1 |
|  |  |  | RT | RT | FIGURE 3.1 |
|  |  |  | Scale |  | Rev. |
| Rev | Description | Date |  |  |  |




## Attachment 6

Replacement Pages for Visual Impact Assessment
the mountain backdrop in the background. As illustrated in Figure 7.1, a portion of the Proposed Development will be screened off by the existing trees and vegetation. In this connection, the Proposed Development will merge into the existing visual composition with minimal negative effect on the visual balance, compatibility, harmony, unity or contrast. Therefore the visual composition would be low.

## Visual Obstruction

7.2.2 From this VP, VSRs are currently enjoying an open view towards the Site with the mountain backdrop in the background. As demonstrated in Figure 7.1, comparing to the OZP compliant scheme existing condition affecting the openness of VSR's views, the Proposed Development with a low building height would not form visual obstruction and therefore the current openness of the sky view and most of the view of the mountain backdrop at this VP will be minimally affected. With proposed landscape and trees, the Proposed Development will alternatively provide positive visual resources to VSRs at this VP. Therefore the visual obstruction would be low.

## Effect on Public Viewers

7.2.3 Due to the close proximity to the Site, VSRs at VP-1 will be inevitably affected, yet in a good way. The existing view of the public viewers from VP-1 consists of a refuse collection point, unorganised space occupied by temporary structures and vehicles, and the abandoned ATV Production Centre. With well-designed layout of buildings, landscape elements, the visual effect on public viewers at this VP brought by the Proposed Development at VP-1 will be enhanced.

## Effect of Visual Resources

7.2.4 The existing refuse collection point, roadside vegetation, and temporary structures in the foreground, the abandoned ATV Production Centre, sky view and mountain backdrop in the background are the major visual resources for VSRs at VP-1. The Proposed Development will inevitably impact the existing visual resources, as temporary structures will be removed. However, the Proposed Development will not degrade the condition, visual quality and character of the assessment area, as it would not partially block the sky view and mountain backdrop. Alternatively, the Proposed Development will remove and partly screen off some of the existing undesirable visual resources. Therefore the visual resources would be enhanced partly enhanced/partly adverse.
7.2.5 In summary, with varied design merits, the resultant visual impact of the Proposed Development viewed from VP-1 is assessed to be enhanced partly enhanced/partly adverse.

### 7.3 Viewing Point 2- Crossroad of Luk Mei Tsuen Road and Hiram's Highway

Visual Composition
7.3.1 The existing view comprises the junction of Luk Mei Tsuen Road and Hiram's Highway, the retaining wall along Hiram's Highway, a big warehouse of the Kin Hing Group, Limited, the area zoned "GB" with rich vegetation and roadside trees along Luk Mei Tsuen Road in the foreground and, mountain backdrop in the background. It is observed that the Proposed Development is located at a ground level higher than VP-2, and the view towards the Proposed Development is mostly blocked by retaining wall along

Hiram's Highway and the roadside vegetation. The Proposed Development would therefore have no impact to the visual composition at this VP.

## Visual Obstruction

7.3.2 From VP-2, the view is dominated by junction of Luk Mei Tsuen Road and Hiram's Highway, the retaining wall along Hiram's Highway, area zoned "GB" with rich vegetation and roadside vegetation. The Photomontage Figure 7.2 illustrates that the Proposed Development cannot be seen at this VP, in this connection, the Proposed Development will not cause visual obstruction or block the openness of this VP, resulting in no impact.

## Effect on Public Viewers

7.3.3 The Proposed Development with a maximum building height of 12 m (+23.70 mPD (Parcel A and B) and +25.97 mPD (Parcel C) is located at a ground level higher than this VP, however due to the rich roadside vegetation and existing structures, the Proposed Development will be shielded in a great extent. In this connection, the views of public viewers at this VP will not be affected. Moreover, given the transient nature of this VP, the visual sensitivity of VSRs at this VP would be low. The visual change brought about by the Proposed Development therefore would be negligible.

