Annex F

Finalised Sewerage Impact Assessment

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

Prepared for

Wing Mau Tea House Limited

Prepared by

Ramboll Hong Kong Limited

PROPOSED REZONING FROM "RESIDENTIAL (GROUP B)1" ZONE TO "RESIDENTIAL (GROUP B)4" ZONE FOR MEDIUM-DENSITY HOUSING DEVELOPMENT TO INCLUDE A FOOTPATH FOR PUBLIC USE AT VARIOUS LOTS AND ADJACENT GOVERNMENT LAND IN DD130, LAM TEI, TUEN MUN

SEWERAGE IMPACT ASSESSMENT



Date 15 July 2024

Prepared by Ken Li

Environmental Consultant

Signed

Approved by Katie Yu

Senior Manager

Signed

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Ramboll Hong Kong Limited

21/F, BEA Harbour View Centre 56 Gloucester Road, Wan Chai, Hong Kong

Tel: (852) 3465 2888 Fax: (852) 3465 2899 Email: hkinfo@ramboll.com

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1. INTRODUCTION

1.1 Background and Objectives

- 1.1.1 The Application Site is located at various lots and adjoining government land in D.D.130, Lam Tei, Tuen Mun.
- 1.1.2 In order to confirm the environmental feasibility of the Proposed Development, Ramboll Hong Kong Limited is commissioned to conduct the Sewerage Impact Assessment based on the information of the Proposed Development.

1.2 Application Site and its Environ

- 1.2.1 With a development area of 8,896m² and zoned as Residential (Group B) 1" (R(B)1) zone, the Application Site is currently a vacant site situated at D.D. 130, Lam Tei bounded by the Castel Peak Road Lam Tei to the east separated by the Tuen Ma Line viaduct and LRT tracks. Existing residential development, Lingrade Garden, is located to the north of the Application Site. Some temporary carpark, open storage and village houses are located to the west of the site separated by a nullah.
- 1.2.2 **Figure 1.1** shows the location of the Application Site and its environ.

1.3 Proposed Development

- 1.3.1 The Proposed Development consists of 5 residential towers ranging from 14 storeys to 27 storeys. A clubhouse is situated below Tower 1 and Tower 3. The number of storeys and building height information of the residential towers are as follow:
 - Tower 1: 27 storeys (+107.8 mPD)
 - Tower 2: 27 storeys (+107.8 mPD)
 - Tower 3: 16 storeys (+66.8 mPD)
 - Tower 4: 14 storeys (+59.8 mPD)
 - Tower 5: 24 storeys (+94.8 mPD)
- 1.3.2 The total numbers of units are 1,385 units with a proposed domestic GFA of 44,480m² (PR5.0) and the tentative population intake year is 2030. Detailed plans of the Proposed Development are presented in **Appendix 1.1**.



2. SEWERAGE IMPACT ASSESSMENT

2.1 Scope of Work

- 2.1.1 The Application site is currently vacant and there is no public sewer network serving the Application Site and the surrounding environment due to the isolation by a nullah.
- 2.1.2 The aim of this study is to evaluate the sewage flow generated from the Proposed Development and assess whether there is adverse sewerage impacts to the surrounding environment.

2.2 Assessment Criteria and Methodology

- 2.2.1 The Commercial and Industrial Floor Space Utilization Survey (CIFSUS) conducted by the Planning Department has been used to determine the worker density for various economic activities and planned usage types.
- 2.2.2 Environmental Protection Department's (EPD's) Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning, Version 1 (GESF) is referred to estimate the quantity of the sewage generated from the Proposed Development and the existing development. Sewage flow parameters and global peaking factors in this document are adopted.
- 2.2.3 According to the Table T-1 of the said EPD Guideline, the unit flow factor (UFF) of resident (Private R2) is 0.27 m³/day, while the UFF for employees for the clubhouse is 0.28m³/day.

2.3 Existing Condition

2.3.1 Currently, there are no public sewerage facilities in the vicinity of the Application Site and the extension of the public sewerage system to Application Site is unlikely as the Application Site is separated from the nearest sewerage manhole by a nullah. It will therefore be necessary to provide on-site treatment facilities before discharging the treated effluent into the nearby nullah.

2.4 Future Estimated Flow

2.4.1 Under the current scheme, the Proposed Development will consist of 1,385 residential units, a clubhouse with a size of 2,002 m² and two swimming pools with an area of 48m² and 108m² respectively. Based on such design parameters, the Average Dry Weather Flow (ADWF) excluding the backwash of the swimming pools is estimated to be 1,028.3 m³/day, as tabulated in **Table 2.1** below and **Appendix 2.1**.

Table 2.1 Estimated Sewage Flow from the Proposed Development

			Remarks
Residential Use			
Total number of residential units	=	1,385	
Total number of residents	=	3,740	people (average household size of 2.7 is assumed)
Design flow	=	270	litre/person/day (Private R2 in Table T-1 of GESF)
Sewage Generation Rate	=	1009.8	m³/day
		·	
Clubhouse		·	



			Remarks
Total GFA	=	2,002	m ²
Assumed floor area per employee	=	30.3	m ² per employee (refer to Table 8 of CIFSUS - Community, Social & Personal Services)
Total number of employees		66	employees
Design flow	=	280	litre/employee/day (refer to Table T-2 of GESF - J11 Community, Social & Personal Services)
Sewage Generation Rate	=	18.5	m³/day
Proposed Adult Swimmin	g Pool		
Assumed area of swimming pool	=	108	m ²
Average depth of water		1.2	m
Volume of swimming pool	=	129.6	m ³
Turnover rate	=	6	hr
Surface loading rate of filter	=	50	m³/m²/hr
Filter areas required		0.4	m ² (Volume of Swimming Pool/Turnover Rate/ Surface Loading Rate of Filter)
Backwash duration	=	5	min/d
Backwash flow rate	=	30	m³/m²/hr
Design flow for swimming pool backwashing	=	1.1	m ³ /day (Backwash flow rate x Filter areas required x Backwash duration)
Design flow for swimming pool backwashing	=	3.6	litre/sec (Backwash flow rate x 1000 / (Backwash duration x 60))
Proposed Children Swimn	ning P		
Assumed area of swimming pool	=	48	m ²
Average depth of water		1.2	m
Volume of swimming pool	=	57.6	m ³
Turnover rate	=	6	hr
Surface loading rate of filter	=	50	m³/m²/hr
Filter areas required		0.2	m ²



			Remarks
			(Volume of Swimming Pool/Turnover Rate/ Surface Loading Rate of Filter)
Backwash duration	=	5	min/d
Backwash flow rate	=	30	m³/m²/hr
Design flow for swimming pool backwashing	=	0.5	m ³ /day (Backwash flow rate x Filter areas required x Backwash duration)
Design flow for swimming pool backwashing	=	1.6	litre/sec (Backwash flow rate x 1000 / (Backwash duration x 60))
Total Flow from the Propo	sed D	evelopmen	t
Flow Rate	=	1,028.3	m³/day
Flow Rate with P _{CIF} (Tuen Mun – 1.1)	=	1131.1	m³/day (refer to Table T-4 of GESF - Tuen Mun - 1.1)
Contributing Population	=	4189	people
Peaking factor	=	4.0	(refer to Table T-5 of GESF for a population of <10,000 incl. stormwater allowance)
Peak flow (excluding backwash of all pools)	=	52.4	litre/sec
Peak flow (including backwash of all pools)	=	57.6	litre/sec

2.5 Proposed Sewage Treatment Facility

- 2.5.1 As the Application Site is not served by public sewerage system currently, it is proposed to provide on-site sewage treatment to ensure sewage can be discharged in a proper manner. Once public sewerage system is connected to the Application Site, the sewage flow from the proposed development may be discharged to the public sewerage system in the future.
- 2.5.2 The use of Sewage Treatment Plant (STP) is recommended for the Proposed Development. The "Guidelines for the Design of Small Sewage Treatment Plants" published by the EPD for the design, construction, operation and maintenance of STP should be followed. The effluent should be treated to acceptable standards as stated in "Technical Memorandum Standards for Effluents Discharged into Drainage and Sewerage systems, Inland and Coastal waters". As the proposed development have a population of more than 2,000, the Applicant will approach EPD to discuss the design of the STP during detailed design stage.
- 2.5.3 Suitable treatment for the sewage generated by the proposed development could practically be provided by a package treatment unit, i.e. treatment facilities which are supplied and installed as complete units, including all treatment stages. For example, one appropriate technology would be Membrane Bioreactor (MBR) units, followed by disinfection to achieve the required effluent standards. The treated effluent can then be discharged of into the drainage system. A sample of the MBR unit is enclosed in **Appendix 2.2**.



- 2.5.4 The design and operation of the STP will be in accordance with the "Guidelines for Design of Small Sewage Treatment Plants" published by the EPD, with adequate capacity to handle three times of ADWF. The area of the STP will be approximately 1,254m² occupying two basement floors. The treated effluent would then be temporary stored in the stormwater storage tank before discharging into the existing nullah to the west of the Application Site. **Figure 2.1** shows the indicative location of the onsite treatment plant. The exact discharge location and pipe design will be subject to further study and agreed with DSD during detailed design stage.
- 2.5.5 The effluent generated from the Proposed Development will go through the process of disinfection to achieve the necessary discharge standards as set out in EPD's Technical Memorandum Standards for Effluents Discharged into Drainage and Sewerage Systems, Inland and Coastal Waters (Table 6 Standards for Effluents Discharged into Group D Inland Waters).
- 2.5.6 As a good practice for sewage treatment facilities, measures will be incorporated into the design to minimise the risk of emergency overflow from the treatment plant. These measures include standby pumps, secure power supplies, appropriate alarms, as well as comprehensive Operation and Maintenance procedures in order to keep the facilities in good working order.
- 2.5.7 The proposed on-site MBR will be fully enclosed with deodorising system, so that the noise and odour emitted from the plant will not have adverse impact upon the surrounding residents.
- 2.5.8 The project proponent will be responsible for the planning, design, implementation and maintenance of the proposed STP and arrangement of the sludge from the proposed STP.
- 2.5.9 The detailed design and size of the MBR unit will be provided in later stage for EPD's approval.



3. CONCLUSION

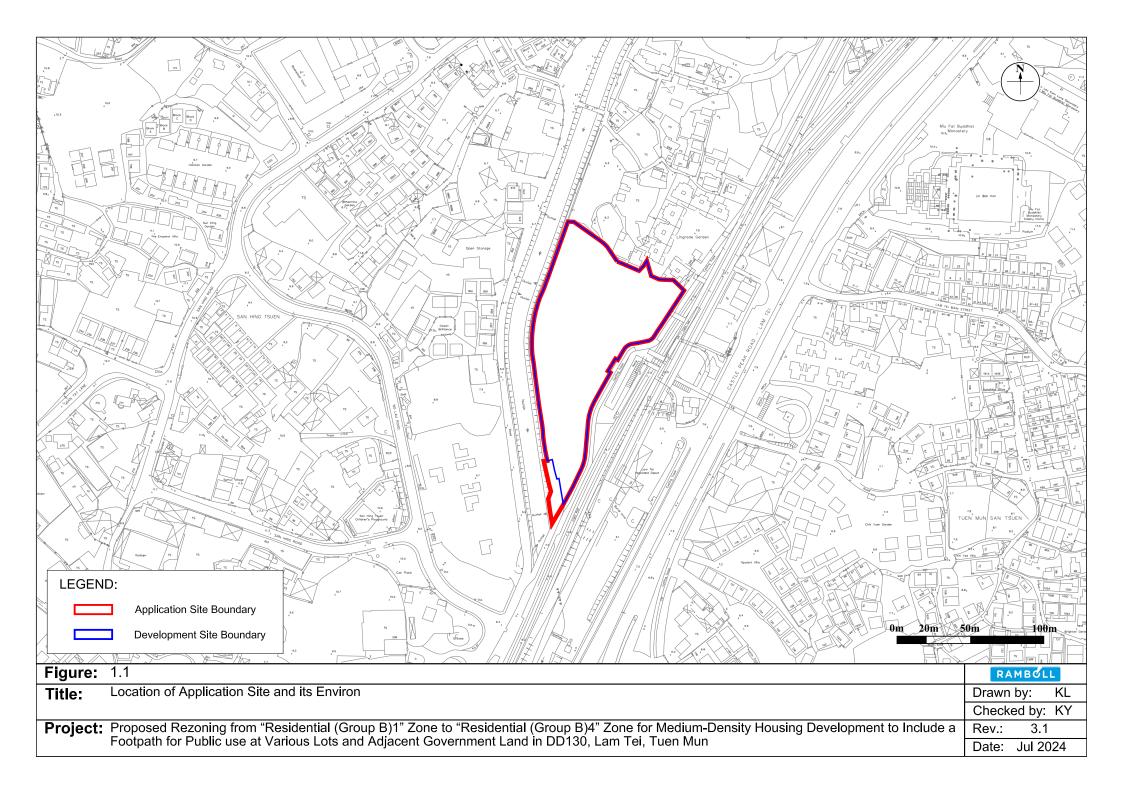
- 3.1.1 A residential development is proposed for the Application Site at D.D.130 Lam Tei, Tuen Mun. The potential sewerage impact has been quantitatively addressed.
- 3.1.2 Since there is no public sewerage system available for the Proposed Development, onsite treatment will be required. Connection to the public sewer will be a long-term option after availability of trunk sewer connecting to the Application Site.
- 3.1.3 An on-site treatment with Membrane Bioreactor (MBR) will be used to treat effluent from the Proposed Development, the treated effluent would be temporary stored in a proposed stormwater storage tank before discharging into the nearby nullah. Relevant statutory guidelines and regulations will be followed and detail design of the MBR unit will be provided in later stage.
- 3.1.4 With the sewage treatment plant and proper maintenance in place, it is concluded that the Proposed Development will not have any unacceptable adverse sewerage impact to the surrounding.