## Effect of Visual Resources

7.3.4 The existing visual resources, such as the sky view, streetscape, and mountain backdrop would not be affected and no change to the quality and character of the assessment area will be caused by the Proposed Development, due to the proposed building heights and topography. This would result in no impact to the visual resources.
7.3.5 In summary, the visual impact of the Proposed Development viewed from VP-2 is assessed to be Negligible.

### 7.4 Viewing Point 3- Car Park of Che Kung Temple

## Visual Composition

7.4.1 The existing view of VP-3 comprises the rich vegetation within the area zoned "GB" and open sky view. The proposed maximum building height is $12 \mathrm{~m}(+23.70 \mathrm{mPD}$ (Parcel A and B) and +25.97 mPD (Parcel C)), which will be entirely screened off by the existing trees. In this connection, the Proposed Development will not form any new visual element or cause any impact on the existing visual composition as shown in Figure7.3, resulting in no impact to the visual composition from this VP.

## Visual Obstruction

7.4.2 The only visual resources viewing from this VP are the mature trees within the area zoned "GB" and the open sky view. As the Proposed Development is situated to the north of the area zoned "GB", the presence of the Proposed Development will not result in any visual obstruction to the existing visual resources with no loss of views or visual openness, resulting in no impact on the visual obstruction

## Effect on Public Viewers

7.4.3 The public viewers of this VP are mostly visitors to Che Kung Temple. These public
viewers will continue to enjoy the open sky and rich vegetation as the Proposed Development cannot be seen at this VP. Hence, the visual sensitivity would be low and the visual change caused by the Proposed Development at this VP would be negligible.

## Effect of Visual Resources

7.4.4 The major visual resources for VSRs at this VP are the mature trees within the area zoned "GB" and the open sky view. As stated above, the Proposed Development cannot be seen at this VP. In this connection, the Proposed Development will neither bring any adverse impact to the condition, visual quality and character of the assessment area nor any on-site and off-site visual impact. There will be no impact on the visual resources from this VP
7.4.5 In summary, the visual impact of the Proposed Development viewed from VP-3 is assessed to be Negligible.
7.5 Viewing Point 4- Ho Chung North Road (Main Road)

Visual Composition
7.5.1 VP-4 is located to the west of the Site, capturing the partial view of the Site with Ho Chung North Road, some temporary structures, the open-air vehicle park and roadside vegetation in the foreground, and the open sky view as backdrop. The existing visual composition is messy and unpleasant, having all the undermaintained temporary structures and cars weltered together. The Proposed Development however would enhance the visual composition by replacing the temporary structures on Site with welldesigned permanent housings as well as additional landscape elements. Therefore the visual composition would be enhanced. [refer to Figure 7.4].

## Visual Obstruction

7.5.2 From this VP, VSRs are currently facing Ho Chung North Road with some temporary structures, open-air vehicle park and roadside vegetation along both sides of the road in the foreground, and the open sky view as backdrop. No significant visual feature is available at this VP, in particularly in terms of coastline, open sea horizon, scenic areas, valued landscape, special landmark and heritage. As illustrated in Figure 7.4, the Proposed Development, would form ne partial visual obstruction and ne partial loss of visual openness of VSRs due to its compatible building heights and mass. This would result in noimpact partly enhanced/partly adverse to the visual obstruction.

## Effect on Public Viewers

7.5.3 The effect of the Proposed Development on the public viewers would be partly enhanced when viewing from this VP, since the Proposed Development would replace the temporary structures and open-air vehicle park at the Parcel C with well-designed permanent house with landscape. The Proposed Development within the Parcel A and B of the Site would be partly shielded off by the existing and proposed roadside trees and structures. Additionally, with consideration of the transient nature of this VP, where VSRs are mainly pedestrian passers-by and vehicle drivers, the visual sensitivity at this VP will be low. The visual change caused by the Proposed Development will be partly enhanced/partly adverse.