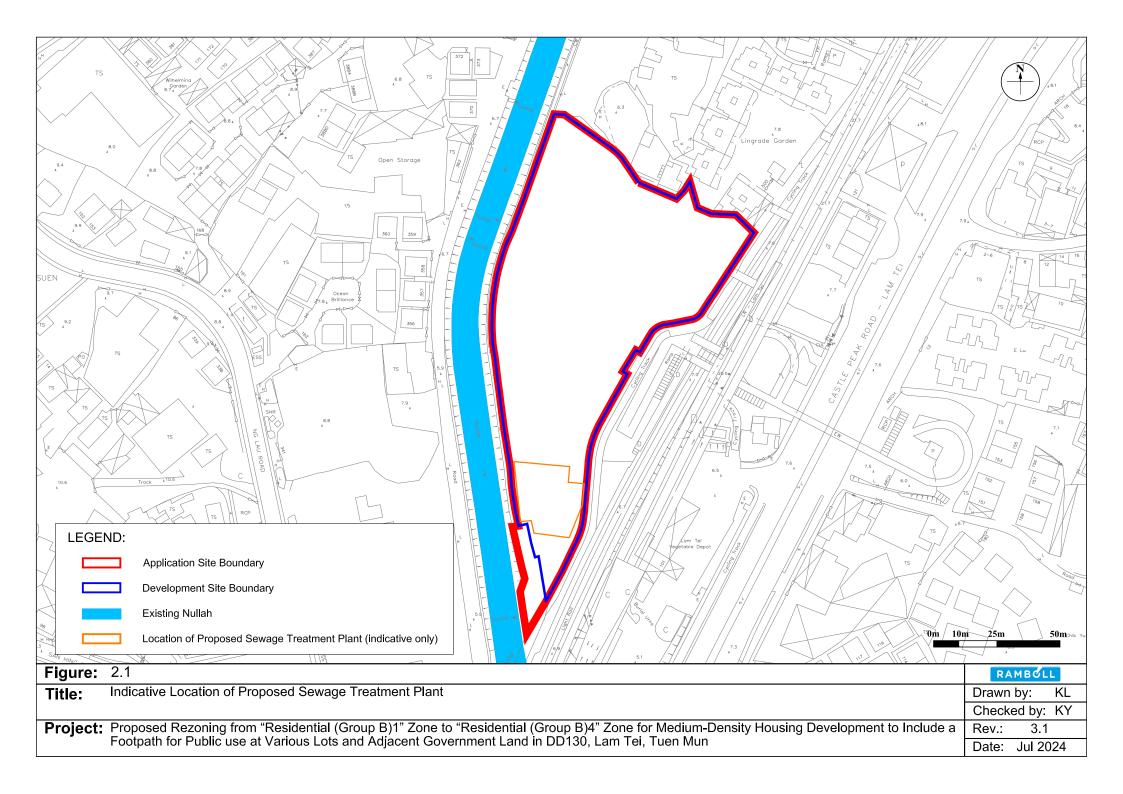


Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

Figures





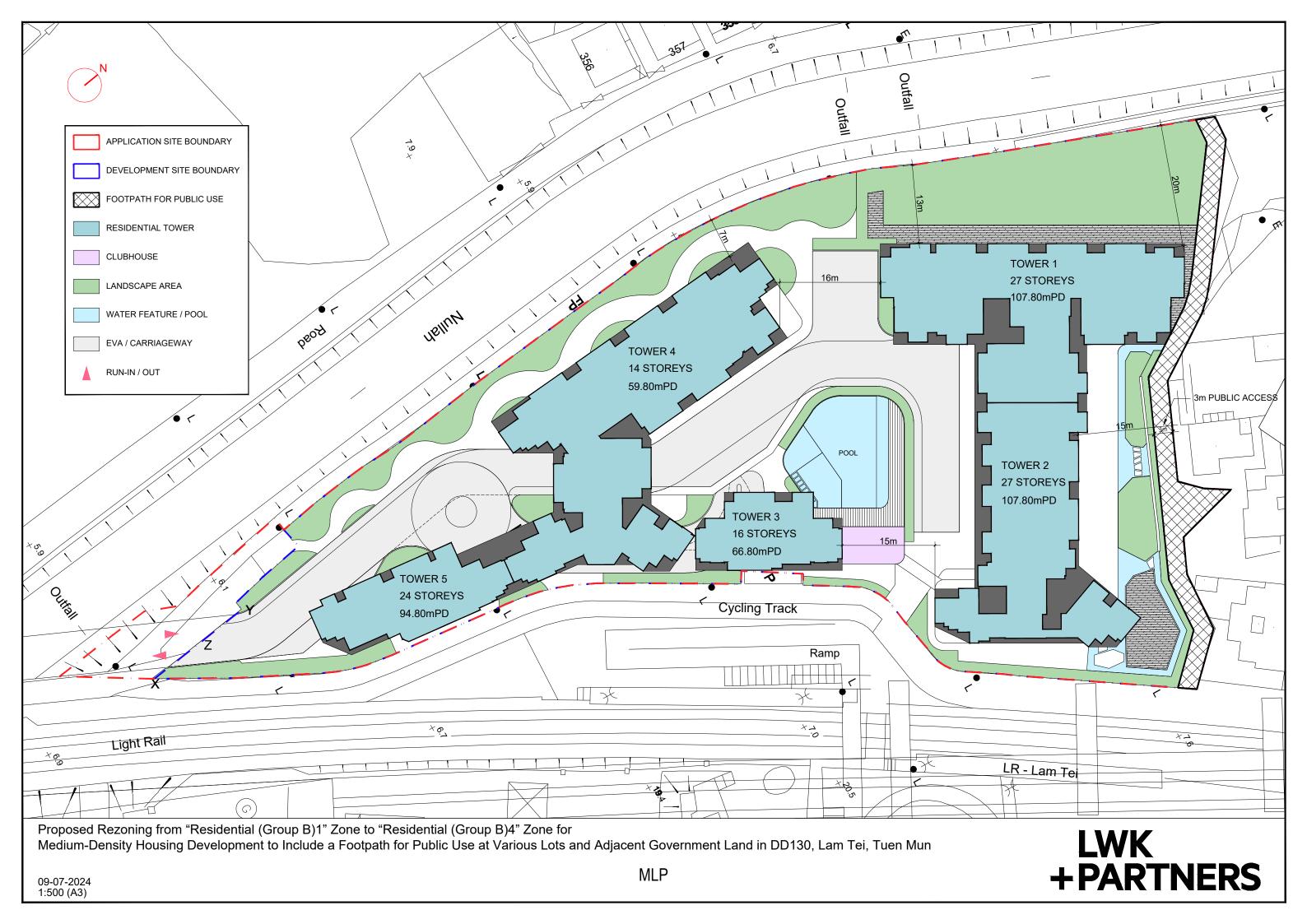