Effect of Visual Resources
7.5.4 The existing visual resources at VP-4 are Luk Mei Tsuen Road, temporary structures, open-air vehicle park, roadside vegetation and sky view at backdrop. The Proposed Development will replace the existing undesirable visual resources with permanent houses with landscapes which would be more visually appealing. Overall, the condition, quality and character of the assessment area would be enhanced as a result of the Proposed Development, as the streetscape would be improved through provision of well-designed buildings, more trees and landscapes.
7.5.5 In summary, the resultant visual impact of the Proposed Development viewed from VP-4 is assessed to be enhanced.

## 8. Conclusion

8.1.1 The Proposed Development for a low-density and low-rise residential development with a rezone of the Subject Site from "Residential (Group D)" ("R(D)"), "Residential (Group $E)$ " ("R(E)" and an area shown as "Road' to "Residential (Group C)3) ("R(C)3" Considering that low building height and its surrounding building height profile, the proposed rezone of site is considered reasonable.
8.1.2 Based on the analysis on the appraisal of visual impact on Visual Composition, Visual Obstruction, Effect on Public Views and Effect on Visual Resources, Table 8.1 below presents the overall visual impact caused by the Proposed Development to the VSRs of each VP.

| Viewing Point | Distance <br> from the <br> site | Visual Sensitive <br> Receivers | Visual <br> Sensitivity | Visual Impact of <br> the Proposed <br> Development |
| :--- | :--- | :--- | :--- | :--- |
| VP1 The Public <br> Toilet on Luk <br> Mei Lane | Short-range | Users of the Public <br> Toilet, pedestrian <br> passers-by, local <br> residents, and <br> vehicle drivers | Medium | Enhanced Partly <br> Enhanced / Partly <br> Adverse |
| VP2 Crossroad <br> of Luk Mei <br> Tsuen Road <br> and Hiram's <br> Highway | Long-range | Pedestrian <br> passers-by, local <br> residents, vehicle <br> drivers, visitors <br> and users of <br> public transport | Low | Negligible |
| VP3 Car Park <br> of Che Kung <br> Temple | Long-range | Visitors, and local <br> residents visiting <br> Che Kong Temple | Low | Negligible |
| VP4 Ho Chung <br> North Road <br> (Main Road) | Medium- <br> range | Vehicle drivers, <br> pedestrian <br> passers-by and <br> local residents | Low | Enhanced Partly <br> Enhanced / Partly |

Table 8.1 Summary of Assessment of Visual Impact at the Viewing Points
8.1.3 While the visual change to VSRs at VP-2 and VP-3 are negligible, the visual impact at VP1 and VP-4 would be enhanced by the Proposed Development. This VIA therefore concludes that overall visual impact of the Proposed Development at the Site to its
surroundings would be enhanced partly enhanced/partly adverse. The Proposed Development will have minimal visual effects to VSRs at a few identified key public viewing points in a positive way, as the Proposed Development will remove some of the existing visual obstructions and provide new visual resources through provision of greenery elements.

## Attachment 7

Replacement Pages for Landscape Proposal
3.1.2. The proposed development includes 8 nos. of residential houses with building heights of about 12 m . The landscape design concept will adopt a modern style. It will use organic forms and shapes as the main elements in order to soften the hard lines of the built forms. Each house has its private garden with a combination of soft and hard landscapes, creating different gathering, recreational and fitness spaces to enrich daily life of the future residents. Plants with different heights and densities are mainly provided along the boundaries of the Site, strengthening privacy and providing shaded for residents. Proposed trees and greenery would be intermixed with the overall landscape design. [refer to Figure 3.2].
3.1.3. The topographical condition of the Site has been considered in the overall design. Since, Parcel A and B of the site is convex in shape with southern portion and northern portion higher than the central portion. Following the natural lay of the land, the carport would be located in the central portion (lower part) of the site to maintain a lower overall building height and to allow the Proposed Development to merge with the natural profiling of the surroundings.
3.1.4. To enhance the local walkability and accessibility, it is proposed to strategically setback the proposed residential development by 1.5 m along the east and north boundaries of the Parcel A and B to create a footpath for public use.