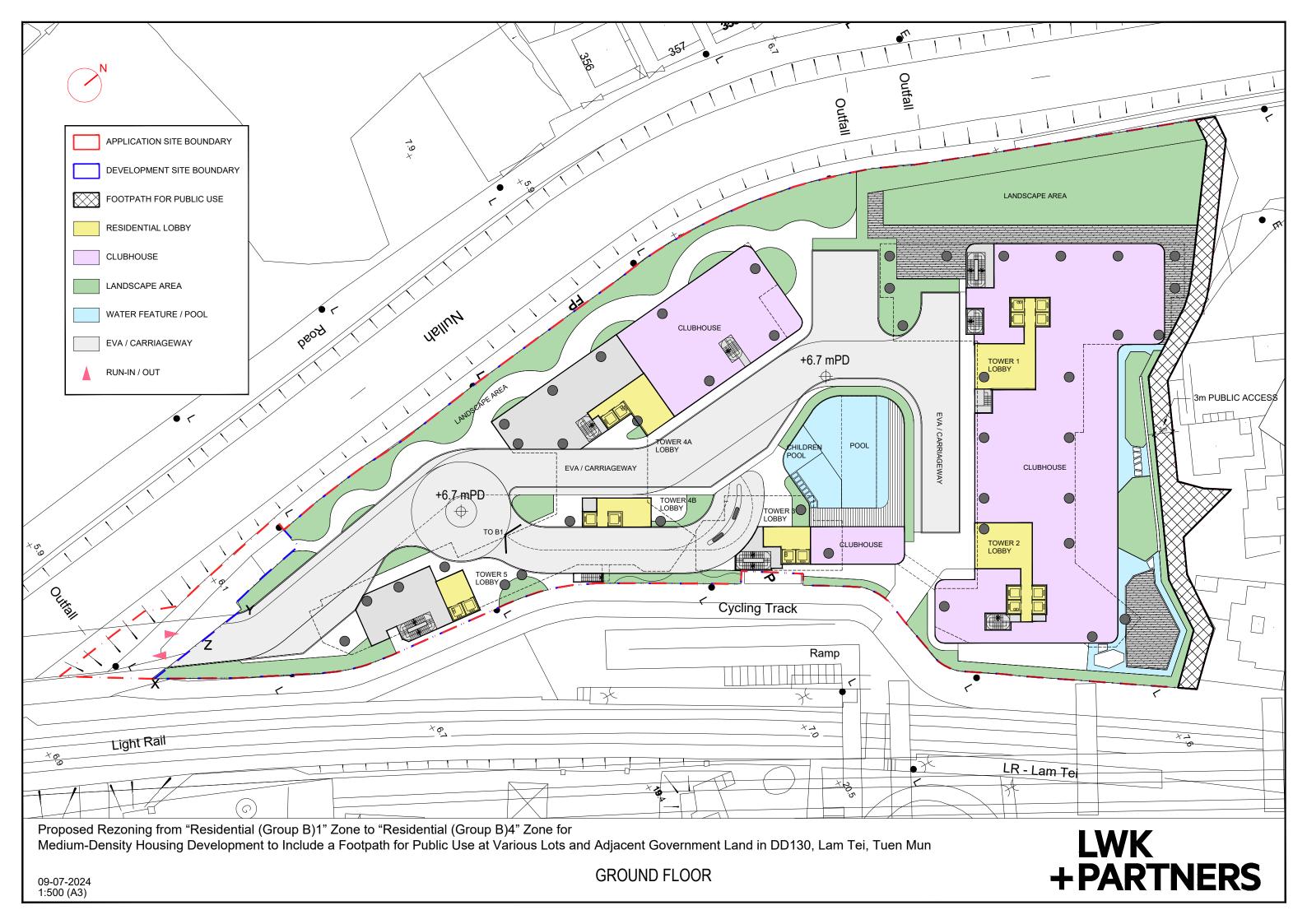
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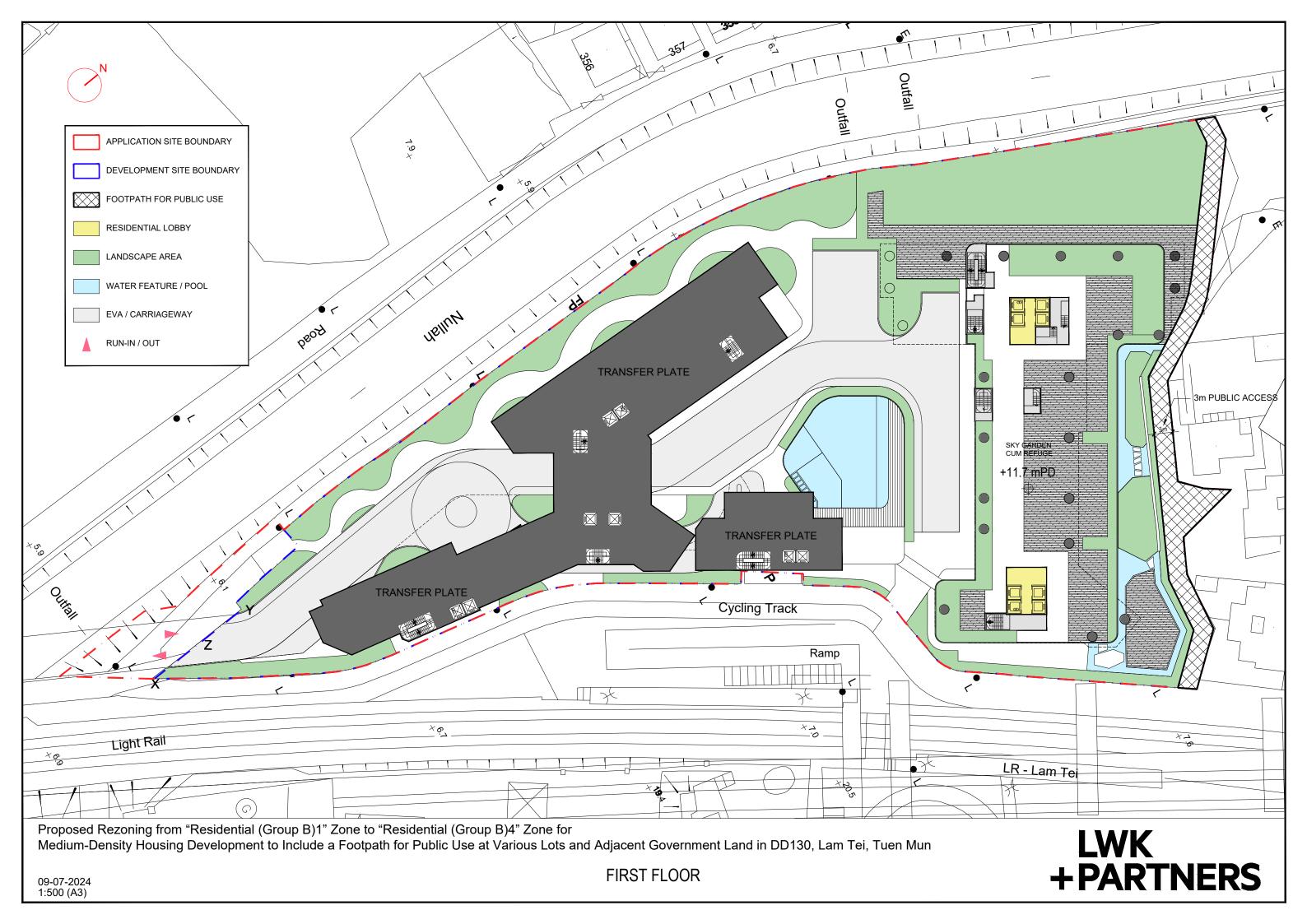
Appendix 1.1

Detailed Layout Plans of the Proposed Development











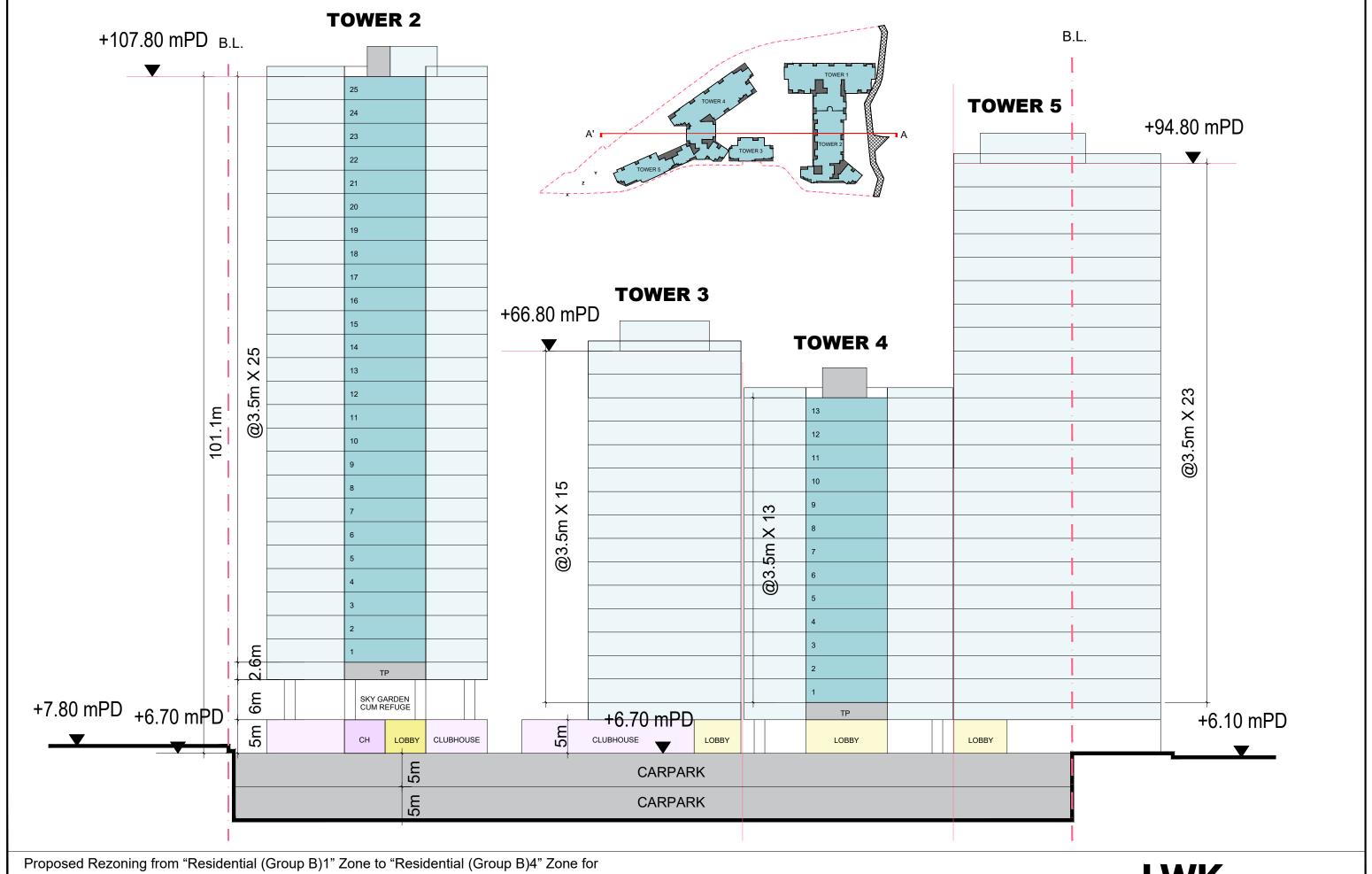
Number of car parking spaces at B1: 119
Total number of car parking spaces: 247

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public Use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