## 4. Landscape Design Concepts

### 4.1. Landscape Design

4.1.1. The aim of the landscape proposals is to not only respond to site conditions, building form and function but to also create private gardens for the future residents. The main factors to be taken into consideration are:

- response to the site context, both in terms of landscape character and visual amenity;
- maximise the opportunities of greening;
- create soft greenery barriers around the Site to enhance privacy and reduce noise pollution from surroundings; and
- careful consideration of future maintenance requirements.
4.1.2. The detail design of the landscape layout should consider the following relevant guidelines/legislations:
- $\quad$ Hong Kong Planning Standards and Guidelines (HKPSG);
- Technical Guidelines on Landscape Treatment and Bio-engineering for Man-made Slopes and Retaining Walls (GEO Publications No. 1/2011);
- Design Manual: Barrier Free Access 2008 (Building Department);
- DEVB TCW No. 3/2012 - Site Coverage of Greenery for Government Building Projects;
- DEVB TCW No. 4/2020 - Tree Preservation; and
- DEVB TCW No. 6/2015- Maintenance of Vegetation and Hard Landscape Features.
- PNPP No. 1/2019 - Processing and Compliance Checking of Landscape Submissions Related to Planning Applications
- JPN No. 3 - Landscape and Site Coverage of Greenery
4.1.3. The principles mentioned below, describe the guidelines applied in formulating the landscape design.


## Response to the Surrounding Context and the Overall Character

4.1.4. The landscape design takes the impacts of the Ho Chung North Road and surrounding industrial uses into full consideration. Through providing boundary walls with vertical greenings along the site boundaries, green noise barriers along Ho Chung North Road would be created to minimise the potential air and noise impact of the road and surroundings industrial uses on the proposed development. Also, the landscape design aims to help integrate the proposed development with its surrounding, while enhancing the landscape and visual amenity at the public frontage. The design of the green noise barrier and its landscape treatment are proposed in Figure 4.1. At the same time, tall evergreen trees would be planted along the boundaries to ensure privacies of the residents.
4.1.5. With reference to the observation during the site visit on $5^{\text {th }}$ July 2023, there were no mature trees local located on the site and upon checking on the Register of Old Valuable Tree records on $27^{\text {th }}$ July 2023 there are no Old and Valuable Trees on the Site.
4.1.6. It is proposed that trees be incorporated into the overall landscape design, while also enhancing the landscape amenity and users' experience. Reference photos to landscape features and vegetation are provided in Figure 4.2.

## Minimal Excavation and Filling of Land Works for the Proposed Development

4.1.7. To minimise the disturbance to the land, existing trees and plantations outside the site boundary, the proposed finished levels of the development will vary within the Site which shall comply with the existing ground profiles. This will significantly reduce the amount of excavation and filling of land works that would be required.

Creation of Private Gardens for Recreational and Amenity Purposes
4.1.8. The private gardens serve as the continuation of living space for the residents. A combination of soft and hard landscape elements is proposed for not only aesthetic but also functional purpose, providing open space for residents to enjoy and use for different amenity activities. Moreover, these planting provisions will help softening the hard lines of the built forms.

## Planting Design Approach

4.1.9. Overall planting design will be consisted of a mix of practicable, ornamental trees, evergreen hedges, and flowering shrubs. Most trees with different heights are proposed along the boundaries of the Site to enhance the privacy of the Site while other soft landscape measures will be provided to ensure the hard lines of the built form being visually softened and screen off unpleasant structures such as the guard houses and the private refuse collection points (PRCP).