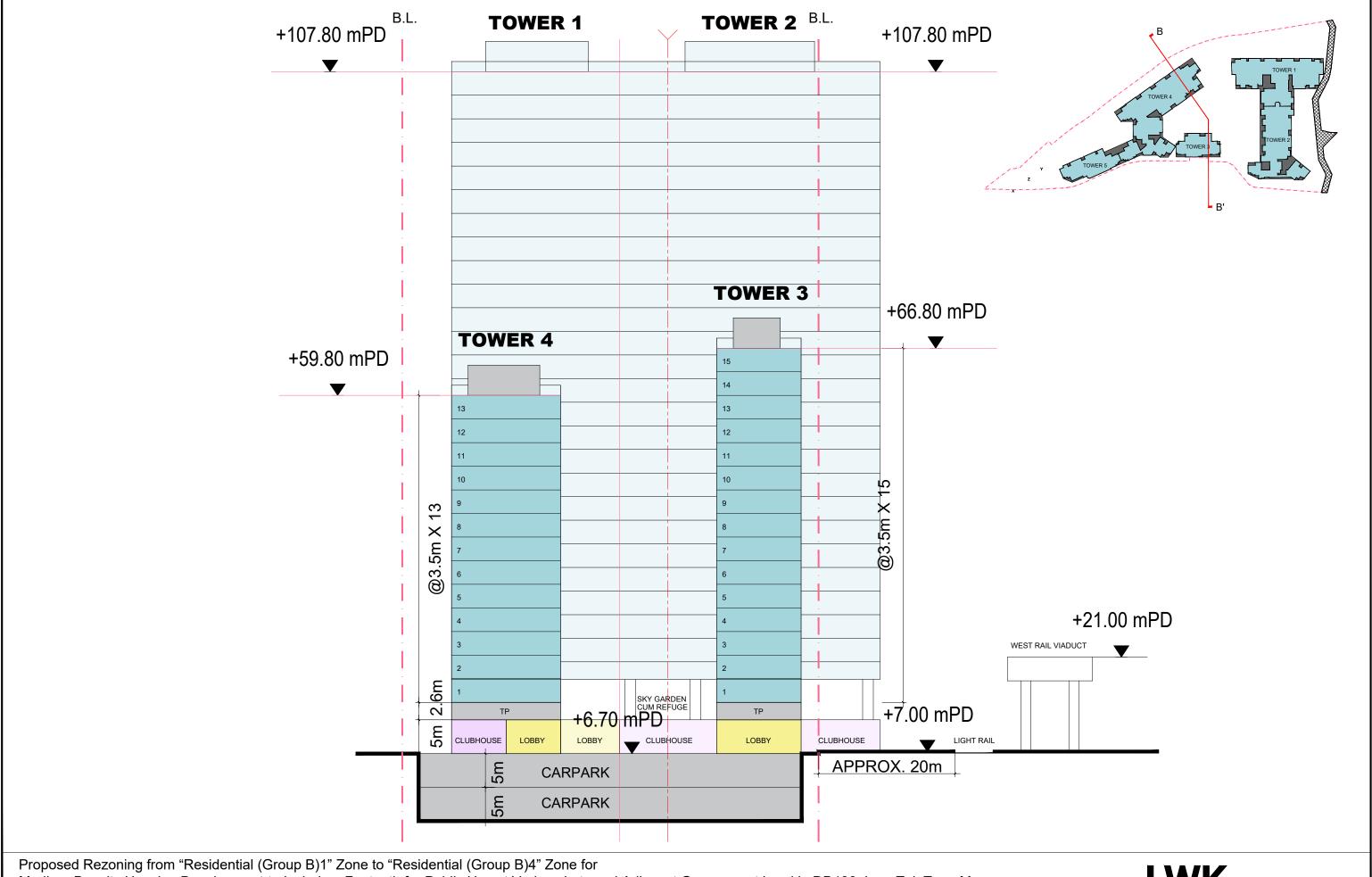


Number of car parking spaces at B1: 128 Total number of car parking spaces: 247

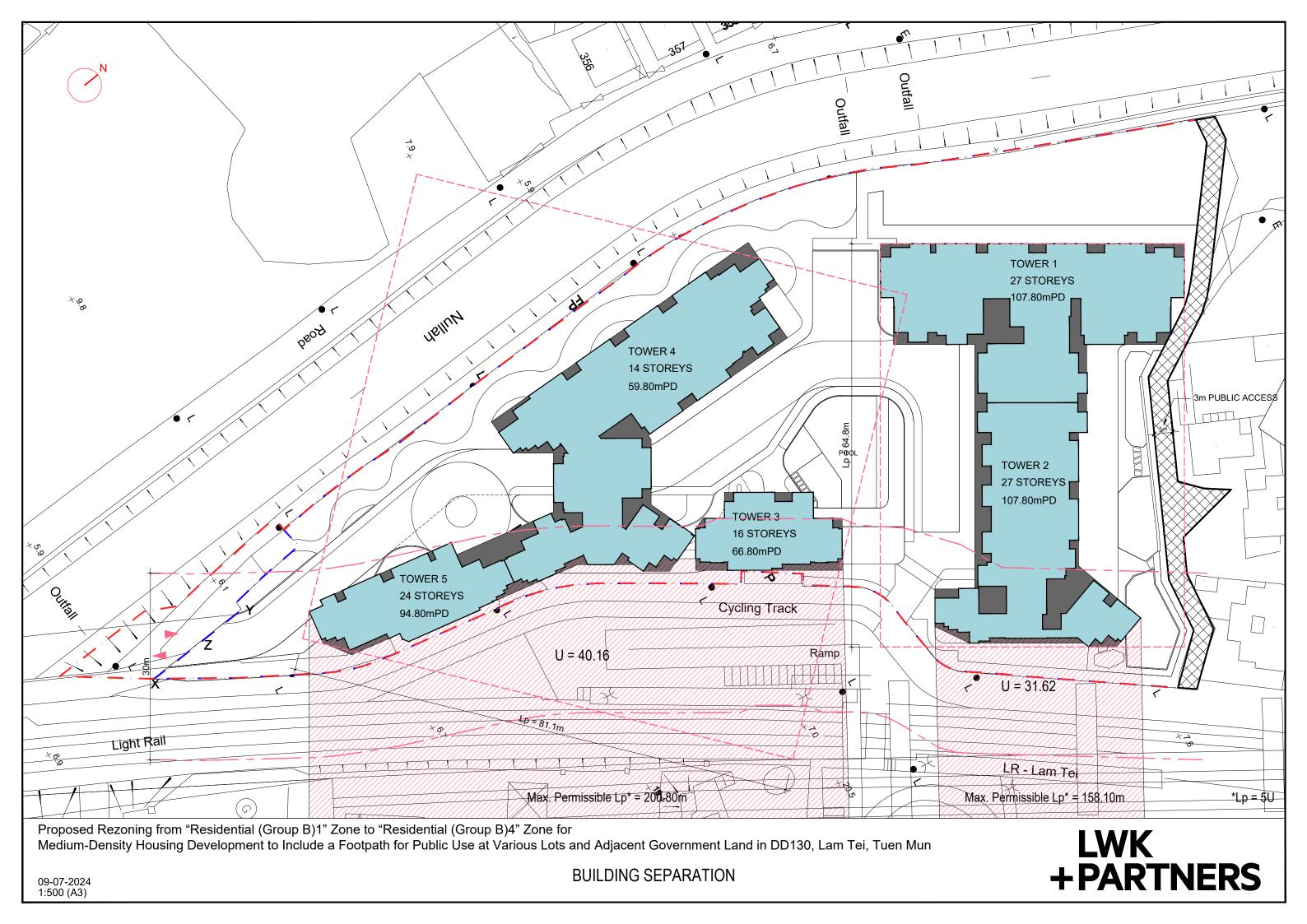
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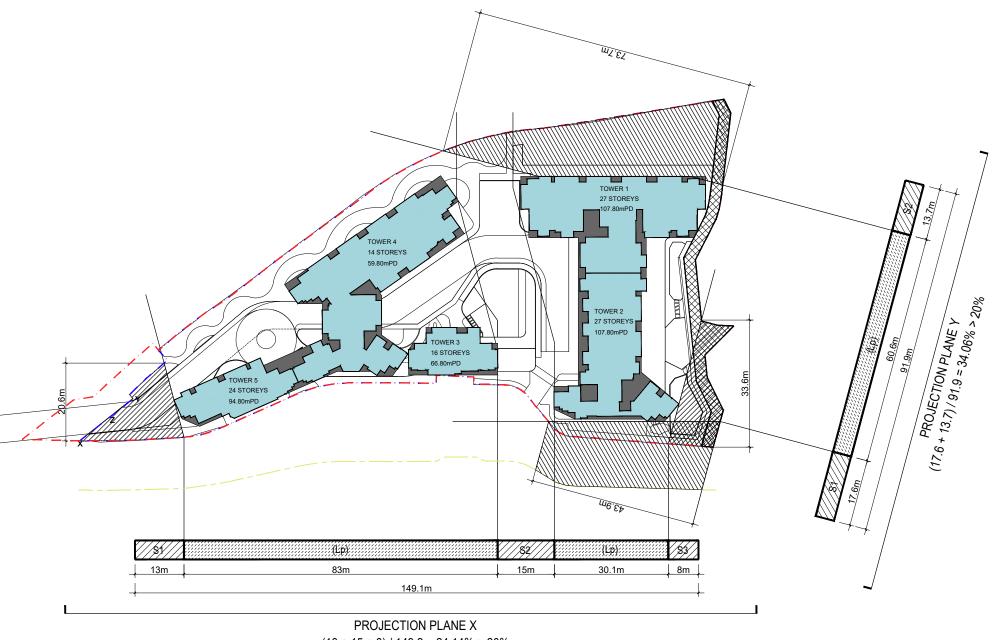
Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public Use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun



Medium-Density Housing Development to Include a Footpath for Public Use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun







(13 + 15 + 8) / 149.2 = 24.14% > 20%

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public Use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

Appendix 2.1

Detailed Sewerage Impact Assessment Calculations



Table 1 Calculation for Sewage Generation Rate of the Proposed Development at the Application Site

Residential Units		
Total number of residential units	=	1.385 units
Total number of residents	=	3740 residents (2.7 people per flat is assumed)
Design flow	=	270 litre/person/day (Private R2 in Table T-1 of GESF)
Sewage generation rate	=	1,009.8 m³/day
Clubhouse		
Total GFA	=	2.002 m ²
Assumed floor area per employee	_	30.3 m ² per employee (refer to Table 8 of CIFSUS - Community, Social & Personal Services)
Total number of employees	=	66 employees
Design flow for employees	=	280 litre/employee/day (refer to Table T-2 of GESF - J11 Community, Social & Personal Services)
Sewage generation rate	=	18.5 m ³ /day
Sewage generation rate	_	10.5 m /day
Proposed Adult Swimming Pool		
Assumed area of swimming pool	=	108 m ²
Average depth of water	=	1.2 m
Volume of swimming pool	=	129.6 m ³
Turnover rate	=	6 hr
Surface loading rate of filter	=	50 m³/m²/hr
Filter areas required	=	0.4 m^2
Backwash duration	=	5 min/d
Backwash flow rate	=	$30 \text{ m}^3/\text{m}^2/\text{hr}$
Design flow for swimming pool backwashing	=	1.1 m³/day
Design flow for swimming pool backwashing	=	3.6 litre/sec
Proposed Children Swimming Pool		
Assumed area of swimming pool	=	48 m²
Average depth of water	=	1.2 m
Volume of swimming pool	=	57.6 m ³
Turnover rate	=	6 hr
Surface loading rate of filter	=	50 m³/m²/hr
Filter areas required	=	0.2 m^2
Backwash duration	=	5 min/d
Backwash flow rate	=	30 m³/m²/hr
Design flow for swimming pool backwashing	=	0.5 m^3 /day
Design flow for swimming pool backwashing	=	1.6 litre/sec
Total Flow from the Proposed Development		
·		1.028,3 m ³ /day
Flow rate (excluding backwash of all pools) Flow rate with P _{CIF} (Tuen Mun - 1.1)	=	,
		1131.1 m ³ /day (refer to Table T-4 of GESF - Tuen Mun - 1.1)
Contributing population	=	4189 people
Peaking factor	=	4.0 (refer to Table T-5 of GESF for a population of <10,000 incl. stormwater allowance)52.4 litre/sec
Peak flow (excluding backwash of all pools)	=	
Peak flow (including backwash of all pools)	=	57.6 litre/sec

Remarks

(1) For job type J11, the "per-employee" unit flow factor takes into account the flows of customers and tenants.

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun

Appendix 2.2

Sample of Membrane Bioreactor



In conventional activated sludge processes, separation of treated water from the sludge is mainly achieved by sedimentation in secondary clarification tank. This requires a considerable amount of space and capital cost, and separation efficiency is low.

Using membrane to replace the secondary clarification tank, suspended solids can be completely removed from treated water, resulting in a high effluent quality. It not only concentrates slow-growing bacteria, but also retains large molecular organic compounds in reactor, thereby increasing contact time with the sludge. Thus, removal rate of COD, BOD, nitrogen and phosphorus will be greatly increased.



Raw wastewater (left) and treated water (right)

Advantanges of MBR

- 1 High effluent quality
 - · Permeate free of suspended solids
- 2 Small footprint, elimination of secondary clarification tank
 - · Only half of the area of conventional activated sludge process is required, and capital cost is reduced
 - · Solve the problem of poor sedimentation in conventional activated sludge process; especially for winter
- 3 Long solid retention time, easy management
 - · Sludge wasting volume is greatly reduced
- 4 High biomass concentration and treatment efficiency
- Operate at biomass concentration of 10,000 20,000 mg/L, COD removal and denitrification occur simultaneously
- 5 Easy to install in the existing aeration tank to become a MBR system
- 6 Easy for operation & maintenance
- 7 Greatly reduce the volume of aeration tank
- 8 Reduce fouling and increase water flux

Case Studies

Wastewater Source	Parameters	Influent	Effluent	Removal Rate (%)
Food Processing	BOD (mg/L)	1590	1	99.9
9	CODcr (mg/L)	2600	7	99.0
	SS/ (mg/L)	380	<1	100
Sauce Production	BOD (mg/L)	3630	5	99.9
	SS/ (mg/L)	380	<1	100
Landfill Leachate	CODcr (mg/L)	~10,000	60	99.4
	SS/ (mg/L)	<500	<1	100
Grey Water Recycling	BOD (mg/L)	<=200	<=10	95.0
	SS/ (mg/L)	<=200	< 1	100

Uses of MBR

- 1 Domestic wastewater treatment / Grey water recycling
 High treatment efficiency, high effluent quality
- 2 Industrial wastewater treatment Suitable for landfill leachate, high BOD food processing wastewater, livestock wastewater, chemical wastewater and pulp and paper wastewater
- 3 Water reuse or recycling
 Direct reuse or purified with reverse osmosis