## Attachment 8

Site Photos Taken on 27 Oct 2023


Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group $\mathrm{E})$ " ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong (Application No. Y/SK HC/6)

Responses to Comments from Environmental Protection Department via Planning Department's email on 07.11.2023 on Further Information 01 (FI01) of the Planning Application No. Y/SK-HC/6 issued on 31.10.2023

| Comments from the Director of Environmental Protection for Environmental <br> Protection Department (EPD) via Planning Department's email on 07.11.2023; |
| :--- | :--- | :--- | :--- | :--- |
| Contact Person: Mr. Alan LI (Tel: 2835 1114) |

## Attachment 1

Extracted Page from Air Quality Impact Appraisal (Ver. C)


## Attachment 2

Extracted Page from Sewerage and Drainage Impact Appraisal (Ver. B)

## 2. Sewerage Impact Appraisal

### 2.1 Scope of Works

The objective of this Sewerage Impact Appraisal (SIA) is to assess whether the capacity of the sewerage networking is sufficient to cope with the peak sewerage flow arising from the proposed comprehensive residential development.

Existing drainage record plan from the Drainage Services Department (DSD) is shown in Figure 2.1.

### 2.2 Existing Sewerage Facilities

According to the drainage record plan, there is no existing public sewerage network serving the Site. [refer to Figure 2.1]. Hence, the Site is an unsewered area at present.

### 2.3 Proposed Sewerage Treatment

In consideration that the Site is unsewered area, it is necessary to consider the provision of an on-site underground Sewerage Treatment Plant, which will be used for treatment of sewerage generated from the Proposed Development.

The applicant will be responsible for the construction, operation and maintenance of the on-site underground Sewerage Treatment Plant and all inter-connecting sewerage pipework (polyethylene pipes) within the Site. The sewerage collected from each house will be discharged to septic tank and soil soakway soakaway pit.

The design, operation and maintenance of the proposed underground Sewerage Treatment Plant are in compliance with EPD's Practice Note for Professional Person (ProPECC) PN 5/93. It is proposed to construct four eight entire underground Sewerage Treatment Plant (involve inlet trap, septic tank, outlet trap, inter-connecting pipes and soil soakway soakaway pit) for proposed houses. The proposed capacity of the each septic tank is 15.98 cu.m and it is greater than the estimated daily water consumption of each proposed house. A reference septic tank is illustrated in Figure 2.2 and the calculation of septic tank are shown in Table 2. For the proposed soil soakway soakaway pit, its size should be determined basing on soil absorption rate and therefore it should be determined in detail design stage.

|  |  | $(\mathrm{mm})$ |
| :--- | :--- | :--- |
| Proposed Septic Tank <br> Capacity | (L-t)x BxD | $(5700-150) 1600 \times 1800=$ <br> $15.98 \mathrm{cu} . \mathrm{m}$ |
| The proposed septic tank <br> System aims to serve one <br> house with 4 Nos. of <br> Person. |  |  |
| Estimate Ultimate per <br> capita daily water <br> consumption | Design Flow Rate <br> x Peak Factor | $0.37 \times 6=2.22$ <br> cu.m/person/day |
| Required Septic Tank <br> Capacity | Nos of Person Per House x <br> estimated daily water | $4 \times 2.22=8.88$ is less <br> Septic Tank Capacity |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

Response to Public Comments received during the period of 08.09.2023 to 29.09 .2023 on the Application No. Y/SK-HC/6. There were 7 comments ${ }^{1}$ received and the comments are as follows:

| Item | Public Comments (PC) | The Applicant's Responses |
| :---: | :---: | :---: |
| PC-1 | After discussions with the villagers, the opinions on the application number Y/SK- HC/6 are as follows: <br> 1. Increase traffic flow: The roads near the application area mainly connect multiple paths to different residential areas, including Greenview Villas and Luk Fung Gardens. Currently, there is a lot of car traffic in the village, and disputes such as conflicts between people and vehicles, car congestion and parking problems arise from time to time. Furthermore, there are many old people, women and children in the village, which poses a certain risk to their safety. <br> 2. Affect the local scenery: The originally approved four 2storey detached houses were rezoned to eight 3-storey detached houses, which will create a walled building effect and affecting the view, blocking the villagers' sight and destroying the village's fengshui. <br> 3. According to the published information, the application content and terms that have been approved in 2021, is it reasonable to apply to change the current land use? | A Traffic Impact Assessment (TIA) has been carried out to assess the potential traffic impact of the Proposed Development. It concludes that the Proposed Development would not cause any significant adverse traffic impact to the vicinity of the Site. Besides, a footpath along Luk Mei Tsuen Road is proposed by the Applicant to improve pedestrian accessibility and safety. <br> A Visual Impact Assessment (VIA) has been carried out to assess the visual impact of the Proposed Development and concludes that the Proposed Development is considered to be fully acceptable in terms of visual impact and will not be incompatible to the surrounding visual context. Instead, it would enhance the visual impact on visual sensitive receivers at certain viewing points. <br> Under Section 12A of Town Planning Ordinance, the application Y/SK-HC/6 for amendment of plan is valid. |

[^3]Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| Item | Public Comments (PC) | The Applicant's Responses |
| :---: | :---: | :---: |
|  | 4. Regardless of the outcome of this application, the applicant must abide by the principle of opening existing roads used by villagers for access, and shall not close roads and block the rights of villagers to use roads after construction, given that it have been used for more than 60 years. <br> Therefore, objections are filed against the above application. | The existing road will be maintained and a dedicated footpath of 1.5 m along the eastern and northern boundaries of Parcels A and B of the Site is proposed to be provided for public use to enhance the walkability and pedestrian accessibility of the surroundings. <br> Noted. |
| PC-2 | I object to this planning application because: <br> 1. Lack of sewage and rainwater drainage systems in the village <br> Flooding is still a serious problem in the district, and the current public drainage facilities still cannot meet the needs of existing residents. During the rainy seasons, residents in low-lying areas will be severely affected. <br> 2. This application will introduce a large amount of vehicle flow <br> The increase in the number of residents will also lead to an increase in the number of people and vehicles. In addition, the application site is within the village road, which is a single-lane road with no entry and exit lines. It is also the main route for villagers to enter and exit. <br> In the future, there is also the chances for the developers to close roads to villagers during the construction or upon completion. | A Drainage Impact Appraisal (DIA) has been carried out to assess the potential impacts on drainage from the Proposed Development. It is anticipated that there will be no serious adverse drainage impact to the existing drainage system after the implementation of the Proposed Development. DD <br> A Traffic Impact Assessment (TIA) has been carried out to assess the potential traffic impact of the Proposed Development. It concludes that the Proposed Development would not cause any significant adverse traffic impact to the vicinity of the Site. <br> According to the Planning Statement, the Applicant intends to grant right of way and to devote the private parts (owned by the Applicant) of Luk Mei Tsuen Road for public use. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong


Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| Item | Public Comments (PC) | The Applicant's Responses |
| :--- | :--- | :--- | \left\lvert\, | 6. Inappropriate location of |
| :--- | :--- |
| planning application notices |
| Both Ho Chung Village and |
| Luk Mei Tsuen in Sai Kung |
| have notice boards at the |
| village entrance, but the |
| planning application notice |
| were only hung on the railings |
| next to the garbage bins, |
| which is inappropriate and |
| villagers were not informed. |$\quad$| Noted. The planning application notices |
| :--- |
| locations were selected by and hung by Town |
| Planning Board. |\right.

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| Item | Public Comments (PC) | The Applicant's Responses |
| :---: | :---: | :---: |
| PC-6 | Object to inclusion of government land as it appears to be a public passage and it should be excluded from site. If not, so then there should be a land swap. Inclusion appears to be intended to dilute the PR. <br> Object to height. Parking should be underground and the height of the villas the same of that of village houses, $8.23 \mathrm{mts}-\mathrm{meters}$, to retain the rural landscape and reduce impact on the views of the mountains. <br> Layout is appalling waste of land, effectively almost $50 \%$ of the site devoted to roads. This greatly reduces the amount of green coverage. Entrance to homes bordering road should be directly from the road, why create additional road surface. Obviously no lessons taken from recent weather events and the folly of cementing over vast tracts of land at the expense of trees and plants. <br> Note that the tree survey does not indicate how many existing trees there are on site, it only refers to "Overall planting design will be consisted of a mix of practicable, ornamental trees, evergreen hedges, and flowering shrubs", indicating zero replacement as ornamental trees are essentially tall shrubs. | The Applicant will undertake a land exchange process of 're-acquired and regrant' upon approval of this rezoning. It is proposed to reacquired land that were previous allotted to the Government for road works and to regrant land. <br> A Visual Impact Assessment (VIA) has been carried out to assess the visual impact of the Proposed Development and concludes that the Proposed Development is considered to be fully acceptable in terms of visual impact and will not be incompatible to the surrounding visual context. Instead, it would enhance the visual impact on visual sensitive receivers at certain viewing points. <br> The road layout for the Proposed Development is mainly to accommodate for Emergency Vehicular Access (EVA). Despite the above, the Proposed Development will provide greenery area of approximately $854 \mathrm{sq} . \mathrm{m}$, giving a total of greenery ratio of over $20 \%$. <br> With reference to the Landscape Proposal in the Planning Statement, there were no mature trees located on the site. In the Planting Proposal, 35 nos. of new trees will be planted, including 19 nos. of Plumeria obtusa, 11 nos. of Ficus benjamina var. princess and 5 nos. of Ravenala madagascariensis Sonn. |

Amendment of Plan to Rezone from "Residential (Group D)" ("R(D)"), "Residential (Group E)" ("R(E))" and an area shown as 'Road' to "Residential (Group C)3) ("R(C)3") on the Approved Ho Chung Outline Zoning Plan No. S/SK-HC/11 Various Lots in Demarcation District 210 and Demarcation District 244 and Adjoining Government land Ho Chung, Sai Kung, New Territories, Hong Kong

| Item | Public Comments (PC) | The Applicant's Responses |
| :---: | :---: | :---: |
| PC-7 | Regarding the application number (Y/SK-HC/6), the Sai Kung Rural Committee has received opinions from villagers, indicating that the application site is at the high level of Ho Chung North Road, where the current junction of Ho Chung North Road and Ho Chung Road is located. Serious flooding has occurred in this area during rainy days (see pictures 1, 2, and 3 of the flooding situation). There is currently no housing construction work, and flooding has already occurred frequently. Villagers have expressed their concern, and therefore hope that the relevant departments will first solve the flooding problem. | Noted. The said pictures are unclear. <br> A set of perimeter U-channels have been installed to solve the flooding problem by Home Affairs Department (HAD). This set of perimeter U-channels have been completed in 2023. <br> It should also be noted, that the Proposed Development would be provided with its own drainage connection and this would not impact the existing perimeter U-channel. <br> Furthermore, a Drainage Impact Appraisal (DIA) has been carried out to assess the potential impacts on drainage from the Proposed Development. It is anticipated that there will be no serious adverse drainage impact to the existing drainage system after the implementation of the development. <br> Should the issued of area flooding persist, the commenter should take up issue with the relevant departments for follow-up. |


[^0]:    ${ }^{1}$ Highways Department's web site (2023) Hiram's Highway Improvement Stage 1

[^1]:    ${ }^{1}$ Highways Department's web site (2023) Hiram's Highway Improvement Stage 1

[^2]:    ${ }^{2}$ It is noted the water mains of the WSD have been designed with pressure of 15 to 30 m for freshwater pipelines. (WSD Performance Pledge 2022/23, https://www.wsd.gov.hk/en/about-us/performance-targets-andachievements/index.html)

[^3]:    ${ }^{1}$ The Chinese comments were translated and included.