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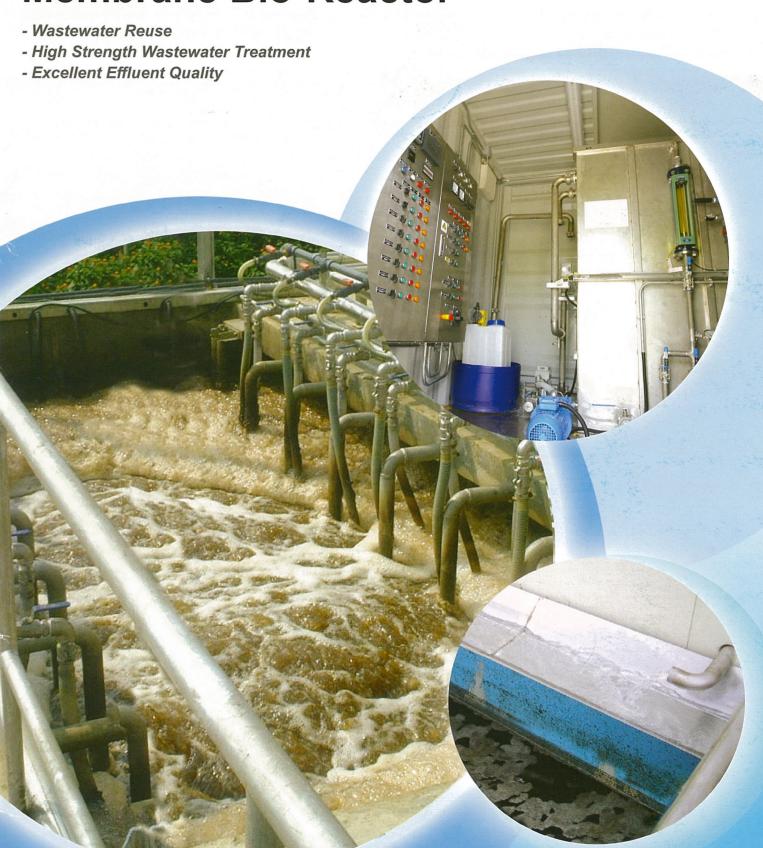
8 Wang Lee Street, Yuen Long Industrial Estate, Yuen Long, N.T., Hong Kong

Tel: (852) 2443 8168 Email: decl@dunwellgroup.com
Fax: (852) 2789 3346 Website: www.dunwellgroup.com



MBR

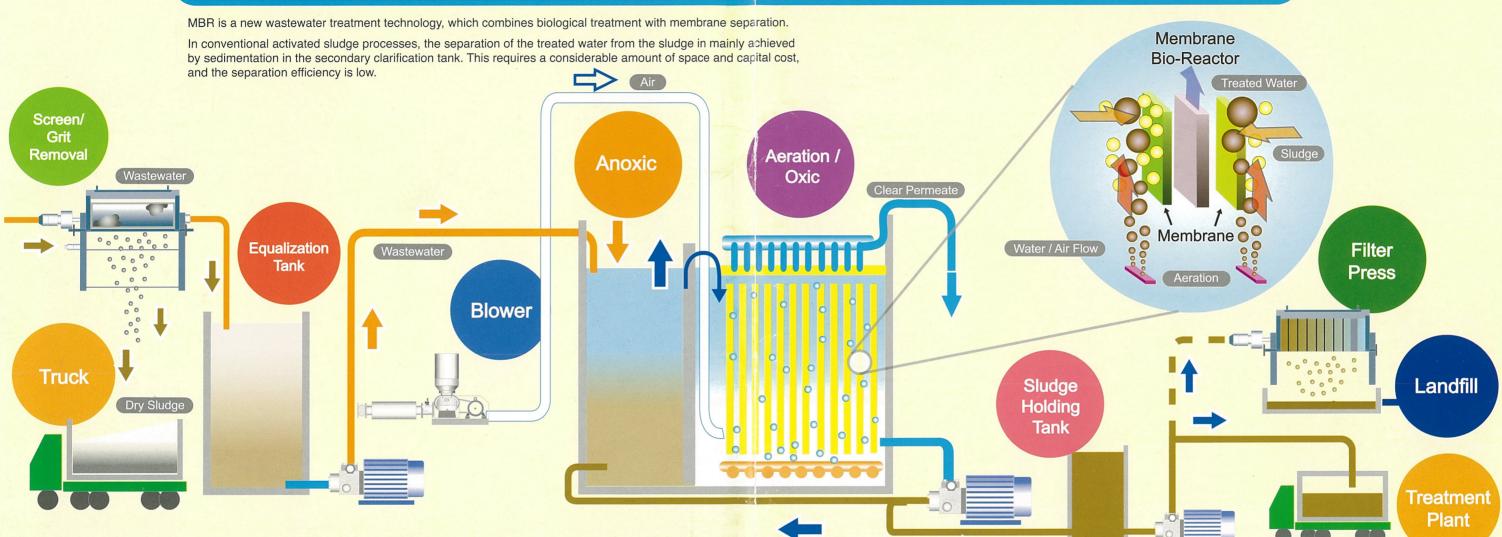
Membrane Bio-Reactor





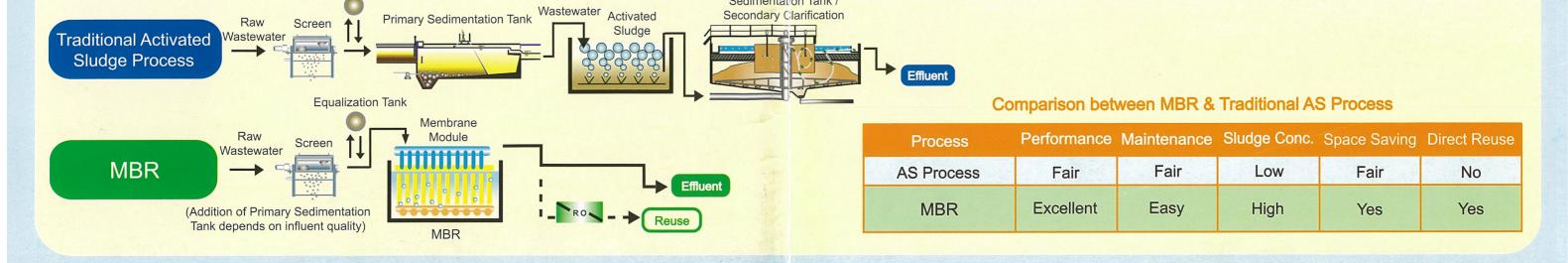
MBR Membrane Bio-Reactor





MBR and Traditional Activated Sludge Process Comparison

Equalization Tank



Sedimentation Tank /